PSE
PETERSON STRUCTURAL ENGINEERS

PORTLAND OFFICE
9400 SW Barnes Rd.
Suite 100
Portland, OR 97225
Phone: 503.292.1635

Structural Design Calculations

Portland Airport DAS Rack Anchorage
Portland, OR

Client Information	Project Site
Michael Smith	Portland International Airport
CTS, LLC	7000 NE Airport Way
8309 SW Cirrus Dr	Portland, OR 97218
Beaverton, OR 97005	45.5914°N, 122.5971°W

Prepared By:
Peterson Structural Engineers
September 4, 2019
Job No. 1901-0239

Endorsement

STRUCTURAL · REGISTERED PROFESSIONAL ENGINEER · 63186PE · OREGON · NOV. 9, 2004 · TRAVIS GREGORY MEFFORD · EXPIRES 12/31/20

Scope: To provide structural calculations for the anchorage design of network equipment racks located at the project location given on the cover page. Elements not specifically referenced in these calculations are outside the scope of this design and are assumed to be designed by others.

References:

1 2014 Oregon Structural Specialty Code (OSSC)
2 SEI/ASCE 7-10, Minimum Design Loads for Buildings and Other Structures, American Society of Civil Engineers (ASCE)
3 2011 Building Code Requirements for Structural Concrete, ACI 318-11 and Commentary (ACI)

Table of Contents

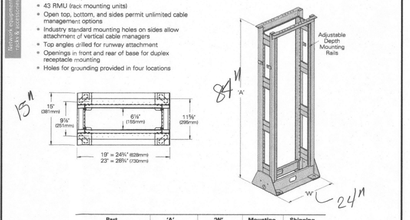
Drawings

Network Equipment Racks & Accessories

Two-Post Relay Rack

Two-Post Seismic Relay Rack
19" & 23" Panel Mounting, Welded Steel

- Independently tested to NEBS GR-63-CORE Seismic Zone 4 standard, and for 900 lbs. (408kg) of equipment
- Independent testing conducted in accordance with ICBO AC156 standard for use up to 900 lbs. (408kg)
- Conformance to EIA/ECA-310-E
- Mounting rail spacing is adjustable from 3" (76mm) to 8" (203mm)
- 43 RMU track mounting units)
- Open top, bottom, and sides permit unlimited cable management options
- Industry standard mounting holes on sides allow attachment of vertical cable managers
- Top angles drilled for runaway attachment
- Openings in front and rear of base for duplex receptacle mounting
- Holes for grounding provided in four locations
- Tubular steel frame provides a high degree of strength to weight ratio
- See page ii for link to OPM approvals
- Concrete installation requires (4) AWA-62-350 (sold separately)
- Finish ___ Flat black powder coat (FB)
  Telco gray powder coat (TG)



Part Number	'A' Height In. (mm)	'W' Spacing In. (mm)	Mounting Spaces	Shipping Weight Lbs. (kg)
19" (482mm) Rack Width				
SB8219084___	84" (2133)	24¾" (629)	43	113 (51.3)
SB8219096	96" (2438)	24¾" (629)	49	124 (56.2)
23" (584mm) Rack Width				
SB8223084___	84" (2133)	28¾" (730)	43	117 (53.1)
SB8223096___	96" (2133)	28¾" (730)	49	127 (57.6)

Specify part number and finish when ordering.

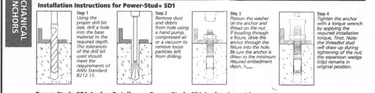
Drawings (cont.)

Power-Stud+® SD1 PRODUCT INFORMATION — Powers FASTENERS

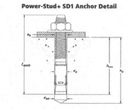
MECHANICAL ANCHORS

INSTALLATION INSTRUCTIONS

Installation Instructions for Power-Stud+ SD1



Power-Stud+ SD1 Anchor Detail



Power-Stud+ SD1 Anchor Assembly

Head Marking

Legend
Letter Code = Length Identification Mark
"+" Symbol = Strength Design Compliant Anchor (see ordering information)
Number Code = Carbon Steel Body and Stainless Steel Expansion Clip (not on 1/4" diameter anchors)

Length Identification

Mark	A	B	C	D	E	F	G	H	J	K	L	M	N	O	P	Q	R	S	T
From	1-1/2"	2"	2-1/2"	3"	3-1/2"	4"	4-1/2"	5"	5-1/2"	6"	6-1/2"	7"	7-1/2"	8"	8-1/2"	9"	9-1/2	10"	11"	12"
Up to but not including	2"	2-1/2"	3"	3-1/2"	4"	4-1/2"	5"	5-1/2"	6"	6-1/2"	7"	7-1/2"	8"	8-1/2"	9"	9-1/2	10"	11"	12"	13"

Length identification mark indicates overall length of anchor.

REFERENCE DATA (ASD)

Installation Specifications for Power-Stud+ SD1 in Concrete[1,2]

Anchor Property/ Setting Information	Notation	Units	Nominal Anchor Diameter 1/4	3/8	1/2	5/8	3/4	3/8	1	1-1/4
Anchor diameter	d_a	in. (mm)	0.250 (6.4)	0.375 (9.5)	0.500 (12.7)	0.625 (15.9)	0.750 (19.1)	0.875 (22.2)	1.000 (25.4)	1.250 (31.8)
Minimum diameter of hole clearance in fixture	d_h	in. (mm)	5/16 (7.5)	7/16 (11.1)	9/16 (14.3)	11/16 (17.5)	13/16 (20.6)	15/16 (23.8)	1-1/16 (27.0)	1-5/16 (33.3)
Nominal drill bit diameter	d_o	in.	1/4" AWG	3/8" AWG	1/2" AWG	5/8" AWG	3/4" AWG	7/8" AWG	1" AWG	1-1/4" AWG
Minimum nominal embedment depth	h_{nom}	in. (mm)	1-1/8 (29)	1-5/8 (41)	2-1/4 (57)	2-3/4 (70)	3-3/8 (86)	4-1/2 (114)	4-1/2 (114)	6-1/2 (165)
Minimum hole depth	h_h	in. (mm)	1-3/4 (44)	2-3/4 (70)	3-1/2 (89)	4-1/4 (108)	5 (127)	6-1/8 (156)	6-1/8 (156)	7-1/2 (190)
Installation torque	T_{inst}	ft.-lbf (N-m)	4 (5)	25 (34)	40 (54)	80 (108)	110 (149)	175 (237)	225 (305)	375 (508)
Torque wrench/socket size	-	in.	7/16	9/16	3/4	15/16	1-1/8	1-5/16	1-1/2	1-7/8
Nut height	h_n	in.	7/32	21/64	7/16	35/64	41/64	3/4	55/64	1-1/16

For SI: 1 inch = 25.4 mm, 1 ft-lbf = 1.356 N-m.
1. The minimum base material thickness should be 1.5h_{nom} or 3", whichever is greater.
2. See Performance Data in Concrete for additional embedment depths.

Design Criteria

Latitude: 45.5914°N Longitude: 122.5971°W

Unit Properties

W_{up} = unit/curb wet weight =	585 lbs	Weight of unit and curb when full
W_u = unit/curb dry weight =	113 lbs	Weight of unit and curb when empty
L_u = unit/curb length =	2.06 ft	Controlling dimension of longest side
B_u = unit/curb width =	1.25 ft	Controlling dimension of shortest side
H_u = unit height =	7.00 ft	Dimension of vertical height
H_c = curb height =	0.00 ft	Dimension of vertical height
H_t = assembly total height =	7.00 ft	$H_u + H_c$
h_a = attachment height =	0.00 ft	Elevation above grade at bottom of curb/unit
L_b = building length =	n/a ft	Parallel to unit length
B_b = building width =	n/a ft	Parallel to unit width
h = mean roof height =	n/a ft	Elevation above grade to mean roof height
COA_{wet} = vertical center of area =	3.50 ft	Assumed (1/2)H_u+H_c
COG_{wet} = vertical center of gravity =	4.67 ft	Assumed (2/3)H_u+H_c

Seismic Loading

Soil Site Class:	D
Risk Category:	IV
S_{1*} =	0.955 g per ATC Hazards, see SH. 5
S_1 =	0.401 g per ATC Hazards, see SH. 5
S_{DS} =	0.712 g per ATC Hazards, see SH. 5
S_{D1} =	0.427 g per ATC Hazards, see SH. 5

Wind Loading

Exposure:
V = basic wind speed = n/a mph

Live Loading

q_h = horizontal partition pressure 5 psf

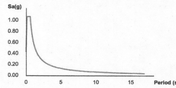
9/3/2019 ATC Hazards by Location

ATC Hazards by Location

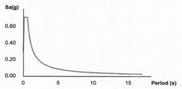
Search Information

Coordinates:	45.591487, -122.597176
Elevation:	ft
Timestamp:	2019-09-03T17:58:14.983Z
Hazard Type:	Seismic
Reference Document:	ASCE7-10
Risk Category:	IV
Site Class:	D

MCER Horizontal Response Spectrum



Design Horizontal Response Spectrum



Basic Parameters

Name	Value	Description
S_S	0.955	MCE_R ground motion (period=0.2s)
S_1	0.401	MCE_R ground motion (period=1.0s)
S_{MS}	1.067	Site-modified spectral acceleration value
S_{M1}	0.641	Site-modified spectral acceleration value
S_{DS}	0.712	Numeric seismic design value at 0.2s SA
S_{D1}	0.427	Numeric seismic design value at 1.0s SA

Additional Information

Name	Value	Description
SDC	D	Seismic design category
F_a	1.118	Site amplification factor at 0.2s
F_v	1.599	Site amplification factor at 1.0s
CR_S	0.896	Coefficient of risk (0.2s)

https://hazards.atcouncil.org/#/seismic?lat=45.591487&lng=-122.597176&address=

Seismic Loading

ASCE 7-10, ⌊

COG_{wet} =	4.67	ft	Assumed (2/3)H_u+H_c	Distance above attachment point to the center of gravity
$h_a = z$ =	0.00	ft		Height of attachment
W_{up} =	585	lbs		Wet weight
h =	n/a	ft		Mean roof height
S_{DS} =	0.712			Short period spectral acceleration
a_p =	1.00			Amplification factor — Table 13.6-1
R_p =	2.50			Response modification factor — Table 13.6-1
Ω_o =	2.50			Overstength factor — Table 13.6-1
I_p =	1.50			Component importance factor — Section 13.1.3
$F_{ph,calc}$ =	100	lb	$0.4 \cdot a_p \cdot S_{DS} \cdot W_{up} / (R_p / I_p) \cdot (1+2 \cdot z/h_r)$	Calculated horizontal seismic force — Eq. 13.3-1
$F_{ph,max}$ =	1000	lb	$1.6 \cdot S_{DS} \cdot I_p \cdot W_{up}$	Maximum horizontal seismic force
$F_{ph,min}$ =	187	lb	$0.3 \cdot S_{DS} \cdot I_p \cdot W_{up}$	Minimum horizontal seismic force
F_{ph} =	187	lb		Design seismic shear (LRFD) — Section 13.3.1
$0.7 \cdot F_{ph}$ =	131	lb		Design seismic shear (ASD)
$F_{ph} \cdot \Omega_o$ =	469	lb		Design seismic shear with overstength (LRFD)
$0.7 \cdot F_{ph} \cdot \Omega_o$ =	328	lb		Design seismic shear with overstength (ASD)

Design Anchors for Fph = 187 lbs. OR Fph·Ωo = 469 lbs. (LRFD)
Design Anchors for 0.7·Fph = 131 lbs. OR 0.7·Fph·Ωo = 328 lbs. (ASD)

M_E =	875	lb-ft	$F_{ph} \cdot COG_{wet}$ Eq. 12.4-4	Overturning moment, due to horizontal load (LRFD)
$0.7 \cdot M_E$ =	612	lb-ft	Eq. 12.4-4	Overturning moment, due to horizontal load (ASD)
$M_E \cdot \Omega_o$ =	2187	lb-ft	(LRFD)	Overturning moment w/overstength, due to horiz. load
$0.7 \cdot M_E \cdot \Omega_o$ =	1531	lb-ft	(ASD)	Overturning moment w/overstength, due to horiz. load

Design Anchors for ME = 875 lb-ft OR ME·Ωo = 2187 lb-ft (LRFD)
Design Anchors for 0.7·ME = 612 lb-ft OR 0.7·ME·Ωo = 1531 lb-ft (ASD)

F_{pv} =	83	lb	$0.25 S_{DS} W_{up}$ — Design uplift seismic force (acts up or down) (LRFD)
$0.7 \cdot F_{pv}$ =	58	lb	Design uplift seismic force (acts up or down) (ASD)
P_u =	443	lb	$0.9 \cdot W_{up} - F_{pv}$ — Factored vert. force (+ indicates load acts down) (LRFD)
P_r =	293	lb	$0.6 \cdot W_{up} - 0.7 \cdot F_{pv}$ — Factored vert. force (+ indicates load acts down) (ASD)

Design Anchors for Pu = 443 lbs. (LRFD)
Design Anchors for Pr = 293 lbs. (ASD)

Live Loading

IBC 2012, ⌊

COA_{wet} =	3.50	ft	Assumed (1/2)H_u+H_c	Height above attachment point to the centroid of A_r
W_u =	113	lb		Dry weight
L_u =	2.06	ft		Length of unit
B_u =	1.25			Width of unit
H_u =	7.00	ft		Total height of unit & curb
A_r =	14.42	ft^2	$B_u \cdot H_t$	Controlling horizontal area normal to pressure IBC 1607.14
q_h =	5.00	psf		Horizontal live load pressure
F_h =	115	lb	$1.6 \cdot q_h \cdot A_r$	Design horizontal live load force (LRFD)
$F_h/1.6$ =	72	lb		Design horizontal live load force (ASD)

Design Anchors for Fh = 115 lbs. (LRFD)
Design Anchors for Fh/1.6 = 72 lbs. (ASD)

M_{ll} =	404	lb-in	$F_h \cdot COA_{wet}$	Overturning moment, due to horizontal load
$M_{ll}/1.6$ =	252	lb-in		Overturning moment, due to horizontal load

Design Anchors for MLL = 404 lbs. (LRFD)
Design Anchors for MLL/1.6 = 252 lbs. (ASD)

P_u =	102	lb	$0.9W_D$ — Factored vertical force (positive indicates load acts down)
P_r =	68	lb	$0.6W_D$ — Factored vertical force (positive indicates load acts down)

Design Anchors for Pu = 102 lbs. (LRFD)
Design Anchors for Pr = 68 lbs. (ASD)

City of Portland
REVIEWED FOR CODE COMPLIANCE
JAN 29 2020
19-106363 DFS DFA
Permit Number

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1 STRUCTURAL CALCULATION PAGES
SCALE: NONE

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PORTLAND INTERNATIONAL AIRPORT
PORTLAND, OREGON

DEFERRED SUBMITTAL

RACK ANCHORAGE CALCULATION SHEETS

PROJ. NO. 17096
11.20.2019

19-106363-DFS-1 FA

DFS-1
Rev 2

PETERSON STRUCTURAL ENGINEERS

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Structural Design Calculations

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EXPIRES 12/31/20

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Base Reactions (LRFD)

min. embement into 4" concrete slab, min f'c = 3000psi, is			
Axial (P_{ut}) =	443 lb	per SH. 6	(0.9D + E)
Shear (V_{ut}) =	187 lbs	per SH. 6	(E)
Moment (M_{ut}) =	875 lb-ft	per SH. 6	(E)
Shear w/ Ω_o ($\Omega_o V_{ut}$) =	469 lbs	per SH. 6	(Ω_oE)
Moment w/ Ω_o ($\Omega_o M_{ut}$) =	2187 lb-ft	per SH. 6	(Ω_oE)
=	26241 in-lb		

Live			
Axial (P_{ut}) =	102 lb	per SH. 7	(0.9D)
Shear (V_{ut}) =	115 lbs	per SH. 7	(1.6L)
Moment (M_{ut}) =	404 lb-ft	per SH. 7	(1.6L)

Anchor Design

(4) 1/2" Ø x Power-Stud+ SD1 wedge expansion anchors w/ 2.5" embement into 4" concrete slab, min f'c = 3000psi, is adequate.

*See Sheets 10 to 12 for design checks.

Anchor Design (cont.)

DEWALT ENGINEERED BY POWERS — DEWALT Design Assist Ver. 1.4.2.0 — Page 1
Sep 03 2019

1. Project Information
Company:
Project Engineer:
Address:
Phone:
Email:
Project Name:
Project Address:
Notes:

2. Selected Anchor Information
Selected Anchor: Power-Stud+ SD1
Brand: DEWALT
Material: 1/2" Ø Medium Carbon Steel
Embedment: h_{ef} 2 in h_{nom} 2.5 in
Approval: ICC-ES ESR-2818
Issued | Revision: Dec,2018 -
Drill Method: Hammer Drilled

3. Design Principles
Design Method: ACI 318-11
Load Combinations: Section 9.2 User Defined Loads
Seismic Loading: Tension D.3.3.4.3(d) Shear D.3.3.5.3(c) Ω_o= User Defined

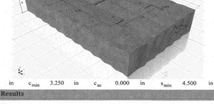
4. Base Material Information
Concrete: Type Cracked Normal Weight Concrete; Strength 3000 psi
Reinforcement: Edge Reinforcement None or < #4 Rebar; Spacing Tension No (Condition B) Shear No (Condition B); Controls Breakout Tension True Shear True
Base Plate: Sizing Thickness 0.1875 in Length 9 in Width 24.75 in; Standoff None Height 0 in; Strength 36000 psi
Profile: None

Input data and results must be checked for agreement with the existing conditions, the standards and guidelines and must be checked for plausibility.

Anchor Design (cont.)

DEWALT ENGINEERED BY POWERS — DEWALT Design Assist Ver. 1.4.2.0 — Page 2
2019_09_03 Anchorage on Idealized Base PL 1901-02XX
Sep 03 2019

5. Geometric Conditions



h_{min} 4.000 in c_{min} 3.250 in c_{ac} 0.000 in s_{min} 4.500 in

6. Summary Results

Tension Loading

Design Proof	Demand (lb)	Capacity (lb)	Utilization	Status	Critical
Steel Strength	968.00	6810.00	0.142	OK	
Concrete Breakout Strength	1936.00	2568.00	0.754	OK	Controls

Shear Loading

Design Proof	Demand (lb)	Capacity (lb)	Utilization	Status	Critical
Steel Strength	117.00	2574.00	0.045	OK	
Concrete Breakout Strength				OK	
Pryout Strength	469.00	7374.00	0.064	OK	Controls

7. Warnings and Remarks

ANCHOR DESIGN CRITERIA IS SATISFIED

- The results of the calculations carried out by means of the DDA Software are based essentially on the data you put in. Therefore, you bear the sole responsibility for the absence of errors in the data and for their relevance for the structure to be built in by you. Moreover, you bear sole responsibility for having the results of the calculation checked and cleared by an design professional/engineer, particularly with regard to compliance with applicable standards, norms and permits, prior to using them for your specific project. The DDA Software serves only as an aid to interpret standards, norms and permits without any guarantee as to the absence of errors, the correctness and the relevance of the results or suitability for a specific application.
- Calculations including seismic design requirements in accordance with ACI 318 are required for anchors in structures assigned to seismic design categories C, D, E and F.
- Under those seismic conditions, the direction of shear may not be predictable. In accordance with ACI 318 the full shear force should be assumed also in reverse direction for a safe design. Load reversal may influence the direction of the controlling concrete breakout strength.

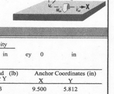
Input data and results must be checked for agreement with the existing conditions, the standards and guidelines and must be checked for plausibility

Anchor Design (cont.)

DEWALT ENGINEERED BY POWERS — DEWALT Design Assist Ver. 1.4.2.0 — Page 3
2019_09_03 Anchorage on Idealized Base PL 1901-02XX
Sep 03 2019

8. Load Conditions

Design Loads / Actions
Nu -443 lb Vux 0 lb Vuy -469 lb
Mux 0 in-lb Muy 0 in-lb Mut 26241 in-lb
Consider Load Reversal X Direction 100% Y Direction 100%



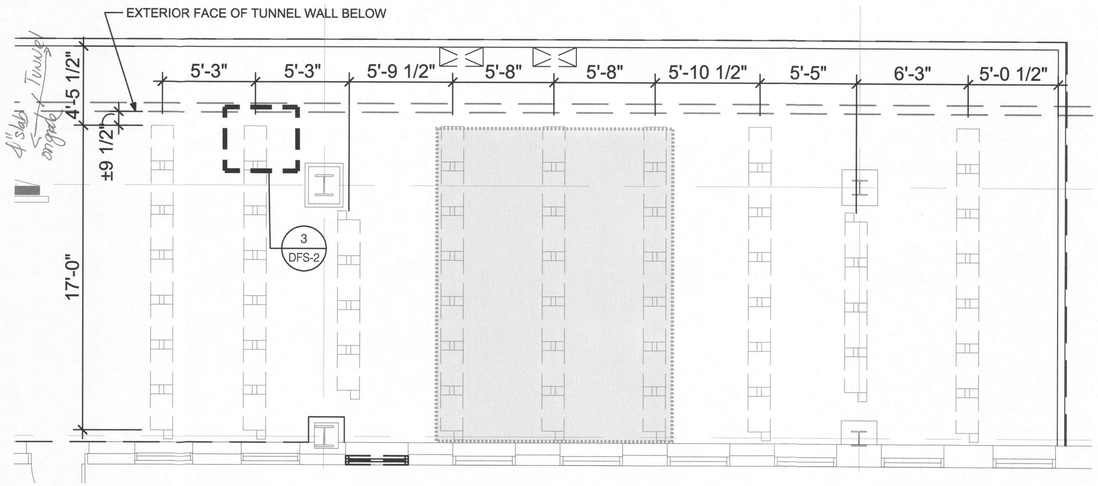
9. Load Distribution

Max. concrete compressive strain: 0.072 %
Max. concrete compressive stress: 313.493 psi
Anchor Eccentricity ex 0 in ey 0 in
Resulting anchor forces / Load distribution

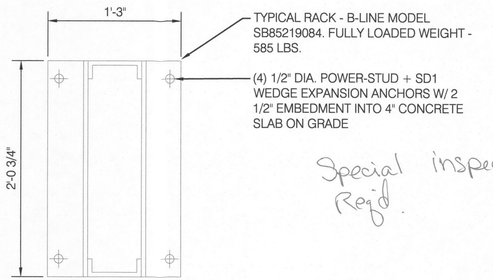
Anchor	Tension Load (lb)	Shear Load (lb)	Component Shear Load (lb) Shear X	Shear Y	Anchor Coordinates (in) X	Y
1	967.97	117.3	0.0	-117.3	9.500	5.812
2	0.00	117.3	0.0	-117.3	9.500	-5.812
3	967.97	117.3	0.0	-117.3	-9.500	5.812
4	0.00	117.3	0.0	-117.3	-9.500	-5.812

Input data and results must be checked for agreement with the existing conditions, the standards and guidelines and must be checked for plausibility

1 STRUCTURAL CALCULATION PAGES, CONT.
SCALE: NONE



2 RACK LAYOUT PLAN
SCALE: 1/4" = 1'-0"



3 RACK ATTACHMENT PLAN, TYPICAL
SCALE: 1 1/2" = 1'-0"

Do not Anchor racks over tunnel.

Special inspection Reqd.

City of Portland REVIEWED FOR CODE COMPLIANCE JAN 29 2020 Permit Number

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DEFERRED SUBMITTAL

RACK INSTALLATION DETAILS

PROJ. NO.
17096
11.20.2019

DFS-2

B2 19-106363 DFS 01 FA 2

City of Portland, Oregon - Bureau of Development Services

1900 SW Fourth Avenue · Portland, Oregon 97201 | 503-823-7300 | www.portlandoregon.gov/bds

Deferred Submittal Requirements and Application

City of Portland
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JAN 29 2020
19-106363 DFS 01 FA
Permit Number

Applicants will provide:

- ☑ A copy of this application
- ☑ Three (3) sets of plans
- ☑ Two (2) set of calculations
- ☑ Two (2) sets of product information
- ☑ Drawings and calculations must be stamped and signed by an Engineer registered in Oregon and approved by the Architect/Engineer of record for the building.
- ☐ Permit fee (paid at time of submittal)
- ☑ If the DFS includes exterior elements, plan views and elevations identifying the location(s) as approved by the Architect and Engineer of Record must be submitted.
- ☐ One (1) copy of your main building permit approved plans (NOTE: Approved plans do not need to be submitted if your project has a development liaison assigned.)

Contractor submittal information:

JEFFR 2
TODD B 3
CHRIS 1
9/19

Contact name Michael Smith

Address 8309 SW Cirrus Dr

City Beaverton State OR Zip Code 97008

Phone 541-520-4342 E-mail msmith@cts1.com

Value of deferred submittal 1000.00 Issued main building permit # 19-106364FA

Job Site Address 7000 NE Airport Wy Portland OR 97218

Description/Scope of work Equipment Racks PDX Terminal DAS Head End Room

19-106363 DFS 01 FA

Fees

Deferred submittal (DFS) fees are collected in addition to the standard building review fee paid on the main building permit. DFS fees cover the cost of the additional processing and review time associated with the design build element.

The DFS fee for processing and reviewing deferred plan submittals is 10 percent of the building permit fee calculated using the value of the particular deferred portion of the project.,

Minimum fee: Residential, one and two family dwelling ...$195 for DFS with valuation of less than or equal to $222,000

Commercial and all other projects .....$510 for DFS with valuation of less than or equal to $680,000

The Bureau of Development Services (BDS) fee schedule is also available on the BDS web site at ***www.portlandoregon.gov/bds*** | select the Fees tab.

Helpful Information

Bureau of Development Services
1900 SW 4th Avenue, Portland, OR 97201

Submit your plans to:
Development Services Center (DSC), First Floor,
For Hours Call 503-823-7310 | Select option 1
or visit www.portlandoregon.gov/bds

Important Telephone Numbers

BDS main number	503-823-7300
DSC automated information line	503-823-7310
Building code information	503-823-1456
BDS 24 hour inspection request line	503-823-7000
Residential information for one and two family dwellings	503-823-7388
City of Portland TTY	503-823-6868