

ELEVATIONS

A-3

INTERIOR PARTITIONS - WOOD STUD (LOAD BEARING)

U334

2-HOUR

920600

LOAD BEARING

WOOD STUDS, RC-1 CHANNEL

STC = 58

USG-810219

ASSEMBLY OPTIONS:

GYPSUM BOARD:

TWO LAYERS OF 5/8 IN. THICK GYPSUM BOARD APPLIED HORIZONTALLY OR VERTICALLY.

RESILIENT CHANNEL:

25 GA. FURRING CHANNELS INSTALLED HORIZONTALLY SPACED 24 IN. O.C.

WOOD STUDS:

2 IN. X 4 IN. WOOD STUDS SPACED MAX. 16 IN. O.C.

BATTS & BLANKETS:

2 IN. THICK MINERAL WOOL INSULATION

INTERIOR PARTITIONS: WOOD STUD (LOAD-BEARING)

FIRE RATING:

2 HOUR

STC:

58

SOUND TEST:

USG-810219

SYSTEM THICKNESS:

6-1/2"

6-1/2"

3"

6-1/2"

3"

CENTER OF FRAMED WALL DETAIL

(TYPICAL)

GYPSUM ON COMMON SEPARATION WALLS SHALL BE CONTINUOUS BEHIND BUILDING ELEMENTS INCLUDING SHOWERS, BATHTUBS, CABINETS, CHASES, SOFFITS, ELECTRICAL PANELS AND STAIR STRINGERS (TYPICAL)

SEE DETAIL 1/44 FOR FULL FIRE PARTITION ASSEMBLY DETAILS

DIMENSION IS TO CENTER OF FIRE PARTITION WHICH IS 6-1/2" WIDE. STUD WALL SHOULD BE OFFSET TO CREATE CENTERING OF FULL ASSEMBLY WHEN DONE (TYPICAL)

6-1/2"

3"

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(TYPICAL)

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1
A4

UPPER LEVEL FLOOR PLAN

1/4" = 1'-0"

ALL EXTERIOR DOORS AND WINDOW OPENINGS ARE 96" RO TO BOTTOM OF HEADER

LOWER LEVEL FLOOR PLAN

1/4" = 1'-0"

CAREFULLY REVIEW ALL DETAILS IN THE FULL SET OF PLANS. ALL DETAILS ARE USED. CONTRACTOR IS RESPONSIBLE FOR FOLLOWING ALL DETAILS. CONTACT SITE SUPERVISOR SHOULD ANY CONFLICTS ARISE

JURISDICTION APPROVAL STAMP (KEEP CLEAR)

DETAIL NUMBER

PAGE NUMBER

SECTION/ DETAIL CALL OUT

\$	SINGLE POLE SWITCH	⚡	EXHAUST FAN 80 CFM
⚡	THREE-WAY SWITCH	📺	TELEPHONE OUTLET
⚡	FOUR-WAY SWITCH	📺	TELEVISION OUTLET
⚡	DUPLEX RECEPTACLE	⚡	ELECTRICAL PANEL
⚡	DUPLEX RECEPTACLE SPLIT WIRE	⚡	ELECTRICAL METER
⚡	220 VOLT RECEPTACLE	⚡	WALL MOUNTED
⚡	JUNCTION BOX	⚡	RECESSED LED
⚡	WATERPROOF	⚡	UNDER CABINET
⚡	GROUND FAULT INTERRUPTER	⚡	CEILING FAN
⚡	CEILING MOUNTED		
⚡	IONIZATION TYPE SMOKE DETECTOR 110V W/ BATT.		
⚡	INTERCONNECTED WITH CARBON MONOXIDE DETECT WHERE REQUIRED		
S	DOOR SENSOR	T	THERMOSTAT

GENERAL NOTES

ALL WORK SHALL COMPLY WITH THE LATEST ADOPTED ISSUE OF THE BUILDING CODE AND ANY APPLICABLE STATE, COUNTY OR LOCAL REGULATIONS THAT MAY BE IN EFFECT AT THE TIME OF CONSTRUCTION.

THE CONTRACTOR IS RESPONSIBLE TO CHECK THE PLANS AND SITE CONDITIONS AND TO NOTIFY THE DESIGNER OF ANY ERRORS OR OMISSIONS PRIOR TO THE START OF CONSTRUCTION.

WRITTEN DIMENSIONS HAVE PRECEDENCE OVER SCALED DIMENSIONS.

ALL POSTS, BEAMS, HEADERS, JOIST AND RAFTERS TO BE #2 DF UNLESS NOTED.

ALL PLATES, SILLS, BLOCKING, BRIDGING ETC. TO BE #3 DF UNLESS NOTED.

UNLESS NOTED ALL EXTERIOR WINDOW AND DOOR HEADERS TO BE PER ENGINEERING SHEETS S1 AND S2

PROVIDE DOUBLE JOIST UNDER ALL PARALLEL WALLS IF APPLIES.

PROVIDE SOLID BLOCKING UNDER ALL BEARING WALLS.

PROVIDE SOLID FRAMING UNDER ALL BEAMS AND HEADERS.

(2) STUDS MIN WITH (3) STUDS AND BUILD UP AND BEAM MEMBERS. SOLID POSTS REQUIRED AS INDICATED ON PLANS.

NAILING SCHEDULES ARE PER ENGINEERING OR BRACED WALL METHODS AS INDICATED ON THE PLANS SHEET S1 AND S2. S2 IF APPLICABLE

DECKS AND BALCONY GUARDRAILS TO BE 36" HIGH WITH MAXIMUM OPENING SPACES NO LARGER THAN THAT WHICH CAN KEEP A 4" SPHERE FROM PASSING THROUGH. (IF DECK OR RAILS APPLY TO PLANS)

ALL WINDOWS WITH IN 18" OF FLOOR OR WITHIN 24" OF DOORS ARE TO BE TEMPERED. WINDOWS LESS THAN 24" OFF FLOOR TO HAVE CHILD PROTECTION

ALL SHOWER AND TUB ENCLOSURES ARE TO HAVE TEMPERED SAFETY

PROVIDE 1/2" "GREEN BOARD" GYPSUM AROUND TUB, SHOWER AND MOISTURE EXCLUDING FIRE PARTITIONS

ALL EXHAUST VENTS TO BE VENTED TO THE OUTDOORS A MINIMUM OF 3' FROM ANY OPENING.

ALL LIGHTING OR ELECTRICAL SHOWN ON PLANS IS MEANT TO BE USED AS A GUIDE ONLY. ELECTRICAL SWITCHES AND OUTLETS ARE TO BE INSTALLED AS PER LOCAL CODE AND OWNERS' BUILDERS REQUIREMENTS.

SMOKE DETECTORS TO BE 110V INTERCONNECTED WITH BATTERY BACKUP AND INSTALLED IN ACCORDANCE WITH THE ELECTRICAL CODE. CARBON SENSING AS REQUIRED BY CODE

PROVIDE GFCI OUTLETS IN KITCHENS, BATHROOMS, GARAGE, AND WP GFCI'S ON EXTERIOR OF HOME ARC FAULT BREAKERS AS REQUIRED BY CODE.

PROVIDE OUTSIDE VENTING OF DRYER AND LABEL PER CODE.

PROVIDE EXTERIOR LANDINGS WITH A LENGTH IN THE DIRECTION OF TRAVEL OF NO LESS THAN 36"

ALL WINDOWS TO BE GLAZED IN ACCORDANCE WITH ENERGY CODE

BEDROOMS AND BASEMENT EGRESS MUST BE MIN OF 621 SQ INCH OF OPENING WITH A MAX SILL HEIGHT OF 44"

APPLY VAPOR BARRIER AT INTERIOR SIDE OF ALL EXTERIOR WALLS PER CODE BENEATH FINISH MATERIAL.

CLEANOUTS IN CRAWL SPACE MUST BE LOCATED WITHIN 5' OF THE ACCESS OPENING PER OPSC 707.8 AND 707.9

PLUMBING FIXTURES WATER SAVINGS RATING REQUIRED PER OPSC TABLE 401.3

BTS DEVELOPMENTS, LLC
1419 BROADWAY ST
VANCOUVER, WA 98663

FLOOR PLANS

PROJECT NAME: CURTIS/ HUNT
LOCATION: CURTIS/ HUNT
9-11-23
VERSION: PRE ENGINEERING SET

A-4

ALL WORK SHALL COMPLY WITH THE LATEST ADOPTED ISSUE OF THE BUILDING CODE AND ANY APPLICABLE STATE, COUNTY OR LOCAL REGULATIONS THAT MAY BE IN EFFECT AT THE TIME OF CONSTRUCTION.

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STAIR FRAMING

CAREFULLY REVIEW ALL DETAILS IN THE FULL SET OF PLANS. ALL DETAILS ARE USED. CONTRACTOR IS RESPONSIBLE FOR FOLLOWING ALL DETAILS. CONTACT SITE SUPERVISOR SHOULD ANY CONFLICTS ARISE

**SHEET "S" SHEETS AT REAR OF THIS
SET FOR DETAILED ENGINEERING
REQUIREMENTS**

(1)	FOOTING	SCHEDULE	REINFORCING BARS	CAPACITY
[F1]		1'-6"x1'-6"x10"	NONE	3375#
[F2]		2'-0"x2'-0"x8"	(2) #4 BARS EACH WAY	5500#
[F3]		2'-3"x2'-3"x8"	(2) #4 BARS EACH WAY	7000#
[F4]		2'-6"x2'-6"x8"	(3) #4 BARS EACH WAY	8600#
[F4]		2'-9"x2'-9"x8"	(3) #4 BARS EACH WAY	10500#
[F5]		3'-0"x3'-0"x8"	(3) #4 BARS EACH WAY	12500#
[F6]		3'-6"x3'-6"x10"	(4) #4 BARS EACH WAY	16000#

(1):3" CLEAR FROM BOTTOM OF FOOTING

FOUNDATION NOTES

1. REFER TO MAIN FLOOR SHEAR WALL PLAN FOR HOLDOWN SIZE.
2. THIS DRAWING IS FOR LATERAL INFORMATION ONLY, REFER TO ARCHITECTURAL PLANS FOR ALL OTHER INFORMATION.
3. 4X6 DFL-#2 POST. POST FOOTING TO BE SEPARATED BY ASPHALT SHINGLE. TYPICAL CRAWL SPACE BEAM TO BE C1: 4X8 DFL-#2.
SINGLE GUSSET PLATE TO BE USED ON BOTH SIDES OF ATTACHMENT TO POST.

MATERIALS:

CONCRETE: MIN. 28-DAY CONCRETE STRENGTH = 2500 psi.
GRADE BEAMS, PIERS, AND SPREAD FOOTINGS SHALL BE POURED ONTO UNDISTURBED, NATIVE SOIL WHICH IS FREE FROM ANY MATERIAL THAT WILL ADVERSELY AFFECT THE SOIL DESIGN BEARING PRESSURE REFERENCED ABOVE. ALL NON-STRUCTURAL WEATHER PROOFING AND FINISH MATERIAL TO BE DETERMINED "BY OTHERS".

SLAB CONTROL JOINTS: PER OWNERS REQUIREMENTS OR DIRECTION:

MISC. SITE PREPARATIONS:

OBTAIN AND OBEY ALL APPLICABLE REGULATIONS REGARDING GRADING AND EXCAVATION. IDENTIFY, MARK, AND PROTECT FROM DAMAGE ALL EXISTING UNDERGROUND PIPES, CONDUITS, AND CABLE (WATER SUPPLY, SANITARY SEWER, STORM SEWER, GAS, STEAM, ELECTRICAL, AND COMMUNICATION CABLE). REMOVE SOIL WITH ORGANIC MATTER. PERFORM BACKFILL AND COMPACTION IN A SYSTEMATIC MANNER, TO ASSURE COMPLETE AND CONSISTENT WORK. IF ANY OVER-EXCAVATION OCCURS, THE EXCAVATION SHALL BE REGRADED TO THE PROPOSED BACKFILL. PROVIDE TESTS AND INSPECTION OF BACKFILL AND COMPACTION. LAYER BACKFILL IN 6 IN. TO 12 IN. INCREMENTS. COMPACT ALL FILL. USE STABILIZED FILL MATERIAL OF AN APPROVED TYPE AND FROM AN APPROVED SOURCE. TEST AND APPROVE MATERIAL DELIVERED FROM OTHER SITES. DO NOT ALLOW ANY DEBRIS TO BE MIXED WITH FILL. CURE CONCRETE TO FULL REQUIRED STRENGTH BEFORE BACKFILLING. PROVIDE DRAINAGE CATCHERS PER ARCHITECTURAL DRAWINGS.

SPECIAL INSPECTION: NONE

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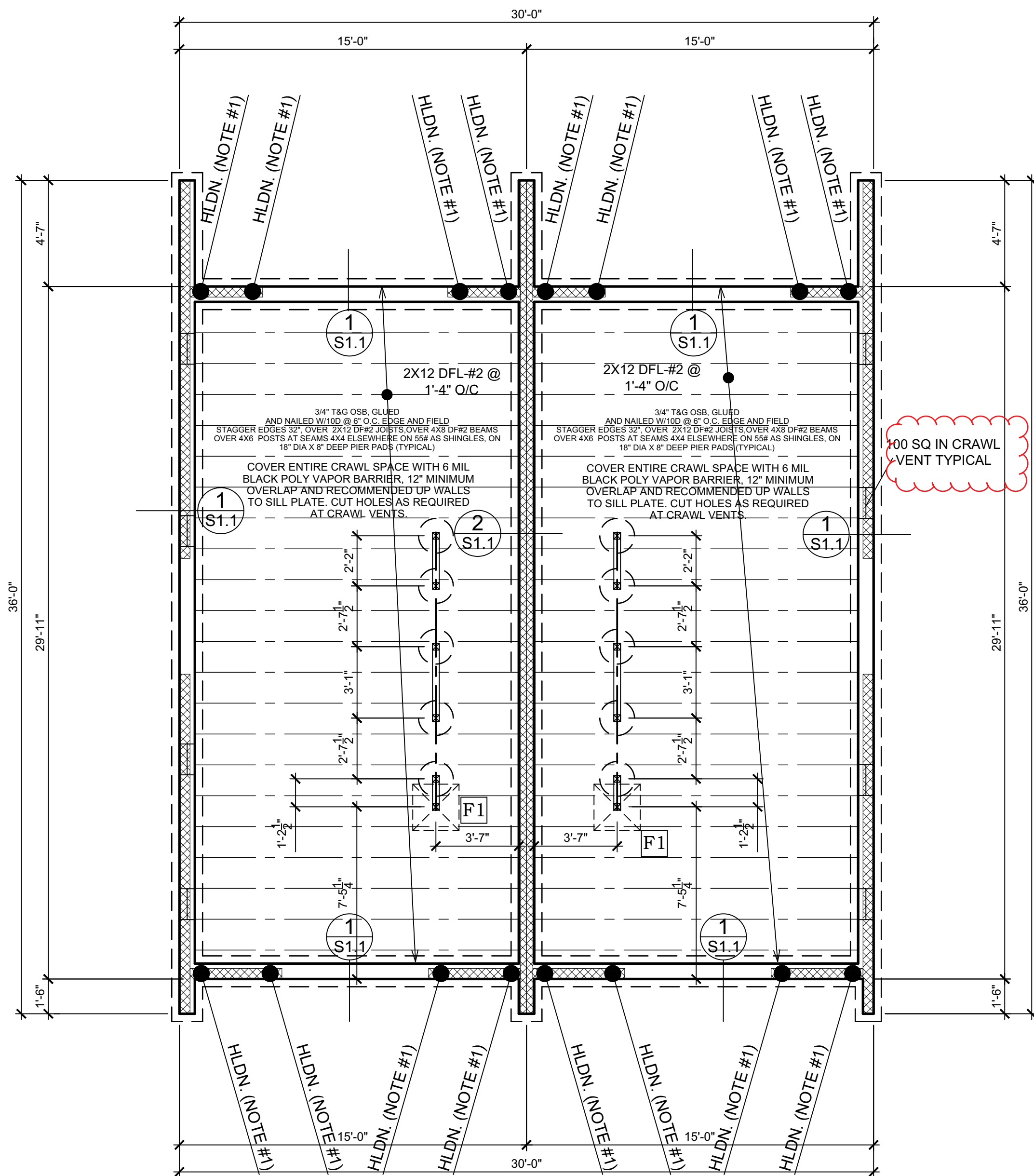
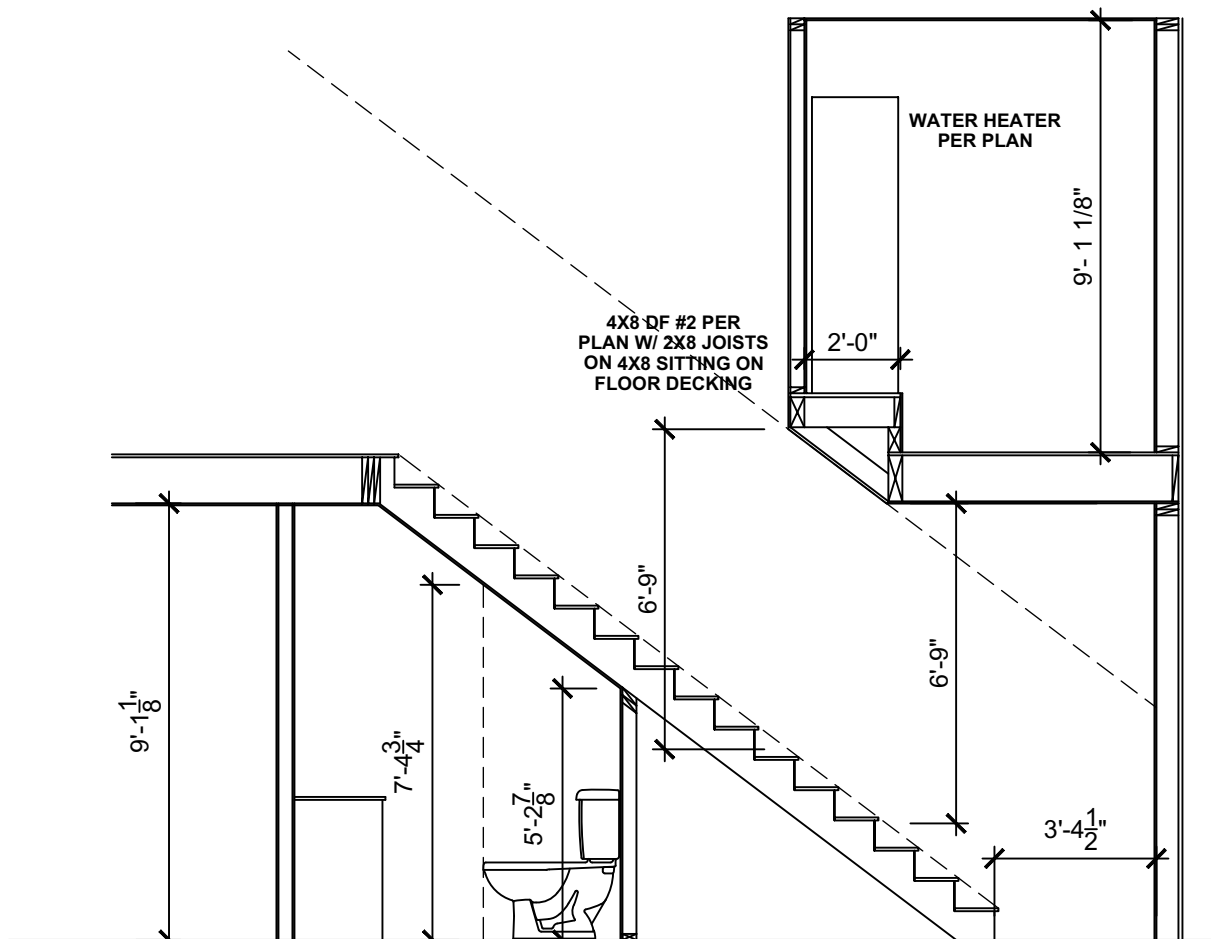
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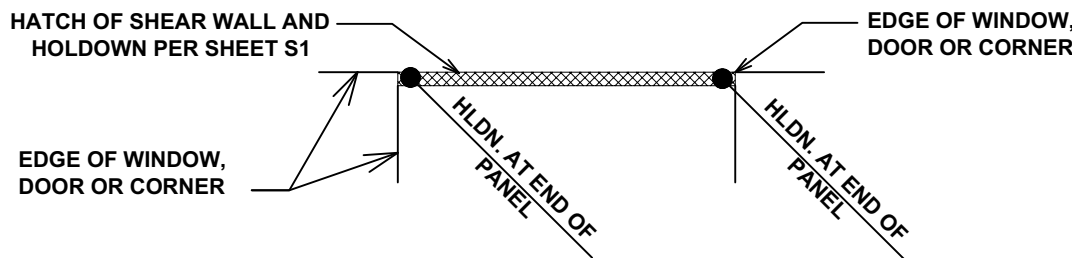
SPECIAL INSPECTION: NONE



FOUNDATION PLAN

1/4" = 1'-0"

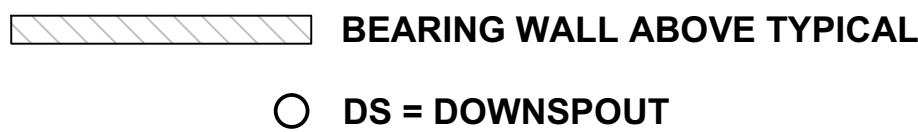
JURISDICTION APPROVAL STAMP (KEEP CLEAR)



SHEAR WALL / HOLDOWN NOTATION DIAGRAM

**SEE SHEETS S1 AND S2 (S2 IF PROVIDED) AT THE BACK OF THIS PLAN SET
FOR ALL DETAILED ENGINEERING AND FRAMING REQUIREMENTS**

SEE DETAILS SHEETS FOR RADON REQUIREMENTS TYPICAL



FOUNDATION NOTES

- FOOTINGS ARE TO BEAR ON UNDISTURBED LEVEL SOIL DEVOID OF ANY ORGANIC MATERIAL AND STEPPED AS REQUIRED TO MAINTAIN THE REQUIRED DEPTH BELOW THE FINAL GRADE.
- SOIL BEARING PRESSURE ASSUMED TO BE 1500 PSF.
- MAXIMUM SLOPE OF CUTS AND FILLS TO BE TWO (2) HORIZONTAL TO ONE (1) VERTICAL FOR BUILDINGS, STRUCTURES, AND FOUNDATIONS
- ANY FILL UNDER GRADE SUPPORTED SLABS TO BE A MIN. OF 4" IN. GRANULAR MATERIAL COMPACTED TO 95%.
- CONCRETE - MIX AND 28 DAY STRENGTH OF CONCRETE.

BASEMENT WALLS & FOUNDATIONS NOT EXPOSED TO WEATHER:	2,500 PSI	MORTAR & GROUT TO BE MIXED PER PERMITS
BASEMENT & INTERIOR SLABS ON GRADE:	2,500 PSI	
<div style="border: 2px solid red; padding: 5px; display: inline-block;"> City of Portland Reviewed for code compliance Date: 01/10/24 3,000 PSI Project #: 23-089944-000-00-RS </div>		
FOUNDATIONS EXPOSED TO THE WEATHER, AND GARAGE SLABS	3,000 PSI	
PORCHES, STEPS, & CARPORT SLABS EXPOSED TO WEATHER:	3,500 PSI	

- [illegible]

CRAWL SPACE VENTING

THE UNDER FLOOR SPACE BETWEEN THE BOTTOM OF THE FLOOR JOISTS AND THE EARTH UNDER ANY BUILDING SHALL HAVE VENTILATION OPENINGS THROUGH THE FOUNDATION WALLS OR EXTERIOR WALLS. THE MINIMUM NET AREA OF VENTILATION OPENING SHALL NOT BE LESS THAN 1 SQ FT FOR EACH 150 SQUARE FEET OF UNDER FLOOR SPACE AREA. ONE VENTILATION OPENING SHALL BE REQUIRED WITHIN THREE FEET OF EACH CORNER OF THE BUILDING. THE OPENINGS SHALL BE COVERED WITH CORROSION RESISTANT WIRE MESH OR EQUIVALENT WITH 1/8 INCH MINIMUM DIMENSION.

CRAWL SIZE #1 = 400 SQ FT
PROVIDE MIN 2.66 SQ FT VENTING/ 100 SQ IN
VENTS BEING USED
PROVIDE MINIMUM 4 CRAWL VENTS AS SHOWN

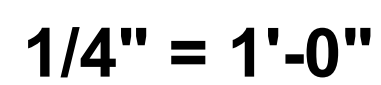
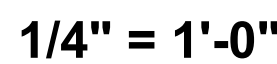
VERIFY THAT CRAWL SPACE VENTS ARE INSTALLED
WITHIN THREE FEET OF EACH CORNER TYPICAL

BTS DEVELOPMENTS, LLC
1419 BROADWAY ST
VANCOUVER, WA 98663

FOUNDATION PLAN

PROJECT NAME: CURTIS/ HUNT
LOCATION: CURTIS/ HUNT
9-11-23
VERSION: PRE ENGINEERING SE

A-5



UPPER FLOOR FRAMING NOTES:

1. HEADERS TO BE 4X8 DFL-#2 (MAX. SPAN 4'-0"), U.N.O.
2. INTERIOR HEADERS TO BE 4X8 DFL-#2 (MAX. SPAN 4'-0"), U.N.O.
3. REFER TO ARCHITECTURAL DRAWINGS FOR ROOF FINISHING SPECIFICATIONS AND VERIFICATION OF ALL DIMENSIONS.
4. TYPICAL EXTERIOR WALL POST TO BE (2) 2X6 DFL-#2 (BUNDLED STUD), FASTEN EACH STUD TOGETHER WITH 16d NAILS @ 12" O.C. TYP. ENTIRE LENGTH OF STUD, U.N.O.
5. TYPICAL INTERIOR WALL POST TO BE (2) 2X4 DFL-#2 (BUNDLED STUD), FASTEN EACH STUD TOGETHER WITH 16d NAILS @ 12" O.C. TYP. ENTIRE LENGTH OF STUD, U.N.O.
6. EXTERIOR POST CAPS TO BE SIMPSON "PC" OR "EPC", IF EXP. USED, CONDITION COAT PER MANUFACTURER'S SPECIFICATIONS WITH EXTERIOR EXPOSURE AND P.T. MATERIAL.
7. (B.W. INTERIOR WALLS)
8. INSTALL (3) 2X6 WITH NISTC28 VERTICAL STRAP AT END OF BEAM TO POST BELOW.

SUBMITTED 11-2-23

A-6

ST S DEVELOPMENTS, LLC
419 BROADWAY ST
VAINICOVER, WA 98663

ROOF PLANS

PROJECT NAME: CURTIS HUNT
LOCATION: CURTIS HUNT
9-11-23
VERSION: PRE-ENGINEERING SET

A-7

JURISDICTION APPROVAL STAMP (KEEP CLEAR)

ROOF NOTES

1) THE TOTAL NET FREE VENTILATION AREA SHALL NOT BE LESS THAN 1/300 OF THE AREA OF THE SPACE VENTILATED, PROVIDED THAT AT LEAST 40% BUT NOT MORE THAN 50% OF THE REQUIRED AREA IS PROVIDED BY VENTILATORS LOCATED IN THE UPPER PORTION OF THE SPACE BEING VENTILATED AT LEAST 3 ABOVE THE GAVE VENT. THE REMAINING BALANCE OF THE REQUIRED VENTING WILL BE PROVIDED BY GAVE VENTS

2) ALTERNATIVE METHOD: VENTILATION SHALL NOT BE LESS THAN 1/150 OF THE AREA OF THE SPACE VENTILATED.

VENTS:

1) ROOF VENTS ARE BASED ON 49 SQ IN NET FREE VENTILATION AREA PER VENT.

2) GAVE VENTS ARE BASED ON 16 SQ IN NET FREE VENTILATION PER VENT.

1. PROVIDE POSITIVE VENTILATION AT EACH END OF EACH RAFTER SPACE AT VAULTED CLG AREAS, AND INSULATION BAFFLES AT EAVE VENTS BETWEEN RAFTERS. RAFTER VENTILATION IS ALSO REQUIRED AT BLOCKING LOCATIONS ABOVE THE PLATE.
2. PROVIDE FIRE BLOCKING, DRAFT STOPS, & FIRE STOPS AS PER OREGON DWELLING SPECIALTY CODE SEC.
3. HIPS, VALLEYS AND RIDGES SHALL NOT BE LESS IN DEPTH THAN THE END CUT OF THE RAFTER.

7" TRUSS HEEL TYPICAL
6" FOR INSULATION PER
CODE AND 1" FOR
VENTILATION TYPICAL

NO OFF VENTS OR ROOF PENETRATIONS IN AREA SHOWN LIKE THIS HATCH WITHIN 48" EITHER DIRECTION OF THE FIRE PARTITION WALL IN ATTIC PROVIDE FIRE TREATED SHEATHING PER DETAILS IN PLAN SET

ROOF FRAMING PLAN

ROOF AND FLOOR FRAMING NOTES:
 (1) HEADERS TO BE RT: 4X8 DFL-#2 (6'-0" MAX SPAN), U.N.O.
 (2) INTERIOR HEADERS TO BE 4X8 DFL-#2 (MAX. SPAN 4'-0"), U.N.O.
 (3) REFER TO ARCHITECTURAL DRAWINGS FOR ROOF FINISHING SPECIFICATIONS AND VERIFICATION OF ALL DIMENSIONS.

BEAM SCHEDULE:

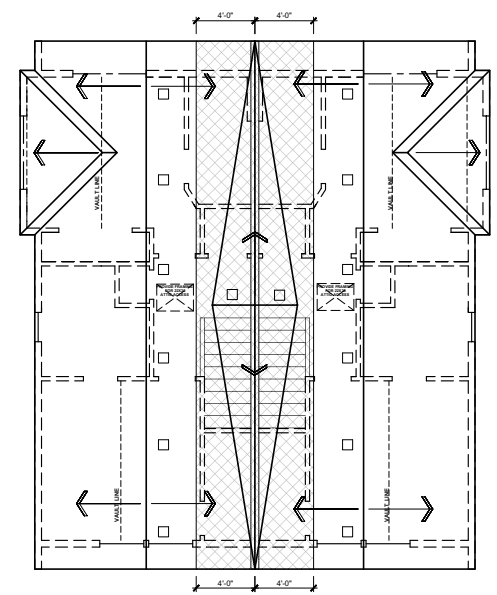
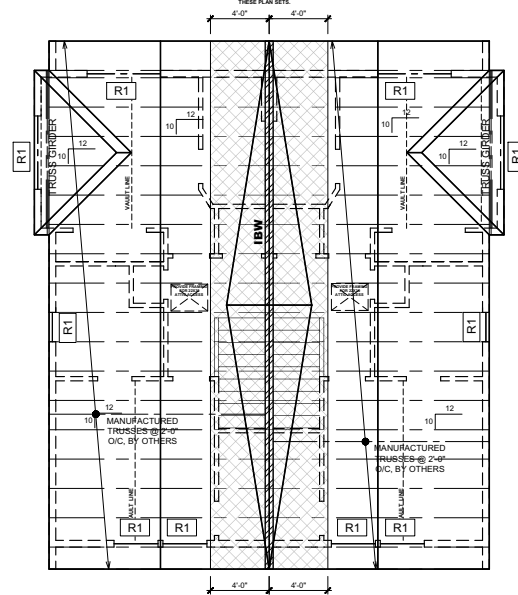
BEAM SIZE	SIZE
R1	4X8 DFL-#2

TRUSS TIES

TRUSS TYPE	SIMPSON TIEDOWN	UPLIFT
TRUSS	H10A	1145#
TRUSS	H2 5A	820#
2PLY GIRDERS	LG12	2050#

NON RATED CRICKET WITH VENTS AS SHOWN.
FIRE RATED PLYWOOD UNDER FOR 4"
EACH SIDE OF VIRTUAL PROPERTY LINE
AS HATCHED HERE. SEE DETAILS ON
THESE PLAN SETS.

NON RATED CRICKET WITH VENTS AS SHOWN
FIRE RATED PLYWOOD UNDER FOR 4"
EACH SIDE OF VIRTUAL PROPERTY LINE
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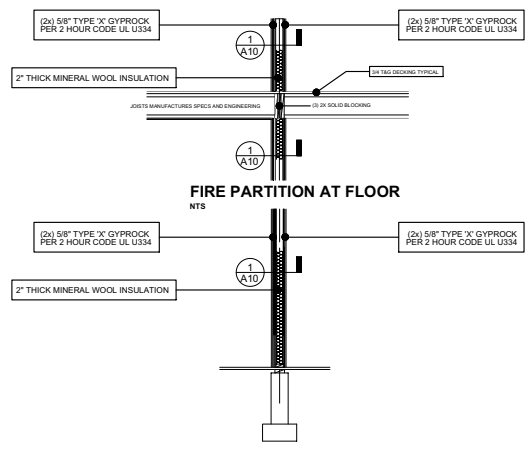
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BLOCKING BETWEEN

1/2" = 1'-0"

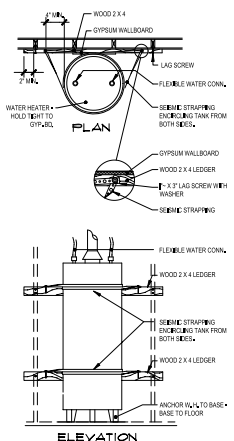


ROOF STRUCTURAL

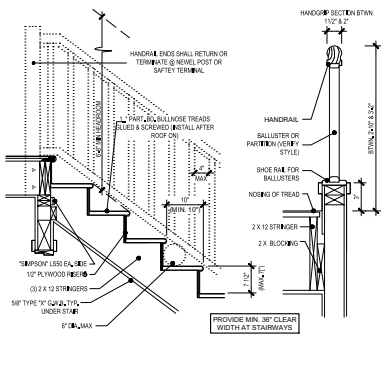
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ROOF LAYOUT PLAN

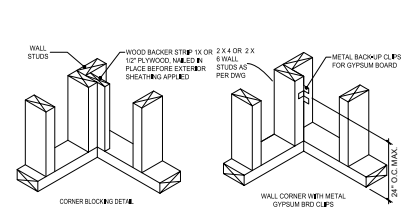
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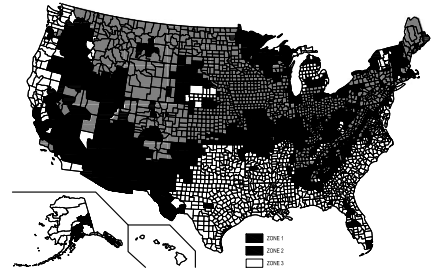
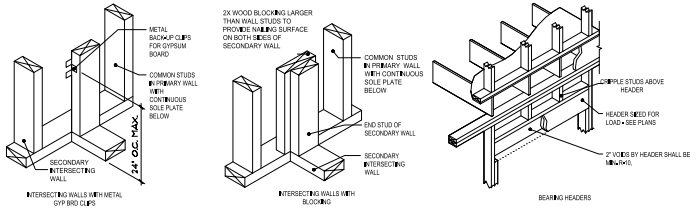
WATER HEATER SUPPORT



STAIR DETAILS



INTERMEDIATE FRAMING DETAILS

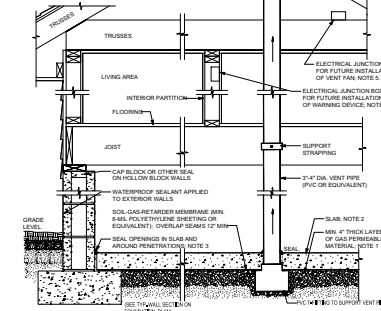


EPA RADON ZONES

PASSIVE SUB SLAB DEPRESSURIZATION RADON CONTROL SYSTEM

NOTES:

1. ALL CONCRETE SLABS THAT COME IN CONTACT WITH THE GROUND SHALL BE LAID OVER A GAS PERMEABLE MATERIAL MADE UP OF EITHER A MINIMUM 4\"/>



SLAB ON-GRADE/BELOW-GRADE (BASEMENTS) SUB-MEMBRANE DEPRESSURIZATION SYSTEM

RADON DETAILS AND NOTES

SLAB SUB-FLOOR PREPARATION

1. A LAYER OF GAS-IMPERMEABLE MATERIAL SHALL BE PLACED UNDER ALL CONCRETE SLABS AND OTHER FLOOR SYSTEMS THAT DIRECTLY CONTACT THE GROUND, AND ARE WITHIN THE WALLS OF THE BUILDING SPACES OF THE BUILDING. THE GAS-IMPERMEABLE LAYER SHALL CONSIST OF ONE OF THE FOLLOWING:
 - A. A UNIFORM LAYER OF CLEAN AGGREGATE A MINIMUM OF 4 INCHES THICK. THE AGGREGATE SHALL CONSIST OF MATERIAL SHALL BE SUFFICIENT TO PASS THROUGH A 1/2\"/>
 - B. A UNIFORM LAYER OF SAND OR GRAVEL A MINIMUM OF 4 INCHES THICK OVERLAPPED BY A LAYER OR STRIPS OF GEOTEXTILE DRAINAGE MATTING DESIGNED TO ALLOW THE LATERAL FLOW OF SOIL GASES.

SOIL-GAS-RETARDER

1. THE SOIL-GAS-RETARDER SHALL BE COVERED WITH A CONTINUOUS LAYER OF MINIMUM 6 MIL POLYETHYLENE SHEETING UNDER THE GROUND COVER SHALL BE LAPPED A MINIMUM OF 6 INCHES AT JOINTS AND SHALL EXTEND TO ALL FOUNDATION WALLS ENCLOSED THE CRAWL SPACE AREA.
2. THE SHEET SHALL BE CLOSED AROUND ANY PIPE, WIRE OR OTHER PENETRATIONS OF THE MATERIAL. SHALL BE SEALED OR COVERED WITH ADDITIONAL SHEETING.

VENT PIPE (RADON)

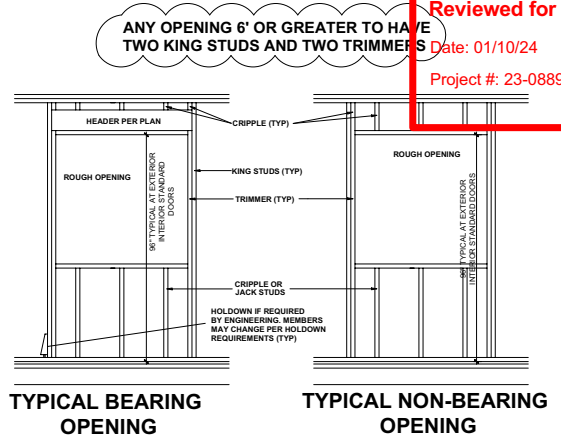
1. A PLUMBING TEE OR OTHER APPROVED CONNECTION SHALL BE INSERTED HORIZONTALLY THROUGH THE SOIL-GAS-RETARDER SHEETING AND CONNECTED TO A 7/8\"/>
2. THE VENT PIPE SHALL BE EXTENDED UP THROUGH THE BUILDING FLOORS TO TERMINATE AT LEAST 10 INCHES ABOVE THE ROOF SURFACE IN A LOCATION AT LEAST 10 FEET AWAY FROM ANY WINDOW OR OTHER OPENING INTO THE CONFINED SPACES OF THE BUILDING THAT IS LESS THAN 2 FEET BELOW THE EXHAUST OUTLET AND 10 FEET FROM ANY WINDOW OR OTHER OPENING.
3. IN BUILDINGS WHERE EXTERIOR ROOF JOISTS OR OTHER BARRIERS SEPARATE THE SOIL-GAS-RETARDER FROM OTHER GAS-IMPERMEABLE MATERIAL, EACH SHALL BE FITTED WITH AN INDIVIDUAL VENT PIPE.
4. ALL VENT PIPE SHALL CONNECT TO A SINGLE VENT THAT TERMINATES ABOVE THE ROOF OR EACH INDIVIDUAL VENT PIPE SHALL TERMINATE ABOVE THE ROOF.
5. ALL COMPONENTS OF THE RADON VENT PIPE SYSTEM SHALL BE INSTALLED TO PROVIDE FOR DRAINAGE TO THE GROUND BENEATH THE SLAB OR SOIL-GAS-RETARDER.
6. RADON VENT PIPES SHALL BE ACCESSIBLE FOR FUTURE PIPE INSTALLATION THROUGH AN ATTIC OR OTHER AREA OUTSIDE THE HABITABLE SPACE, OR AN APPROVED ROOF TOP ELECTRICAL SUPPLY MAY BE PROVIDED FOR FUTURE USE FOR A POWERED RADON VENT FAN.
7. ALL EXPOSED AND UNINSULATED RADON VENT PIPES SHALL BE IDENTIFIED WITH AT LEAST ONE LABEL ON EACH FLOOR AND ACCESSIBLE ATTIC. THE LABEL SHALL READ "RADON REDUCION SYSTEM".

POWER SOURCE REQUIREMENT

1. TO ACCOMMODATE FUTURE INSTALLATION OF AN ACTIVE SUBMEMBRANE OR SIMILAR DEPRESSURIZATION SYSTEM, AN ELECTRICAL "BOX" TERMINATED IN AN APPROVED LOCATION OF THE BUILDING SHALL BE INSTALLED OUTSIDE THE BUILDING. THE VENT PIPES MAY BE CONNECTED TO A SINGLE VENT TERMINATING ABOVE THE ROOF OR EACH VENT PIPE INDIVIDUALLY CONNECT TO TERMINATE ABOVE THE ROOF (SEE VENT PIPE NOTES).

COMBINATION FOUNDATIONS

1. COMBINATION FOUNDATIONS (SPACE OR SLAB-ON-GRADE/BELOW-GRADE) FOUNDATIONS SHALL HAVE SEPARATE RADON REDUCION SYSTEMS IN EACH TYPE OF FOUNDATION AREA. A SOIL-GAS-RETARDER AND RADON VENT PIPES SHALL BE INSTALLED OUTSIDE THE BUILDING. THE VENT PIPES MAY BE CONNECTED TO A SINGLE VENT TERMINATING ABOVE THE ROOF OR EACH VENT PIPE INDIVIDUALLY CONNECT TO TERMINATE ABOVE THE ROOF (SEE VENT PIPE NOTES).



TYPICAL BEARING OPENING

TYPICAL NON-BEARING OPENING

City of Portland
Reviewed for code compliance
Date: 01/10/24
Project #: 23-088944-000-00-RS

IT'S DEVELOPMENTS, LLC
419 BROADWAY ST
VAILICOVER, WA 98663

BUILDING DETAILS

PROJECT NAME: CLIMATE HUNT
LOCATION: CLIMATE HUNT
DATE: 01/10/24
DESIGNED BY: ENGINEERING SET

A-8

CODE: 2019 OSSC (ASCE 7-16)
USE OR OCCUPANCY OF BUILDINGS AND STRUCTURES RISK CATEGORY (TABLE 1604.5): II
WIND SPEED V (ATC, TABLE 1603.9): 98 MPH EXPOSURE B (LOAD CASE 16-12)
SEISMIC DESIGN CATEGORY: D (TABLE 1613.2.5 (1) AND (2))
SITE CLASS: D (STIFF SOIL, PER ATC)
ROOF SNOW LOAD: 25 PSF (SEAQ SNOW LOAD)
ROOF DEAD LOAD: 17 PSF
SOIL BEARING PRESSURE: 1500 PSF
SOIL PASSIVE SOIL PRESSURE: (LOAD INCREASE 1.33 FOR WIND/SEISMIC APPLICATION):
GRAVEL BACKFILL - 200 PSF (OSSC 1806.2 PRESUMPTIVE)
CONCRETE BACKFILL - 100 PSF

1. WALL STUDS TO BE 2X6 DFD @ 12" O.C., TYPICAL U.N.C.
2. ROOF SHEATHING TO BE $\frac{1}{2}$ " A.P. RATED CD SHEATHING OR OSB. INSTALL PANELS ALONG SLOPE. SPACE 84 NAILS MAXIMUM 6" O.C. ALONG PANEL EDGES. FOR OTHER CONDITIONS, SPACE 84 NAILS MAXIMUM 12" O.C. ON INTERMEDIATE SUPPORTS.
3. TYPICAL WALL SHEATHING (T&N TO BE $\frac{1}{2}$ " A.P. RATED CD SHEATHING OR OSB. ALL PANEL EDGES TO BE NAIL SCHEDULE. SPACE 84 NAILS MAXIMUM 6" O.C. ALONG PANEL EDGES. FOR OTHER CONDITIONS AND PANEL THICKNESSES, SPACE 84 NAILS MAXIMUM 12" O.C. ON INTERMEDIATE SUPPORTS.
4. FLOOR SHEATHING (T&N TO BE $\frac{1}{2}$ " A.P. RATED CD SHEATHING OR OSB. SPACE 84 NAILS MAXIMUM 6" O.C. ALONG PANEL EDGES. FOR OTHER CONDITIONS, SPACE 84 NAILS MAXIMUM 12" O.C. ON INTERMEDIATE SUPPORTS.
5. SILL PLATE TO BE 2X PT. U.N.C. (REFER TO SILL BOLT SPACING IN SCHEDULE BELOW).

ONE PLACE		NAIL	
0	1	113	113
1	2	114	114
2	3	115	115
3	4	116	116
4	5	117	117
5	6	118	118
6	7	119	119
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8	9	121	121
9	10	122	122
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98	99	211	211
99			

HOLDOWN NOTATION	SIMPSON® HOLDOWN TYPE	INSTALLATION INSTRUCTIONS
2	HDU2 (3075S)	STU: STU X 8 X MIN. IF REINFORCEMENT IS CONCRETE, ANCHOR IS TO BE INSTALLED PLUMB AND LOCATED AGAINST THE CENTER LINE OF THE WALL. WALL STUDS MIN. 24" ON CENTER. PARTED STUD: PARTED STUD TO BE INSTALLED PLUMB AND LOCATED AGAINST THE CENTER LINE OF THE WALL. WALL STUDS MIN. 24" ON CENTER. PARTED STUD TOGETHER WITH THE NAIL(S) IF A/C" DENTH RIGID OF STUD. INSTALL ALSO PER MANUFACTURER'S SPECIFICATION.
4	HDU4 (456S)	STU: STU X 8 X MIN. IF REINFORCEMENT IS CONCRETE, ANCHOR IS TO BE INSTALLED PLUMB AND LOCATED AGAINST THE CENTER LINE OF THE WALL. WALL STUDS MIN. 24" ON CENTER. PARTED STUD: PARTED STUD TO BE INSTALLED PLUMB AND LOCATED AGAINST THE CENTER LINE OF THE WALL. WALL STUDS MIN. 24" ON CENTER. PARTED STUD TOGETHER WITH THE NAIL(S) IF A/C" DENTH RIGID OF STUD. INSTALL ALSO PER MANUFACTURER'S SPECIFICATION.
5	HDU5 (564S)	STU: STU X 8 X MIN. IF REINFORCEMENT IS CONCRETE, ANCHOR IS TO BE INSTALLED PLUMB AND LOCATED AGAINST THE CENTER LINE OF THE WALL. WALL STUDS MIN. 24" ON CENTER. PARTED STUD: PARTED STUD TO BE INSTALLED PLUMB AND LOCATED AGAINST THE CENTER LINE OF THE WALL. WALL STUDS MIN. 24" ON CENTER. PARTED STUD TOGETHER WITH THE NAIL(S) IF A/C" DENTH RIGID OF STUD. INSTALL ALSO PER MANUFACTURER'S SPECIFICATION.
8	HDU8	STU: STU X 8 X MIN. IF REINFORCEMENT IS CONCRETE, ANCHOR IS TO BE INSTALLED PLUMB AND LOCATED AGAINST THE CENTER LINE OF THE WALL. WALL STUDS MIN. 24" ON CENTER. PARTED STUD: PARTED STUD TO BE INSTALLED PLUMB AND LOCATED AGAINST THE CENTER LINE OF THE WALL. WALL STUDS MIN. 24" ON CENTER. PARTED STUD TOGETHER WITH THE NAIL(S) IF A/C" DENTH RIGID OF STUD. INSTALL ALSO PER MANUFACTURER'S SPECIFICATION.
28	MSTC28 (1535S)	STU: STU X 8 X MIN. IF REINFORCEMENT IS CONCRETE, ANCHOR IS TO BE INSTALLED PLUMB AND LOCATED AGAINST THE CENTER LINE OF THE WALL. WALL STUDS MIN. 24" ON CENTER. PARTED STUD: PARTED STUD TO BE INSTALLED PLUMB AND LOCATED AGAINST THE CENTER LINE OF THE WALL. WALL STUDS MIN. 24" ON CENTER. PARTED STUD TOGETHER WITH THE NAIL(S) IF A/C" DENTH RIGID OF STUD. INSTALL ALSO PER MANUFACTURER'S SPECIFICATION.
40	MSTC40 (3070S)	STU: STU X 8 X MIN. IF REINFORCEMENT IS CONCRETE, ANCHOR IS TO BE INSTALLED PLUMB AND LOCATED AGAINST THE CENTER LINE OF THE WALL. WALL STUDS MIN. 24" ON CENTER. PARTED STUD: PARTED STUD TO BE INSTALLED PLUMB AND LOCATED AGAINST THE CENTER LINE OF THE WALL. WALL STUDS MIN. 24" ON CENTER. PARTED STUD TOGETHER WITH THE NAIL(S) IF A/C" DENTH RIGID OF STUD. INSTALL ALSO PER MANUFACTURER'S SPECIFICATION.
52	MSTC52 (4510S)	STU: STU X 8 X MIN. IF REINFORCEMENT IS CONCRETE, ANCHOR IS TO BE INSTALLED PLUMB AND LOCATED AGAINST THE CENTER LINE OF THE WALL. WALL STUDS MIN. 24" ON CENTER. PARTED STUD: PARTED STUD TO BE INSTALLED PLUMB AND LOCATED AGAINST THE CENTER LINE OF THE WALL. WALL STUDS MIN. 24" ON CENTER. PARTED STUD TOGETHER WITH THE NAIL(S) IF A/C" DENTH RIGID OF STUD. INSTALL ALSO PER MANUFACTURER'S SPECIFICATION.

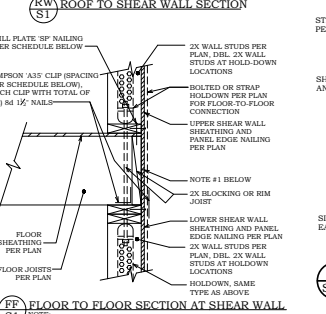
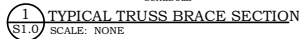
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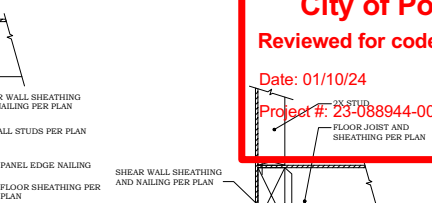
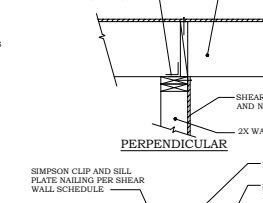
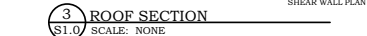
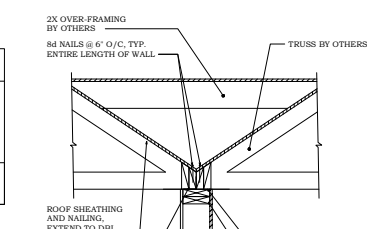
1) HOLDOWNS TO BE FASTENED TO DOUBLE STUDS (CONTINUOUS FROM SILL PLATE TO DOUBLE TOP PLATE) AT PANEL ENDS. WALL STUDS SHOULD HAVE PANEL EDGE NAILING FROM SHEAR WALL SHEATHING.

2) IF HOLDOWNS 2, 5, 6, AND 8 ARE INSTALLED FROM FLOOR TO FLOOR, REFER TO DETAIL WF/FS1.

3) U.N. INSTALL 11-14 CONTINUOUS HORIZONTAL TOP BAR 3" DOWN FROM TOP OF WALL AT ALL HOLDOWN ANCHORS. EXTEND BAR MIN. 5'-0" PAST HOLDOWN IN BOTH DIRECTIONS (END BAR ARGUED AT CORNER CONDITION). FOR THIS 10'-0" SECTION INSTALL 11-14 VERTICAL BAR @ 24" O.C. THE HOLDOWN ANCHORS TO HORIZONTAL TOP BAR.

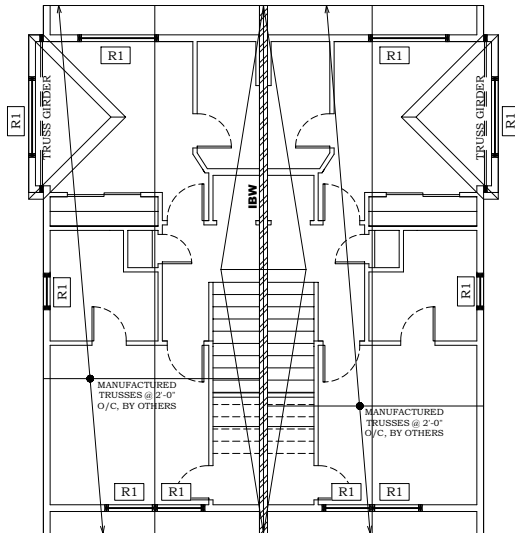
Diagram illustrating the minimum continuous panel length for a shear wall. The diagram shows a horizontal section of a wall with a central "MINIMUM CONTINUOUS PANEL LENGTH" section. This section is flanked by "SHEAR END OF PANEL" and "FIXED END OF PANEL" regions. A "HOLDOWN TYPE" is shown at the fixed end, with a dimension "d" indicating the distance from the edge of the window, door, or corner to the holdown. The "EDGE OF WINDOW, DOOR, OR CORNER" is marked at both ends of the wall section.





BEAM PER PLAN

PROJECT NAME
HUNT LOT 2
WALL AND COLUMN FOUNDATION PLAN



ROOF FRAMING PLAN

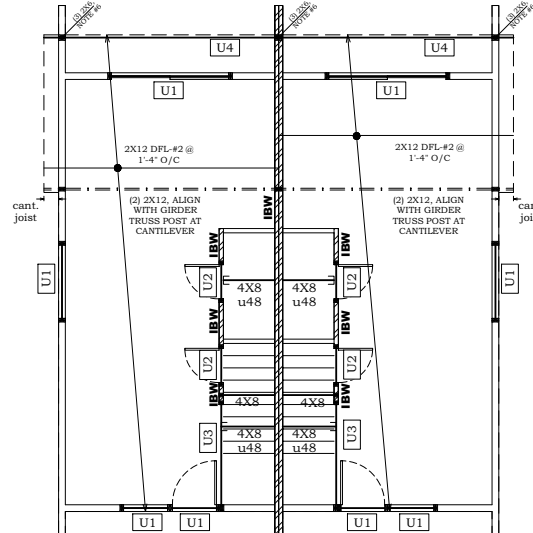
ROOF AND FLOOR FRAMING NOTES:
(1) HEADERS TO BE R1: 4X8 DFL-#2 @ 6'-0" MAX SPAN, U.N.O.
(2) INTERIOR HEADERS TO BE 4X8 DFL-#2 (MAX. SPAN 4'-0") U.N.O.
(3) REFER TO ARCHITECTURAL DRAWINGS FOR ROOF FINISHING SPECIFICATIONS AND VERIFICATION OF ALL DIMENSIONS.

BEAM SCHEDULE:

BEAM SIZE	SIZE
R1	4X8 DFL-#2

TRUSS TIES:

TRUSS TYPE	SIMPSON TIE/DOWEL	UPLIFT
TRUSS	R10A	1140#
TRUSS	R12.5A	200#
CHG. GIRDERS	U272	200#

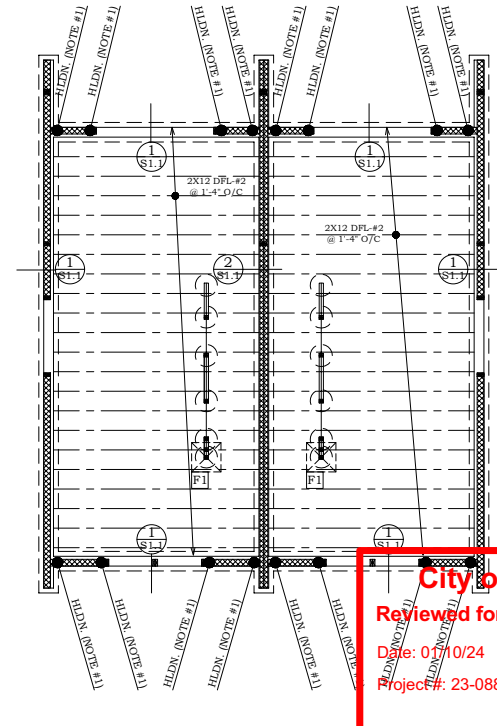


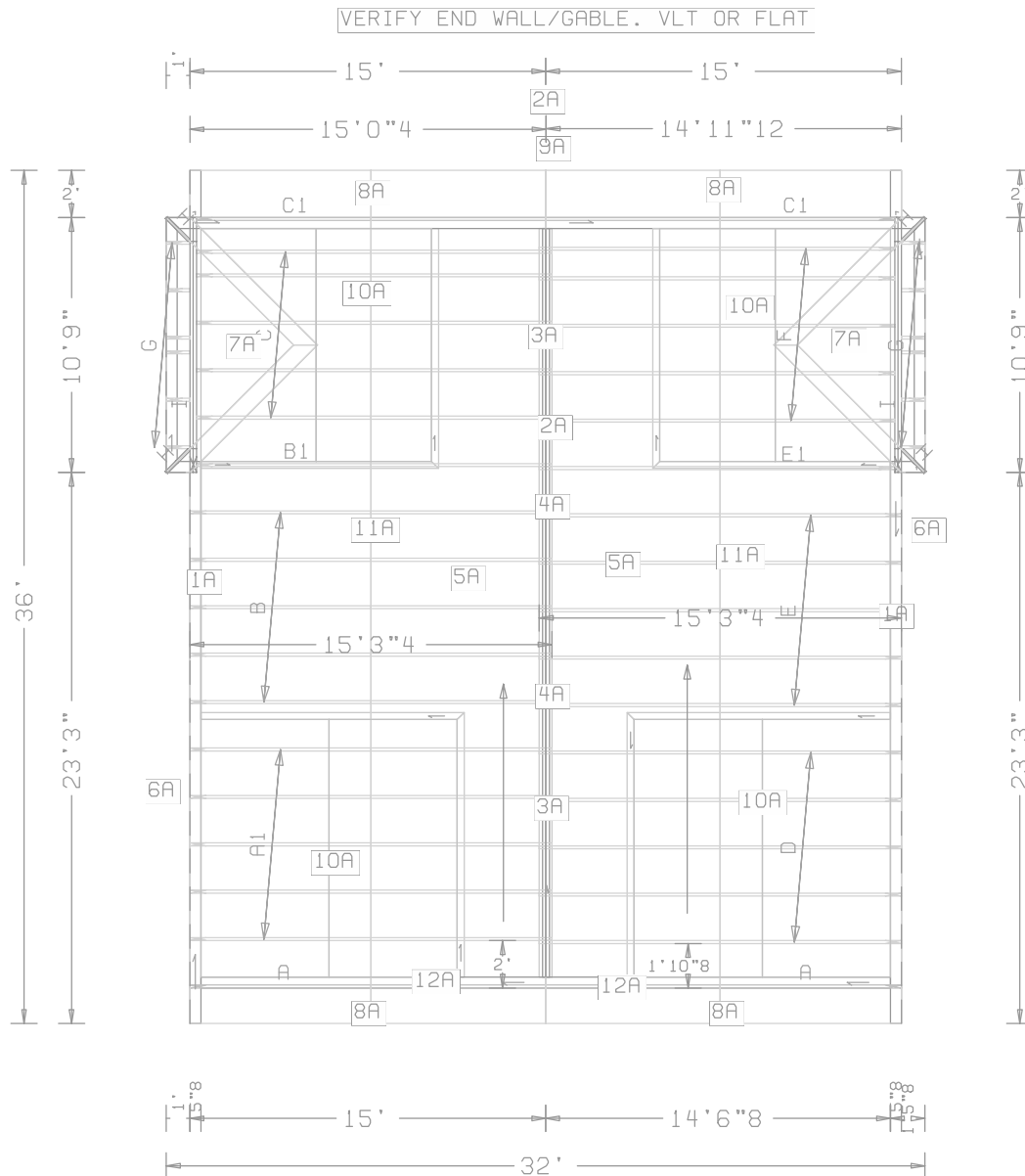
UPPER FLOOR FRAMING PLAN

UPPER FLOOR FRAMING NOTES:
(1) HEADERS TO BE 4X8 DFL-#2 (MAX. SPAN 4'-0") U.N.O.
(2) INTERIOR HEADERS TO BE 4X8 DFL-#2 (MAX. SPAN 4'-0") U.N.O.
(3) REFER TO ARCHITECTURAL DRAWINGS FOR ROOF FINISHING SPECIFICATIONS AND VERIFICATION OF ALL DIMENSIONS.
(4) TYPICAL EXTERIOR WALL POST TO BE (2) 2X6 DFL-#2 BUNDLED STUD, FASTEN EACH STUD TOGETHER WITH 16d NAILS @ 12" O/C TYP. ENTIRE LENGTH OF STUD, U.N.O.
(5) TYPICAL INTERIOR WALL POST TO BE (2) 2X4 DFL-#2 BUNDLED STUD, FASTEN EACH STUD TOGETHER WITH 16d NAILS @ 12" O/C TYP. ENTIRE LENGTH OF STUD, U.N.O.
(6) EXTERIOR POST CAPS TO BE SIMPSON PC OR EPC, IF EXPOSED CONDITION COAT PER MANUFACTURER'S SPECIFICATIONS WITH EXTERIOR EXPOSURE AND P7 MATERIAL.
(7) IBW - INTERIOR BEARING WALL.
(8) INSTALL (3) 2X6 WITH MATCHES VERTICAL STRIP AT END OF BEAM TO POST BELOW.

BEAM SCHEDULE:

BEAM SIZE	SIZE
U1	4X8 DFL-#2
U2	4X8 DFL-#2
U3	4X12 DFL-#2
U4	8" X 10" GLB 24F-V4





TRUSS NOTES :

- 1A - 7" HEEL PER PLAN
- 2A - 6-1/2" WIDE WALL ASSEMBLY. 2X4 WALL IS SET PER PLAN AT 3" TO CL FROM RIGHT SIDE OF TOTAL ASSEMBLY. VERIFY
- 3A - TRUSSES SIDE STEP EACH OTHER OVER THE WALL HERE.
- 4A - TRIPPLE BLOCKS CENTERED ON 6-1/2" ASSEMBLY
- 5A - HAND FRAME CRICKET ON SITE
- 6A - NO EAVES PER PLAN. VERIFY
- 7A - HAND FRAME OVER-LAY ON SITE
- 8A - FRAME THIS AREA PER PLAN ON SITE
- 9A - CENER LINE OF 2X4 BEARING WALL
- 10A - 6/12 VLT CENTERED IN ROOM
- 11A - FLAT CLG HERE
- 12A - GABLES BUTT TO BUTT HEEL. LEFT SIDE RUN START IS 24". LEFT SIDE IS 1'-10-1/2" .

City of Portland
Reviewed for code compliance
Date: 01/10/24
Project #: 23-088944-000-00-RS

Customer: BTS Development LLC
Desc: BTS N Hunt project
ADDRESS: xxxx N Hunt st
Designed by: BILLY BREESE

JOB NO:
0823082

PAGE NO:
1 OF 1



City of Portland

Reviewed for code compliance

Date: 8/21/2023

Project #: 23-088944-000150-RS

Alpine, an ITW Company

155 Harlem Ave

North Building, 4th Floor

Glenview, IL 60025

Phone: (800)877-3678 (916)387-0116
Fax: (916)387-1110
www.alpineitw.com

Site Information:	Page 1:
Customer: Truss Components of Oregon, Inc.	Job Number: 0823082
Job Description: BTS N Hunt project	
Address: xxxx N Hunt st, PORTLAND, OR	

Job Engineering Criteria:			
Design Code: IBC 2018		IntelliVIEW Version: 22.02.01	
		JRef #: 1XS571750005	
Wind Standard: ASCE 7-16	Wind Speed (mph): 120	Design Loading (psf): 42.00	
Building Type: Closed			

This package contains general notes pages, 13 truss drawing(s) and 3 detail(s).

Item	Drawing Number	Truss
1	223.23.1709.41710	A
3	223.23.1709.46793	B
5	223.23.1711.19470	C
7	223.23.1710.33060	D
9	223.23.1710.38060	E1
11	223.23.1710.46673	G
13	223.23.1711.45443	I
15	GABRST160118	

Item	Drawing Number	Truss
2	223.23.1709.44577	A1
4	223.23.1709.49530	B1
6	223.23.1710.30063	C1
8	223.23.1710.36063	E
10	223.23.1710.41287	F
12	223.23.1711.05997	H
14	A12030ENC160118	
16	GBLLETIN0118	

General Notes

Truss Design Engineer Scope of Work, Design Assumptions and Design Responsibilities:

The design responsibilities assumed in the preparation of these design drawings are those specified in ANSI/TPI 1, Chapter 2; and the National Design Standard for Metal Plate Connected Wood Truss Construction, by the Truss Plate Institute. The truss component designs conform to the applicable provisions of ANSI/TPI 1 and NDS, the National Design Specification for Wood Construction by AWC. The truss component designs are based on the specified loading and dimension information furnished by others to the Truss Design Engineer. The Truss Design Engineer has no duty to independently verify the accuracy or completeness of the information provided by others and may rely on that information without liability. The responsibility for verification of that information remains with others neither employed nor controlled by the Truss Design Engineer. The Truss Design Engineer's seal and signature on the attached drawings, or cover page listing these drawings, indicates acceptance of professional engineering responsibility solely for the truss component designs and not for the technical information furnished by others which technical information and consequences thereof remain their sole responsibility.

The suitability and use of these drawings for any particular structure is the responsibility of the Building Designer in accordance with ANSI/TPI 1 Chapter 2. The Building Designer is responsible for determining that the dimensions and loads for each truss component match those required by the plans and by the actual use of the individual component, and for ascertaining that the loads shown on the drawings meet or exceed applicable building code requirements and any additional factors required in the particular application. Truss components using metal connector plates with integral teeth shall not be placed in environments that will cause the moisture content of the wood in which plates are embedded to exceed 19% and/or cause corrosion of connector plates and other metal fasteners.

The Truss Design Engineer shall not be responsible for items beyond the specific scope of the agreed contracted work set forth herein, including but not limited to: verifying the dimensions of the truss component, calculation of any of the truss component design loads, inspection of the truss components before or after installation, the design of temporary or permanent bracing and their attachment required in the roof and/or floor systems, the design of diaphragms or shear walls, the design of load transfer connections to and from diaphragms and shear walls, the design of load transfer to the foundation, the design of connections for truss components to their bearing supports, the design of the bearing supports, installation of the truss components, observation of the truss component installation process, review of truss assembly procedures, sequencing of the truss component installation, construction means and methods, site and/or worker safety in the installation of the truss components and/or its connections.

This document may be a high quality facsimile of the original engineering document which is a digitally signed electronic file with third party authentication. A wet or embossed seal copy of this engineering document is available upon request.

Temporary Lateral Restraint and Bracing:

Temporary lateral restraint and diagonal bracing shall be installed according to the provisions of BCSI chapters B1, B2, B7 and/or B10 (Building Component Safety Information, by TPI and SBCA), or as specified by the Building Designer or other Registered Design Professional. The required locations for lateral restraint and/or bracing depicted on these drawings are only for the permanent lateral support of the truss members to reduce buckling lengths, and do not apply to and may not be relied upon for the temporary stability of the truss components during their installation.

Permanent Lateral Restraint and Bracing:

The required locations for lateral restraint or bracing depicted on these drawings are for the permanent lateral support of the truss members to reduce buckling lengths. Permanent lateral support shall be installed according to the provisions of BCSI chapters B3, B7 and/or B10, or as specified by the Building Designer or other Registered Design Professional. These drawings do not depict or specify installation/erection bracing, wind bracing, portal bracing or similar building stability bracing which are parts of the overall building design to be specified, designed, and detailed by the Building Designer.

Connector Plate Information:

Alpine connector plates are made of ASTM A653 or ASTM A1063 galvanized steel with the following designations, gauges and grades: W=Wave, 20ga, grade 40; H=High Strength, 20ga, grade 60; S=Super Strength, 18ga, grade 60. Information on model code compliance is contained in the ICC Evaluation Service report ESR-1118, available on-line at www.icc-es.org.

Fire Retardant Treated Lumber:

Fire retardant treated lumber must be properly re-dried and maintained below 19% or less moisture level through all stages of construction and usage. Fire retardant treated lumber may be more brittle than untreated lumber. Special handling care must be taken to prevent breakage during all handling activities.

General Notes (continued)

Key to Terms:

Information provided on drawings reflects a summary of the pertinent information required for the truss design. Detailed information on load cases, reactions, member lengths, forces and members requiring permanent lateral support may be found in calculation sheets available upon written request.

BCDL = Bottom Chord standard design Dead Load in pounds per square foot.

BCLL = Bottom Chord standard design Live Load in pounds per square foot.

CL = Certified lumber.

Des Ld = total of TCLL, TCDL, BCLL and BCDL Design Load in pounds per square foot.

FRT = Fire Retardant Treated lumber.

FRT-DB = D-Blaze Fire Retardant Treated lumber.

FRT-DC = Dricon Fire Retardant Treated lumber.

FRT-FP = FirePRO Fire Retardant Treated lumber.

FRT-FL = FlamePRO Fire Retardant Treated lumber.

FRT-FT = FlameTech Fire Retardant Treated lumber.

FRT-PG = PYRO-GUARD Fire Retardant Treated lumber.

FRT-PR = ProWood Fire Retardant Treated lumber.

g = green lumber.

HORZ(LL) = maximum Horizontal panel point deflection due to Live Load, in inches.

HORZ(TL) = maximum Horizontal panel point long term deflection in inches, due to Total Load, including creep adjustment.

HPL = additional Horizontal Load added to a truss Piece in pounds per linear foot or pounds.

Ic = Incised lumber.

FJ = Finger Jointed lumber.

L/# = user specified divisor for limiting span/deflection ratio for evaluation of actual L/defl value.

L/defl = ratio of Length between bearings, in inches, divided by the vertical Deflection due to creep, in inches, at the referenced panel point. Reported as 999 if greater than or equal to 999.

Loc = Location, starting location of left end of bearing or panel point (joint) location of deflection.

Max BC CSI = Maximum bending and axial Combined Stress Index for Bottom Chords for all load cases.

Max TC CSI = Maximum bending and axial Combined Stress Index for Top Chords for all load cases.

Max Web CSI = Maximum bending and axial Combined Stress Index for Webs for all load cases.

NCBCLL = Non-Concurrent Bottom Chord design Live Load in pounds per square foot.

PL = additional Load applied at a user specified angle on a truss Piece in pounds per linear foot or pounds.

PLB = additional vertical load added to a Bottom chord Piece of a truss in pounds per linear foot or pounds

PLT = additional vertical load added to a Top chord Piece of a truss in pounds per linear foot or pounds.

PP = Panel Point.

R = maximum downward design Reaction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

-R = maximum upward design Reaction, in pounds, from all specified gravity load cases, at the identified location (Loc).

Rh = maximum horizontal design Reaction in either direction, in pounds, from all specified gravity load cases, at the indicated location (Loc).

RL = maximum horizontal design Reaction in either direction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

Rw = maximum downward design Reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the identified location (Loc).

TCDL = Top Chord standard design Dead Load in pounds per square foot.

TCLL = Top Chord standard design Live Load in pounds per square foot.

U = maximum Upward design reaction, in pounds, from all specified non-gravity (wind or seismic) load cases, at the indicated location (Loc).

VERT(CL) = maximum Vertical panel point deflection in inches due to Live Load and Creep Component of Dead Load in inches.

VERT(CTL) = maximum Vertical panel point deflection ratios due to Live Load and Creep Component of Dead Load, and maximum long term Vertical panel point deflection in inches due to Total load, including creep adjustment.

VERT(LL) = maximum Vertical panel point deflection in inches due to Live Load.

VERT(TL) = maximum Vertical panel point long term deflection in inches due to Total load, including creep adjustment.

W = Width of non-hanger bearing, in inches.

Refer to ASCE-7 for Wind and Seismic abbreviations.

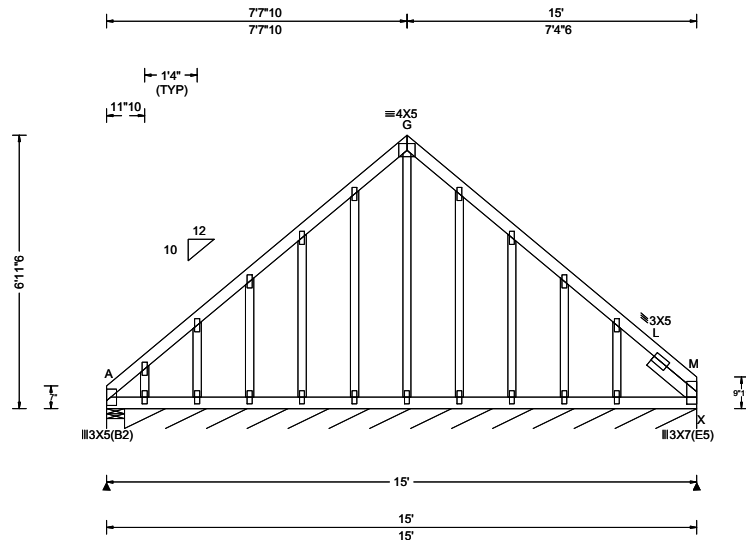
Uppercase Acronyms not explained above are as defined in TPI 1.

References:

1. AWC: American Wood Council; 222 Catoctin Circle SE, Suite 201; Leesburg, VA 20175; www.awc.org
2. ICC: International Code Council; www.iccsafe.org.
3. Alpine, a division of ITW Building Components Group Inc.: 155 Harlem Ave, North Building, 4th Floor, Glenview, IL 60025; www.alpineitw.com.
4. TPI: Truss Plate Institute, 2670 Crain Highway, Suite 203, Waldorf, MD 20601; www.tpinst.org.
5. SBCA: Wood Truss Council of America, 6300 Enterprise Lane, Madison, WI 53719; www.sbcacomponents.com.

SEQN: 357924 GABL Ply: 1 Job Number: 0823082
 FROM: BB Qty: 2 BTS N Hunt project
 Truss Label: A

Case R-1113-0 Ref: 1X557175003P13
 Drawn: 223.23.1709.41710
 Date: 07/10/24 / JAK 08/11/2023
 Project #: 23-000944-000-00-RS



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 25.00	Wind Std: ASCE 7-16	Pg: 25.0 Ct: 1.1 CAT: II	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 120 mph	Pf: 21.2 Ce: 1.1	VERT(LL): 0.002 L 999 360	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: - Cs: 0.93	VERT(CL): 0.005 L 999 240	A 93 /- /- /115 /54 /130
BCDL: 7.00	Risk Category: II	Snow Duration: 1.15	HORZ(LL): 0.004 L - -	X* 114 /- /- /49 /15 /-
Des Ld: 42.00	EXP: B Kzt: NA		HORZ(TL): 0.005 L - -	Wind reactions based on MWFRS
NCBCLL: 10.00	Mean Height: 21.76 ft	Building Code:	Creep Factor: 2.0	A Brg Wid = 5.5 Min Req = 1.5 (Truss)
Soffit: 2.00	TCDL: 4.2 psf	IBC 2018	Max TC CSI: 0.071	X Brg Wid = 174 Min Req = -
Load Duration: 1.15	BCDL: 4.2 psf	TPI Std: 2014	Max BC CSI: 0.025	Bearings A & A are a rigid surface.
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Varies by Ld Case	Max Web CSI: 0.103	Members not listed have forces less than 375#
	C&C Dist a: 3.00 ft	FT/RT: 2(0)/4(0)		
	Loc. from endwall: Any	Plate Type(s):		
	GCpi: 0.18	WAVE	VIEW Ver: 22.02.01.1115.14	
	Wind Duration: 1.60			

Lumber

Top chord: 2x4 HF #2;
 Bot chord: 2x4 HF #2;
 Webs: 2x3 HF #2;
 Rt Slider: 2x4 HF Standard + HF Stud; block length = 1.500'

Plating Notes

All plates are 1.5X4 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0" span opposite face. Top chord may be notched 1.5" deep X 3.5" at 48" o/c along top edge. DO NOT OVERCUT. No knots or other lumber defects allowed within 12" of notches. Do not notch in overhang or heel panel, unless specified otherwise. Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.



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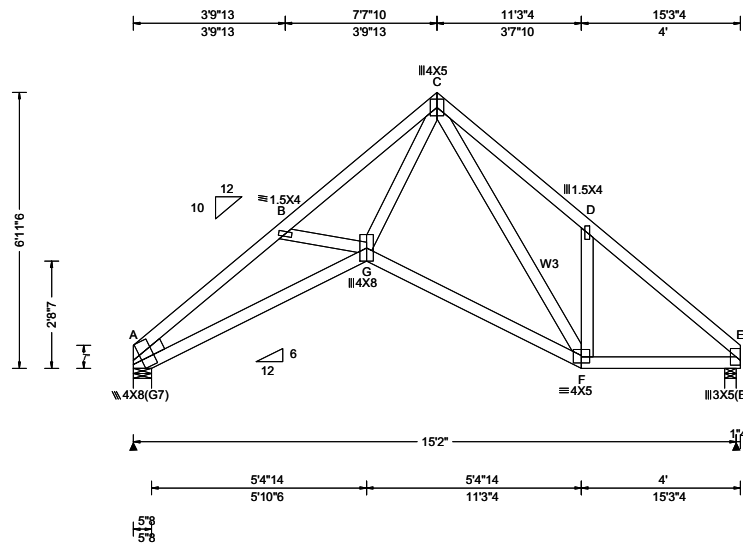
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 P. O. Box 468
 CORNELIUS OR 97113
 (503)357-2118



155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025

SEQN: 357926 COMN Ply: 1 Job Number: 0823082
 FROM: BB Qty: 5 BTS N Hunt project
 Truss Label: A1

Case R-1113 Order 1X55175003
 Drawn: 223.23.1709.44577
 Date: 07/10/24
 / JAK 08/11/2023
 Project #: 23-000944-000-00-RS



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 7.00 Des Ld: 42.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 21.76 ft TCDL: 4.2 psf BCDL: 4.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: 25.0 Ct: 1.1 CAT: II Pf: 21.2 Ce: 1.1 Lu: - Cs: 0.93 Snow Duration: 1.15 Building Code: IBC 2018 TPI Std: 2014 Rep Fac: Yes FT/RT: 2(0)/4(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.074 G 999 360 VERT(CL): 0.134 G 999 240 HORZ(LL): 0.069 E - - HORZ(TL): 0.125 E - - Creep Factor: 2.0 Max TC CSI: 0.430 Max BC CSI: 0.603 Max Web CSI: 0.530 VIEW Ver: 22.02.01.1115.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 688 - / - / 322 / 48 / 116 E 706 - / - / 335 / 48 / - Wind reactions based on MWFRS A Brg Wid = 5.5 Min Req = 1.5 (Truss) E Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings A & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 114 - 1638 C - D 178 - 813 B - C 74 - 1356 D - E 73 - 849

Lumber

Top chord: 2x4 HF #2;
 Bot chord: 2x4 HF #2;
 Webs: 2x4 :HF Standard + HF Stud;; W3 2x4 HF #2;
 Lt Stub Wedge: 2x4 HF Standard + HF Stud;

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right cantilever is exposed to wind

Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - G	1288 -56	F - E	847 -11
G - F	631 0		

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.
G - C	1040 0



Renews: 6/30/2025
 08/11/2023

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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org

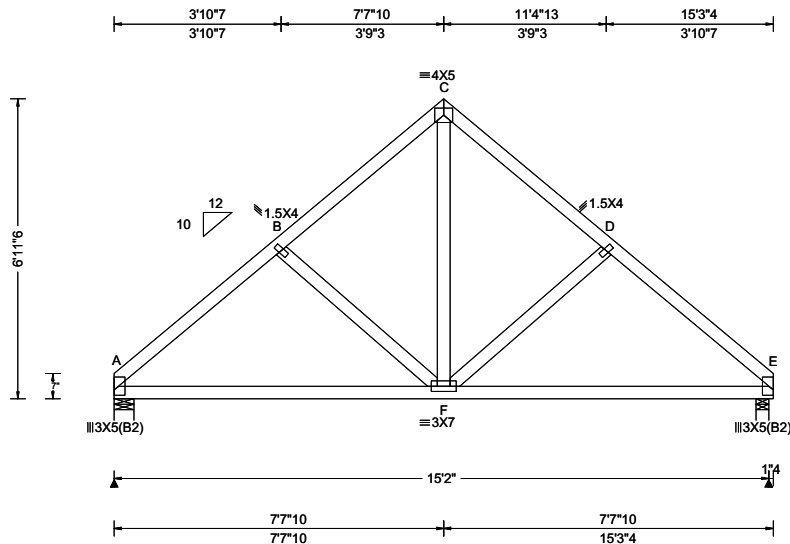
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 P. O. Box 468
 CORNELIUS OR 97113
 (503)357-2118



155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025

SEQN: 357928 COMN Ply: 1 Job Number: 0823082
 FROM: BB Qty: 5 BTS N Hunt project
 Truss Label: B

Case R-1113 Order 1X557175003114
 Drawn: 223.23.1709.46793
 Date: 01/10/24
 / JAK 08/11/2023
 Project #: 23-000944-000-00-RS



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg,Pf in PSF)	Defl/CSI Criteria	Maximum Reactions (lbs)
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 7.00 Des Ld: 42.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 21.76 ft TCDL: 4.2 psf BCDL: 4.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: 25.0 Ct: 1.1 CAT: II Pf: 21.2 Ce: 1.1 Lu: - Cs: 0.93 Snow Duration: 1.15 Building Code: IBC 2018 TPI Std: 2014 Rep Fac: Yes FT/RT:2(0)/4(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.026 D 999 360 VERT(CL): 0.047 D 999 240 HORZ(LL): 0.013 E - - HORZ(TL): 0.023 E - - Creep Factor: 2.0 Max TC CSI: 0.169 Max BC CSI: 0.385 Max Web CSI: 0.189 VIEW Ver: 22.02.01.1115.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 678 /- /- /320 /10 /116 E 697 /- /- /329 /10 /- Wind reactions based on MWFRS A Brg Wid = 5.5 Min Req = 1.5 (Truss) E Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings A & E are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 49 -829 C - D 64 -613 B - C 64 -613 D - E 48 -831

Lumber

Top chord: 2x4 HF #2;
 Bot chord: 2x4 HF #2;
 Webs: 2x4 :HF Standard + HF Stud;;

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right cantilever is exposed to wind

Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords	Tens.Comp.	Chords	Tens. Comp.
A - F	566 -23	F - E	825 0

Maximum Web Forces Per Ply (lbs)

Webs	Tens.Comp.
C - F	378 -28



Renews: 6/30/2025
 08/11/2023

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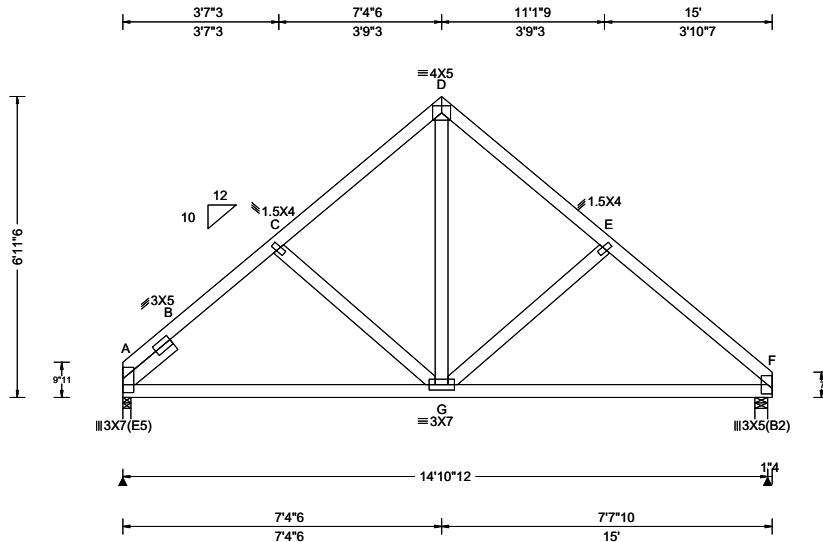
For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

Truss Components of Oregon, Inc.
 P. O. Box 468
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 (503)357-2118



155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025

SEQN: 357931 COMN Ply: 1 Job Number: 0823082
 FROM: BB Qty: 1 BTS N Hunt project
 Truss Label: B1



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 25.00	Wind Std: ASCE 7-16	Pg: 25.0 Ct: 1.1 CAT: II	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 120 mph	Pf: 21.2 Ce: 1.1	VERT(LL): 0.025 E 999 360	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: - Cs: 0.93	VERT(CL): 0.044 E 999 240	A 664 /- /- /312 /10 /116
BCDL: 7.00	Risk Category: II	Snow Duration: 1.15	HORZ(LL): 0.012 F - -	F 687 /- /- /325 /10 /-
Des Ld: 42.00	EXP: B Kzt: NA		HORZ(TL): 0.022 F - -	Wind reactions based on MWFRS
NCBCLL: 10.00	Mean Height: 21.76 ft	Building Code:	Creep Factor: 2.0	A Brg Wid = 2.3 Min Req = 1.5 (Truss)
Soffit: 2.00	TCDL: 4.2 psf	IBC 2018	Max TC CSI: 0.209	F Brg Wid = 3.5 Min Req = 1.5 (Truss)
Load Duration: 1.15	BCDL: 4.2 psf	TPI Std: 2014	Max BC CSI: 0.376	Bearings A & F are a rigid surface.
Spacing: 24.0 "	MWFRS Parallel Dist: h/2 to h	Rep Fac: Yes	Max Web CSI: 0.182	Members not listed have forces less than 375#
	C&C Dist a: 3.00 ft	FT/RT: 2(0)/4(0)		Maximum Top Chord Forces Per Ply (lbs)
	Loc. from endwall: not in 9.00 ft	Plate Type(s):	VIEW Ver: 22.02.01.1115.14	Chords Tens.Comp. Chords Tens. Comp.
	GCpi: 0.18	WAVE		A - B 96 -859 D - E 64 -601
	Wind Duration: 1.60			B - C 47 -754 E - F 48 -819
				C - D 64 -601

Maximum Bot Chord Forces Per Ply (lbs)				
Chords	Tens.Comp.	Chords	Tens. Comp.	
A - G	544 -26	G - F	810	0

Lumber

Top chord: 2x4 HF #2;
 Bot chord: 2x4 HF #2;
 Webs: 2x4 :HF Standard + HF Stud;;
 Lt Slider: 2x4 HF Standard + HF Stud; block length = 1.500'

Loading

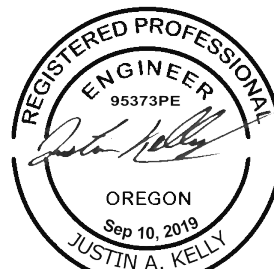
Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right cantilever is exposed to wind

Wind loading based on both gable and hip roof types.



Renews: 6/30/2025
 08/11/2023

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For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

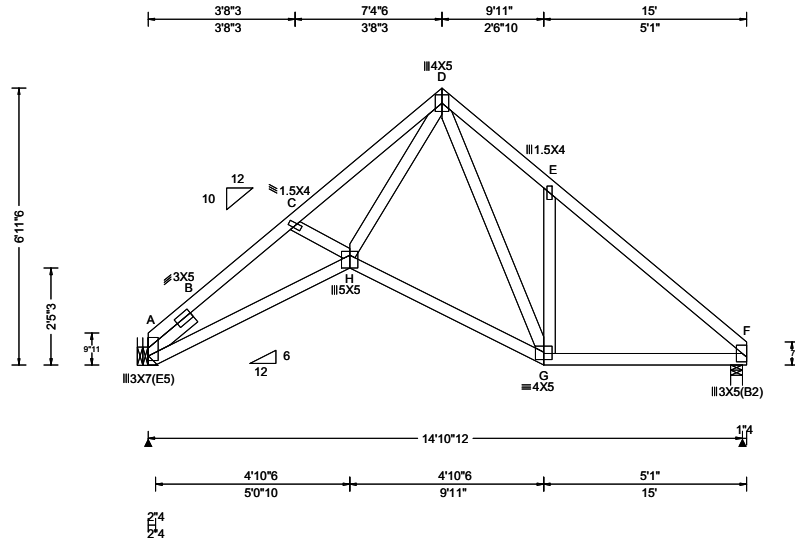
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 P. O. Box 468
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 (503)357-2118



155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025

SEQN: 357933 COMN Ply: 1 Job Number: 0823082
 FROM: BB Qty: 5 BTS N Hunt project
 Truss Label: C

Case R-1113-0 Ref: 1X5571750003 P-19
 Draw No: 223.23.1711.19470
 Date: 07/10/24 / JAK 08/11/2023
 Project #: 23-000944-000-00-RS



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 7.00 Des Ld: 42.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 21.76 ft TCDL: 4.2 psf BCDL: 4.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: 25.0 Ct: 1.1 CAT: II Pf: 21.2 Ce: 1.1 Lu: - Cs: 0.93 Snow Duration: 1.15 Building Code: IBC 2018 TPI Std: 2014 Rep Fac: Yes FT/RT: 2(0)/4(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.061 H 999 360 VERT(CL): 0.111 H 999 240 HORZ(LL): 0.054 F - - HORZ(TL): 0.098 F - - Creep Factor: 2.0 Max TC CSI: 0.279 Max BC CSI: 0.342 Max Web CSI: 0.517 VIEW Ver: 22.02.01.1115.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 675 /- /- /319 /45 /116 F 692 /- /- /329 /48 /- Wind reactions based on MWFRS A Brg Wid = - Min Req = - F Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearing F is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 95 - 1563 D - E 181 - 739 B - C 101 - 1502 E - F 76 - 808 C - D 100 - 1339

Lumber

Top chord: 2x4 HF #2;
 Bot chord: 2x4 HF #2;
 Webs: 2x4 :HF Standard + HF Stud;
 Lt Slider: 2x4 HF Standard + HF Stud; block length = 1.500'

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right cantilever is exposed to wind

Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

Chords Tens.Comp. Chords Tens. Comp.

A - H 1199 -48 G - F 754 0
 H - G 544 0

Maximum Web Forces Per Ply (lbs)

Webs Tens.Comp.

H - D 1015 0



Renews: 6/30/2025
 08/11/2023

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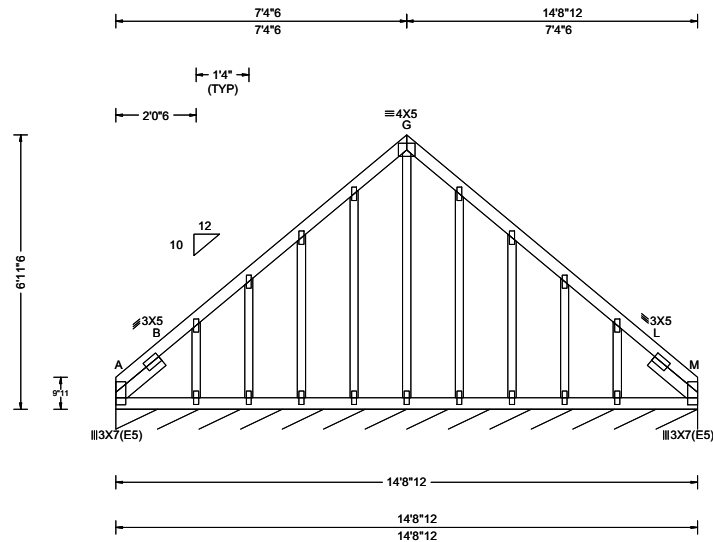
Truss Components of Oregon, Inc.
 P. O. Box 468
 CORNELIUS OR 97113
 (503)357-2118



155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025

SEQN: 357944 GABL Ply: 1 Job Number: 0823082
 FROM: BB Qty: 2 BTS N Hunt project
 Truss Label: C1

Case R-1113 Order 1X557175003P12
 Draw No: 223.23.1710.30063
 Date: 01/10/24
 / JAK 08/11/2023
 Project #: 23-008944-000-00-RS



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs), or *=PLF
TCLL: 25.00	Wind Std: ASCE 7-16	Pg: 25.0 Ct: 1.1 CAT: II	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 120 mph	Pf: 21.2 Ce: 1.1	VERT(LL): 0.002 B 999 360	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: - Cs: 0.93	VERT(CL): 0.004 B 999 240	M* 117 /- /- /45 /9 /9
BCDL: 7.00	Risk Category: II	Snow Duration: 1.15	HORZ(LL): -0.003 B - -	Wind reactions based on MWFRS
Des Ld: 42.00	EXP: B Kzt: NA		HORZ(TL): 0.004 B - -	M Brg Wid = 176 Min Req = -
NCBCLL: 10.00	Mean Height: 21.88 ft	Building Code:	Creep Factor: 2.0	Bearing A is a rigid surface.
Soffit: 2.00	TCDL: 4.2 psf	IBC 2018	Max TC CSI: 0.070	Members not listed have forces less than 375#
Load Duration: 1.15	BCDL: 4.2 psf	TPI Std: 2014	Max BC CSI: 0.027	
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Varies by Ld Case	Max Web CSI: 0.100	
	C&C Dist a: 3.00 ft	FT/RT:2(0)/4(0)		
	Loc. from endwall: Any	Plate Type(s):	VIEW Ver: 22.02.01.1115.14	
	GCpi: 0.18	WAVE		
	Wind Duration: 1.60			

Lumber

Top chord: 2x4 HF #2;
 Bot chord: 2x4 HF #2;
 Webs: 2x3 HF #2;
 Lt Slider: 2x4 HF Standard + HF Stud; block length = 1.500'
 Rt Slider: 2x4 HF Standard + HF Stud; block length = 1.500'

Plating Notes

All plates are 1.5X4 except as noted.

Loading

Truss designed to support 1-0-0 top chord outlookers and cladding load not to exceed 5.00 PSF one face and 24.0' span opposite face. Top chord may be notched 1.5" deep X 3.5" at 48" o/c along top edge. DO NOT OVERCUT. No knots or other lumber defects allowed within 12" of notches. Do not notch in overhang or heel panel, unless specified otherwise. Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.
 Wind loading based on both gable and hip roof types.

Additional Notes

See DWGS A12030ENC160118, GBLLETIN0118, & GABRST160118 for gable wind bracing and other requirements.



Renews: 6/30/2025
 08/11/2023

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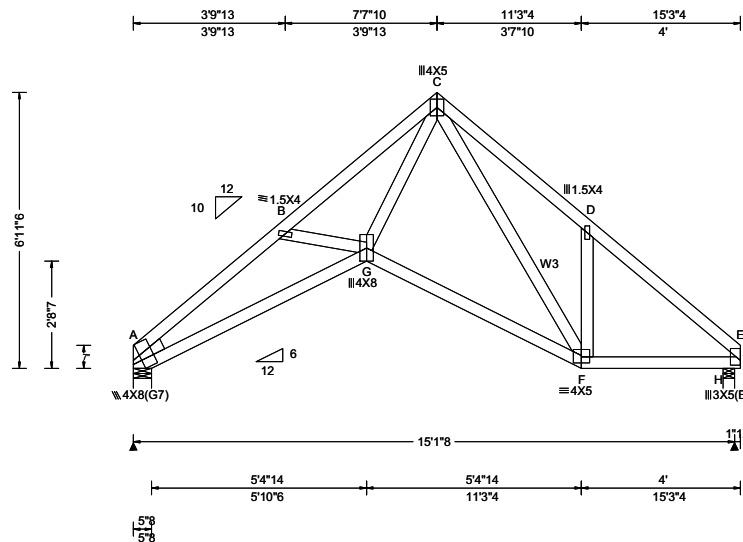
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 P. O. Box 468
 CORNELIUS OR 97113
 (503)357-2118



155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025

SEQN: 357935 COMN Ply: 1 Job Number: 0823082
 FROM: BB Qty: 5 BTS N Hunt project
 Truss Label: D

Case R-1113 Order 1X557175003P16
 Drawn: 223.23.1710.33060
 Date: 07/10/24
 / JAK 08/11/2023
 Project #: 23-000944-000-00-RS



Loading Criteria (psf)
 TCCL: 25.00
 TCDL: 10.00
 BCCL: 0.00
 BCDL: 7.00
 Des Ld: 42.00
 NCBCCL: 10.00
 Soffit: 2.00
 Load Duration: 1.15
 Spacing: 24.0 "

Wind Criteria
 Wind Std: ASCE 7-16
 Speed: 120 mph
 Enclosure: Closed
 Risk Category: II
 EXP: B Kzt: NA
 Mean Height: 21.76 ft
 TCDL: 4.2 psf
 BCDL: 4.2 psf
 MWFRS Parallel Dist: 0 to h/2
 C&C Dist a: 3.00 ft
 Loc. from endwall: Any
 GCpi: 0.18
 Wind Duration: 1.60

Snow Criteria (Pg,Pf in PSF)
 Pg: 25.0 Ct: 1.1 CAT: II
 Pf: 21.2 Ce: 1.1
 Lu: - Cs: 0.93
 Snow Duration: 1.15
 Building Code:
 IBC 2018
 TPI Std: 2014
 Rep Fac: Yes
 FT/RT:2(0)/4(0)
 Plate Type(s):
 WAVE

Defl/CSI Criteria
 PP Deflection in loc L/def L/#
 VERT(LL): 0.074 G 999 360
 VERT(CL): 0.135 G 999 240
 HORZ(LL): 0.069 E - -
 HORZ(TL): 0.126 E - -
 Creep Factor: 2.0
 Max TC CSI: 0.429
 Max BC CSI: 0.601
 Max Web CSI: 0.529
 VIEW Ver: 22.02.01.1115.14

▲ Maximum Reactions (lbs)							
Gravity				Non-Gravity			
Loc	R+	/ R-	/ Rh	/ Rw	/ U	/ RL	
A	686	/-	/-	/321	/48	/116	
H	708	/-	/-	/337	/48	/-	
Wind reactions based on MWFRS							
A	Brg Wid = 5.5		Min Req = 1.5 (Truss)				
H	Brg Wid = 3.5		Min Req = 1.5 (Truss)				
Bearings A & H are a rigid surface.							
Members not listed have forces less than 375#							
Maximum Top Chord Forces Per Ply (lbs)							
Chords		Tens.Comp.		Chords		Tens. Comp.	
A - B	114	-1633	C - D	178	-803		
B - C	73	-1351	D - E	73	-839		

Maximum Bot Chord Forces Per Ply (lbs)				
Chords	Tens.Comp.		Chords	Tens. Comp.
A - G	1284	-57	F - E	876 -11
G - F	628	0		

Maximum Web Forces Per Ply (lbs)		
Webs	Tens.Comp.	
G - C	1037	0

Lumber

Top chord: 2x4 HF #2;
 Bot chord: 2x4 HF #2;
 Webs: 2x4 :HF Standard + HF Stud;; W3 2x4 HF #2;
 Lt Stub Wedge: 2x4 HF Standard + HF Stud;

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right cantilever is exposed to wind

Wind loading based on both gable and hip roof types.



Renews: 6/30/2025
 08/11/2023

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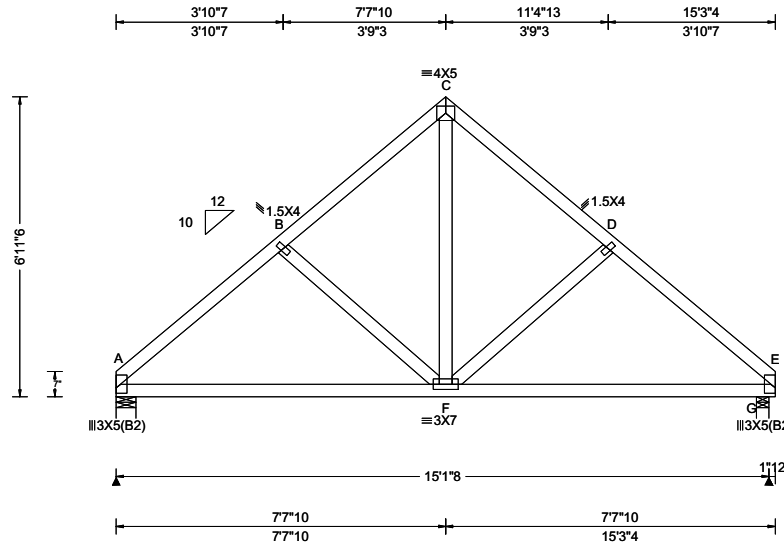
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 P. O. Box 468
 CORNELIUS OR 97113
 (503)357-2118



155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025

SEQN: 357937 COMN Ply: 1 Job Number: 0823082
 FROM: BB Qty: 5 BTS N Hunt project
 Truss Label: E

Case R-1113 Order 1X5571750003
 Draw No: 223.23.1710.36063
 Date: 01/10/24
 / JAK 08/11/2023
 Project #: 23-000944-000-00-RS



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 7.00 Des Ld: 42.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 21.76 ft TCDL: 4.2 psf BCDL: 4.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCp: 0.18 Wind Duration: 1.60	Pg: 25.0 Ct: 1.1 CAT: II Pf: 21.2 Ce: 1.1 Lu: - Cs: 0.93 Snow Duration: 1.15 Building Code: IBC 2018 TPI Std: 2014 Rep Fac: Yes FT/RT: 2(0)/4(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.028 D 999 360 VERT(CL): 0.051 D 999 240 HORZ(LL): 0.013 E - - HORZ(TL): 0.024 E - - Creep Factor: 2.0 Max TC CSI: 0.185 Max BC CSI: 0.413 Max Web CSI: 0.186 VIEW Ver: 22.02.01.1115.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 676 /- /- /319 /10 /116 G 698 /- /- /331 /9 /- Wind reactions based on MWFRS A Brg Wid = 5.5 Min Req = 1.5 (Truss) G Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings A & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 49 -825 C - D 64 -609 B - C 64 -609 D - E 48 -827

Lumber

Top chord: 2x4 HF #2;
 Bot chord: 2x4 HF #2;
 Webs: 2x4 :HF Standard + HF Stud;;

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right cantilever is exposed to wind

Wind loading based on both gable and hip roof types.



Renews: 6/30/2025
 08/11/2023

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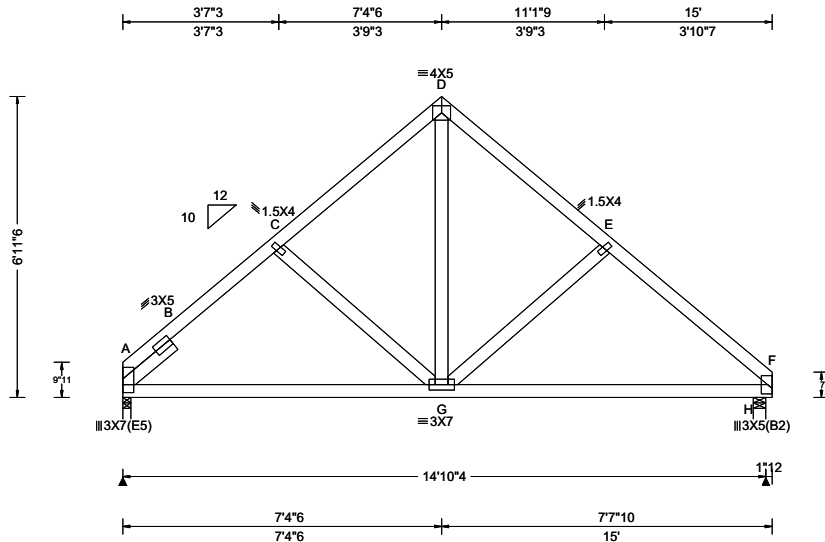
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 P. O. Box 468
 CORNELIUS OR 97113
 (503)357-2118



155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025

SEQN: 357940 COMN Ply: 1 Job Number: 0823082
 FROM: BB Qty: 1 BTS N Hunt project
 Truss Label: E1

Case R-1113 Order 1X55175003
 Drawn: 223.23.1710.38060
 Date: 01/10/24
 / JAK 08/11/2023
 Project #: 23-000944-000-00-RS



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 7.00 Des Ld: 42.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 21.76 ft TCDL: 4.2 psf BCDL: 4.2 psf MWFRS Parallel Dist: h/2 to h C&C Dist a: 3.00 ft Loc. from endwall: not in 9.00 ft GCpi: 0.18 Wind Duration: 1.60	Pg: 25.0 Ct: 1.1 CAT: II Pf: 21.2 Ce: 1.1 Lu: - Cs: 0.93 Snow Duration: 1.15 Building Code: IBC 2018 TPI Std: 2014 Rep Fac: Yes FT/RT: 2(0)/4(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.027 E 999 360 VERT(CL): 0.049 E 999 240 HORZ(LL): 0.013 F - - HORZ(TL): 0.023 F - - Creep Factor: 2.0 Max TC CSI: 0.210 Max BC CSI: 0.405 Max Web CSI: 0.179 VIEW Ver: 22.02.01.1115.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL A 662 -/- /- /311 /9 /116 H 689 -/- /- /327 /9 /- Wind reactions based on MWFRS A Brg Wid = 2.3 Min Req = 1.5 (Truss) H Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearings A & H are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 96 -854 D - E 64 -596 B - C 47 -750 E - F 48 -814 C - D 64 -598

Maximum Bot Chord Forces Per Ply (lbs)				
Chords	Tens.Comp.	Chords	Tens. Comp.	
A - G	542 -26	G - F	842 0	

Lumber

Top chord: 2x4 HF #2;
 Bot chord: 2x4 HF #2;
 Webs: 2x4 :HF Standard + HF Stud;;
 Lt Slider: 2x4 HF Standard + HF Stud; block length = 1.500'

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right cantilever is exposed to wind

Wind loading based on both gable and hip roof types.



Renews: 6/30/2025
 08/11/2023

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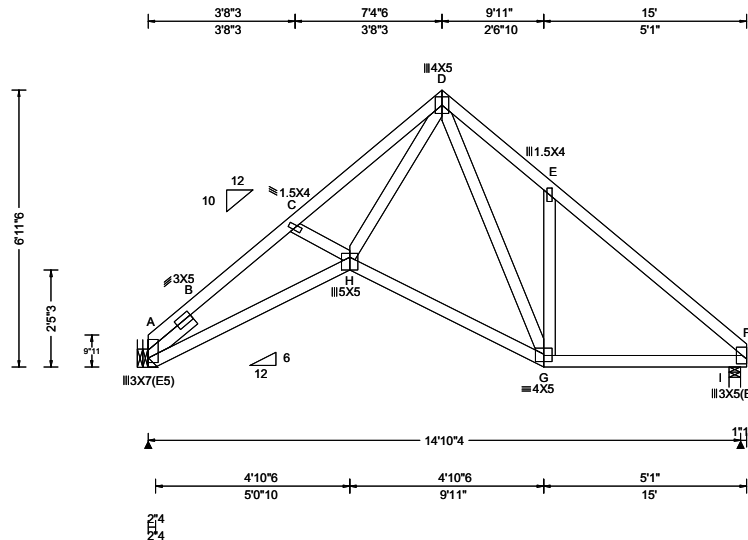
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 P. O. Box 468
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 (503)357-2118



155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025

SEQN: 357942 COMN Ply: 1 Job Number: 0823082
 FROM: BB Qty: 5 BTS N Hunt project
 Truss Label: F

Case R-1113-0 Ref: 1X557175003 P-1113
 Draw No: 223.23.1710.41287
 Date: 07/10/24 / JAK 08/11/2023
 Project #: 23-000944-000-00-RS



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 7.00 Des Ld: 42.00 NCBCLL: 10.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 21.76 ft TCDL: 4.2 psf BCDL: 4.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: 25.0 Ct: 1.1 CAT: II Pf: 21.2 Ce: 1.1 Lu: - Cs: 0.93 Snow Duration: 1.15 Building Code: IBC 2018 TPI Std: 2014 Rep Fac: Yes FT/RT: 2(0)/4(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.061 H 999 360 VERT(CL): 0.112 H 999 240 HORZ(LL): 0.054 F - - HORZ(TL): 0.098 F - - Creep Factor: 2.0 Max TC CSI: 0.259 Max BC CSI: 0.441 Max Web CSI: 0.516 VIEW Ver: 22.02.01.1115.14	Gravity Loc R+ / R- / Rh / Rw / U / RL A 673 - / - / 318 / 45 / 116 I 694 - / - / 332 / 47 - Wind reactions based on MWFRS A Brg Wid = - Min Req = - I Brg Wid = 3.5 Min Req = 1.5 (Truss) Bearing I is a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 94 - 1558 D - E 181 - 731 B - C 101 - 1497 E - F 76 - 801 C - D 99 - 1334

Lumber

Top chord: 2x4 HF #2;
 Bot chord: 2x4 HF #2;
 Webs: 2x4 :HF Standard + HF Stud;;
 Lt Slider: 2x4 HF Standard + HF Stud; block length = 1.500'

Loading

Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right cantilever is exposed to wind

Wind loading based on both gable and hip roof types.

Maximum Bot Chord Forces Per Ply (lbs)

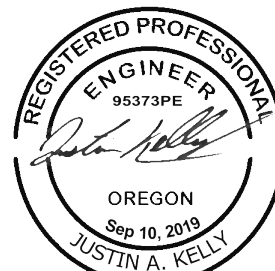
Chords Tens.Comp. Chords Tens. Comp.

A - H 1195 - 48 G - F 785 0
 H - G 541 0

Maximum Web Forces Per Ply (lbs)

Webs Tens.Comp.

H - D 1013 0



Renews: 6/30/2025
 08/11/2023

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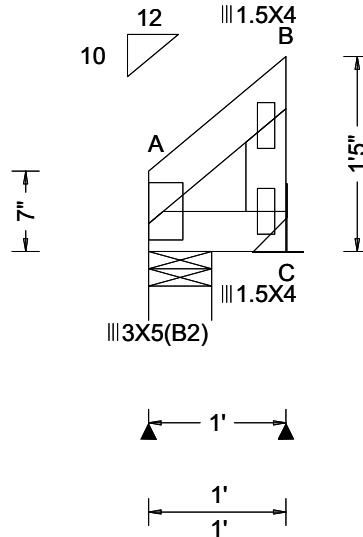
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 (503)357-2118



155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025

SEQN: 357920 EJAC Ply: 1 Job Number: 0823082
 FROM: BB Qty: 12 BTS N Hunt project
 Truss Label: G

Drawn: 223.23.1710.46673
 Date: 01/10/24
 / JAK 08/11/2023
 Project #: 23-000944-000-00-RS



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 25.00	Wind Std: ASCE 7-16	Pg: 25.0 Ct: 1.1 CAT: II	PP Deflection in loc L/defl L/#	Gravity Non-Gravity
TCDL: 10.00	Speed: 120 mph	Pf: 21.2 Ce: 1.1	VERT(LL): NA	Loc R+ / R- / Rh / Rw / U / RL
BCLL: 0.00	Enclosure: Closed	Lu: - Cs: 0.93	VERT(CL): NA	A 48 /- /- /21 /- /18
BCDL: 7.00	Risk Category: II	Snow Duration: 1.15	HORZ(LL): -0.000 B - -	C 43 /- /- /29 /14 /-
Des Ld: 42.00	EXP: B Kzt: NA		HORZ(TL): 0.000 B - -	Wind reactions based on MWFRS
NCBCLL: 10.00	Mean Height: 19.00 ft	Building Code:	Creep Factor: 2.0	A Brg Wid = 5.5 Min Req = 1.5 (Truss)
Soffit: 2.00	TCDL: 4.2 psf	IBC 2018	Max TC CSI: 0.014	C Brg Wid = - Min Req = -
Load Duration: 1.15	BCDL: 4.2 psf	TPI Std: 2014	Max BC CSI: 0.006	Bearing A is a rigid surface.
Spacing: 24.0 "	MWFRS Parallel Dist: 0 to h/2	Rep Fac: Yes	Max Web CSI: 0.007	Members not listed have forces less than 375#
	C&C Dist a: 3.00 ft	FT/RT:2(0)/4(0)		
	Loc. from endwall: Any	Plate Type(s):		
	GCpi: 0.18			
	Wind Duration: 1.60	WAVE	VIEW Ver: 22.02.01.1115.14	

Lumber

Top chord: 2x4 HF #2;
 Bot chord: 2x4 HF #2;
 Webs: 2x4 :HF Standard + HF Stud;;

Loading

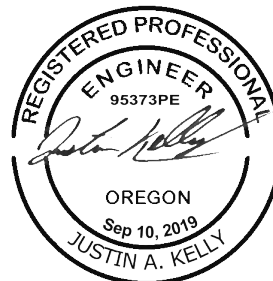
Bottom chord checked for 10.00 psf non-concurrent live load.

Wind

Wind loads based on MWFRS with additional C&C member design.

Right end vertical not exposed to wind pressure.

Wind loading based on both gable and hip roof types.



Renews: 6/30/2025
 08/11/2023

WARNING READ AND FOLLOW ALL NOTES ON THIS DRAWING!

IMPORTANT FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS

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Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from this drawing, any failure to build the truss in conformance with ANSI/TPI 1, or for handling, shipping, installation and bracing of trusses. A seal on this drawing or cover page listing this drawing, indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec.2.

For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbccomponents.com; ICC: iccsafe.org; AWC: awc.org

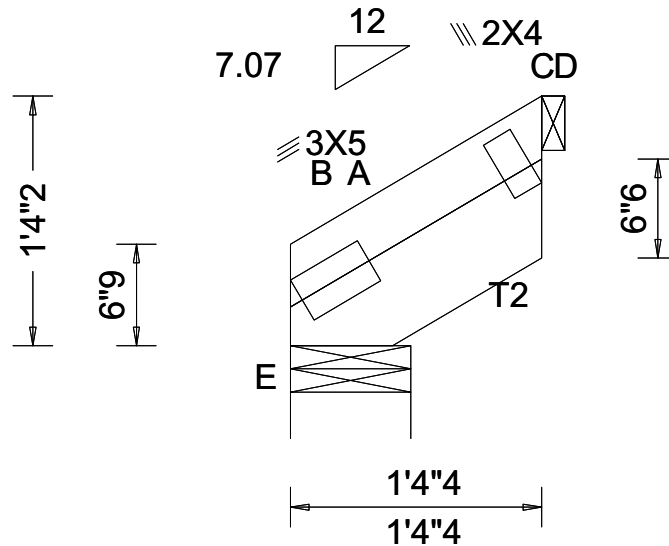
Truss Components of Oregon, Inc.
 P. O. Box 468
 CORNELIUS OR 97113
 (503)357-2118



155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025

SEQN: 357922 CALF Ply: 1 Job Number: 0823082
 FROM: BB Qty: 4 BTS N Hunt project
 Truss Label: H

Case R-1113 Order 1X557175003115
 Drawn: 223.23.1711.05997
 Date: 01/10/24
 / JAK 08/11/2023
 Project #: 23-000944-000-00-RS



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 7.00 Des Ld: 42.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 0.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 0.00 ft TCDL: 4.2 psf BCDL: 4.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: NA GCpi: 0.18 Wind Duration: 0.00	Pg: 25.0 Ct: 1.1 CAT: II Pf: 21.2 Ce: 1.1 Lu: - Cs: 1.00 Snow Duration: 1.15 Building Code: IBC 2018 TPI Std: 2014 Rep Fac: Yes FT/RT: 2(0)/4(0) Plate Type(s): WAVE	PP Deflection in loc L/defl L/# VERT(LL): 0.000 - - 360 VERT(CL): 0.000 A 999 240 HORZ(LL): 0.000 - - - HORZ(TL): 0.000 C - - Creep Factor: 2.0 Max TC CSI: 0.000 Max BC CSI: 0.000 Max Web CSI: 0.000 VIEW Ver: 22.02.01.1115.14	Gravity Non-Gravity Loc R+ / R- / Rh / Rw / U / RL E 1 /- /- /- /- /- C 0 /- /- /- /- /- Wind reactions based on MWFRS E Brg Wid = 7.8 Min Req = 1.5 (Truss) C Brg Wid = 1.5 Min Req = - Bearing E is a rigid surface. Members not listed have forces less than 375#

Lumber

Top chord: 2x4 HF #2; T2 2x6 HF #2;
 Bot chord: ;

Additional Notes

Shim all supports to solid bearing.

Engineer of record shall approve this design, including all loads, reactions, and connections prior to fabrication. Use of this truss is the responsibility of owner.



Renews: 6/30/2025
 08/11/2023

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 For more information see these web sites: Alpine: alpineitw.com; TPI: tpinst.org; SBCA: sbcacomponents.com; ICC: iccsafe.org; AWC: awc.org

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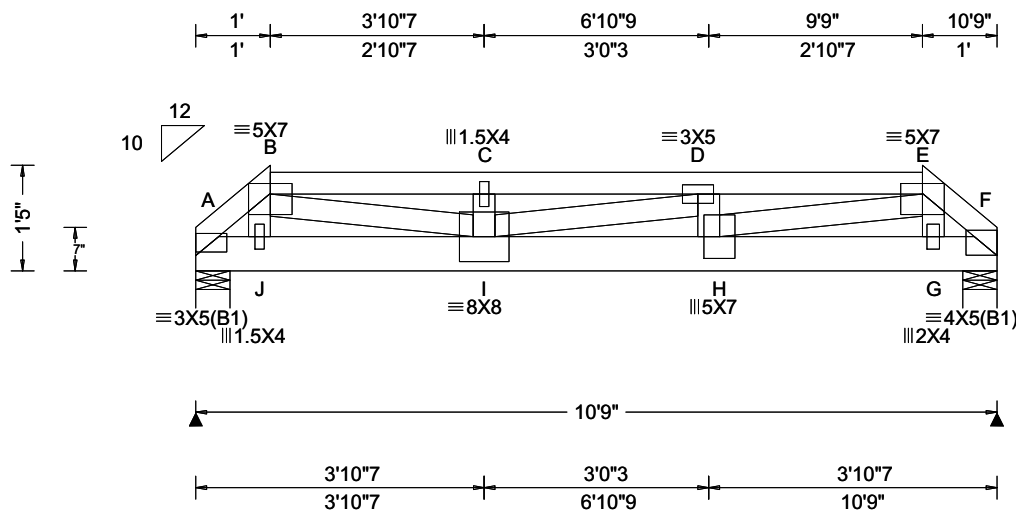


155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025

SEQN: 357947 HIPS Ply: 2 Job Number: 0823082
 FROM: BB Qty: 2 BTS N Hunt project
 Truss Label: I

Case R-1113 Order 1X55175003
 Drawn: 223.23.1711.45443
 Date: 07/10/24
 / JAK 08/11/2023
 Project #: 23-000944-000-00-RS

2 Complete Trusses Required



Loading Criteria (psf)	Wind Criteria	Snow Criteria (Pg, Pf in PSF)	Defl/CSI Criteria	▲ Maximum Reactions (lbs)
TCLL: 25.00 TCDL: 10.00 BCLL: 0.00 BCDL: 7.00 Des Ld: 42.00 NCBCLL: 0.00 Soffit: 2.00 Load Duration: 1.15 Spacing: 24.0 "	Wind Std: ASCE 7-16 Speed: 120 mph Enclosure: Closed Risk Category: II EXP: B Kzt: NA Mean Height: 19.00 ft TCDL: 4.2 psf BCDL: 4.2 psf MWFRS Parallel Dist: 0 to h/2 C&C Dist a: 3.00 ft Loc. from endwall: Any GCpi: 0.18 Wind Duration: 1.60	Pg: 25.0 Ct: 1.1 CAT: II Pf: 21.2 Ce: 1.1 Lu: - Cs: varies Snow Duration: 1.15 Building Code: IBC 2018 TPI Std: 2014 Rep Fac: Varies by Ld Case FT/RT: 2(0)/4(0) Plate Type(s): WAVE	PP Deflection in loc L/def L/# VERT(LL): 0.086 C 999 360 VERT(CL): 0.146 C 862 240 HORZ(LL): 0.016 B - - HORZ(TL): 0.026 B - - Creep Factor: 2.0 Max TC CSI: 0.400 Max BC CSI: 0.708 Max Web CSI: 0.942 VIEW Ver: 22.02.01.1115.14	Gravity Loc R+ / R- / Rh / Rw / U / RL Non-Gravity A 1857 -/- /- /- /153 -/ F 2333 -/- /- /- /185 -/ Wind reactions based on MWFRS A Brg Wid = 5.5 Min Req = 1.5 (Truss) F Brg Wid = 5.5 Min Req = 1.9 (Truss) Bearings A & G are a rigid surface. Members not listed have forces less than 375# Maximum Top Chord Forces Per Ply (lbs) Chords Tens.Comp. Chords Tens. Comp. A - B 99 - 1219 D - E 220 - 2772 B - C 211 - 2632 E - F 117 - 1501 C - D 211 - 2632

Lumber

Top chord: 2x4 HF #2;
 Bot chord: 2x6 HF #2;
 Webs: 2x4 :HF Standard + HF Stud;;

Nailnote

Nail Schedule: 0.120"x3", min. nails
 Top Chord: 1 Row @ 12.00" o.c.
 Bot Chord: 2 Rows @ 5.50" o.c. (Each Row)
 Webs : 1 Row @ 4" o.c.
 Use equal spacing between rows and stagger nails
 in each row to avoid splitting.

Special Loads

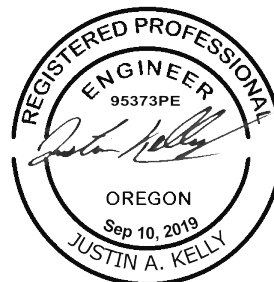
——(Lumber Dur.Fac.=1.15 / Plate Dur.Fac.=1.15)
 TC: From 76 plf at 0.00 to 76 plf at 1.00
 TC: From 38 plf at 1.00 to 38 plf at 9.75
 TC: From 76 plf at 9.75 to 76 plf at 10.75
 BC: From 7 plf at 0.00 to 7 plf at 10.75
 TC: 1 lb Conc. Load at 1.01, 9.74
 BC: 43 lb Conc. Load at 1.06, 3.06, 5.06, 5.69
 7.69, 9.69
 BC: 675 lb Conc. Load at 2.31, 4.31, 6.31, 8.31
 9.31

Purlins

In lieu of structural panels use purlins to brace all flat
 TC @ 24" oc.

Wind

Wind loads and reactions based on MWFRS.
 Wind loading based on both gable and hip roof types.



Renews: 6/30/2025
 08/11/2023

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Truss Components of Oregon, Inc.
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 (503)357-2118



155 Harlem Ave
 North Building, 4th Floor
 Glenview, IL 60025

Gable Stud Reinforcement Detail

ASCE 7-16: 120 mph Wind Speed, 30' Mean Height, Enclosed, Exposure C, Kzt = 1.00

Or 100 mph Wind Speed, 30' Mean Height, Partially Enclosed, Exposure C, Kzt = 1.00

Or 100 mph Wind Speed, 30' Mean Height, Enclosed, Exposure D, Kzt = 1.00

Max Gable Vertical Length	2x4 Gable Vertical Spacing		Brace Grade		No Braces	(1) 1x4 "L" Brace #										(2) 2x4 "L" Brace #										(1) 2x6 "L" Brace #										(2) 2x6 "L" Brace #										(1) 2x6 "L" Brace #										(2) 2x6 "L" Brace #									
	Species	Grade	#1 / #2	Group A		Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B	Group A	Group B																																			
24" o.c.	SPF	HF	Standard	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	11' 5"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"																																		
				7' 2"	7' 8"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"																																	
				7' 2"	7' 7"	9' 1"	9' 5"	10' 10"	11' 4"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"																																	
				6' 2"	8' 2"	8' 2"	8' 9"	10' 10"	11' 4"	12' 10"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"																																
				7' 11"	8' 2"	9' 4"	9' 8"	11' 1"	11' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"																																
	SP	DFL	Standard	7' 10"	8' 1"	9' 3"	9' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"																																	
				6' 6"	6' 11"	8' 7"	9' 2"	10' 11"	11' 4"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"																																
				6' 6"	6' 11"	8' 7"	9' 2"	10' 11"	11' 4"	13' 6"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"																															
				5' 9"	6' 1"	7' 7"	8' 2"	10' 4"	11' 1"	12' 10"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"																														
				8' 11"	9' 3"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"																														
16" o.c.	SPF	HF	Standard	8' 10"	9' 3"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"																																	
				8' 9"	9' 2"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"																															
				7' 6"	8' 0"	10' 1"	10' 9"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"																														
				9' 1"	9' 5"	10' 8"	11' 1"	12' 8"	13' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"																														
				8' 11"	9' 3"	10' 7"	11' 0"	12' 7"	13' 1"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"																														
	SP	DFL	Standard	7' 11"	8' 5"	10' 6"	10' 11"	12' 6"	13' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"																															
				7' 10"	8' 4"	10' 5"	10' 10"	12' 5"	12' 11"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"																														
				9' 10"	10' 2"	11' 7"	12' 1"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"																														
				9' 10"	10' 2"	11' 7"	12' 1"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"																														
				9' 10"	10' 2"	11' 7"	12' 1"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"																													
12" o.c.	SPF	HF	Standard	9' 8"	10' 1"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"																																
				9' 8"	10' 1"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"																														
				8' 8"	9' 3"	11' 6"	11' 11"	13' 8"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"																													
				10' 0"	10' 4"	11' 9"	12' 2"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"																												
				9' 10"	10' 2"	11' 7"	12' 1"	13' 10"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"																												
	SP	DFL	Standard	9' 2"	9' 9"	11' 6"	12' 0"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"																													
				9' 2"	9' 9"	11' 6"	12' 0"	13' 9"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"	14' 0"																																												

ASCE 7-16: 120 mph, 30' Mean Height, Closed, Exposure C
Common Residential Gable End Wind Bracing Requirements - Stiffeners

120 mph, 30ft. Mean Hgt, ASCE 7-16, Enclosed, Exp C, or
100 mph, 30ft. Mean Hgt, ASCE 7-16, Enclosed, Exp D, or
100 mph, 30ft. Mean Hgt, ASCE 7-16, Part. Enclosed, Exp C,
Kzt = 1.00, Wind TC DL=5.0 psf, Wind BC DL=5.0 psf.

Lateral chord bracing requirements
Top: Continuous roof sheathing
Bot: Continuous ceiling diaphragm

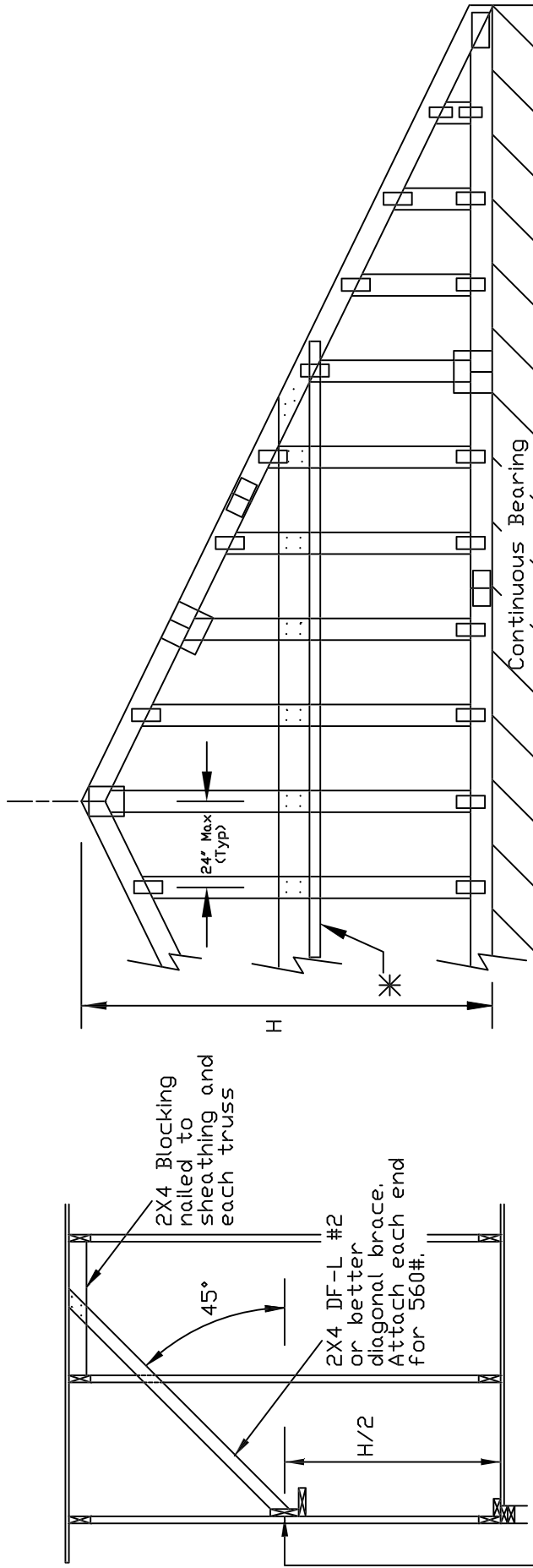
See Engineer's sealed design referencing this detail
for lumber, plates, and other information not shown
on this detail.

Nails: 10d box or gun (0.128"x3",min) nails.

H Less than 4'6" - no stud bracing required
H Greater than 4'6" to 7'6" in length
provide a 2x6 stiffback at mid-height and brace
to roof diaphragm every 6'0" (see detail below or
refer to DRWG A12030ENC160118).

H Greater than 7'6" to 12'0" max:
provide a 2x6 stiffback at mid-height and brace
to roof diaphragm every 4'0" (see detail below or
refer to DRWG A12030ENC160118).

* Optional 2x L-reinforcement attached
to stiffback with 10d box or gun
(0.128" x 3", min.) nails @ 6" o.c.



IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING!
INSTALLERS: FURNISH THIS DRAWING TO ALL CONTRACTORS INCLUDING THE INSTALLERS.
Trusses require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI Building Components Group Inc. TPI and SBCA for safety practices prior to performing these functions. Installers shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have a properly attached rigid ceiling. Locations shown for permanent lateral restraint of webs of truss and position of standard plate positions. Refer to drawings 10d42 for standard plate positions.
Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from the design shown on this drawing. The truss is to be installed in accordance with ANSI/TPI 1, or for handling, shipping, installation & bracing of trusses.
A seal on this drawing or cover page listing this drawing. Indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec2.
For more information see this job's general notes page and these web sites:
ALPINE: www.alpinetw.com TPI: www.tpi.org SBCA: www.sbcacomponents.com ICD: www.icdcsa.org

REGISTERED PROFESSIONAL
ENGINEER
95373PE
Justin A. Kelley
OREGON
Sep 10, 2019
Renews: 6/30/2025
08/11/2023

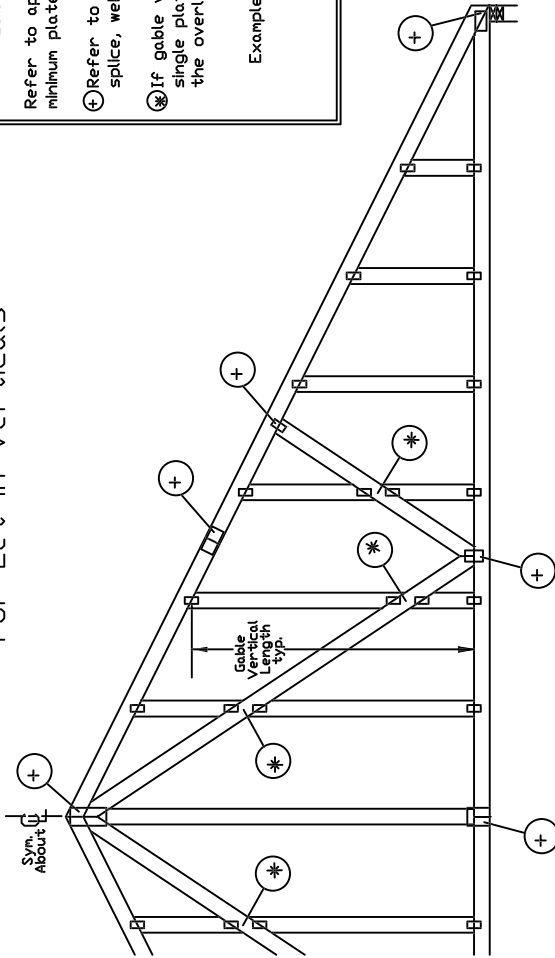
2x6 #2 Stiffback attached to each Stud w/ (4) 10d box or gun (0.123" x 3", min.) nails.

24" Max (Typ)

Continuous Bearing

REF	GE WALL
DATE	01/02/2018
DRWG	GABRST160118
MAX. TOT. LD. 60 PSF	
MAX. SPACING	

Gable Detail For Let-in Verticals

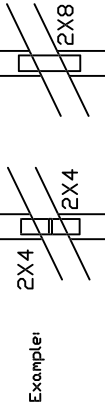


Gable Truss Plate Sizes

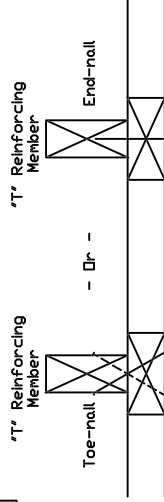
Refer to appropriate Alpine gable detail for minimum plate sizes for vertical studs.

⊕ Refer to Engineered truss design for peak, splice, web, and heel plates.

⊗ If gable vertical plates overlap, use a single plate that covers the total area of the overlapped plates to span the web.



'T' Reinforcement Attachment Detail



To convert from 'L' to 'T' reinforcing members, multiply 'T' increase by length (based on appropriate Alpine gable detail).

Maximum allowable 'T' reinforced gable vertical length is 14' from top to bottom chord. 'T' reinforcing member material must match size, specie, and grade of the 'L' reinforcing member.

Web Length Increase w/ 'T' Brace

'T' Reinf.	'T' Mbr. Size	Increase
2x4	2x6	30 %
2x6	2x8	20 %

Example:

ASCE 7-10 Wind Speed = 120 mph
Mean Roof Height = 30 ft, Kzt = 100
Gable Vertical = 24' o.c. SP #3
'T' Reinforcing Member Size = 2x4
'T' Brace Increase (From Above) = 30% = 1.30
(1) 2x4 'L' Brace Length = 8' 7"
Maximum 'T' Reinforced Gable Vertical Length
1.30 x 8' 7" = 11' 2"

Provide connections for uplift specified on the engineered truss design.

Attach each 'T' reinforcing member with

End Driven Nails:

- 10d Common (0.148"x 3", min) Nails at 4' o.c. plus
- (4) nails in the top and bottom chords.

Toenailed Nails:

- 10d Common (0.148"x 3", min) Toenails at 4' o.c. plus
- (4) toenails in the top and bottom chords.

This detail to be used with the appropriate Alpine gable detail for ASCE wind load.

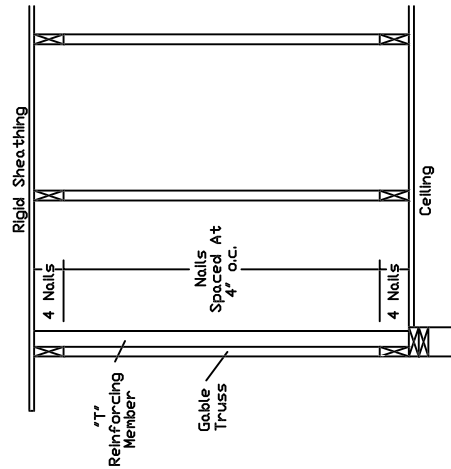
ASCE 7-05 Gable Detail Drawings

A13015051014, A12015051014, A1015051014, A10030051014, A14015051014, A13030051014, A12030051014, A1030051014, A10030051014, A14030051014

ASCE 7-10 & ASCE 7-16 Gable Detail Drawings

A11515ENC100118, A12015ENC100118, A14015ENC100118, A16015ENC100118, A18015ENC100118, A20015ENC100118, A20015SPED100118, A11530ENC100118, A12030ENC100118, A14030ENC100118, A16030ENC100118, A18030ENC100118, A20030ENC100118, A20030SPED100118, S11515ENC100118, S12015ENC100118, S14015ENC100118, S16015ENC100118, S18015ENC100118, S20015ENC100118, S20015SPED100118, S11530ENC100118, S12030ENC100118, S14030ENC100118, S16030ENC100118, S18030ENC100118, S20030ENC100118, S20030SPED100118

See appropriate Alpine gable detail for maximum unreinforced gable vertical length.



IMPORTANT: READ AND FOLLOW ALL NOTES ON THIS DRAWING! INSTALLERS require extreme care in fabricating, handling, shipping, installing and bracing. Refer to and follow the latest edition of BCSI Building Components Group Inc. TPI and SBCA for safety practices prior to performing these functions. Trusses shall provide temporary bracing per BCSI. Unless noted otherwise, top chord shall have properly attached structural sheathing and bottom chord shall have bracing installed per BCSI sections 83, 87 or 810, as applicable. Apply plates to each face of truss and position as shown above and on the Joint Details, unless noted otherwise. Refer to drawings 1001-2 for standard plate positions.

Alpine, a division of ITW Building Components Group Inc. shall not be responsible for any deviation from the truss design shown on this drawing. The truss is to be installed in accordance with the installation & bracing of trusses. A seal on this drawing or cover page listing this drawing. Indicates acceptance of professional engineering responsibility solely for the design shown. The suitability and use of this drawing for any structure is the responsibility of the Building Designer per ANSI/TPI 1 Sec2.

For more information see this job's general notes page and these web sites: ALPINE: www.alpinetw.com TPI: www.tpihq.org SBCA: www.sbcacomponents.com ICD: www.icdcsa.org



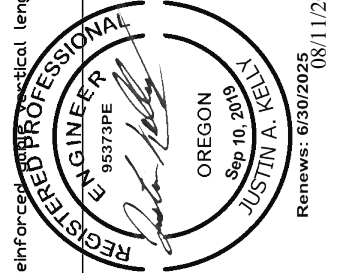
155 Harlem Ave
North Building, 4th Floor
Glenview, IL 60025

City of Portland
Reviewed for code compliance

Date: 01/02/2018
Project: 21188004-000-00-RS

REF LET-N VERT
DATE 01/02/2018
DRWG GBLT-TIN0118

MAX. TOT. LD. 60 PSF
DUR. FAC. ANY
MAX. SPACING 24.0"



Renews: 6/30/2025