

Development Services

From Concept to Construction

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APPEAL SUMMARY

Status: Decision Rendered - Held over from ID 20280 (4/24/19) for additional information

Appeal ID: 20421	Project Address: 2510 NE Sandy Blvd
Hearing Date: 5/22/19	Appellant Name: Shea Gilligan
Case No.: B-008	Appellant Phone: 6462630186
Appeal Type: Building	Plans Examiner/Inspector: Geoffrey Harker
Project Type: commercial	Stories: 5 Occupancy: R-2, M Construction Type: III-B
Building/Business Name: Atomic Orchard	Fire Sprinklers: Yes - throughout
Appeal Involves: Reconsideration of appeal	LUR or Permit Application No.:
Plan Submitted Option: pdf [File 1] [File 2] [File 3] [File 4] [File 5] [File 6]	Proposed use: Mixed Use

APPEAL INFORMATION SHEET

Appeal item 1

Code Section 705.8.6.2

Requires	<p>When a new building is to be erected adjacent to an existing building, all openings in the exterior wall of the new building are required to be not less than 3/4 hour when these openings are less than 15 feet vertically above the roof of the existing building or structure. The opening protectives are required where the distance between the building or structures is less than 15 feet. When the roof of the new building is at lower elevation from the existing building, the roof construction of the new building shall have a fire-resistance rating of not less than 1 hour for a minimum of 10 feet from the exterior wall facing the new building. The entire length and span of the supporting elements for the fire-resistance-rated roof assembly shall also have a fire-resistance rating of not less than 1 hour. The roof protections are required where the distance between the buildings or structures is less than 15 feet.</p>
Proposed Design	<p>The west elevation of this site, 2510 NE Sandy, shows exterior exit stair, Stair A, of which a 9'-0" wide x 15'-0" high portion is located within 15' vertically above the roof of the existing building. There are 2 overhead doors also within this zone. (see Exhibit 1 for the west elevation and Exhibit 2 for a partial site plan showing the two adjacent properties).</p>

There is an existing one story building at 2500 NE Sandy Blvd., which is built to the property line in all directions. Appellant proposes that 2500 NE Sandy is treated as the "new" building, since it is owned by the same owner as 2510 NE Sandy and therefore can be modified to be code compliant. Appellant proposes that all of the roof area which is within 15' of 2510 NE Sandy Blvd. be modified to have a 1 hour roof construction (see Exhibit 3 for the one hour detail and Exhibit 4 for the roof plan for 2500 NE Sandy Blvd). This roof area on the 2500 NE Sandy Blvd. project includes an area 42"(north-south) x 10'-0" (east west) from the 2510 NE Sandy Blvd. north property line. Heavy timber beams and columns, and concrete pilasters in the 2500 NE Sandy Blvd. site meet 1 hour rating equivalency per char calculations for this area of the roof. (Exhibit 5)

Reconsideration Text: A survey has been added showing the two adjacent site plans, with the 1 hour roof portion of the 2500 NE Sandy Blvd. site identified.

Reason for alternative Treating the existing building as the new building provides equivalent fire and life safety regardless of the order in which the buildings are constructed. Additional information including a 1 hour rated roof ceiling assembly is provided, in addition to the roof framing plan for 2500 NE Sandy Blvd. Appeal ID 15124 is a similar appeal which has been granted.

Reconsideration Text: A survey has been added showing the two adjacent site plans, with the 1 hour roof portion of the 2500 NE Sandy Blvd. site identified.

Appeal item 2

Code Section 1026.5 Exterior Exit Stairways and Ramps, Location

Requires Exterior exit stairways and ramps shall have a minimum fire separation distance of 10 feet measured from the exterior edge of the stairway or ramp, including landings, to adjacent lot lines and from other buildings on the same lot unless the adjacent building exterior walls and openings are protected in accordance with Section 705 based on fire separation distance.

Proposed Design The west stair (Stair A) is located within 10' of the property line, however the adjacent lot is owned by the same owner as this site. The adjacent property to the west, 2500 NE Sandy Blvd, is a one story building with an existing two hour wall next to Stair A. This 2 hour concrete wall will provide protection for the exterior stair below the roof line of the adjacent building. A no-build easement is proposed above the roof of the adjacent building to provide the required 10 foot separation above the roof of 2500 NE Sandy Blvd. (see Exhibit 2). Stair A is additionally protected by the 1 hour roof assembly proposed in Appeal Item 1.

Reconsideration Text: A survey has been added showing the two adjacent site plans, with the 1 hour roof portion of the 2500 NE Sandy Blvd. site identified.

Reason for alternative The two hour concrete wall provides an equivalent level of fire and life safety for the area below the parapet of 2500 NE Sandy Blvd. for occupants of both buildings and the no-build easement above the existing roof will provide the required clearance for the occupants using Stair A above the roof of the adjacent building. Since both sites are owned by the same owner, it is possible to make both designs safe and compliant for occupants of both buildings.

Reconsideration Text: A survey has been added showing the two adjacent site plans, with the 1 hour roof portion of the 2500 NE Sandy Blvd. site identified.

APPEAL DECISION

1. One hour fire rated roof construction of existing lower building in buildings with vertical exposure on separate lots: Granted provided the building permit for the 1 hour fire resistance rating of the roof structure is finalized prior to the issuance of the Certificate of Occupancy for the proposed building.

Note: The submitted char calculations were not reviewed as part of this appeal. They will be reviewed during structural review of the building permit submittal documents.

Appellant may contact John Butler (503 823-7339) with questions.

2. Exterior exit stair with less than 10 feet of fire separation distance and no build easement: Granted provided the easement is a minimum of 10 feet wide.

Appellant must contact Nancy Thorington (503-823-7023) for more information prior to writing the no build easement.

The easement must include language that establishes the edge of the easement on the adjacent lot as the implied property line for the purpose of determining fire separation distance for future development on the adjacent lot.

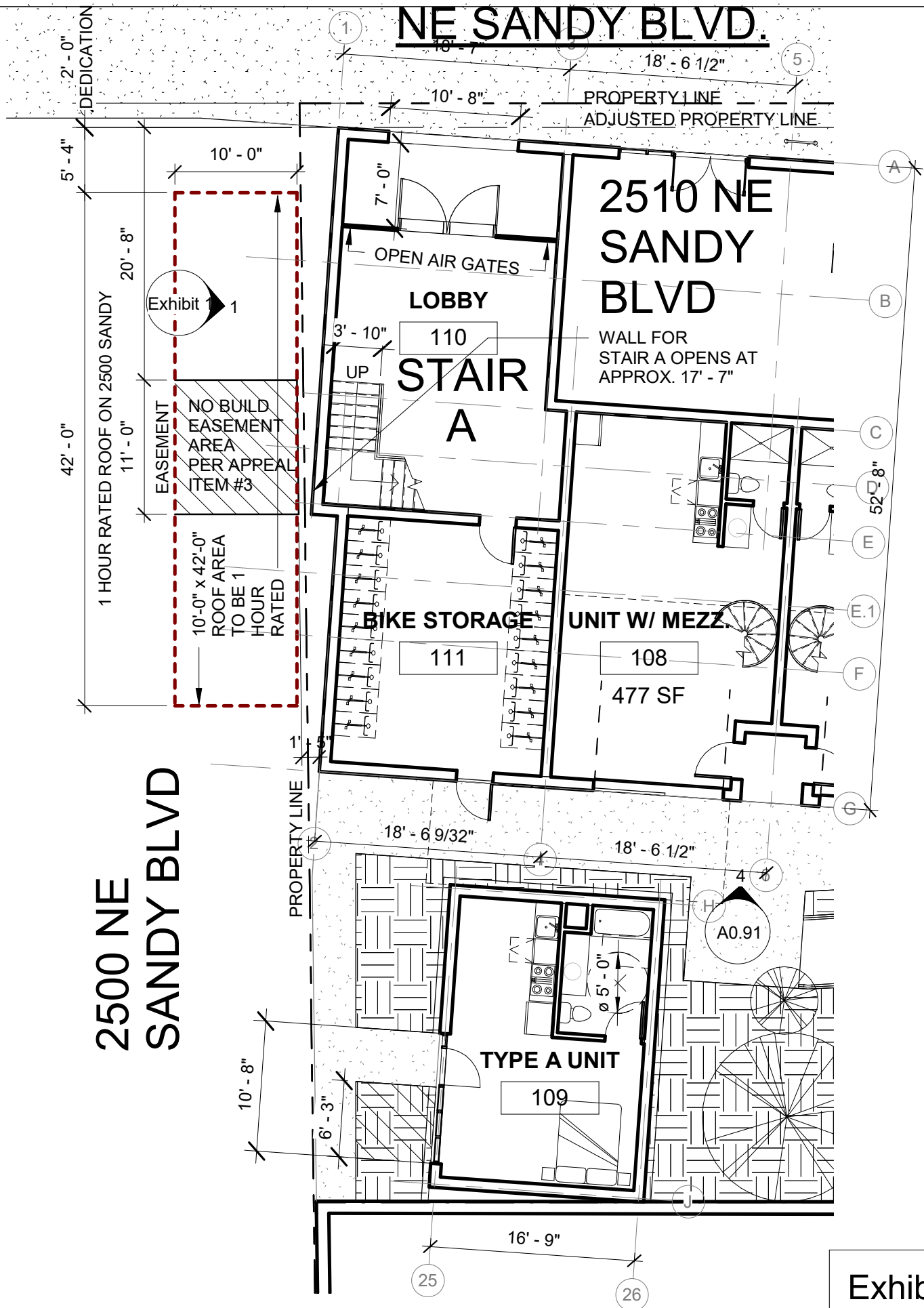
The unrecorded easement must be reviewed and approved by BDS prior to recording. A copy of the recorded easement must then be provided to BDS prior to plan review approval.

The Administrative Appeal Board finds with the conditions noted, that the information submitted by the appellant demonstrates that the approved modifications or alternate methods are consistent with the intent of the code; do not lessen health, safety, accessibility, life, fire safety or structural requirements; and that special conditions unique to this project make strict application of those code sections impractical.

Pursuant to City Code Chapter 24.10, you may appeal this decision to the Building Code Board of Appeal within 180 calendar days of the date this decision is published. For information on the appeals process and costs, including forms, appeal fee, payment methods and fee waivers, go to www.portlandoregon.gov/bds/appealsinfo, call (503) 823-7300 or come in to the Development Services Center.

2500 NE
SANDY BLVD

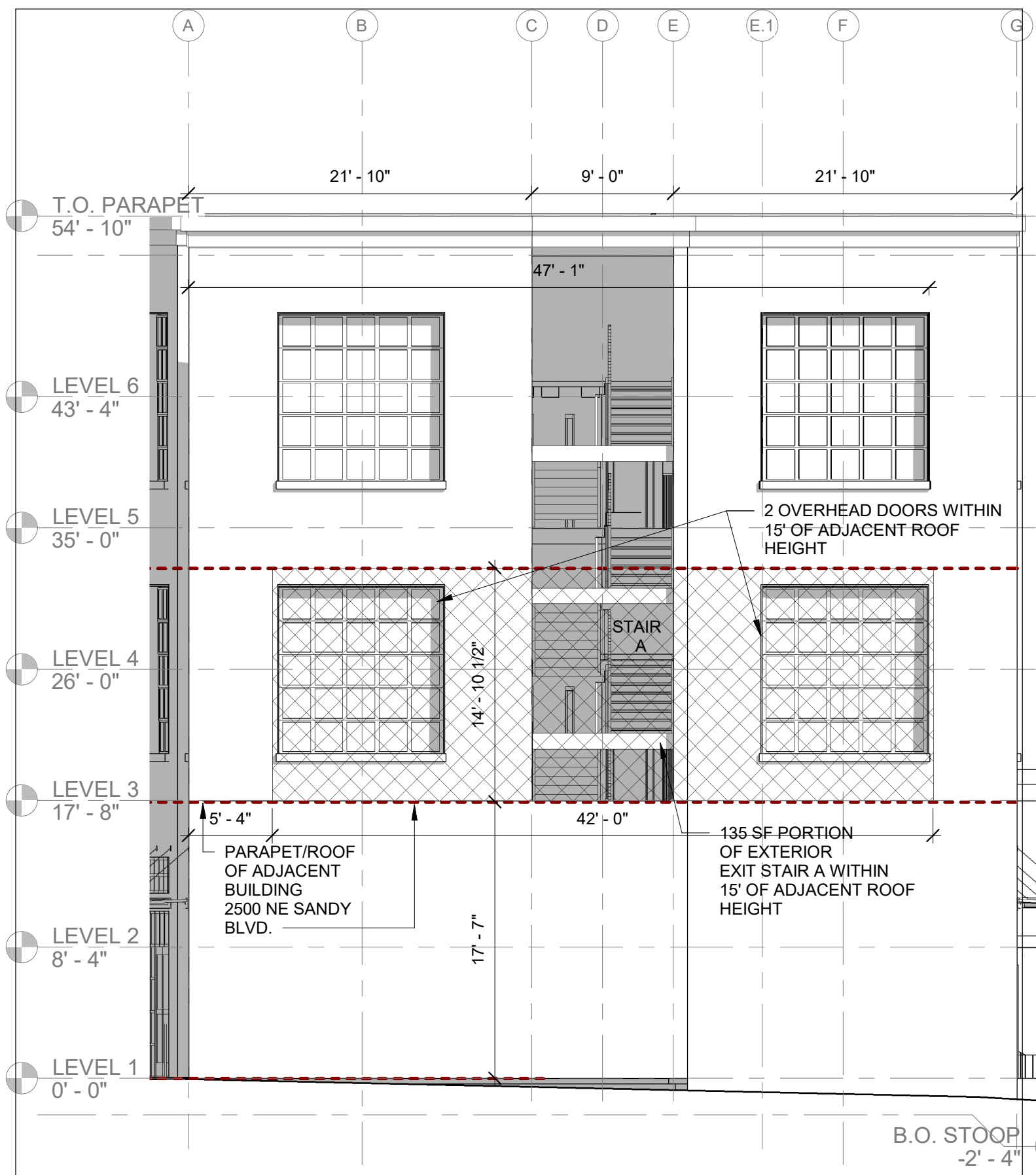
NE SANDY BLVD.



1 PARTIAL FIRST FLOOR/SITE PLAN

Exhibit 2

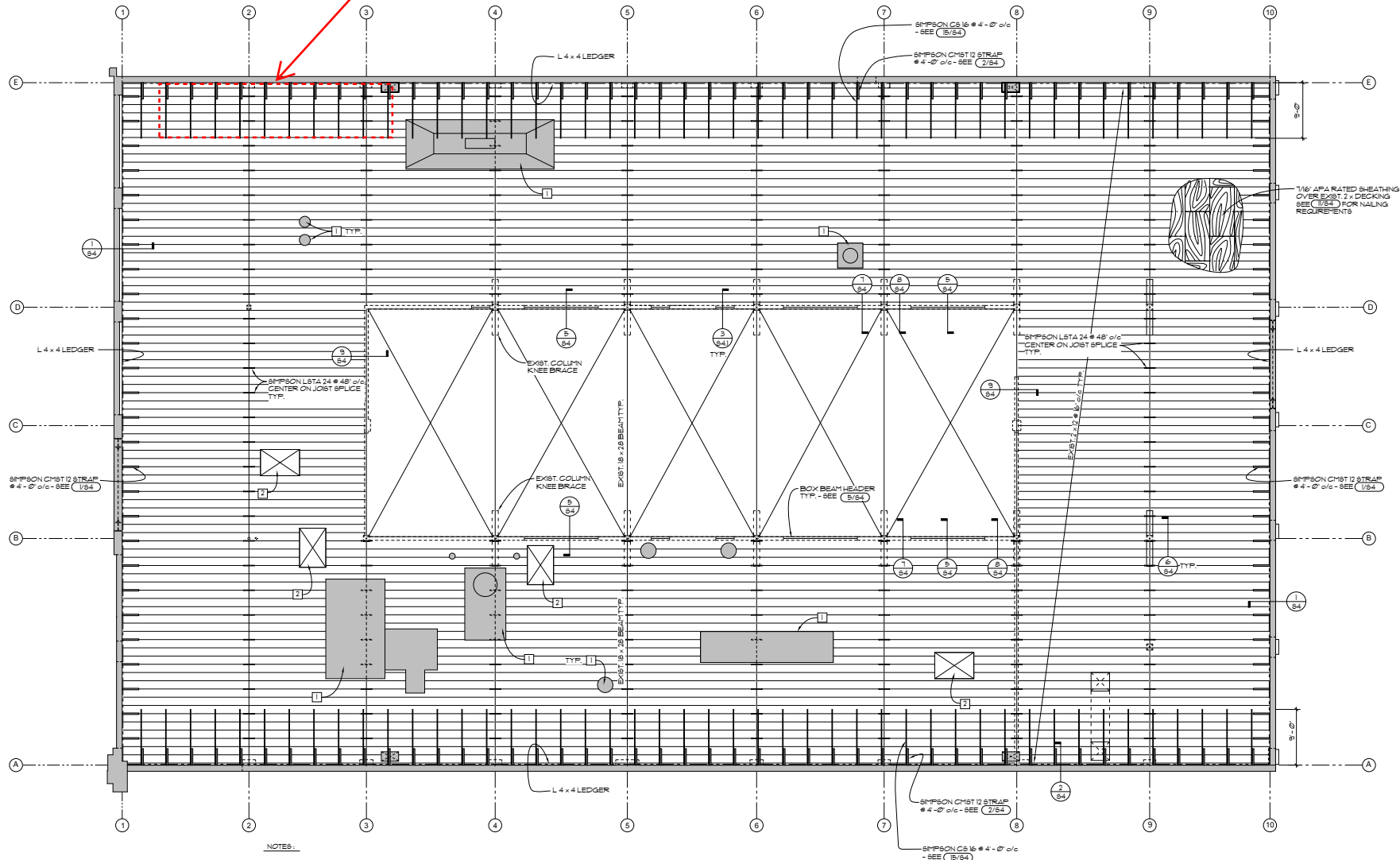
Scale 3/32" = 1'-0"



1 WEST ELEVATION Appeal

Exhibit 1
Scale 1/8" = 1'-0"

AREA OF 1 HR ROOF



ROOF FRAMING PLAN

1/8" = 1'-0"



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503-607-0481, FAX 503-607-0486

NEW CRUSHER COURT
2500 NE SANDY BLVD
PORTLAND, OR 97209

PERMIT:

ISSUE: SPACE PLAN

ISSUE DATE: 12/29/15

DRAWN: CN

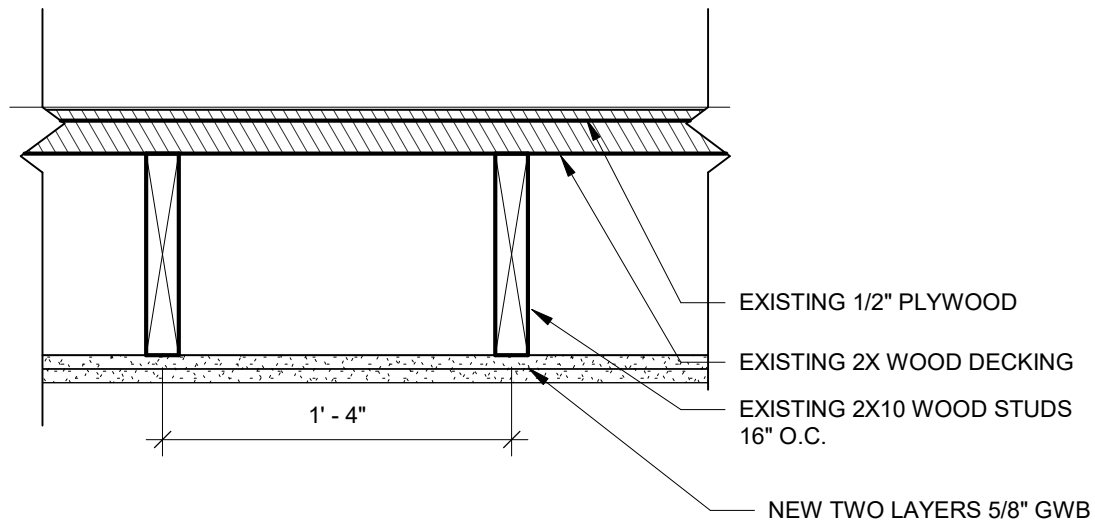
APPROVED: MV

REVISIONS:

SCALE: AS NOTED

Exhibit #4

S2



1 HR RATING: GA #RC 2601

① 1 HR ROOF CEILING ASSEMBLY
1 1/2" = 1'-0"

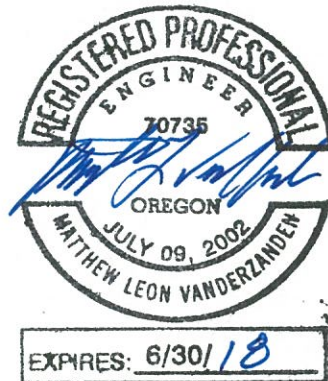
BASE LAYER 5/8" GYP APPLIED
AT RIGHT ANGLES TO 2 X 10 WOOD JOISTS
WITH 1 1/4" TYPE W OR S DRYWALL SCREWS 24" O.C.

FACE LAYER APPLIED AT RIGHT ANGLES TO JOISTS
WITH 1 7/8" TYPE W OR S DRYWALL SCREWS 12" O.C.
AT JOINTS AND INTERMEDIATE JOISTS

SUPPLEMENTAL STRUCTURAL CALCULATIONS

FOR

Char Calculations
New Crusher Court
2500 NE Sandy Blvd.
Portland, OR



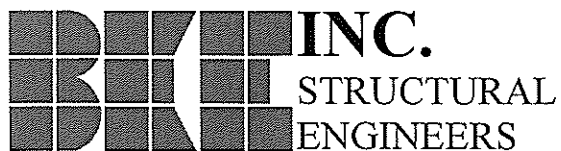
Project: CHAR CALC. @ New New Crusher Court

Client: Brett Schulz Architect Proj. No.: 15-440

Date: 05/2017 By: MLV Sheet No.: _____

DESIGN SUMMARY

THESE CALCULATIONS ARE TO DETERMINE THE ADEQUACY OF THE EXISTING HEAVY TIMBER BEAMS AND COLUMNS TO RESIST A 1 HR FIRE. CHAPTER 16 OF THE 2012 NDS WAS USED FOR THESE CALCULATIONS.



Project: CHAR Calc @ New New Crusher Court

Client: Brett Schulz Architect

Proj. No.: 15-440

Date: 05/2017

By: MLV

Sheet No.: Summary

BEAMS

(NDS CH. 16)

$$b = 11''$$

$$h = 27''$$

$$\beta_{eff} = 1.8$$

$$a_{char} = 1.8$$

$$b_{char} = 11 - 2(1.8) = 7.4 \text{ in}$$

$$h_{char} = 27 - 1.8 = 25.2 \text{ in}$$

$$S_{char} = \frac{7.4(25.2)^2}{6} = 783.2 \text{ in}^3$$

$$F_b = 1000 \text{ PSI}$$

$$C_F = 1.0$$

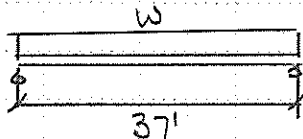
$$C_V = .929(.922)(.94) = 0.8$$

$$C_L = 1.0$$

$$C_d = 1.15$$

$$F'_{b_{CHAR}} = 1000(2.85)(.8)(1.15) \\ = \underline{2622 \text{ PSI}}$$

$$W = 21.5(15+25) \\ = 860 \text{ PIF}$$



$$m = 147168$$

$$f_b = \frac{m}{S_{char}} = \frac{147168(12)}{783.2} = 2254.8$$

$$f_{b_{char}} < F'_{b_{CHAR}} \therefore \text{OK}$$

COLUMNS

$$b = 11''$$

$$d = 11''$$

$$\beta_{eff} = 1.8$$

$$a_{char} = 1.8$$

$$b = d = 11 - 2(1.8) = 7.4''$$

$$P_{DL} = 21.5(37)(15) = 11932$$

$$P_{SL} = 21.5(37)(25) = 19888$$

11x11 OK SEE OUTPUT

Wood Column

Lic. # : KW-06007984

Licensee : BK ENGINEERS INC

Description : -None-

Code References

Calculations per

Load Combinations Used : ASCE 7-10

General Information

Analysis Method :		Allowable Stress Design		Wood Section Name		11x11 char	
End Fixities		Top & Bottom Pinned		Wood Grading/Manuf.		Graded Lumber	
Overall Column Height		14.666 ft		Wood Member Type		Sawn	
(Used for non-slender calculations)							
Wood Species		Douglas Fir - Larch		Exact Width		7.40 in Allow Stress Modification Factors	
Wood Grade		No.1		Exact Depth		7.40 in Cf or Cv for Bending 1.0	
Fb - Tension		1200 psi	Fv	170 psi	Area		54.760 in^2 Cf or Cv for Compression 1.0
Fb - Compr		1200 psi	Ft	825 psi	Ix		249.888 in^4 Cf or Cv for Tension 1.0
Fc - Prll		1000 psi	Density	31.2 pcf	Iy		249.888 in^4 Cm : Wet Use Factor 1.150
Fc - Perp		625 psi					Ct : Temperature Factor 1.0
E : Modulus of Elasticity . . .		x-x Bending	y-y Bending	Axial			Cfu : Flat Use Factor 1.0
Basic		1600	1600	1600 ksi			Kf : Built-up columns 1.0 NDS 15.3.2
Minimum		580	580				Use Cr : Repetitive ? No
Brace condition for deflection (buckling) along columns :							
				X-X (width) axis :		Unbraced Length for X-X Axis buckling = 14.666 ft, K = 1.0	
				Y-Y (depth) axis :		Unbraced Length for X-X Axis buckling = 14.666 ft, K = 1.0	

Applied Loads

Service loads entered. Load Factors will be applied for calculations.

Column self weight included : 174.007 lbs * Dead Load Factor

AXIAL LOADS . . .

Axial Load at 14.666 ft, D = 11.933, S = 19.888 k

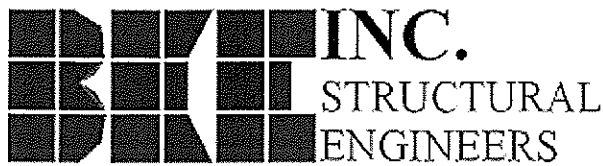
DESIGN SUMMARY

Bending & Shear Check Results

PASS	Max. Axial+Bending Stress Ratio =	0.8817 : 1	Maximum SERVICE Lateral Load Reactions . .		
	Load Combination	+D+S+H	Top along Y-Y	0.0 k	Bottom along Y-Y 0.0 k
	Governing NDS Formula	Comp Only, f_c/F_c'	Top along X-X	0.0 k	Bottom along X-X 0.0 k
	Location of max. above base	0.0 ft	Maximum SERVICE Load Lateral Deflections . .		
	At maximum location values are . . .		Along Y-Y	0.0 in at 0.0 ft above base	
	Applied Axial	31.995 k	for load combination : n/a		
	Applied Mx	0.0 k-ft	Along X-X	0.0 in at 0.0 ft above base	
	Applied My	0.0 k-ft	for load combination : n/a		
	Fc : Allowable	662.68 psi	Other Factors used to calculate allowable stresses . .		
PASS	Maximum Shear Stress Ratio =	0.0 : 1		Bending	Compression Tension
	Load Combination	+0.60D+0.70E+0.60H			
	Location of max. above base	14.666 ft			
	Applied Design Shear	0.0 psi			
	Allowable Shear	272.0 psi			

Load Combination Results

Load Combination	C _D	C _P	Maximum Axial + Bending Stress Ratios			Maximum Shear Ratios		
			Stress Ratio	Status	Location	Stress Ratio	Status	Location
+D+H	0.900	0.668	0.3678	PASS	0.0 ft	0.0	PASS	14.666 ft
+D+L+H	1.000	0.629	0.3513	PASS	0.0 ft	0.0	PASS	14.666 ft
+D+Lr+H	1.250	0.544	0.3250	PASS	0.0 ft	0.0	PASS	14.666 ft
+D+S+H	1.150	0.576	0.8817	PASS	0.0 ft	0.0	PASS	14.666 ft
+D+0.750Lr+0.750L+H	1.250	0.544	0.3250	PASS	0.0 ft	0.0	PASS	14.666 ft
+D+0.750L+0.750S+H	1.150	0.576	0.7447	PASS	0.0 ft	0.0	PASS	14.666 ft
+D+0.60W+H	1.600	0.452	0.3056	PASS	0.0 ft	0.0	PASS	14.666 ft
+D+0.70E+H	1.600	0.452	0.3056	PASS	0.0 ft	0.0	PASS	14.666 ft
+D+0.750Lr+0.750L+0.450W+H	1.600	0.452	0.3056	PASS	0.0 ft	0.0	PASS	14.666 ft
+D+0.750L+0.750S+0.450W+H	1.600	0.452	0.6821	PASS	0.0 ft	0.0	PASS	14.666 ft
+D+0.750L+0.750S+0.5250E+H	1.600	0.452	0.6821	PASS	0.0 ft	0.0	PASS	14.666 ft
+0.60D+0.60W+0.60H	1.600	0.452	0.1834	PASS	0.0 ft	0.0	PASS	14.666 ft



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 and then using the "Printing &
 Title Block" selection.
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 ENERCALC, INC. 1983-2017, Build:6.17.2.28, Ver:6.17.2.28

Wood Column

Lic. #: KW-06007984

Licensee: BK ENGINEERS INC

Description: --None--

Load Combination Results

Load Combination	C _D	C _P	Maximum Axial + Bending Stress Ratios			Maximum Shear Ratios		
			Stress Ratio	Status	Location	Stress Ratio	Status	Location
+0.60D+0.70E+0.60H	1.600	0.452	0.1834	PASS	0.0 ft	0.0	PASS	14.666 ft

Maximum Reactions

Note: Only non-zero reactions are listed.

Load Combination	X-X Axis Reaction		Y-Y Axis Reaction		Axial Reaction
	@ Base	@ Top	@ Base	@ Top	@ Base
+D+H		k		k	12.107 k
+D+L+H		k		k	12.107 k
+D+Lr+H		k		k	12.107 k
+D+S+H		k		k	31.995 k
+D+0.750Lr+0.750L+H		k		k	12.107 k
+D+0.750L+0.750S+H		k		k	27.023 k
+D+0.60W+H		k		k	12.107 k
+D+0.70E+H		k		k	12.107 k
+D+0.750Lr+0.750L+0.450W+H		k		k	12.107 k
+D+0.750L+0.750S+0.450W+H		k		k	27.023 k
+D+0.750L+0.750S+0.5250E+H		k		k	27.023 k
+0.60D+0.60W+0.60H		k		k	7.264 k
+0.60D+0.70E+0.60H		k		k	7.264 k
D Only		k		k	12.107 k
Lr Only		k		k	k
L Only		k		k	k
S Only		k		k	19.888 k
W Only		k		k	k
E Only		k		k	k
H Only		k		k	k

Maximum Deflections for Load Combinations

Load Combination	Max. X-X Deflection	Distance	Max. Y-Y Deflection	Distance
+D+H	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+L+H	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+Lr+H	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+S+H	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+0.750Lr+0.750L+H	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+0.750L+0.750S+H	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+0.60W+H	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+0.70E+H	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+0.750Lr+0.750L+0.450W+H	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+0.750L+0.750S+0.450W+H	0.0000 in	0.000 ft	0.000 in	0.000 ft
+D+0.750L+0.750S+0.5250E+H	0.0000 in	0.000 ft	0.000 in	0.000 ft
+0.60D+0.60W+0.60H	0.0000 in	0.000 ft	0.000 in	0.000 ft
+0.60D+0.70E+0.60H	0.0000 in	0.000 ft	0.000 in	0.000 ft
D Only	0.0000 in	0.000 ft	0.000 in	0.000 ft
Lr Only	0.0000 in	0.000 ft	0.000 in	0.000 ft
L Only	0.0000 in	0.000 ft	0.000 in	0.000 ft
S Only	0.0000 in	0.000 ft	0.000 in	0.000 ft
W Only	0.0000 in	0.000 ft	0.000 in	0.000 ft
E Only	0.0000 in	0.000 ft	0.000 in	0.000 ft
H Only	0.0000 in	0.000 ft	0.000 in	0.000 ft

Wood Column

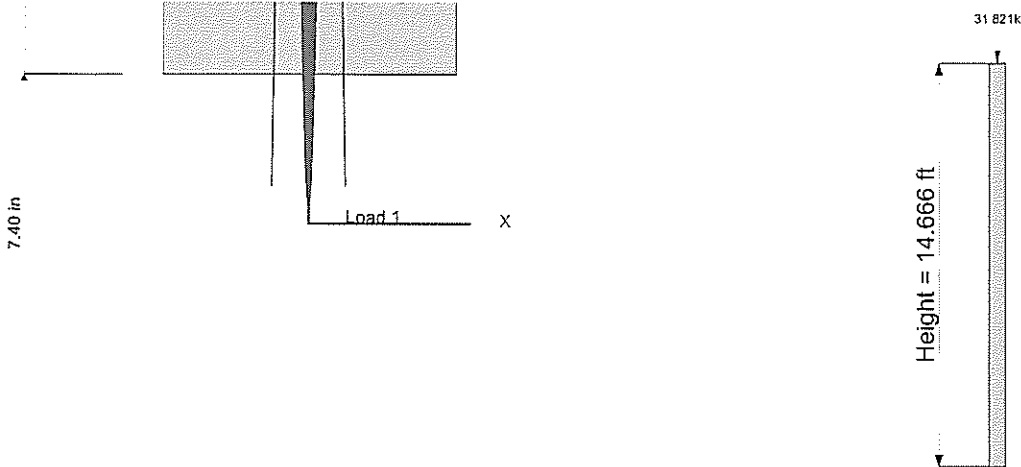
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 ENERCALC, INC. 1983-2017, Build:6.17.2.28, Ver:6.17.2.28

Lic. # : KW-06007984

Licensee : BK ENGINEERS INC

Description : --None--

Sketches



Loads are total entered value. Arrows do not reflect absolute direction.

