

CERTIFIED FOR

JOB NAME: ARLENE SCHNITZER

CUSTOMER: MASON OREGON

CUSTOMER P.O.:

MASON E.O.: 36969

REQUIRED: (1) TAG: CT-1

TYPE

WFSL

BASE

M

MASON INDUSTRIES, Inc.

Manufacturers of Vibration Control Products

350 Rabro Drive

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FAX 631/348-0279

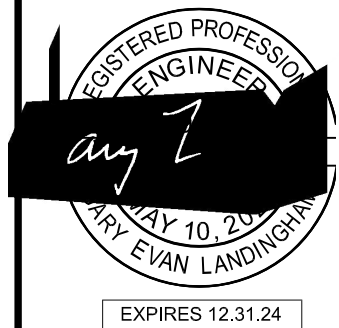
TELEX 96-8464

2101 W. Crescent Ave., Suite D

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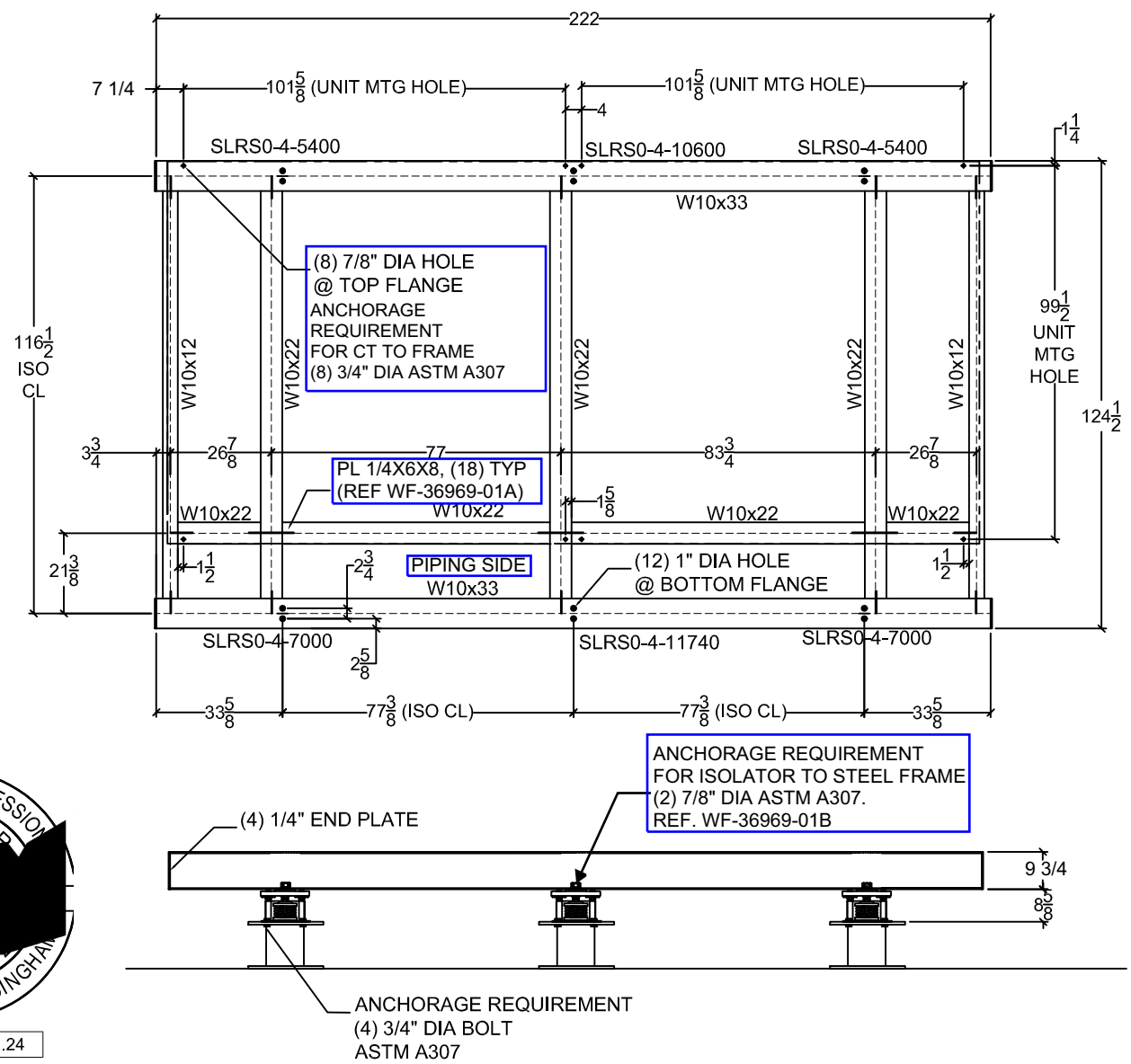


EXPIRES 12.31.24

UNIT: BAC-FXV-0818A-36H-L
STEEL WT.: 2480 LBS
EQUIPMENT WT.: 29220 LBS
PIPE WT.: 2200 LBS
ISOLATOR REF. DWG.: WF-SLRS2
SPECIFIED DEFL: 2.0"

City of Portland
Reviewed for code compliance
Date: 01/05/24
Project #: 23-084757-DFS-01-FA

REV-04: NN, 11/20/23
REV-03: NN, 09/29/23
REV-02: NN, 09/28/23
DWG. NO: WF-36969-01-4



DWN	CHKT	DATE
NN	TH	5/23/19

A1

A1



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City of Portland

Reviewed for Code Compliance

Date: 01/08/12

Project # 23-034357-SPP-01-F1

JOB NAME ARLENE SCHNEIDER

CUSTOMER MASON OREGON

CUSTOMER P.O.

MASON M.I. EO 36969

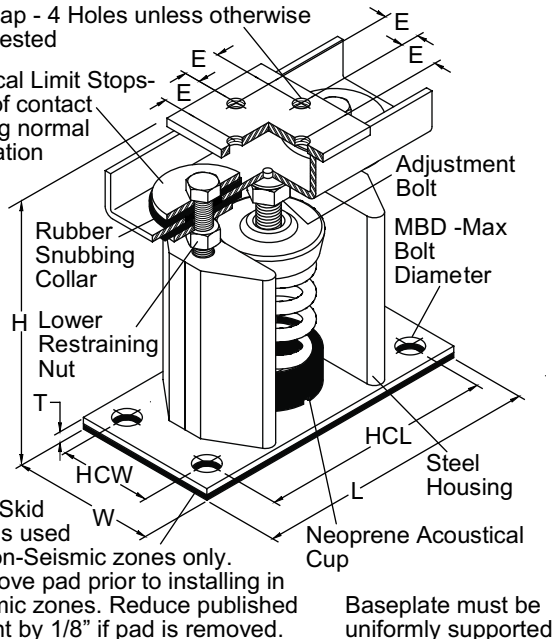
DWG. NO. WF-SLRS01

SLRSO

1" DEFLECTION
B, 1, 2 & 4
SERIES SPRING
MOUNTS

"D" Tap - 4 Holes unless otherwise requested

Vertical Limit Stops-
Out of contact
during normal
operation



SPRING DATA

Size	Spring OD	Free Height	Ratio K _x /K _y	Ratio OD/OH
B	2 3/8	4	0.70-0.80	0.80 - 1.25
C	2 7/8	4 1/8	0.90-1.10	0.92

Illustration shows SLRSO-B housing which contains one (1) B spring. Not shown is SLRSO-1 housing which contains one (1) C2 spring, SLRSO-2 housing which contains two (2) C2 springs and SLRSO-4 which contains four (4) C2 springs.

All springs have an additional travel to solid equal to 50% of the rated deflection.

Housing load ratings expressed in G's are based on tests with bolted connections to steel top and bottom.

TYPE SLRSO RATINGS

Size	Rated Capacity (lb)	Rated Defl. (in)	Spring Constant (lb/in)	Max. Horiz. Housing G Rating	Spring Color
SLRSO-B-65	65	2.10	31	21.6	Brown
SLRSO-B-85	85	2.10	40	16.5	Wht/Blk
SLRSO-B-115	115	2.00	57	12.2	Silver
SLRSO-B-150	150	2.00	75	9.3	Orange
SLRSO-B-280	280	1.60	174	5.0	Green
SLRSO-B-450	450	1.31	344	3.1	Red
SLRSO-B-750	750	1.12	670	1.9	White
SLRSO-B-1000	1000	1.00	1000	1.4	Blue
SLRSO-1-1000	1000	1.00	1000	4.4	Black
SLRSO-1-1350	1350	1.00	1350	3.3	Yellow
SLRSO-1-1750	1750	1.00	1750	2.5	Black*
SLRSO-1-2100	2100	1.00	2100	2.1	Yellow*
SLRSO-1-2385	2385	1.00	2385	1.9	Yellow**
SLRSO-1-2650	2650	1.00	2650	1.7	Red*
SLRSO-1-2935	2935	1.00	2935	1.5	Red**
SLRSO-2-3500	3500	1.00	3500	1.8	Black*
SLRSO-2-4200	4200	1.00	4200	1.5	Yellow*
SLRSO-2-4770	4770	1.00	4770	1.3	Yellow**
SLRSO-4-5400	5400	1.00	5400	2.0	Yellow
SLRSO-4-7000	7000	1.00	7000	1.6	Black*
SLRSO-4-8400	8400	1.00	8400	1.3	Yellow*
SLRSO-4-9540	9540	1.00	9540	1.2	Yellow**
SLRSO-4-10600	10600	1.00	10600	1.1	Red*
SLRSO-4-11740	11740	1.00	11740	0.9	Red**

* with RED inner spring ** with GREEN inner spring

TYPE SLRSO DIMENSIONS (inches)

Size	L	W	H	T	MBD	HCW	HCL	D	E
SLRSO-B	8 1/2	4 1/4	8 3/4	3/8	5/8	2 3/4	7	1/2	1 1/8
SLRSO-1	9 1/2	5 1/4	8 3/4	3/8	5/8	3 1/2	7 1/2	5/8	1 3/8
SLRSO-2	14	5 1/4	8 3/4	3/8	5/8	3 1/2	12 1/4	5/8	1 3/8
SLRSO-4	13 3/4	8	8 3/4	3/8	3/4	6 1/4	11	7/8	1 3/8



EXPIRES 12.31.24



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TELEX 96-8464

Date: 01/15/24

Project # 23-084757-DES-01-PA

CERTIFIED FOR

JOB NAME: ARLENE SCHNITZER

CUSTOMER: MASON OREGON

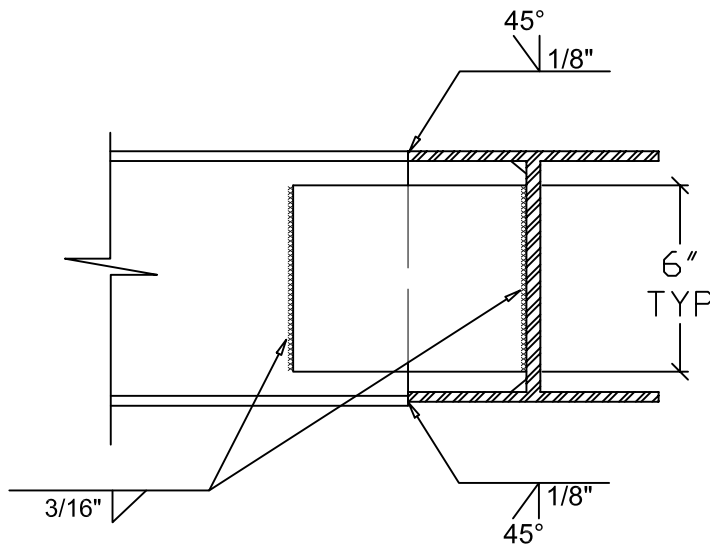
CUSTOMER P.O.: JOB NO.:

MASON E.O.: 36969

DWG. NO.: WF-36969-01A

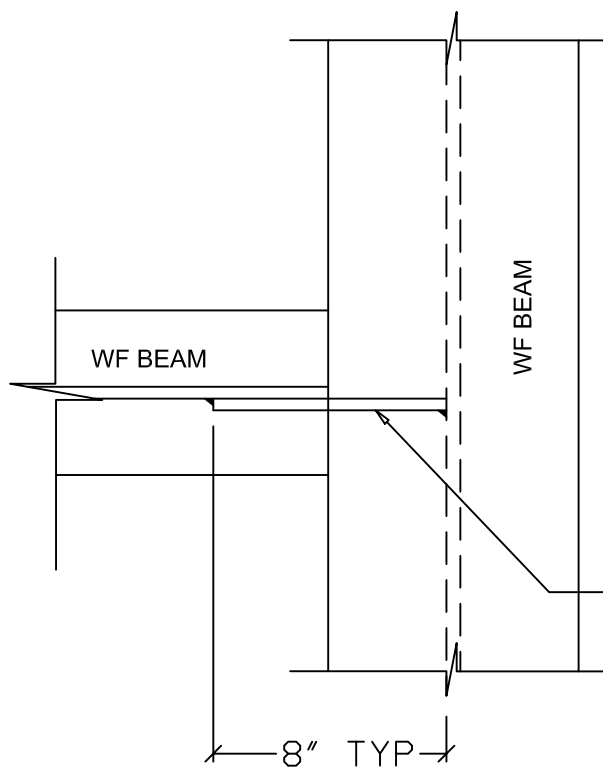
TAG CT-1

BEAM TO BEAM WELDED CONNECTION DETAILS



WF BEAM SIZE	CONNECTING PLATE SIZE
W10x12	PL 1/4 x 6 x 8
W10x22	PL 1/4 x 6 x 8
W10x33	PL 1/4 x 6 x 8

NOTE: ALL WELD ELECTRODES TO BE 70XX



NOTE:
MECHANICAL CONTRACTOR
TO WORK OUT WITH INSPECTOR
FOR SPECIAL INSPECTION
SUBJECT TO WELDS



EXPIRES 12.31.24

(NOT TO SCALE)

DWN NN	CHKD	DATE 09/28/23
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DWG. NO. WF-36969-01A

City of Portland

Reviewed for code compliance

Date: 01/05/24

Project #: 23-084737-DPS-01-FA



MASON INDUSTRIES, Inc.

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TELEX 96-8464

CERTIFIED FOR

JOB NAME: ARLENE SCHNITZER

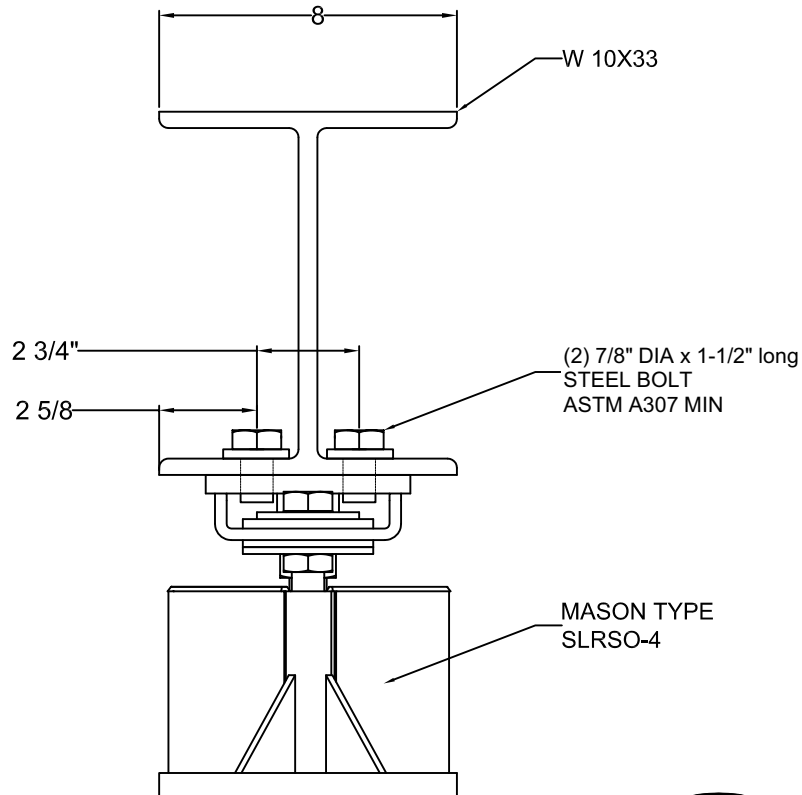
CUSTOMER: MASON OREGON

CUSTOMER P.O.: JOB NO.:

MASON E.O.: 36969 DWG. NO.: WF-36969-01B

TAG: CT-1

CONNECTION DETAILS FOR ISOLATOR TO STEEL FRAME



EXPIRES 12.31.24

DWN	CHKD	DATE
NN		09/28/23

DWG. NO.
WF-36969-01B



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City of Portland
 Reviewed for code compliance

Date: 01/05/24

Project #: 23-08257-CUSTOMER

SEISMIC OVERTURNING CALCULATIONS

JOB NAME:

ARLENE SCHNITZER

CUSTOMER:

MASON OREGON

EO NUMBER:

####

EQUIPMENT TAGS:

CT

9/29/2023

DWG. NO: C01A-1

SEISMIC DESIGNS FOR NONSTRUCTURAL COMPONENT PER ASCE 7-10 OR 7-16, SECTION 13.3.1

(All Units Are in Lbs and Inches)

Max. Component Weight, W_p =	33900	
Component Amplification Factor, a_p =	2.5	(ASCE 7, Table 13.6-1)
Component Response Modification Factor, R_p =	2.0	(ASCE 7, Table 13.6-1)
*Design Overstrength Factor, Ω_o =	1.0	(ASCE 7, Table 13.6-1) for Concrete Anchorage Designs Only
Spectral Acceleration, S_{ds} =	0.680	(ASCE 7, Section 11.4.4 or per Structural Plans)
Component Importance Factor, I_p =	1.00	(ASCE 7, Section 13.1.3 or per Structural Plans)
Ratio of Elevation Height vs. Mean Roof Height, z/h =	1.00	(0 for Grade Level or Below, 1 for Roof Level)

Horizontal Seismic Force (LRFD):

ASD Conversion Factor = 0.7

$$F_{ph} = \frac{0.4a_p S_{ds} W_p}{\left(\frac{R_p}{I_p}\right)} \left(1 + 2\frac{z}{h}\right) = 34578 \quad (\text{ASCE 7, Equation 13.3-1}) = 24205 \quad (\text{ASD Level})$$

$$F_{ph} (\text{minimum}) = 0.3S_{ds} I_p W_p = 6916 \quad (\text{ASCE 7, Equation 13.3-3}) = 4841 \quad (\text{ASD Level})$$

$$F_{ph} (\text{maximum}) = 1.6S_{ds} I_p W_p = 36883 \quad (\text{ASCE 7, Equation 13.3-2}) = 25818 \quad (\text{ASD Level})$$

$$F_{ph} (\text{governing}) = 34578 = 24205 \quad (\text{ASD Level})$$

$$\text{Horizontal Seismic "G" Force Ratio} = 1.02 = 0.71 \quad (\text{ASD Level})$$

Vertical Seismic Component (LRFD):

$$F_{pv} = 0.2S_{ds} W_p = 4610 = 3227 \quad (\text{ASD Level})$$

Component Seismic Mounts Layout & Critical Overturning Angle:

Seismic Mount Model = SLRS

Total Number of Seismic Mounts, N = 6

Maximum Length Between Isolators/Mounts, b_1 = 155

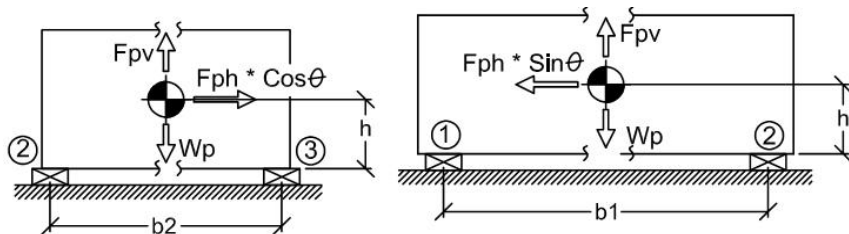
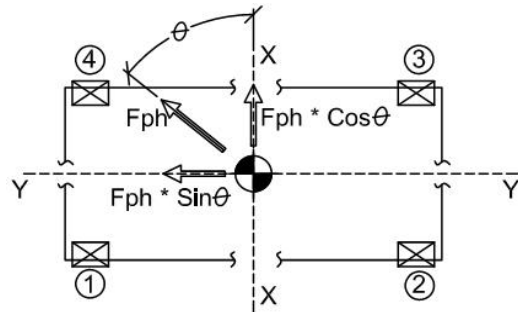
Maximum Width Between Seismic Mounts, b_2 = 117

Combined C.G. Height of Component, h = 90

$$I_{xx} = \frac{N(N+2)}{12(N-2)} b_1^2 = 23948$$

$$I_{yy} = \frac{N}{4} b_2^2 = 20358$$

$$\theta = \tan^{-1} \left(\frac{I_{yy} b_1}{I_{xx} b_2} \right) = \frac{0.85}{48} \quad \text{rad deg}$$



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DWN: NN

DWG. NO.: C01A-1



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City of Portland
 Reviewed for Code Compliance

Date: 01/05/24

Project #: 23-084767-005-0110

SEISMIC OVERTURNING CALCULATIONS

JOB NAME: ARLENE SCHNITZER
 CUSTOMER: MASON OREGON
 EO NUMBER: 36969
 EQUIPMENT TAGS: CT
 11/20/2023 DWG. NO: C01B-1

Based On Seismic Horizontal Force Applied on Component At a Critical Angle, θ

Maximum Uplift at Seismic Mount Location #2:

$$(LRFD) \quad P_t = \frac{0.9W_p - F_{pv}}{N} - \frac{F_{ph} * \cos \theta * h * b_2}{2 * I_{YY}} - \frac{F_{ph} * \sin \theta * h * b_1}{2 * I_{XX}} = -9114$$

$$(ASD) \quad P_{t(ASD)} = \frac{0.6W_p - F_{pv(ASD)}}{N} - \frac{F_{ph(ASD)} * \cos \theta * h * b_2}{2 * I_{YY}} - \frac{F_{ph(ASD)} * \sin \theta * h * b_1}{2 * I_{XX}} = -6549$$

Maximum Compression at Seismic Mount Location #4:

$$(LRFD) \quad P_c = \frac{1.2W_p + F_{pv}}{N} + \frac{F_{ph} * \cos \theta * h * b_2}{2 * I_{YY}} + \frac{F_{ph} * \sin \theta * h * b_1}{2 * I_{XX}} = 20979$$

$$(ASD) \quad P_{c(ASD)} = \frac{W_p + F_{pv(ASD)}}{N} + \frac{F_{ph(ASD)} * \cos \theta * h * b_2}{2 * I_{YY}} + \frac{F_{ph(ASD)} * \sin \theta * h * b_1}{2 * I_{XX}} = 15589$$

Maximum Shear Per Seismic Mount Location:

$$(LRFD) \quad P_s = \frac{F_{ph}}{N} = 5763$$

$$(ASD) \quad P_{s(ASD)} = \frac{F_{ph(ASD)}}{N} = 4034$$



Steel Bolt Anchorage Design per AISC, 14th Edition, Sections J3.6 & J3.7 (ASD):

Height of Mount, H = 8.75

Min. Bolt Spacing, B = 6.25

Anchor Bolt Diameter, d = 3/4

Grade/Quality of Steel Bolt = A307

EXPIRES 12.31.24
 Ab = 0.442 (Bolt Unthreaded Area)

$$\text{Max. Tension per Bolt, } T_{bolt} = \frac{P_t}{4} + \frac{P_s * H}{2B} = 4461$$

$$\text{Max. Shear per Bolt, } V_{bolt} = P_s/4 = 1009$$

$$A_t = 0.334$$

$$A_k = 0.323$$

$$F_{nt} = 45000$$

$$F_{nv} = 24000$$

$$f_t = T_{bolt}/A_t = 13357$$

$$f_v = V_{bolt}/A_k = 3122$$

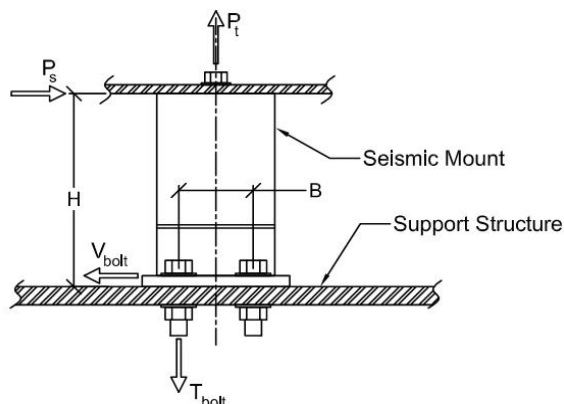
$$F'_{nt} = (1.3 * F_{nt} - \frac{2 * F_{nt}}{F_{nv}} * f_v) \leq F_{nt} = 45000$$

$$T_{allow} = (F'_{nt} * A_b)/2 = 9940$$

$$V_{allow} = (F_{nv} * A_b)/2 = 5301$$

$$(T_{bolt}/T_{allow}) = 0.45 \quad \text{OK}$$

$$(V_{bolt}/V_{allow}) = 0.19 \quad \text{OK}$$



DWN: NN

DWG. NO.: C01B-1

City of Portland

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Date: 01/05/24

Project: ~~CE-2019-01-FA~~



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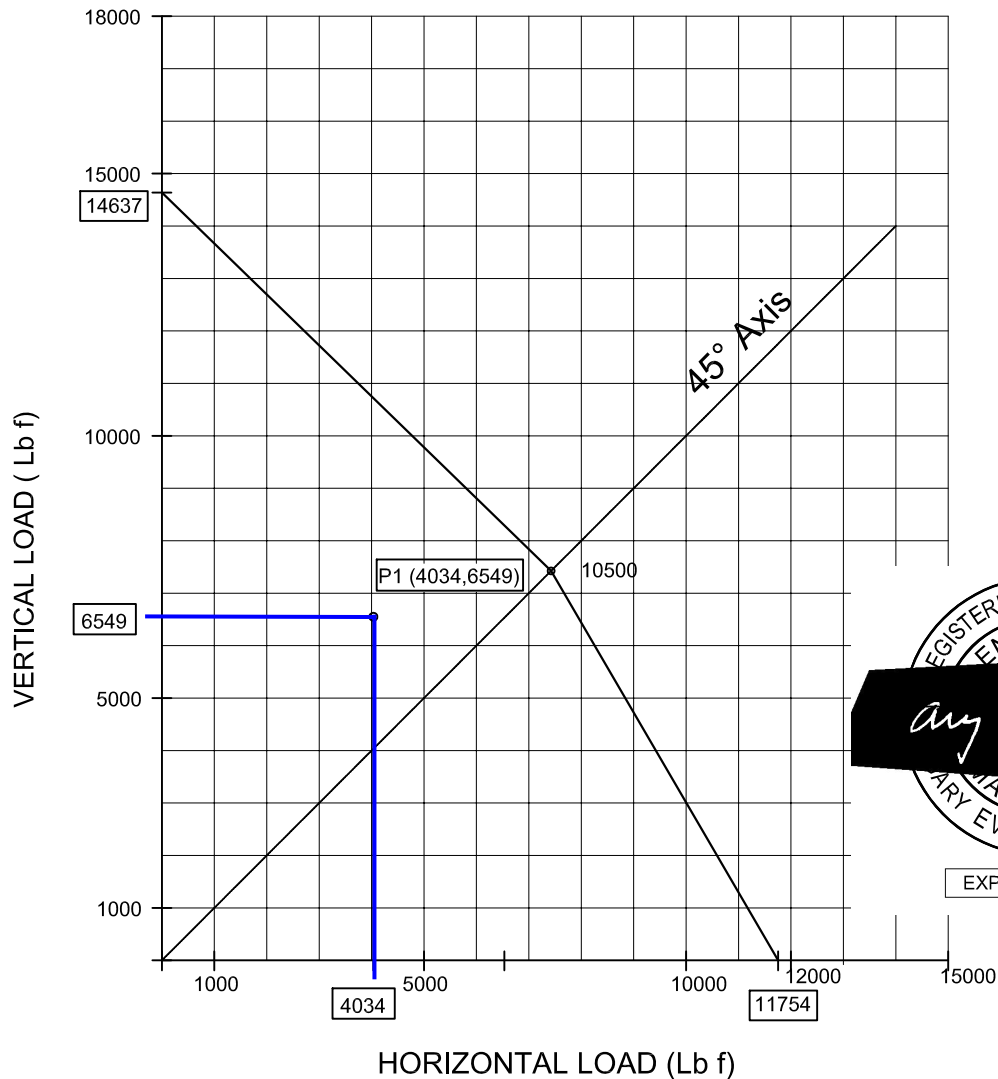
MASON E.O. : 36969

DWG.: WF-36969-C01C

Comparison of calculated loads vs rated loads on SLRSO-4 isolator

(REF. A-30070-1 & 30072-1)

Equipment Tag	Total Weight	Number of Isolators	Actual Vertical Load Per Isolator (Pt)	Actual Horizontal Load Per Isolator (Ps)	Plotted Load Point
CT-1	33900 LBS	(6)	6549 LBS (REF. C01B-1)	4034 LBS (REF. C01B-1)	P1



EXPIRES 12.31.24

DWN NN CHKD DATE 11/20/23

DWG. NO. WF-36969-C01C