







MiTek, Inc.

400 Sunrise Ave., Suite 270 Roseville, CA 95661 916.755.3571

Re: 23012314-A REVIVE NW

23-063170-DFS-01-RS

The truss drawing(s) referenced below have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Trus-way Inc.

Pages or sheets covered by this seal: R78835208 thru R78835210

My license renewal date for the state of Oregon is June 30, 2025.



October 14,2023

Johnson, Andrew

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job	Tru	SS	Truss Type	Qty	Ply	REVIVE NW	City of Portland
23012314-A	A01		GABLE	1	1		Reviewed for code compliance
Trus-Way, Vanc	ouver, WA	- 98661,		8	.720 s Au	Job Reference (optional) g 11 2023 MiTek Industries, Inc	Date: 01/25/24 Tue Oct 10 15:19:33 2023 Page 1
		- <u>1-0-</u> 0 1-0-0	<u>3-1-7 6-6-3 10-0-0</u> 3-1-7 3-4-13 3-5-13	ID:M5svgSOGNRI <u>13-5-13</u>	brUJv5xr9 16-10-9 3-4-13	J6yWqkg-P?5tBCVfdbVj0fmDR <u>20-0-0 21-0-0</u> 3-1-7 1-0-0	:Um9QFWVH2SeJKMSISPWIyUn6e
		100		6x8 =	0 4 10		Scale = 1:70.6
				F			
		6 1200 12 1200 12 6 6 6 7 8 9 0 3x	5x6 % 4x6 E 3x6 6x8 % C AN C C C C C C C C C C C C C	R Q P O 3x6 10x14 3x6 3x6	4x6 II H 3x6	اا 3x6 6x8 م ا AO ر 3x6 ل ا ا أ أ أ أ أ أ أ أ أ أ أ أ أ أ أ أ أ	
			6-6-3 6-8-0 10-0-0	13-4-0 13-5-1	3 2	20-0-0	
Plate Offsets (X,Y)	[B:0-0-5	,0-0-4], [C:0-1-5,0-0-8], [6-6-3 0-1 ¹ -13 3-4-0 [:0-1-5,0-0-8], [J:0-0-6,0-0-1], [P:0-3-0	3-4-0 0-1-1: 0,0-0-4], [T:0-3-0,0-0	3 0-13], [Y:(6-6-4)-1-9,0-1-0]	
LOADING (psf)	25.0	SPACING-	2-0-0 CSI .	DEFL.	in	(loc) l/defl L/d	PLATES GRIP
Snow (Pf/Pg) 20.8/3 TCDL	0.0	Plate Grip DOL Lumber DOL	1.15 TC 0.08 1.15 BC 0.06	Vert(LL) Vert(CT)	0.00	J n/r 120 K n/r 90	MT20 220/195
BCLL BCDL	0.0 * 0.0	Rep Stress Incr Code IRC2018/ ⁻	YES WB 0.07 TPI2014 Matrix-S	Horz(CT) 0.01	J n/a n/a	Weight: 243 lb FT = 8%
TOP CHORD 2x4 I C-F,I BOT CHORD 2x4 I O-U: WEBS 2x4 I OTHERS 2x4 I REACTIONS. All (lb) - Max Max	9F No.1&E F-I: 2x8 DF 9F No.1&E 2x12 DF Stud/Si 9F Stud	8tr G *Except* 5 SS 8tr G *Except* SS dd G (20-0-0. 81(LC 11) uplift 100 lb or less at jo 123(LC 12) reactions 250 lb or less ept B=438(LC 2), U=579	int(s) B, J, T, P except U=-123(LC 12 at joint(s) S, T, V, W, X, Q, P, N, M, L (LC 23), J=438(LC 2), O=570(LC 24)), , R	Structur Rigid ce 1 Brace	al wood sheathing directly ap eiling directly applied or 10-0- at Jt(s): Y	plied or 6-0-0 oc purlins. 0 oc bracing.
FORCES. (lb) - Ma TOP CHORD B-0 BOT CHORD B-3 R-5 L-M	 Comp./I =-454/38 =-64/345, =-59/292 =0/271, J =-402/10 	Max. Ten All forces 25(C-D=-317/55, D-E=-319 W-X=-64/345, V-W=-64 Q-R=-59/292, P-Q=-59/ -L=0/271 B-H-Q=-368/109) (Ib) or less except when shown. //133, G-H=-319/133, H-I=-273/33, I-J /345, U-V=-65/349, T-U=-64/297, S-T 292, O-P=-64/297, N-O=0/275, M-N=	l=-436/29 =-59/292, 0/271,			
NOTES- 1) Unbalanced roof li 2) Wind: ASCE 7-16; II; Exp B; Enclose 13-0-0, Exterior(2) exposed;C-C for n 3) Truss designed fo Gable End Details 4) TCLL: ASCE 7-16 DOL=1.15); Is=1.0 5) This truss has been non-concurrent wi 6) All plates are 2x4 7) Plates checked for 8) Gable requires coo 9) Gable requires coo 9) Gable requires coo 9) Gable requires coo 9) Gable studs space 10) This truss has been 11) * This truss has been 12) * This truss has been 13) * This truss has been 14) * This truss has been 15) * This truss has been 16) * This truss has been 17) * This truss has been 18) * This truss has been 19) * This truss has been 10) This truss has been 10) * This truss has been 11) * This truss has been 12) * This truss has been 13) * This truss has been 14) * This truss has been 15) * This truss has been 16) * This truss has been 17) * This truss has been 18) * This truss has been 19) * This truss has been 10) * This truss has been 10) * This truss has been 11) * This truss has been 12) * This truss has been 13) * This truss has been 14) * This truss has been 15) * This truss has been 16) * This truss has been 17) * This truss has been 17) * This truss has been 18) * This truss has been 18) * This truss has been 19) * This truss has been 19) * This truss has been 19) * This truss has been 10) * Th	ve loads h Vult=140 i; MWFRS i) 13-0-0 i embers a wind load as applic: Pr=25.0 ; Rough C n designe h other liv MT20 unle a plus or tinuous b d at 1-4-0 en design een desig e bottom (10.0 psf ty design pa	ave been considered for mph (3-second gust) Vas 6 (directional) and C-C C 0 18-0-0, Corner(3E) 18- nd forces & MWFRS for 1s in the plane of the trus able, or consult qualified posf (roof LL: Lum DOL=1 at B; Fully Exp.; Ce=0.9 d for greater of min roof e loads. ses otherwise indicated. minus 4 degree rotation ottom chord bearing. oc. ed for a 10.0 psf bottom ined for a live load of 20. chord and any other mer) on member(s). D-E, G-	this design. d=111mph; TCDL=4.2psf; BCDL=6.0 orner(3E) -1-0-0 to 2-0-0, Exterior(2N -0-0 to 21-0-0 zone; cantilever left and reactions shown; Lumber DOL=1.60 is only. For studs exposed to wind (n building designer as per ANSI/TPI 1. .15 Plate DOL=1.15); Pg=30.0 psf; PI ; Cs=1.00; Ct=1.10 ; Cs=1.00; Ct=1.10 ; ive load of 12.0 psf or 2.00 times flat about its center. chord live load nonconcurrent with an Opsf on the bottom chord in all areas nbers, with BCDL = 10.0psf. H, E-Y, G-Y; Wall dead load (5.0psf) N THIS AND INCLUDED MITEK REFERENCE P	Opsf; h=25ft; B=45ft;) 2-0-0 to 7-0-0, Coi d right exposed ; en- plate grip DOL=1.6(ormal to the face), s f=20.8 psf (Lum DO roof load of 20.8 ps hy other live loads. where a rectangle 3 on member(s).D-U	L=24ft; e mer(3R) f d vertical eee Stand L=1.15 P f on overf f on overf H-O 23 BEFORI	ave=2ft; Cat. 7-0-0 to left and right ard Industry late hangs by 2-0-0 wide	93437PE 93437PE 93437PE OREGON 979CH 13,2018 OREGON 979CH 13,2018 OREGON 970CH 14,2023
Design valid for use a truss system. Befo building design. Bra is always required fi fabrication, storage, and BCSI Building	only with Mi re use, the t cing indicate r stability an delivery, ere Componen	Tek® connectors. This design is suilding designer must verify the d is to prevent buckling of indiv d to prevent collapse with poss ction and bracing of trusses an t Safety Information availabl	s based only upon parameters shown, and is for applicability of design parameters and properly divala truss web and/or chord members only. Ac bible personal injury and property damage. For gr d truss systems, see ANSI/TPI1 Quality Criter e from the Structural Building Component Assor	r an individual building co / incorporate this design i dditional temporary and p general guidance regardii ria and DSB-22 availab ciation (www.sbcscompo	mponent, no nto the over ermanent b ng the le from Trus nents.com)	ot all racing s Plate Institute (www.tpinst.org)	400 Sunrise Ave., Suite 270 Roseville, CA 95661 916.755.3571 / MITGeL/US.com

						City of Doutlond
Job	Truss	Truss Type	Qty	Ply	REVIVE NW	City of Portland
23012314-A	A01	GABLE	1	1		Reviewed for code compliance
					Job Reference (optional)	Date: 01/25/24
Trus-Way, Vancouver,	WA - 98661,		8	.720 s Aug	11 2023 MiTek Industries, Inc	Tue Oct 10 15:19:34 2023 Page 2
		ID:M5svg	SOGNRIb	rUJv5xr9j6	yWqkg-tBfFPYWHOveZeoKP1	x??fide5FgNfiNfiaV4PCyF8YUnfea

NOTES-

13) A plate rating reduction of 20% has been applied for the green lumber members.

14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) B, J, T, P except (jt=lb) U=123, O=123.
15) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
16) Attic room checked for L/360 deflection.



Job		Truss	Truss Type			Qty	Ply	REVIVI	E NW		City of Po	rtland
23012314-A		A02	ATTIC			15	1				Reviewed for code	compliance
Trus-Way,	Vancouver,	WA - 98661,				8.7	720 s Aug	Job Ref 11 2023	erence (o 8 MiTek In	ptional) dustries, Inc	Date: 01/25/24 Tue Oct 10 15:19:35 202	3 Page 1
		1 <u>-0-0</u>	3-1-7 6-	<u>6-3 10-0-0</u>	ID:M5svg	9SOGNRIb -5-13	rUJv5xr9j 16-10-9	i6yWqkg 9	-MNDdcu 20-0-0	21-0-0	cZfWEEtKFf4gy6AieU3xW	PayUn6c
		1-0-0	3-1-7 3-4	-10 -0-10		5-15	3-4-13		3-1-7	1-0-0		Scale = 1:65 2
					6x8 —							00010 1.00.2
		т			F							
				6x6 1/		5x	β 📏					
			4:	6 E		G						
				D	N 2x4	× / /	M 4x0 II H					
		5.			244 11	A	\backslash					
		10	6x8 //					e	6x8 🔨			
		12.00 12	c	10-10				\mathbf{X}	× ×			
			U					// `				
		E B	/							L L		
			1	M		L	+		Ì			
		3x6	=	10x14 =		10x14	=		3x(6 =		
		F	6-6-3	<u>6-8-0</u> 0-1-13	13-4-0	13-5-7	1 <u>3</u>	20-0-0				
Plate Offsets ()	K,Y) [B:0	0-6-0,0-0-11], [J:0-6-0,0-0-1	1]			-	-					
TCLL (roof)	t) 25.0	SPACING- Plate Grip DOL	2-0-0 1.15	CSI. TC 0.17		DEFL. Vert(LL)	in -0.05	(loc) L-T	l/defl >999	L/d 240	PLATES MT20	GRIP 220/195
TCDL	20.8/30.0	Lumber DOL Rep Stress Inc	1.15 • YES	BC 0.25 WB 0.27		Vert(CT) Horz(CT)	-0.11 0.02	L-T J	>999 n/a	180 n/a		
BCDL	10.0	Code IRC2018	/TPI2014	Matrix-MS		Wind(LL)	-0.04	M-Q	>999	360	Weight: 162 lb	FT = 8%
LUMBER- TOP CHORD	2x4 DF No.	.1&Btr G *Except*			BRACIN TOP CH	G- ORD	Structura	al wood	sheathing	g directly ap	pplied or 5-7-9 oc purlins.	
BOT CHORD	C-F,F-I: 2x 2x4 DF No.	8 DF SS .1&Btr G *Except*			BOT CH	ORD	Rigid cei 1 Brace	iling dire at Jt(s):	ectly appli N	ied or 10-0-0	0 oc bracing.	
WEBS	L-M: 2x12 I 2x4 DF Stu	DF SS id/Std G										
REACTIONS.	(size)	B=0-5-8, J=0-5-8										
	Max Horz Max Uplift	B=-281(LC 10) B=-67(LC 12), J=-67(LC 12)	<u>2)</u>									
	Max Grav	B=1189(LC 23), J=1189(L	5 24)									
TOP CHORD) - Max. Con B-C=-144	np./Max. Ten All forces 2 40/63, C-D=-1316/74, D-E=	-858/132, G-H=-85	51 when shown. 3/132, H-I=-1316/74	, I-J=-1440)/63						
WEBS	B-M=0/11 C-M=-300	0/131, D-M=-3/583, E-N=-9	o 74/191, G-N=-974/	191, I-L=-301/131, H	I-L=-3/584							
NOTES-	na of live los	de heurs heers eensidered f	anthia design									
2) Wind: ASCE	7-16; Vult=	140mph (3-second gust) V	asd=111mph; TCDI	_=4.2psf; BCDL=6.0	psf; h=25f	t; B=45ft; I	_=24ft; ea	ave=4ft;	Cat.			
, Interior(1)	13-0-0 to 18-	-0-0, Exterior(2E) 18-0-0 to	21-0-0 zone; cantil	ever left and right ex	(posed ; er	nd vertical	left and r	ight	3-0-0		DED PROFE	
3) TCLL: ASCE	E 7-16; Pr=2	5.0 psf (roof LL: Lum DOL=	1.15 Plate DOL=1.	15); Pg=30.0 psf; P1	=20.8 psf	(Lum DOL	=1.15 Pla	ate			TENGINEE	5/0
4) This truss ha	as been desi ent with othe	igned for greater of min roc er live loads	f live load of 12.0 p	sf or 2.00 times flat	roof load o	f 20.8 psf	on overh	angs		PE	104	E
5) Plates check 6) This truss ha	ked for a plu	s or minus 4 degree rotatio	n about its center. chord live load non	concurrent with any	other live	loads					9343/PE	
7) * This truss will fit between the second sec	has been de en the botto	signed for a live load of 20 m chord and any other me	Opsf on the bottom nbers.	chord in all areas w	here a rec	tangle 3-6	-0 tall by	2-0-0 wi	ide	A	ritefter	yan
 8) Ceiling dead 9) Bottom chor 	l load (10.0 p d live load (4	psf) on member(s). D-E, G- 40.0 psf) and additional bot	H, E-N, G-N; Wall tom chord dead loa	dead load (5.0psf) c d (0.0 psf) applied c	on member only to roon	(s).D-M, H n. L-M	I-L			11	A OREGON	8//
10) A plate rati 11) Provide me	ng reduction echanical co	n of 20% has been applied t nnection (by others) of trus	or the green lumbe s to bearing plate c	r members. apable of withstandi	ng 100 lb ι	uplift at joir	nt(s) B, J.			P	ARCH 13,20	S
12) This truss i referenced	is designed i standard Al	in accordance with the 201 NSI/TPI 1	3 International Resi	dential Code sectior	ns R502.11	.1 and R8	02.10.2 a	and			REW JOHN	
13) Attic room	checked for	L/360 deflection.								F	VDIDEC: Jun 20 C	0.05
										E	October 14.2	023

e Institute (www.tpinst.org) Bank State St

Job		Truss	Truss Type	Qtv	Plv	REVIVE NW	City of Portland	
23012314-A		A03	GABLE	1	1		Reviewed for code compliance	
Trus-Way	Vancouver	NA - 98661			720 s Auc	Job Reference (optional)	Date: 01/25/24 Tue Oct 10 15:19:37 2023, Page 1	
nuo muy,	vanoouvor,	-1-0-	0 3-1-7 6-6-3 10-0-0	ID:M5svgSOGNRIb	UJv5xr9j6 16-10-9	yWqkg-ImKO1aYAgq08VG3_g	425199923#11908982781085379068	
		1-0-0	3-1-7 3-4-13 3-5-13	3-5-13	3-4-13	3-1-7 1-0-0		
				6x8 =			Scale = 1:70.6	
		Ţ		F				
		с с 12.00 12 с с ц с с с с в с с с с с с с с с с с с	5x6 // 4x6 II E 3x6 II 6x8 // C AN C AN C C AN C C AN C C C C C C C C C C C C C	Axe G V V V V V V V V V V V V V V V V V V	4x6 H 3x6 H 3x6 N	ا 3x6 II AO المح المح المح المح المح المح المح المح		
			3x6 6-6-3 6-8-0 10-0-0 6-6-3 0-1 ¹ 13 3-4-0	3x6 3x6 <u>13-4-0 13-5-1</u> <u>3-4-0 0-1¹-13</u>	<u>3 2</u> 3 (0-0-0		
Plate Offsets (X	(,Y) [B:0-	0-6,0-0-1], [C:0-1-5,0-0-8],	[I:0-1-5,0-0-8], [J:0-1-3,0-1-8], [P:0-3-	0,0-0-4], [T:0-3-0,0-0)-13], [Y:0	-1-9,0-1-0]		
LOADING (psf TCLL (roof)) 25.0	SPACING- Plate Grip DOL	2-0-0 CSI. 1.15 TC 0.08	DEFL. Vert(LL)	in 0.00	(loc) l/defl L/d J n/r 120	PLATES GRIP MT20 220/195	
Snow (Pf/Pg) 2 TCDL	20.8/30.0	Lumber DOL Rep Stress Incr	1.15 BC 0.06 YES WB 0.07	Vert(CT) Horz(CT	0.00	K n/r 90 J n/a n/a		
BCLL BCDL	0.0 * 10.0	Code IRC2018/	TPI2014 Matrix-S	1012(01	, 0.01	0 11/4 11/4	Weight: 243 lb FT = 8%	
TOP CHORD BOT CHORD WEBS OTHERS REACTIONS. (lb) -	LUMBER- TOP CHORD 2x4 DF No.1&Btr G *Except* C-F,F-I: 2x8 DF SS BOT CHORD 2x4 DF No.1&Btr G *Except* O-U: 2x12 DF SS WEBS 2x4 DF Stud/Std G OTHERS 2x4 DF Stud/Std G OTHERS 2x4 DF Stud/Std G REACTIONS. All bearings 20-0-0. (ib) - Max Horz B=281(LC 11) Max Uplift All uplift 100 lb or less at joint(s) B, J, T, P except U=-123(LC 12), O=-123(LC 12) Max Grav All reactions 250 lb or less at joint(s) S, T, V, W, X, Q, P, N, M, L, R							
FORCES. (Ib) TOP CHORD BOT CHORD	- Max. Com B-C=-454/ B-X=-64/3 R-S=-59/2 L-M=0/27	p./Max. Ten All forces 25 38, C-D=-317/55, D-E=-31: 45, W-X=-64/345, V-W=-64 92, Q-R=-59/292, P-Q=-59 1, J-L=0/271	0 (Ib) or less except when shown. 9/133, G-H=-319/133, H-I=-273/33, I- /345, U-V=-65/349, T-U=-64/297, S-7 /292, O-P=-64/297, N-O=0/275, M-N=	J=-436/29 =-59/292, 0/271,				
NOTES- 1) Unbalanced 2) Wind: ASCE II; Exp B; Em 13-0-0, Exter exposed;C-C 3) Truss design Gable End D 4) TCLL: ASCE DOL=1.15); J 5) This truss ha non-concurre 6) All plates are 7) Plates check 8) Gable require 9) Gable studs 10) This truss h 11) *	roof live load 7-16; Vult=1 closed; MWF closed; MWF closed; MWF ror(ZN) 13-0 C for member led for wind l retails as app s 5-10; Pr=25 s=1.0; Roug as been desig ent with othe e 2x4 MT20 t red for a plus es continuou spaced at 1- nas been des has been des has has has has has has has has has ha	Is have been considered fo 40mph (3-second gust) Va FRS (directional) and C-C O to 18-0-0, Corner(3E) 18 is and forces & MWFRS for oads in the plane of the tru- plicable, or consult qualified 0.0 psf (roof LL: Lum DOL=: h Cat B; Fully Exp.; Ce=0.5 gned for greater of min roof r live loads. or minus 4 degree rotation s bottom chord bearing. 4-0 oc. igned for a 10.0 psf bottom signed for a live load of 20 om chord and any other me psf) on member(s). D-E, G	this design. sd=111mph; TCDL=4.2psf; BCDL=6.1 orner(3E) -1-0-0 to 2-0-0, Exterior(2N -0-0 to 21-0-0 zone; cantilever left an reactions shown; Lumber DOL=1.60 ss only. For studs exposed to wind (r building designer as per ANSI/TPI 1. .15 Plate DOL=1.15); Pg=30.0 psf; P ; Cs=1.00; Ct=1.10 live load of 12.0 psf or 2.00 times flat about its center. chord live load nonconcurrent with an .0psf on the bottom chord in all areas mbers, with BCDL = 10.0psf. H, E-Y, G-Y; Wall dead load (5.0psf)	Dpsf; h=25ft; B=45ft;) 2-0-0 to 7-0-0, Cord d right exposed; end, plate grip DOL=1.60 iormal to the face), s f=20.8 psf (Lum DO roof load of 20.8 psf hy other live loads. where a rectangle 3 on member(s).D-U, PAGE MII-7473 rev. 1/2/20	L=24ft; e ner(3R) 7 d vertical ee Stand L=1.15 Pl f on overh -6-0 tall b H-0 23 BEFORE	ave=2ft; Cat. -0-0 to eft and right ard Industry ate angs y 2-0-0 wide	STERED PROFESSION 93437PE 93437PE OREGON NO NO NO NO NO NO NO NO NO NO NO NO N	
Design valid a truss system building design is always req fabrication, so and BCSI B	for use only with m. Before use, to gn. Bracing ind juired for stabilit torage, delivery uilding Compo	1 MiTek® connectors. This design he building designer must verify th cated is to prevent buckling of indi y and to prevent collapse with pose erection and bracing of trusses ar nent Safety Information availab	s based only upon parameters shown, and is for e applicability of design parameters and proper- dividual truss web and/or chord members only. A lible personal injury and property damage. For d truss systems, see ANSI/TPI1 Quality Crite le from the Structural Building Component Asso	r an individual building co y incorporate this design i dditional temporary and p general guidance regardir ria and DSB-22 availabl ciation (www.sbcscompor	mponent, no nto the over ermanent br ig the e from Truss nents.com)	it all acing s Plate Institute (www.tpinst.org)	400 Sunrise Ave., Suite 270 Roseville, CA 95661 916.755.3571 / MiTek-US.com	

						City of Doutlond
Job	Truss	Truss Type	Qty	Ply	REVIVE NW	City of Portiand
23012314-A	A03	GABLE	1	1		Reviewed for code compliance
					Job Reference (optional)	Date: 01/25/24
Trus-Way, Vancouver,	WA - 98661,		8	.720 s Aug	11 2023 MiTek Industries, Inc	Tue Oct 10 15:19:37 2023 Page 2
		ID:M5svg	SOGNRIb	rUJv5xr9j6	yWqkg-ImKO1aYAgq08VG3	427,100027#120299827790768
NOTES-		-				

13) A plate rating reduction of 20% has been applied for the green lumber members.

14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) B, J, T, P except (jt=lb) U=123, O=123.
15) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
16) Attic room checked for L/360 deflection.









General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- 1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- . Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 5. Cut members to bear tightly against each other
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- 11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient
- 20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- The design does not take into account any dynamic or other loads other than those expressly stated.





MiTek, Inc.

400 Sunrise Ave., Suite 270 Roseville, CA 95661 916.755.3571

Re: 23012317-A REVIVE NW 23-063180-DFS-01-RS

The truss drawing(s) referenced below have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Trus-way Inc.

Pages or sheets covered by this seal: R78835328 thru R78835332

My license renewal date for the state of Oregon is June 30, 2025.



October 14,2023

Johnson, Andrew

IMPORTANT NOTE: The seal on these truss component designs is a certification that the engineer named is licensed in the jurisdiction(s) identified and that the designs comply with ANSI/TPI 1. These designs are based upon parameters shown (e.g., loads, supports, dimensions, shapes and design codes), which were given to MiTek or TRENCO. Any project specific information included is for MiTek's or TRENCO's customers file reference purpose only, and was not taken into account in the preparation of these designs. MiTek or TRENCO has not independently verified the applicability of the design parameters or the designs for any particular building. Before use, the building designer should verify applicability of design parameters and properly incorporate these designs into the overall building design per ANSI/TPI 1, Chapter 2.

Job	Truss	Truss Type	Qtv Pl	V REVIVE NW	City of Portland
23012317-A	A01	GABLE	1	1	Reviewed for code compliance
Trus-Way Vancouver	WA - 98661		8 72(Job Reference (optional) 0 s Aug 11 2023 MiTek Industries Inc	Date: 01/25/24 Tue Oct 10 15:26:32 2023 Page 1
nuo muj, tunouton,	1-6-0.	3-1-6 6-6-3 10-9-0	ID:RRxF4lpSXe3Wu329 14-11-12	9MN0hivyUrAS-XIp4RYK?yIshcNUkG 18-4-10 21-6-0 23-0-0	v229x64vfti&7_961170R948v9UR05
	1-6-0	3-1-6 3-4-13 4-2-13	4-2-13	3-4-13 3-1-6 1-6-0	
			6x8 =		Scale = 1:73.3
			F		
	€2 12.00 [12]	5x6 // 8x8 // 4x6 II 3x6 II 6x8 // C		4x6 H 3x6 6x8	
	E B GA			κığ	
	3x6	S = X W V U T S	R Q P O	N M L 3x6 🛨	
		10x14 — 3x6 3x6	3x6 3x6	_	
		6-6-3 6-8-0 10-9-0	14-10-0 14-11-1	12 21-6-0	
Plate Offsets (X,Y) [B:0	⊦ 2-6-6,0-0-12], [J:0-6-4,0-0-12	6-6-3 0-1 ⁻¹ -13 4-1-0], [Y:0-1-9,0-1-0], [AA:1-4-8,0-2-3], [AI	<u>4-1-0</u> 0-1-13 :1-4-8,0-2-3]	3 6-6-4	
LOADING (psf)	SPACING-	2-0-0 CSI .	DEFL	in (loc) l/defl l/d	PLATES GRIP
TCLL (roof) 25.0 Snow (Pf/Pg) 20.8/30.0	Plate Grip DOL	1.15 TC 0.17	Vert(LL)	-0.01 K n/r 120	MT20 220/195
TCDL 10.0 BCLL 0.0	* Rep Stress Incr	YES WB 0.08	Horz(CT)	0.01 J n/a n/a	
BCDL 10.0	Code IRC2018/	I PI2014 Matrix-S			Weight: 259 lb F I = 8%
LUMBER- TOP CHORD 2x4 DF No	.1&Btr G *Except*		BRACING- TOP CHORD SI	tructural wood sheathing directly ap	plied or 6-0-0 oc purlins.
C-F,F-I: 2x BOT CHORD 2x4 DF No	8 DF SS 0.1&Btr G *Except*		BOT CHORD R JOINTS 1	Rigid ceiling directly applied or 10-0- Brace at Jt(s): Y) oc bracing.
O-U: 2x12 WEBS 2x4 DF Stu	DF SS ud/Std G				
OTHERS 2x4 DF Stu	ud/Std G				
REACTIONS. All bearin	rgs 21-6-0.				
(ib) - Max Horz Max Uplift	All uplift 100 lb or less at jo	int(s) X, L except B=-105(LC 12),			
Max Grav	All reactions 250 lb or less	at joint(s) S, T, V, W, X, Q, P, N, M, L	, R		
	except B=552(LC 2), U=622	2(LC 23), J=552(LC 2), O=609(LC 24)			
FORCES. (Ib) - Max. Cor TOP CHORD B-C=-538	mp./Max. Ten All forces 25 8/21, C-D=-400/40, D-E=-41	0 (lb) or less except when shown. 5/137, G-H=-415/137, H-I=-367/15, I-J	=-525/0		
BOT CHORD B-X=-54/ R-S=-50/	/410, W-X=-54/410, V-W=-54 /333, Q-R=-50/333, P-Q=-50	/410, U-V=-56/414, T-U=-53/336, S-T /333, O-P=-53/336, N-O=0/345, M-N=	=-50/333, D/341,		
L-M=0/34 WEBS D-U=-419	41, J-L=0/341 9/141, H-O=-391/141				
NOTES- 1) Unbalanced roof live loa 2) Wind: ASCE 7-16; Vult= II; Exp B; Enclosed; MW 13-9-0, Exterior(2N) 13- exposed;C-C for member 3) Truss designed for wind	ads have been considered fo 140mph (3-second gust) Va /FRS (directional) and C-C C 9-0 to 20-0-0, Corner(3E) 20 ers and forces & MWFRS for I loads in the plane of the true	this design. sd=111mph; TCDL=4.2psf; BCDL=6.0 orner(3E) -1-6-0 to 1-6-0, Exterior(2N -0-0 to 23-0-0 zone; cantilever left and reactions shown; Lumber DOL=1.60 p ss only. For studs exposed to wind (no	psf; h=25ft; B=45ft; L≕) 1-6-0 to 7-9-0, Corner I right exposed ; end ve olate grip DOL=1.60 ormal to the face), see	24ft; eave=2ft; Cat. r(3R) 7-9-0 to ertical left and right Standard Industry	STERED PROFESSION ENGINEER 93437PE
4) TCLL: ASCE 7-16; Pr=2	25.0 psf (roof LL: Lum DOL=	.15 Plate DOL=1.15); Pg=30.0 psf; Pf	=20.8 psf (Lum DOL=1	1.15 Plate	milling
DOL=1.15); Is=1.0; Rou 5) This truss has been des	igh Cat B; Fully Exp.; Ce=0.9 igned for greater of min roof	; Cs=1.00; Ct=1.10 live load of 12.0 psf or 2.00 times flat i	roof load of 20.8 psf on	n overhangs	4 OREGON 8
non-concurrent with oth 6) All plates are 2x4 MT20	er live loads. unless otherwise indicated.			NY.	RCH 13,20 CO
 Plates checked for a plu Gable requires continuo 	is or minus 4 degree rotation ous bottom chord bearing.	about its center.			REW JOHN
9) Gable studs spaced at 1 10) This truss has been de	I-4-0 oc. esigned for a 10.0 psf bottom	chord live load nonconcurrent with an	v other live loads.	_	
11) * This truss has been of will fit between the both	designed for a live load of 20	Opsf on the bottom chord in all areas	where a rectangle 3-6-	0 tall by 2-0-0 wide	XPIRES: Jun 30, 2025 October 14 2023
വില്യാവിക്കുന്നത്തിന്നെ നല്ലാവി (10.0	D psf) on member(s). D-E, G	H, E-Y, G-Y; Wall dead load (5.0psf)	on member(s).D-U, H-	-0	
WARNING - Verify desi	gn parameters and READ NOTES C	N THIS AND INCLUDED MITEK REFERENCE P	AGE MII-7473 rev. 1/2/2023 E	BEFORE USE.	
a truss system. Before use building design. Bracing in is always required for stabil	, the building designer must verify th dicated is to prevent buckling of indi- lity and to prevent collapse with poss	e applicability of design parameters and properly vidual truss web and/or chord members only. Ac ible personal injury and property damage. For g	incorporate this design into the distance regarding the distance reg	he overall banent bracing he	400 Sunrise Ave., Suite 270

fabrication, storage, delivery, erection and bracing of trusses and truss systems, see ANSI/TP11 Quality Criteria and DSB-22 available from Truss Plate Institute (www.tpinst.org) and BCSI Building Component Safety Information available from the Structural Building Component Association (www.sbcscomponents.com)

400 Sunrise Ave., Suite 270 Roseville, CA 95661 916.755.3571 / MiTek-US.com

						City of Doutlond
Job	Truss	Truss Type	Qty	Ply	REVIVE NW	City of Portland
23012317-A	A01	GABLE	1	1		Reviewed for code compliance
					Job Reference (optional)	Date: 01/25/24
Trus-Way, Vancouver,	WA - 98661,		8	.720 s Aug	11 2023 MiTek Industries, Inc	Tue Oct 10 15:26:32 2023 Page 2
		ID:RRxF4	lpSXe3Wu	329MN0hi	vyUrAS-Xlp4RYK?ylshcNUkG	v229x64vft1A7-96717CRS48v9UR65
NOTES-						

13) A plate rating reduction of 20% has been applied for the green lumber members.

14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) X, L except (jt=lb) B=105, U=123, J=105, O=123.
 15) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 16) Attic room checked for L/360 deflection.



All and the set of the se	lah		Truco	Truce Ture	0.5	Div		City of Portland
Zeitz 1.4 jac jartic is jack Harden in the set of th	JOD		Truss		Qly	Piy	REVIVE NW	Reviewed for code compliance
Thurship, Vancouve: WA-9881. 572 # 4.01 1202 Mit inclusion: During the Construction of the Construction o	23012317-A		A02	ATTIC	15	1	Job Reference (optional)	Date: 01/25/24
u = transformer (Step 1 and 1	Trus-Way,	Vancouver,	WA - 98661, <mark>-1-6-0</mark> - 1-6-0	<u>3-1-6 6-6-3 10-9-0</u> 3-1-6 3-4-13 4-2-13	8 ID:RRxF4lpSXe3 14-11-12 4-2-13	.720 s Aug Wu329MN + 18-4- 3-4-1	11 2023 MiTek Industries, Ind 0hivyUrAS-TgxqsELFUv6Psgr 10 21-6-0 23-0-0 13 3-1-6 1-6-0	Tue Oct 10 15:26:34 2023 Page 1 รงใช้วัตรุษธิญญาติจากระบบการเรื่องให้เรื่องให้เรื่องให้เรื่องให้เรื่องให้เรื่องให้เรื่องให้เรื่องให้เรื่องให้เร
Junc								Scale = 1:71.4
Image: State of the state					6x8 —			- 1.7 1. 1
1 1			1.2.3	4x6 % 5x6 % E 4x6 U D 6x8 %	F 4x N 2x4	6 % 5x6 % 4x6 H	 6x8 \\	
Bit Control			12.00 12	C C	[
$ \begin{array}{c c c c c c c c c c c c c c c c c c c $			¯ °A ∕ ₿	M		L	8 3	
Paile 0:6-3 0:6-10 14:112 21:6-0 Paile 0:64:3 0:47:3 0:4-4 0:4-4 0:4-4 ChODING (righ) 25:0 Paile 0:67:0:27:0.2-51 [S:0-2-10,2-20, L0:0-6.0,0-1:5] 0:4-4 0:4-4 PLATES GRIP TCUL (root) 25:0 Paile 0:67:0:27:0:27 Vert(11) -0.07 L >999 240 MT20:0:0:0:0:0:0:0:0:0:0:0:0:0:0:0:0:0:0:			2x6	= 10x14 =	10	x14 =	3x6 = ∨	
Plate Offsets (X/Y)- (Bit) 6-0.0-0-15[, [Ei0-2-7.0-2-5], [Gi0-2-13,0-2-0], [J0-6-0,0-0-15] CU1			F	<u>6-6-3 6-8-0</u> 6-6-3 0-1-13	<u>14-10-0</u> 14- 8-2-0 0-	11-12	21-6-0	
LOADING (InF) TCLL (root) 25.0 Pinte Grip DoL 1.15 DCLL (root) 25.0 Pinte Grip DoL 1.15 DCL 1.00 CCLL 0.00 CCL 0.00 CCL 0.00 CCL 0.00 CCL 0.00 CCL 0.00 CCL 0.00 CCL 0.00 CCL 0.00 CCL 0.00 CCL 0.00 CCL 0.00 CCL 0.00 CCL 0.00 CCL 0.00 CCL 0.00 CCL 0.00 CCL 0.00 CCL 0.00 CCL CCL 0.00 CCL 0.00 CCL CCL 0.00 CCL 0.00 CCL 0.00 CCL 0.00 CCL 0.00 CCL CCL 0.00 CCL CCL 0.00 CCL CCL 0.00 CCL CCL CCL CCL CCL CCL CCL C	Plate Offsets (2	X,Y) [B:0	-6-0,0-0-15], [E:0-2-7,0-2-5]	, [G:0-2-13,0-2-0], [J:0-6-0,0-0-15]	020 0	1 10	004	
BCUL 10.0 Image: Control of the second	LOADING (ps TCLL (roof) Snow (Pf/Pg) TCDL BCLL	f) 25.0 20.8/30.0 10.0 0.0 *	SPACING- Plate Grip DOL Lumber DOL Rep Stress Incr Code IRC2018/	2-0-0 CSI. 1.15 TC 0.27 1.15 BC 0.25 YES WB 0.33 TPI2014 Matrix-MS	DEFL. Vert(LL) Vert(CT) Horz(CT Wind(LL	in -0.07 -0.15) 0.03) -0.05	(loc) l/defl L/d L >999 240 L-T >999 180 J n/a n/a M-Q >999 360	PLATES GRIP MT20 220/195 Weight: 179 lb FT = 8%
TOP CHORD CHORD C CHORD 2/4 DF No 18Br G *Except* L-M: 2x1 DF No 18Br G *Except* L-M: 2x1 DF No 18Br G *Except* L-M: 2x1 DF S TOP CHORD SUBJECT NOR DOINTS Structural wood sheathing directly applied or 54-4 oc purtins. BOT CHORD Val DF No 18Br G *Except* L-M: 2x1 DF S WEBS 2x4 DF Stud/Std G REACTIONS. (size) B=0-5-8, 1-0-5-8 Max Horz B=13(LC 11) Max torg B=13(LC 23), J=1341(LC 24) FORCES. (b) - Max. Comp.Max. Ten - All forces 250 (b) or less except when shown. DOP CHORD B-C-1601/42, C-D=1461/54, D=E=95/139, CH==95/139, H==-1460/54, LJ==-1600/42 BOT CHORD B-C-401/42, C-D=-1461/54, D=E=95/2139, CH==95/2139, H==-1460/54, LJ==-1600/42 BOT CHORD B-C-401/123, L-M=01/030, J==01122 WEBS VBES C-M=-3671/126, D-M=0682, E-M=-1222/212, G-M==-1222/212, G-M==-160/42 VIDES 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vullet140mph (3-second gust) Vasd=111mph; TCDL=4.2ps; BCDL=6.0ps; h=261; B=461; L=241; eave=41; Cat. II: Exp B: Enclosed; MWFRS (directional) and C-C Exterior(2E) -1-640 to 1-6-0, Interior(1) 1-6-00 to 7-9-0, Exterior(2R) 7-9-0 to 13-9-0, Interior(1) 1-9-0 to 7-9	LUMBER-	10.0			BRACING-	,		
 REACTIONS. (size) B=0-5-8, J=0-5-8 Max Horz B=313(LC 11) Max Upith B=-89(LC 12), J=-89(LC 12) Max Grav B=1341(LC 23), J=1341(LC 24) FORCES. (lb) - Max Comp./Max Ten All forces 250 (lb) or less except when shown. TOP CHORD B-C=-1601/42, C-D=-1461/54, D-E=-952/139, H-I=-1460/54, I-J=-1600/42 BOT CHORD B-M=-01/23, L-M=01/030, J=1=0/1122 WEBS C-M=-367/126, D-M=0/682, E-N=-1222/212, G-N=-1222/212, I-L==-368/126, H-L=0/662 NOTES. 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vull=140mph (3-second gust) Vasd=111mph; TCDL=4.2psf; BCDL=6.0psf; h=25f; B=45f; L=24f; eave=4f; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 7-9.0, Exterior(2F) 20-0-0 to 2-0-0-200; catilitever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 psf; h=2208 psf (Lum DOL=1.15 Piate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; C==1.00; CI=1.10 3) TCLL: ASCE 7-16; PV2D opt foro ZLL: Lum DOL=1.15); Pg=3.0.0 psf, Pf=20.8 psf (Lum DOL=1.15 Piate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; C==1.00; CI=1.10 4) This truss has been designed for a nive load of 12.0 psf or 2.00 times flat roof load of 20.8 psf on overhangs non-concurrent with other live loads. 5) Paltes checked for a pius or minus 4 degree rotation about its center. 6) This truss has been designed for a lou of 20.0 psf on thord live load of 12.0 psf or 2.00 times flat roof load of 2.0.8 psf on overhangs non-concurrent with other live loads. 7) This truss has been designed for a lou of 20.0 psf on thord live load on (0.0 psf) applied only to room. L-M 10) Aplate rating reduction of 20% has been applied for the green lumber members. 10) Ecling data load (10.0 psf) ont member(9). D-K, G-H; Wall dead load (5.0psf) on member(9).D-M, H-L 9) Bottom chord live load (0.0 psf)	TOP CHORD BOT CHORD WEBS	2x4 DF No. C-F,F-I: 2x8 2x4 DF No. L-M: 2x12 I 2x4 DF Stu	.1&Btr G *Except* 8 DF SS .1&Btr G *Except* DF SS Id/Std G		TOP CHORD BOT CHORD JOINTS	Structur Rigid ce 1 Brace	al wood sheathing directly a iling directly applied or 10-0 at Jt(s): N	pplied or 5-4-4 oc purlins. -0 oc bracing.
 FORCES. (lb) - Max. Comp./Max. Ten All forces 250 (lb) or less except when shown. TOP CHORD B-C=-1601/42, C-D=-1461/54, D-E=-952/139, G-H=-952/139, H=-1460/54, I-J=-1600/42 BOT CHORD B-M=0/1023, L-I=0/1030, J-L=0/1030, J-L=0/1023, G-H=-952/139, H=-1460/54, I-J=-1600/42 WEBS C-M=-367/126, D-M=0/682, E-N=-1222/212, G-N=-1222/212, I-L=-368/126, H-L=0/682 NOTES- Wind: ASCE 7-16; Vull=140mph (3-second gust) Vasd=111mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. It; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) 1-6-0 to 1-6-0, Interior(1) 1-6-0 to 7-9-0, Exterior(2R) 7-9-0 to 13-9-0, Interior(1) 13-9-0 to 20-0; 0 z3-0-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 TOLL: ASCE 7-16; Vrugh Cat B; Fully Exp: Ce=0; Ce=1.00 C: [=1.10] This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.8 psf on overhangs non-concurrent with other live loads. This truss has been designed for a 10.0 psf bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will ft between the bottom chord and any other members. Celling dead load (10.0 psf) on member(s). D-E, C+H, E-N, G-N; Wall dead load (5.0 psf) on member(s).D-M, H-L Bottom chord live load (20.0 psf) ant additional bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will ft between the bottom chord and any other members. Celling dead load (10.0 psf) on member(s). D-E, C+H, E-N, G-N; Wall dead load (5.0 psf) on member(s).D-M, H-L Bottom chord live load (20.0 psf) ant additional bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will ft between the bottom chord and any other members. Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) B, J.	REACTIONS.	(size) Max Horz Max Uplift Max Grav	B=0-5-8, J=0-5-8 B=313(LC 11) B=-89(LC 12), J=-89(LC 12 B=1341(LC 23), J=1341(LC) 24)				
 NOTES- 1) Unbalanced roof live loads have been considered for this design. 2) Wind: ASCE 7-16; Vult=140mph (3-second gust) Vasd=111mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) -1-6-0 to 1-6-0, Interior(1) 1-6-0 to 7-9-0, Exterior(2R) 7-9-0 to 13-9-0, Interior(1) 13-9-0 to 20-0-0, Exterior(2E) 20-0-0 to 23-0-0 zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=30.0 psf; Pf=20.8 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp; Ce=0.9; Cs=1.00; Ct=1.10 4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.8 psf on overhangs non-concurrent with other live loads. 5) Plates checked for a plus or minus 4 degree rotation about its center. 6) This truss has been designed for a 10.0 psf bottom chord ine load nonconcurrent with any other live loads. 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members. 8) Ceiling dead load (10.0 psf) on member(s). D-E, G-H, E-N, G-N; Wall dead load (5.0psf) on member(s).D-M, H-L 9) Bottom chord live load d07 the green lumber members. 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) B, J. 12) This truss is designed for L300 deflection. 	FORCES. (Ib TOP CHORD BOT CHORD WEBS) - Max. Con B-C=-160 B-M=0/12 C-M=-367	np./Max. Ten All forces 25 01/42, C-D=-1461/54, D-E=- 273, L-M=0/1030, J-L=0/112 7/126, D-M=0/682, E-N=-12	0 (lb) or less except when shown. 952/139, G-H=-952/139, H-I=-1460/54 2 22/212, G-N=-1222/212, I-L=-368/126,	ŀ, I-J=-1600/42 , H-L=0/682			
EXPIRES: Jun 30, 2025	NOTES- 1) Unbalanced 2) Wind: ASCE II; Exp B; Er, , Interior(1) exposed; C-1 3) TCLL: ASCI DOL=1.15); 4) This truss h r) Plates checl 6) This truss h 7) * This truss will fit betwe 8) Ceiling dead 9) Bottom chor 10) A plate rati 11) Provide mu 12) This truss referenced 13) Attic room	I roof live loa E 7-16; Vult= rclosed; MW 13-9-0 to 20- C for membe E 7-16; Pr=2! Is=1.0; Roug as been desi rent with othe ked for a plus as been desi has been desi has been desi has been desi d load (10.0 g d load (10.0 g d load (20.0 g d load (20.0 g d load A) d load (20.0 g d load (20.0 g d load A) d load (20.0 g d load (20.0 g d load A) d load (20.0 g d load (20.0 g d load A) d load (20.0 g d l	ds have been considered fo 140mph (3-second gust) Va FRS (directional) and C-C E 0-0, Exterior(2E) 20-0-0 to 2 res and forces & MWFRS for 5.0 psf (roof LL: Lum DOL= gh Cat B; Fully Exp.; Ce=0.5 gined for greater of min roof er live loads. s or minus 4 degree rotation igned for a 10.0 psf bottom of signed for a live load of 20.0 m chord and any other mem psf) on member(s). D-E, G-H 40.0 psf) and additional botto n of 20% has been applied for nnection (by others) of truss in accordance with the 2018 NSI/TPI 1. L/360 deflection.	r this design. sd=111mph; TCDL=4.2psf; BCDL=6.0 xterior(2E) -1-6-0 to 1-6-0, Interior(1) 23-0-0 zone; cantilever left and right ex reactions shown; Lumber DOL=1.60 1.15 Plate DOL=1.15); Pg=30.0 psf; Pi ; Cs=1.00; Ct=1.10 live load of 12.0 psf or 2.00 times flat about its center. thord live load nonconcurrent with any lpsf on the bottom chord in all areas w bers. 4, E-N, G-N; Wall dead load (5.0psf) of m chord dead load (0.0 psf) applied c or the green lumber members. to bearing plate capable of withstandi International Residential Code section	Dpsf; h=25ft; B=45ft; 1-6-0 to 7-9-0, Exte xposed ; end vertica plate grip DOL=1.60 f=20.8 psf (Lum DO roof load of 20.8 ps r other live loads. r/here a rectangle 3- on member(s).D-M, only to room. L-M ing 100 lb uplift at jo ns R502.11.1 and R	L=24ft; ea rior(2R) 7- il left and ri) L=1.15 Pl f on overh 6-0 tall by H-L int(s) B, J 802.10.2 -	ave=4ft; Cat. -9-0 to 13-9-0 right ate - angs 2-0-0 wide - and	STERED PROFESSION 93437PE 93437PE WMMMMM OREGON NO NO NO NO NO NO NO NO NO NO NO NO N

400 Sunrise Ave., Suite 270 Roseville, CA 95661 916.755.3571 / MiTek-US.com

Job	Truss	Truss Type	Qty	Ply	REVIVE NW	City of Portland
23012317-A	A03	GABLE	1	1		Reviewed for code compliance
Trus-Way, Vancouver	 , WA - 98661,		8	.720 s Aug	Job Reference (optional) 11 2023 MiTek Industries, Inc	Date: 01/25/24 Tue Oct 10 15:26:36 2023 Page 1
	<u>-1-6-0</u>	ID: 3-1-6 6-6-3 10-9-0	RRxF4lpSXe3Wu32	29MN0hivy 18-4-1	yUrAS-Q33aHvNV0WM65_nV\ 0	177KikebEn3wF30773QTFin9UnT9
	'1-6-0 '	3-1-6 ' 3-4-13 ' 4-2-13	4-2-13	3-4-1	3 ' 3-1-6 '1-6-0'	
			6x8 =			Scale = 1:73.3
	T	5x6 1⁄2	F A			
		8x8 1/	4×6	3 %		
		4x6 E	G	8x8 📏		
		3x6 II D		4x6 \ H	1	
	-2- -2-	3x6			3x6	
	5	6x8 // 0 0			5x0 6x8 \\	
	12.00 12					
		AP			AQ	
	B B B				ι κις	
	3x6	S = X W V U T S	RQP	0	N M L 3x6 =	
		10x14 = 3x6 3x6	3x6 10) 3x6 3x6	x14 =		
		6-6-3 6-8-0 10-9-0	14-10-0 14-1	11-12	21-6-0	
Plate Offsets (X,Y) [B:	+ 0-6-2,0-0-12], [J:0-6-0,0-0-11	6-6-3 0-1-13 4-1-0 [, [Y:0-1-9,0-1-0], [AA:1-4-8,0-2-3], [AI:1	<u>4-1-0 0-</u> <u>4-1-0 0-</u>	1-13	6-6-4	
LOADING (psf)	SPACING-	2-0-0 CSI .	DEFL.	in	(loc) l/defl L/d	PLATES GRIP
Snow (Pf/Pg) 20.8/30.0	Plate Grip DOL Lumber DOL	1.15 TC 0.17 1.15 BC 0.09	Vert(LL) Vert(CT)	-0.01 -0.01	K n/r 120 K n/r 90	MT20 220/195
BCLL 0.0	* Rep Stress Incr Code IRC2018/	YES WB 0.08 TPI2014 Matrix-S	Horz(CT) 0.01	J n/a n/a	Weight: 259 lb FT = 8%
LUMBER-			BRACING-			-
TOP CHORD 2x4 DF N C-F,F-I: 2	o.1&Btr G *Except* x8 DF SS		TOP CHORD BOT CHORD	Structur Rigid ce	al wood sheathing directly ap iling directly applied or 10-0-	plied or 6-0-0 oc purlins. 0 oc bracing.
BOT CHORD 2x4 DF N O-U: 2x12	o.1&Btr G *Except* 2 DF SS		JOINTS	1 Brace	at Jt(s): Y	°
WEBS 2x4 DF St OTHERS 2x4 DF St	ud/Std G ud/Std G					
REACTIONS. All beari	ngs 21-6-0.					
(lb) - Max Horz Max Uplif	: B=-313(LC 10) t All uplift 100 lb or less at jo	int(s) X, L except B=-105(LC 12),				
Max Grav	U=-123(LC 12), J=-105(LC All reactions 250 lb or less	12), O=-123(LC 12) at joint(s) S, T, V, W, X, Q, P, N, M, L,	R			
	except B=552(LC 2), U=622	2(LC 23), J=552(LC 2), O=609(LC 24)				
FORCES. (lb) - Max. Co TOP CHORD B-C=-53	mp./Max. Ten All forces 25 8/21, C-D=-400/40, D-E=-41	0 (lb) or less except when shown. 5/137, G-H=-415/137, H-I=-367/15, I-J=	-525/0			
BOT CHORD B-X=-54 R-S=-50	/410, W-X=-54/410, V-W=-54 //333, Q-R=-50/333, P-Q=-50	/410, U-V=-56/414, T-U=-53/336, S-T= /333, O-P=-53/336, N-O=0/345, M-N=0/	-50/333, /341,			
L-M=0/3 WEBS D-U=-41	41, J-L=0/341 9/141, H-O=-391/141					
NOTES-						CRED PROFFO
1) Unbalanced roof live lo 2) Wind: ASCE 7-16; Vult	ads have been considered fo =140mph (3-second gust) Va	^r this design. sd=111mph; TCDL=4.2psf; BCDL=6.0p	sf; h=25ft; B=45ft;	L=24ft; ea	ave=2ft; Cat.	DENGINEED SO
II; Exp B; Enclosed; MV 13-9-0, Exterior(2N) 13	VFRS (directional) and C-C C -9-0 to 20-0-0, Corner(3E) 20	orner(3E) -1-6-0 to 1-6-0, Exterior(2N) -0-0 to 23-0-0 zone; cantilever left and	1-6-0 to 7-9-0, Cor right exposed ; end	rner(3R) 7 d vertical l	7-9-0 to left and right	0242705
exposed;C-C for memb 3) Truss designed for win	ers and forces & MWFRS for d loads in the plane of the tru	reactions shown; Lumber DOL=1.60 pl ss only. For studs exposed to wind (not	ate grip DOL=1.60 rmal to the face), s) see Standa	ard Industry	93437PE
Gable End Details as a 4) TCLL: ASCE 7-16; Pr=	pplicable, or consult qualified 25.0 psf (roof LL: Lum DOL=	building designer as per ANSI/TPI 1. .15 Plate DOL=1.15); Pg=30.0 psf; Pf=	20.8 psf (Lum DO	L=1.15 PI	ate	mum
DOL=1.15); Is=1.0; Ro 5) This truss has been de	ugh Cat B; Fully Exp.; Ce=0.9 signed for greater of min roof	; Cs=1.00; Ct=1.10 live load of 12.0 psf or 2.00 times flat ro	oof load of 20.8 ps	f on overh	langs	1 OREGON 8
non-concurrent with oth 6) All plates are 2x4 MT20	ner live loads.) unless otherwise indicated.				Z	ARCH 13,20 0
7) Plates checked for a pl8) Gable requires continue	us or minus 4 degree rotation ous bottom chord bearing.	about its center.				REW JOHNS
9) Gable studs spaced at10) This truss has been d	1-4-0 oc. esigned for a 10.0 psf bottom	chord live load nonconcurrent with any	other live loads.		-	
 * This truss has been will fit between the bo 	designed for a live load of 20 ttom chord and any other me	.0psf on the bottom chord in all areas w nbers, with BCDL = 10.0psf.	here a rectangle 3	8-6-0 tall b	y 2-0-0 wide	October 14,2023
ට්බාtiබස්මාගාන්දකල්ණ2d (10	0 psf) on member(s). D-E, G	H, E-Y, G-Y; Wall dead load (5.0psf) o	n member(s).D-U,	H-O		
WARNING - Verify des Design valid for use only v	ign parameters and READ NOTES (with MiTek® connectors. This design the building designer must verify the	N THIS AND INCLUDED MITEK REFERENCE PA s based only upon parameters shown, and is for a a applicability of design parameters and property.	GE MII-7473 rev. 1/2/20 an individual building co	mponent, no	USE. ot	MiTok
building design. Bracing i is always required for stat	ndicated is to prevent buckling of indi ility and to prevent collapse with pos	vidual truss web and/or chord members only. Add ible personal injury and property damage. For ge	litional temporary and p neral guidance regardir	ermanent br	acing	400 Sunrise Ave., Suite 270
tabrication, storage, delive and BCSI Building Com	ry, erection and bracing of trusses an ponent Safety Information available	a truss systems, see ANSI/TPI1 Quality Criteria le from the Structural Building Component Associa	a and USB-22 available ation (www.sbcscompo	ie from Truss nents.com)	s Plate Institute (www.tpinst.org)	Roseville, CA 95661 916.755.3571 / MiTek-US.com

						City of Doutlond
Job	Truss	Truss Type	Qty	Ply	REVIVE NW	City of Portiand
23012317-A	A03	GABLE	1	1		Reviewed for code compliance
					Job Reference (optional)	Date: 01/25/24
Trus-Way, Vancouver	, WA - 98661,		8	.720 s Aug	11 2023 MiTek Industries, Inc	Tue Oct 10 15:26:36 2023 Page 2
		ID:RRxF4lp	SXe3Wu3	29MN0hivy	/UrAS-Q33aHvNV0WM65_nV	I7♥KANGBEnGWFG0C73QTFinyUnt9
NOTES						

13) A plate rating reduction of 20% has been applied for the green lumber members.

14) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) X, L except (jt=lb) B=105, U=123, J=105, O=123.
 15) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 16) Attic room checked for L/360 deflection.





LOADING (psf) TCLL (roof) 25.0 Snow (Pf/Pg) 20.8/30.0 TCDL 10.0 PCUL 0.0 *	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES	CSI. TC 0.13 BC 0.05 WB 0.00	DEFL. Vert(LL) Vert(CT) Horz(CT)	in (0.00 0.00 0.00	(loc) A A D	l/defl n/r n/r n/a	L/d 120 90 n/a	PLATES MT20	GRIP 220/195
BCDL 10.0	Code IRC2018/TPI2014	Matrix-P						Weight: 11 lb	FT = 8%
	•								

LUMBER-

TOP CHORD2x4 DF No.1&Btr GBOT CHORD2x4 DF No.1&Btr GWEBS2x4 DF Stud/Std G

BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 2-11-0 oc purlins, except end verticals. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) D=2-11-0, B=2-11-0 Max Horz B=61(LC 13) Max Uplift D=-18(LC 11), B=-39(LC 14) Max Grav D=150(LC 19), B=204(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

- Wind: ASCE 7-16; Vult=140mph (3-second gust) Vasd=111mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=2ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Corner(3E) zone; cantilever left and right exposed; end vertical left and right exposed; C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
- 3) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=30.0 psf; Pf=20.8 psf (Lum DOL=1.15 Plate DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10
- 4) Unbalanced snow loads have been considered for this design.
- 5) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.8 psf on overhangs non-concurrent with other live loads.
- 6) Plates checked for a plus or minus 4 degree rotation about its center.
- 7) Gable requires continuous bottom chord bearing.
- 8) Gable studs spaced at 1-4-0 oc.
- 9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 10) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 11) A plate rating reduction of 20% has been applied for the green lumber members.
- 12) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) D, B.
- 13) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



EXPIRES: Jun 30, 2025 October 14,2023





		2-	-11-0		1			
LOADING (psf) TCLL (roof) 25.0 Snow (Pf/Pg) 20.8/30.0 TCDL 10.0 BCLL 0.0 BCDL 10.0	SPACING- 2-0-0 Plate Grip DOL 1.15 Lumber DOL 1.15 Rep Stress Incr YES Code IRC2018/TPI2014	CSI. TC 0.09 BC 0.08 WB 0.00 Matrix-MP	DEFL. Vert(LL) Vert(CT) Horz(CT) Wind(LL)	in (ld -0.00 D -0.01 D 0.00 0.00 D	bc) l/defl)-G >999)-G >999 B n/a)-G >999	L/d 240 180 n/a 360	PLATES MT20 Weight: 9 lb	GRIP 220/195 FT = 8%

LUMBER-

TOP CHORD 2x4 DF No.1&Btr G BOT CHORD 2x4 DF No.1&Btr G BRACING-TOP CHORD

BOT CHORD

Structural wood sheathing directly applied or 2-11-0 oc purlins. Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (size) C=Mechanical, B=0-5-8, D=Mechanical

Max Horz B=60(LC 14) Max Uplift C=-34(LC 14), B=-29(LC 14)

Max Grav C=108(LC 19), B=209(LC 19), D=52(LC 7)

Max Grav C=108(LC 19), B=209(LC 19), D=52(LC 7)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-

1) Wind: ASCE 7-16; Vult=140mph (3-second gust) Vasd=111mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; B=45ft; L=24ft; eave=4ft; Cat. II; Exp B; Enclosed; MWFRS (directional) and C-C Exterior(2E) zone; cantilever left and right exposed ; end vertical left and right

- exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
- 2) TCLL: ASCE 7-16; Pr=25.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=30.0 psf; Pf=20.8 psf (Lum DOL=1.15 Plate
- DOL=1.15); Is=1.0; Rough Cat B; Fully Exp.; Ce=0.9; Cs=1.00; Ct=1.10

3) Unbalanced snow loads have been considered for this design.

4) This truss has been designed for greater of min roof live load of 12.0 psf or 2.00 times flat roof load of 20.8 psf on overhangs non-concurrent with other live loads.

- 5) Plates checked for a plus or minus 4 degree rotation about its center.
- 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
- 8) A plate rating reduction of 20% has been applied for the green lumber members.
- 9) Refer to girder(s) for truss to truss connections.
- 10) Refer to girder(s) for truss to truss connections.
- 11) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) C, B.
- 12) This truss is designed in accordance with the 2018 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.



EXPIRES: Jun 30, 2025 October 14,2023

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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- 1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
- . Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- 5. Cut members to bear tightly against each other
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
- 11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- 14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- 15. Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- 17. Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- 20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.
- 21. The design does not take into account any dynamic or other loads other than those expressly stated.