

Development Services

From Concept to Construction

Phone: 503-823-7300 Email: bds@portlandoregon.gov 1900 SW 4th Ave, Portland, OR 97201

More Contact Info (<http://www.portlandoregon.gov/bds/article/519984>)



APPEAL SUMMARY

Status: Decision Rendered

Appeal ID: 31454

Project Address: 6123 N Marine Dr

Hearing Date: 4/12/23

Appellant Name: Jessie Reeves

Case No.: B-001

Appellant Phone: 3604335154

Appeal Type: Building

Plans Examiner/Inspector: Joe Thornton

Project Type: commercial

Stories: 1 **Occupancy:** 10 **Construction Type:** Tenant Improvement

Building/Business Name: Rodda Paint

Fire Sprinklers: Yes - Warehouse

Appeal Involves: other: Fire

LUR or Permit Application No.: IVR Number: 4892439

Plan Submitted Option: pdf [File 1] [File 2] [File 3]

Proposed use: Storing Paint

APPEAL INFORMATION SHEET

Appeal item 1

Code Section

Fire Code Appeal

Requires

OSSC 2702.1.1/PFC 1203.1.1 Stationary generators. Stationary emergency and standby power generators required by this code shall be listed in accordance with UL 2200.

Code Modification or Alternate Requested

The intent behind the appeal is to get approval to use the Wacker G25 in lieu of the permanent generator until the permanent generator is received in June 2023.

Proposed Design

The generator set shall conform to the requirements of the following codes and standards, as applicable. The generator set shall include necessary features to meet the requirements of these standards. CSA C282, 2009 Emergency Electrical Power Supply for Buildings

Reason for alternative

The Wacker g25 is similar to the permanent unit being offered. The intent behind the appeal is to get approval to use the Wacker G25 in lieu of the permanent generator until the permanent generator is received in June 2023. CSA C282, 2009 Emergency Electrical Power Supply for Buildings

APPEAL DECISION

Use of temporary emergency generator until the permanent generator is received in June 2023: Granted as proposed.

The Administrative Appeal Board finds that the information submitted by the appellant demonstrates that the approved modifications or alternate methods are consistent with the intent of the code; do not lessen health,

safety, accessibility, life, fire safety or structural requirements; and that special conditions unique to this project make strict application of those code sections impractical.

Pursuant to City Code Chapter 24.10, you may appeal this decision to the Building Code Board of Appeal within 90 calendar days of the date this decision is published. For information on the appeals process, go to www.portlandoregon.gov/bds/appealsinfo, call (503) 823-6251 or come in to the Development Services Center.



**WACKER
NEUSON**
all it takes!

G25

Mobile Generators



Versatile and reliable power, when and where you need it

The G25 mobile generator offers contractors and rental houses an excellent power solution. The compact, sound-attenuated model provides single and three-phase power for construction, commercial, industrial and special event applications where quiet, reliable power is needed. A digital controller monitors and protects the generator and engine while providing continuous operating data on an easy to read LCD display. Large capacity fuel tank for long run times and durable features for long life.

- Ideally suited for construction sites where durability, reliability and flexibility are required to supply continuous power for applications such as lighting, cranes, concrete mixers and conveyor belts.
- Brushless, oversized alternators with separate excitation winding and AVR (Automatic Voltage Regulation) provide superior motor starting and enhanced ability to run non-linear loads.
- Single switch start and simple, uncluttered control panel provide unmatched ease of use.
- Automatic remote start capability standard which is ideal for temporary standby applications.
- Factory-installed customized options are available. These include, but are not limited to, a fluid containment system, cold weather options, a camlock panel, etc. Also available are optional trailers that are fully equipped and highway ready.



Technical specifications

	G25 (T4i)	G25 (T4F)	G25 (T4i) 600V- Canada Only
Dimensions			
L x W x H Operating (Skid) in	76.6 x 35 x 44.5	76.6 x 35 x 44.5	76.6 x 35 x 44.5
L x W x H Shipping (Skid) in	81 x 37.5 x 56.6	81 x 37.5 x 56.6	81 x 37.5 x 56.6
L x W x H Shipping/Operating (Trailer) in	133 x 60 x 69	133 x 60 x 69	133 x 60 x 69
Weights			
Weight Dry (Skid) lb	1,911	1,952	1,911
Weight Operating (Skid) lb	2,332	2,373	2,332
Weight Shipping (Skid) lb	2,031	2,072	2,031
Weight Operating (Trailer) lb	2,802	2,843	2,802
Weight Shipping (Trailer) lb	2,381	2,422	2,381
Operating data			
Trailer coupling Type	2in Ball	2in Ball	2in Ball
Standby output kW	20.4	21.4	19.5
Prime output kW	19.5	19.5	18.7
Voltage 3 Phase V	240/480	240/480	240/480/600
Current 3 Phase (Prime) A	58/29	58/29	58/29/23
Current 1 Phase (Prime) A	120/240	120/240	120/240
Voltage 3 Phase V	208, 220, 240, 416, 440, 480	208 / 220 / 240 / 416 / 440 / 480	208, 220, 240, 416, 440, 480, 600
Voltage 1 Phase V	120, 127, 139, 240, 254, 277	120, 127, 139, 240, 254, 277	120, 127, 139, 240, 254, 277, 346
Circuit breaker (Main) A	70	70	70
Frequency Hz	60	60	60
Power factor 1 Phase cos Φ 1~	1	1	1
Power factor 3 Phase cos Φ 3~	0.8	0.8	0.8
Sound level (LwA) at Prime (23 ft / 7 m) dB(A)	65	65	65
Receptacles -20 Amp GFI Duplex	2	2	2
Receptacles -50 Amp Twist Lock	2	2	2
Engine / Motor			
Engine / Motor	Liquid-Cooled, 4 Cylinders	Liquid-Cooled, 4 Cylinders	Liquid-Cooled, 4 Cylinders
Engine / Motor manufacturer	Isuzu	Isuzu	Isuzu
Emissions Certified	Tier 4i	Tier 4	Tier 4i



	G25 (T4i)	G25 (T4F)	G25 (T4i) 600V- Canada Only
Displacement in ³	133	133	133
Operating performance SAE J1995, ISO 8528-1 hp	35.4	31.5	35.4
at rpm rpm	1,800	1,800	1,800
Coolant Capacity US qt	11.9	13.3	11.9
Engine oil Capacity US qt	8.5	11	8.5
Fuel type	Diesel	#1/#2 ULSD	Diesel
Fuel consumption (Prime Load) US gal/h	1.8	1.8	1.8
Tank capacity -Fuel US gal	58.4	58.4	58.4
Quantity (Battery)	1	1	1
Battery V	12	12	12
Generator model	Mecc Alte, Brushless w/AVR	Mecc Alte, Brushless	Mecc Alte, Brushless w/AVR
Voltage control No Load to Full Load +/- %	1	1	1
Voltage control Steady State +/- %	0.2	0.2	0.2
Generator insulation (class)	H	H	H
Frequency (Generator Speed) Hz	60	60	60
Runtime at 100% Prime Load h	32.2	28.2	32.2
Runtime at 75% Prime Load h	24.15		

Please note

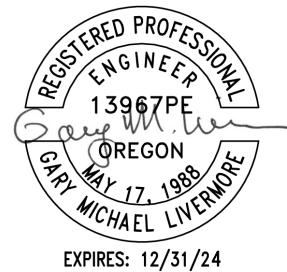
that product availability can vary from country to country. It is possible that information / products may not be available in your country. More detailed information on engine power can be found in the operator's manual; the stated power may vary due to specific operating conditions.

Subject to alterations and errors excepted. Applicable also to illustrations.

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Occupancy Change for Rodda Paint, Portland, Oregon

Structural Calculations:
Generator Attachment 1-7



Revision
03/16/23
Project Number: 220090.00



Generator

Seismic, ASCE 7-16

$$S_{DS} := 0.663$$

13.3, Table 13.5-1 Generators, batteries, ...

$$a_p := 1 \quad R_p := 2.5 \quad \Omega := 2$$

13.1.3

$$I_p := 1.0$$

13.3.1

$$F_p := \frac{0.4 \cdot a_p \cdot S_{DS}}{\left(\frac{R_p}{I_p}\right)} \cdot (1 + 2 \cdot 0) = 0.11$$

$$F_{p_max} := 1.6 \cdot S_{DS} \cdot I_p = 1.06$$

$$F_{p_min} := 0.3 \cdot S_{DS} \cdot I_p = 0.2 \quad \text{Governs}$$

$$\boxed{F_p} := F_{p_min} = 0.2$$

Generator

$$W_p := 2373 \text{ lb} \quad X := 50 \cdot \text{in} \quad Y := 33 \text{ in} \quad Z := 24 \text{ in}$$

$$\bar{F}_p := F_p \cdot W_p = 472 \text{ lb}$$

Overturning

$$\text{LC: } 0.9D - Ev + Eh = > [0.9 - 0.2(0.663)]D + E = > 0.77D + E$$

$$M_{ot} := F_p \cdot Z = 944 \text{ lb} \cdot \text{ft}$$

$$M_{res} := 0.77 \cdot \left(W_p \cdot \frac{Y}{2} \right) = 2512 \text{ lb} \cdot \text{ft}$$

$$\frac{M_{ot}}{M_{res}} = 0.38 < 1.0, \text{ no overturning}$$

Soil Bearing

$$\text{LC: } 1.0D + 0.7Ev + 0.7Eh = > [1.0 + 0.7(0.2)0.663]D + 0.7E = > 1.09D + 0.7E$$

$$M := 0.7 F_p \cdot Z = 661 \text{ lb} \cdot \text{ft}$$

$$P := 1.09 W_p = 2587 \text{ lb}$$

$$e := \frac{M}{P} = 3 \text{ in} < Y/6 = 10.5 \text{ in}$$

$$SB := \frac{\frac{P}{Y \cdot X} + \frac{6 \cdot P \cdot e}{Y^2 \cdot X}}{2} = 176 \frac{\text{lb}}{\text{ft}^2} < 1500 \text{ psf}$$

Anchorage

$$\text{LC: } 0.9D - Ev + Eh = > [0.9 - 0.2(0.663)]D + 2E = > 0.77D + 2E$$

$$TC := \left(\frac{M_{ot} \cdot \Omega}{Y \cdot 2} \right) = 343 \text{ lb} \quad \text{tension force, divide by 2 anchors}$$

$$\bar{P} := 0.77 \frac{W_p}{4} = 457 \text{ lb} \quad \text{divide by 4 locations}$$

$$T := P - TC = 114 \text{ lb} \quad \text{uplift}$$

$$V := \frac{F_p}{4} = 118 \text{ lb} \quad \text{resisted by 4 anchors}$$

--by calculation use Simpson Strong-Tie 1/2" dia.
Titen HD, nominal height 4"



Anchor Designer™
Software
Version 3.1.2301.3

Company:		Date:	3/16/2023
Engineer:		Page:	3
Project:			
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-14
Units: Imperial units

Anchor Information:

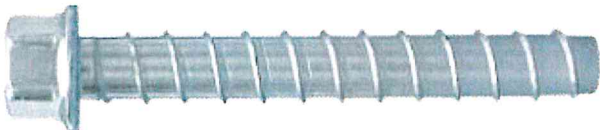
Anchor type: Concrete screw
Material: Carbon Steel
Diameter (inch): 0.500
Nominal Embedment depth (inch): 4.000
Effective Embedment depth, h_{ef} (inch): 2.990
Code report: ICC-ES ESR-2713
Anchor category: 1
Anchor ductility: No
 h_{min} (inch): 6.25
 C_{ac} (inch): 4.50
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 12.00
State: Cracked
Compressive strength, f'_c (psi): 4500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental edge reinforcement: Not applicable
Reinforcement provided at corners: No
Ignore concrete breakout in tension: No
Ignore concrete breakout in shear: No
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Recommended Anchor

Anchor Name: Titen HD® - 1/2"Ø Titen HD, h_{nom} : 4" (102mm)
Code Report: ICC-ES ESR-2713





Anchor Designer™ Software

Version 3.1.2301.3

Company:		Date:	3/16/2023
Engineer:		Page:	4
Project:			
Address:			
Phone:			
E-mail:			

Load and Geometry

Load factor source: ACI 318 Section 5.3

Load combination: not set

Seismic design: Yes

Anchors subjected to sustained tension: Not applicable

Ductility section for tension: 17.2.3.4.3 (d) is satisfied

Ductility section for shear: 17.2.3.5.3 (c) is satisfied

Ω_0 factor: not set

Apply entire shear load at front row: No

Anchors only resisting wind and/or seismic loads: Yes

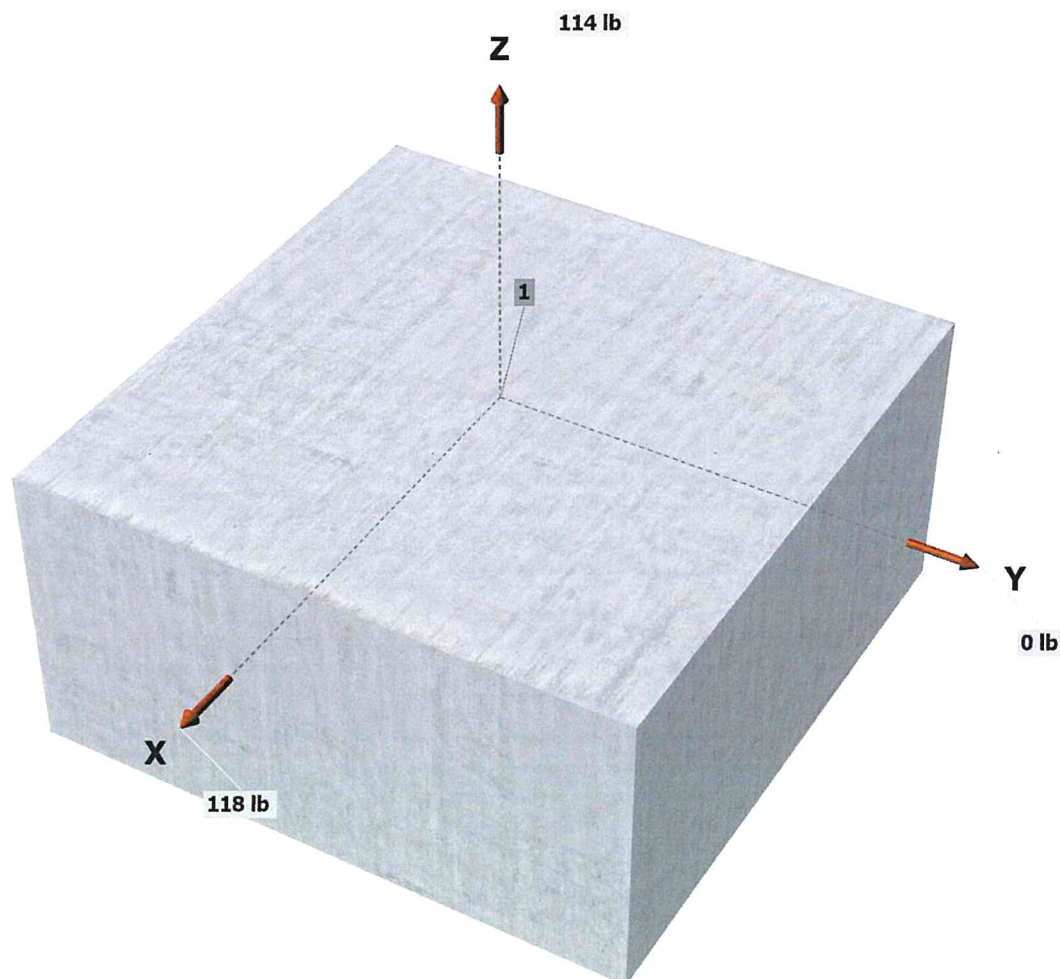
Strength level loads:

N_{ua} [lb]: 114

V_{uax} [lb]: 118

V_{uay} [lb]: 0

<Figure 1>



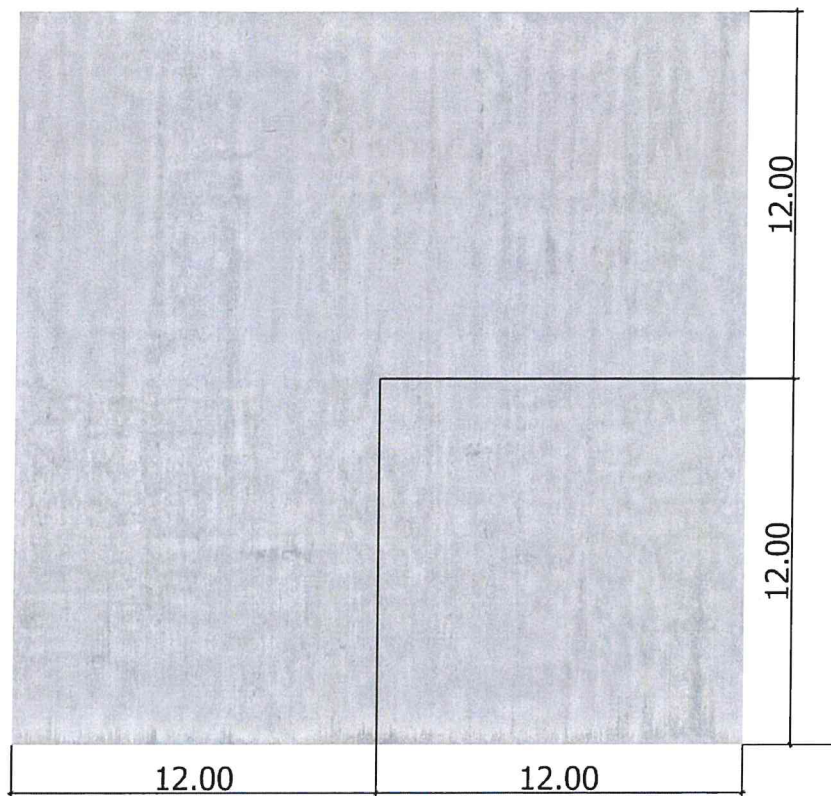
Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:		Date:	3/16/2023
Engineer:		Page:	5
Project:			
Address:			
Phone:			
E-mail:			

<Figure 2>



Company:		Date:	3/16/2023
Engineer:		Page:	6
Project:			
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, √(V _{uax}) ² +(V _{uay}) ² (lb)
1	114.0	118.0	0.0	118.0
Sum	114.0	118.0	0.0	118.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 114

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis, e'_{nx} (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e'_{ny} (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e'_{vx} (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e'_{vy} (inch): 0.00

4. Steel Strength of Anchor in Tension (Sec. 17.4.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
20130	0.65	13085

5. Concrete Breakout Strength of Anchor in Tension (Sec. 17.4.2)

$N_b = k_c \lambda_a \sqrt{f'_c} h_{ef}^{1.5}$ (Eq. 17.4.2.2a)

k _c	λ _a	f' _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	4500	2.990	5896

$0.75 \phi N_{cb} = 0.75 \phi (A_{Nc} / A_{Nco}) \Psi_{ed,N} \Psi_{c,N} \Psi_{cp,N} N_b$ (Sec. 17.3.1 & Eq. 17.4.2.1a)

A _{Nc} (in ²)	A _{Nco} (in ²)	C _{a,min} (in)	Ψ _{ed,N}	Ψ _{c,N}	Ψ _{cp,N}	N _b (lb)	φ	0.75φN _{cb} (lb)
80.46	80.46	12.00	1.000	1.00	1.000	5896	0.65	2874

8. Steel Strength of Anchor in Shear (Sec. 17.5.1)

V _{sa} (lb)	φ _{grout}	φ	φ _{grout} φV _{sa} (lb)
4790	1.0	0.60	2874

9. Concrete Breakout Strength of Anchor in Shear (Sec. 17.5.2)

Shear perpendicular to edge in x-direction:

$V_{bx} = \min[7(l_e / d_a)^{0.2} \sqrt{d_a \lambda_a} \sqrt{f'_c} C_{a1}^{1.5}; 9 \lambda_a \sqrt{f'_c} C_{a1}^{1.5}]$ (Eq. 17.5.2.2a & Eq. 17.5.2.2b)

l _e (in)	d _a (in)	λ _a	f' _c (psi)	C _{a1} (in)	V _{bx} (lb)
2.99	0.500	1.00	4500	8.00	10744

$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \Psi_{ed,V} \Psi_{c,V} \Psi_{h,V} V_{bx}$ (Sec. 17.3.1 & Eq. 17.5.2.1a)

A _{Vc} (in ²)	A _{Vco} (in ²)	Ψ _{ed,V}	Ψ _{c,V}	Ψ _{h,V}	V _{bx} (lb)	φ	φV _{cbx} (lb)
288.00	288.00	1.000	1.000	1.000	10744	0.70	7521

Shear parallel to edge in y-direction:

$V_{bx} = \min[7(l_e / d_a)^{0.2} \sqrt{d_a \lambda_a} \sqrt{f'_c} C_{a1}^{1.5}; 9 \lambda_a \sqrt{f'_c} C_{a1}^{1.5}]$ (Eq. 17.5.2.2a & Eq. 17.5.2.2b)

l _e (in)	d _a (in)	λ _a	f' _c (psi)	C _{a1} (in)	V _{bx} (lb)
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Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Anchor Designer™ Software

Version 3.1.2301.3

Company:		Date:	3/16/2023
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Address:			
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2.99	0.500	1.00	4500	8.00	10744		
$\phi V_{cbv} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx}$ (Sec. 17.3.1, 17.5.2.1(c) & Eq. 17.5.2.1a)							
A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbv} (lb)
288.00	288.00	1.000	1.000	1.000	10744	0.70	15042

10. Concrete Pryout Strength of Anchor in Shear (Sec. 17.5.3)

$$\phi V_{cp} = \phi k_{cp} N_{cb} = \phi k_{cp} (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \quad (\text{Sec. 17.3.1 \& Eq. 17.5.3.1a})$$

k_{cp}	A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕV_{cp} (lb)
2.0	80.46	80.46	1.000	1.000	1.000	5896	0.70	8254

11. Results

Interaction of Tensile and Shear Forces (Sec. 17.6)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	114	13085	0.01	Pass	
Concrete breakout	114	2874	0.04	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	118	2874	0.04	Pass (Governs)	
T Concrete breakout x+	118	7521	0.02	Pass	
Concrete breakout y+	118	15042	0.01	Pass	
Pryout	118	8254	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. 17.6.2	0.00	0.04	4.1%	1.0	Pass

1/2"Ø Titen HD, hnom:4" (102mm) meets the selected design criteria.

12. Warnings

- Per designer input, ductility requirements for tension have been determined to be satisfied – designer to verify.
- Per designer input, ductility requirements for shear have been determined to be satisfied – designer to verify.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



CITY OF
PORTLAND, OREGON
 BUREAU OF FIRE & RESCUE
BUREAU OF DEVELOPMENT SERVICES
 1900 SW 4TH Avenue Suite 2100
 Portland, Oregon 97201

FIRE SAFETY PLAN REVIEW CHECKSHEET

Application #: **21-042315-DFS-01-CO**

Commercial Building Permit

Review Date: March 2, 2023

To:	PWP CONTRACT OR	JH KELLY LLC *JESSIE REEVES* JH KELLY LLC 821 3rd Ave Longview, WA 98632	Work	(360) 423-5510
			Home	(503) -
			Email	jreeves@jhkelly.com
From:	Fire Inspector	Joe Thornton	Phone/Cell	503-209-5977
			Email	Joe.Thornton@portlandoregon.gov
cc:	OWNER	PORT OF PORTLAND 6107 N MARINE DR #3 PORTLAND, OR 97203-6409		

PROJECT INFORMATION

Street Address:	6123 N MARINE DR
Description of Work:	SINGLE PDF - DFS FOR GENERATOR & ITS FOUNDATION & ATTACHMENTS

The following items are in apparent violation of the specific codes or laws noted.

Item #	Location on plans	Code Section	Clarification / Correction Required Please submit a written plan of correction for each review comment. If major revisions are required, revised plans for that portion may be necessary to resubmit. All comments should be responded to in the drawings (not just the written plan of correction) unless otherwise indicated. Please indicate where in the drawings the requested information may be found. Please cloud or highlight changes to the plan set for identification of corrections made.
1.	S101	City of Portland, Title 31	Application for separate permits shall be obtained from the Fire Marshal's Office, 1300 SE Gideon Street, prior to the installation of the following: fire sprinklers, fire alarm systems, fire pumps, underground fire lines, fixed extinguishing systems, in-building radio enhancement systems, stationary generators and hazardous material tanks and related equipment. Please list the separate permit from the Fire Marshal's Office for the hazardous materials tank (diesel) permit.
2.	S101	PFC 312	Guard posts shall comply with all of the following requirements: 1. Constructed of steel not less than 4 inches (102 mm) in diameter and concrete filled. 2. Spaced not more than 4 feet (1219 mm) between posts on center. 3. Set not less than 3 feet (914 mm) deep in a concrete footing of not less than a 15-inch (381 mm) diameter. 4. Set with the top of the posts not less than 3 feet (914 mm) above ground. 5. Located not less than 3 feet (914 mm) from the protected object.

FIRE SAFETY PLAN REVIEW CHECKSHEET	Application #	21-042315-DFS-01-CO
	Review Date:	March 2, 2023

			Please show bollards installed in accordance with this code section. Please provide a section view detail of the bollard installation.
3.	S101	PFC 5704.2.7.3.1 <i>No Action Required at this time. Information only</i>	The drawings do not show the covered outdoor storage area to the east as indicated in the reference only approved set. The tank vents are required to be 5' from building openings including this outdoor storage area if is open on the west side. This will be vetted with the FS permit for the hazardous materials tank requested above in comment #1.
4.		PFC 105.4.1.1	<u>Examination of Documents</u> Additional clarification and/or corrections may be required upon further review and may appear as new items in subsequent review comments.

To respond to this Checksheet, you may need to revise your plans, your supporting documents, or provide additional information. Please see BDS website directions located here: <https://www.portland.gov/bds/permit-review-process/submitting-corrections>. When you finish with your changes, please submit your updated plans and supporting documents. Make sure to include the attached Checksheet Response Form. Visit the BDS Permit Review Process website for more helpful information and available services: <https://www.portland.gov/bds/permit-review-process>

If you want to report a delay, a regulatory conflict or other issue that you have been unable to resolve with your City review team, please visit <https://www.portland.gov/bds/development-permit-processes/report-problem>

If you have questions about this Checksheet, please contact me at the email address or phone number listed above. To check the status of your project, go to <https://www.portlandmaps.com/advanced/?action=permits>. Or you may request the status to be faxed to you, by calling 503.823.7000 and selecting option 4. Please have your IVR number and fax number available.

Appeals: Pursuant to City Code Chapters 31.10 and 28.03, you may appeal any code provision cited in this Checksheet to the Administrative Board of Appeals within 180 calendar days of the review date. For information on the appeals process, costs, including forms, appeal fee and payment methods, the following information is available: For Fire Code appeals go to www.portlandoregon.gov/fire/31187, call (503) 823-3712 or come in to the Fire Marshal's Office, 1300 SE Gideon St. For Building Code appeals go to www.portland.gov/bds/file-appeal or call (503) 823-7300 for assistance. If you have questions or are not sure if your appeal should be a Fire Code or Building Code appeal, please call the Fire Plan Reviewer listed above. Permit application expiration will not be extended pending resolution of any administrative appeal.

