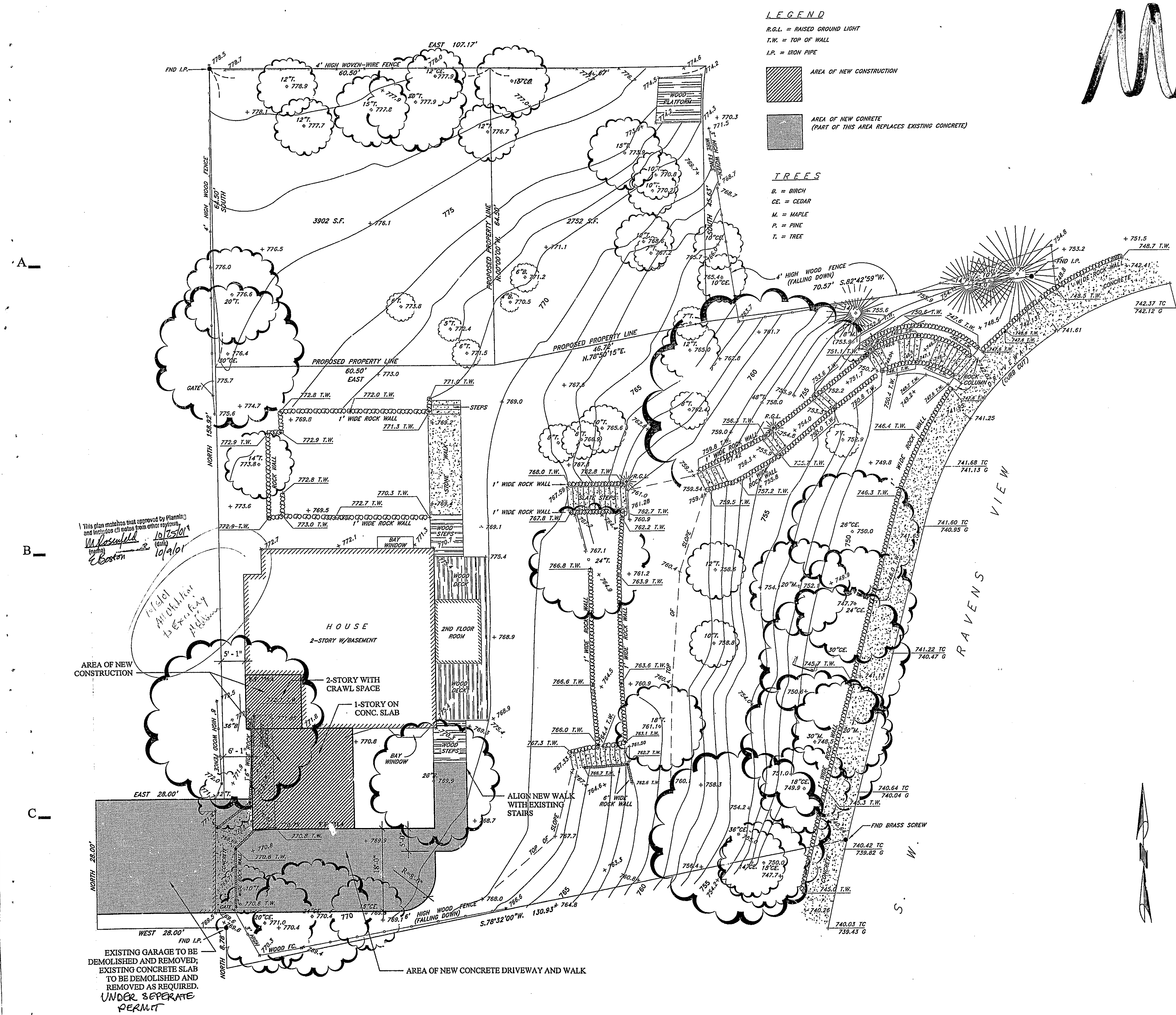

C1 SITE PLAN
1" = 10'-0"

PROJECT LOCATION:
2611 SW RAVENSVIEW DRIVE
PORTLAND, OREGON 97201

PROJECT DESCRIPTION:
ADD 12' X 12' TWO STORY ADDITION TO REAR CORNER OF HOUSE TO INCLUDE FAMILY ROOM EXTENSION AT MAIN FLOOR AND MASTER BATH AT UPPER FLOOR. ADD 22' X 22' SINGLE STORY ATTACHED GARAGE AT SAME CORNER AS TWO STORY ADDITION. REMOVE ONE WALL AT MAIN FLOOR KITCHEN, INCREASE KITCHEN WINDOW SIZE, IMPROVE TWO BEAMS AT EXISTING KITCHEN OPENINGS.

CONTRACTOR:
OWNER/CONTRACTOR
PATRICK DOLAN
2611 SW RAVENSVIEW DRIVE
PORTLAND, OREGON 97201
PHONE: 503-969-8000

DESIGN:
ESLINGER JONES & ASSOCIATES
1414 WASHINGTON STREET
OREGON CITY, OREGON 97045
PHONE: 503-655-1222
FAX: 503-655-4700
CONTACT: TERRY AMUNDSON

STRUCTURAL ENGINEER:
LEE ENGINEERING, INC.
1300 JOHN ADAMS STREET
OREGON CITY, OREGON 97045
PHONE: 503-655-1342
FAX: 503-655-1360
CONTACT: SHANE EARP

OWNER:
PATRICK AND ANNE DOLAN
2611 SW RAVENSVIEW DRIVE
PORTLAND, OREGON 97201
PHONE: 503-969-8000

GENERAL NOTES:

1. *PRESCRIPTIVE PATH 1* INSULATION STANDARDS:

FLAT CEILING AREAS:	R-38
VAULTED CEILING AREAS:	R-30
WALLS:	R-21
BASEMENT WALLS:	R-21
UNDER FLOOR:	R-25
SLAB ON GRADE:	R-15 (PERIMETER)
EXTERIOR DOORS:	
ENTRY:	U=0.54
OTHER:	U=0.20
1-LITE GLASS:	U=0.40
WINDOWS:	U=0.40

2. CONTRACTOR(S) VERIFY ALL NOTES, EXISTING DIMENSIONS, AND CONDITIONS OF PLANS RELATIVE TO SITE CONDITIONS PRIOR TO COMMENCEMENT OF WORK.
3. WRITTEN DIMENSIONS SHALL TAKE PRECEDENCE OVER SCALED DIMENSIONS. DO NOT SCALE INFORMATION FROM DRAWINGS.
4. ALL EXPOSED INSULATION TO HAVE A FLAME SPREAD RATING LESS THAN 25 AND A SMOKE DENSITY RATING LESS THAN 450.
5. PROVIDE NEW SMOKE DETECTOR AT EACH FLOOR OF ADDITION. INTERCONNECTED AND TIED INTO ELECTRICAL SYSTEM.
6. CONTRACTOR(S) SHALL BE SOLELY RESPONSIBLE FOR COMPLYING WITH ALL OSHA AND SAFETY REQUIREMENTS.

INSTALL AT LEAST ONE SMOKE DETECTOR IN EACH SLEEPING ROOM, IN AREAS IMMEDIATELY OUTSIDE OF SLEEPING ROOMS AND AT LEAST ONE ON EACH LEVEL

GARAGE ROOF TRUSSES TO BE DEFERRED SUBMITTAL

City of Portland
APPROVED
OCT 25 2001
01-159450-RS
Permit Number

DRAWING INDEX:

A1.1 SITE PLAN / GENERAL INFORMATION
A2.1 MAIN FLOOR PLAN / CROSS SECTION
A2.2 UPPER FLOOR PLAN / CROSS SECTION
A4.1 EXTERIOR ELEVATIONS
S1 STRUCTURAL NOTES / FOUNDATION PLAN
S2 STRUCTURAL PLANS / DETAILS

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eslinger jones & associates
503.655.1222
www.eslingerjones.com

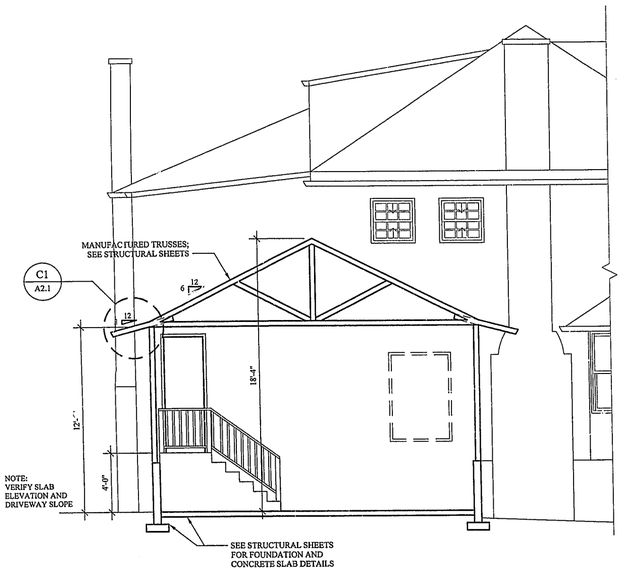
Patrick and Anne Dolan
Portland, Oregon
SITE PLAN / GENERAL INFORMATION

REV. DATE: 10.25.01
REV. DATE:

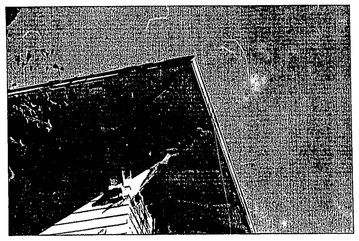
DATE: 09.12.01
CHECK: TLA
DRAWN BY: TLA
PROJECT #: Dolan

SHEET
A1.1

01-159450-RS
MICRO
3
C. 3
BIN 50



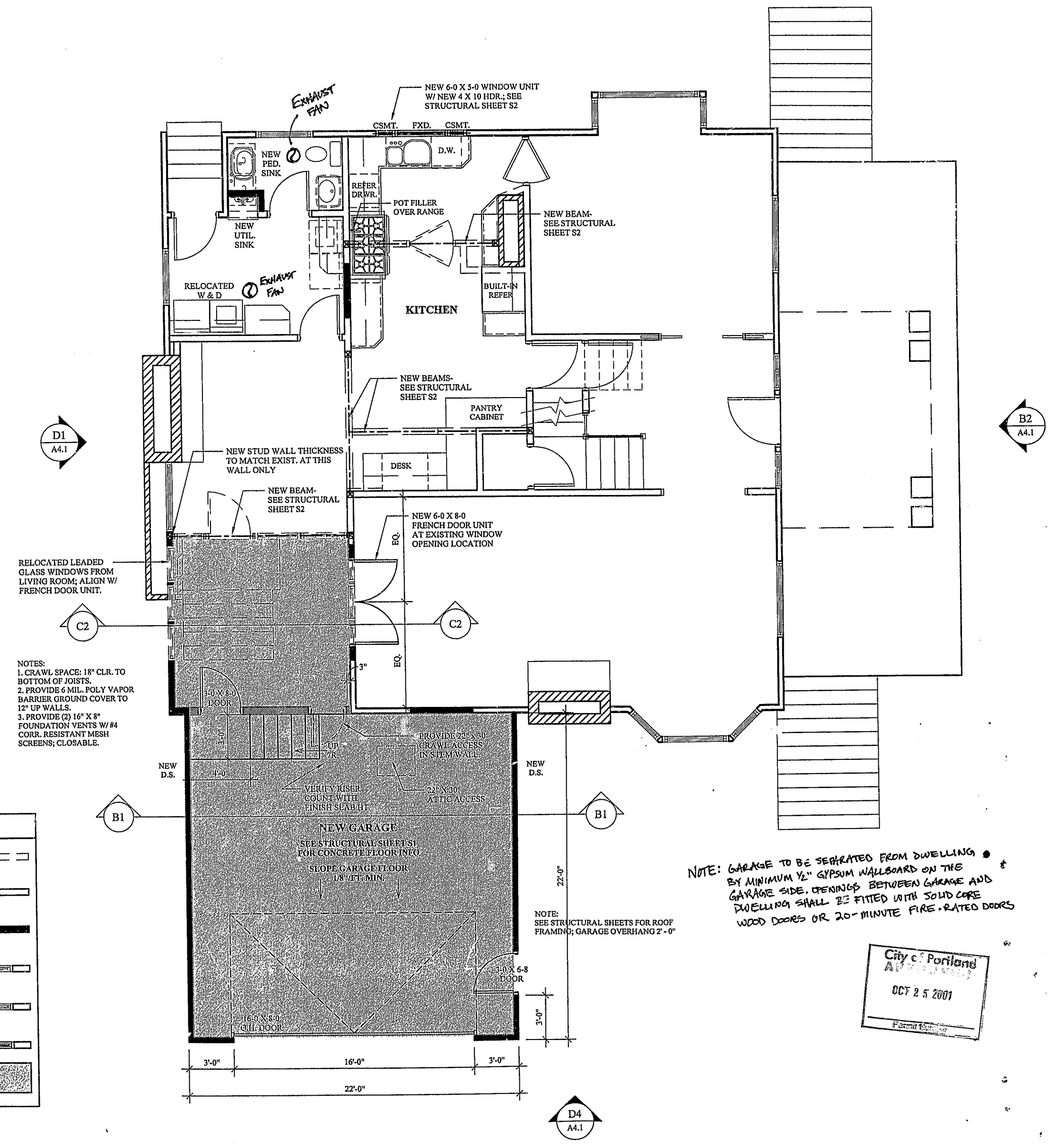
B1 CROSS SECTION
1/4" = 1'-0"



C1 EAVE DETAIL (MATCH EXISTING EAVES & CORNER BRACKETS)
NO SCALE

LEGEND:

- EXISTING WALLS TO BE REMOVED:
- EXISTING WALLS TO REMAIN:
- NEW WALLS:
- EXISTING DOORS/WINDOWS TO BE REMOVED:
- EXISTING DOORS/WINDOWS TO REMAIN:
- NEW DOORS/WINDOWS:
- AREA OF NEW CONSTRUCTION:



NOTE: GARAGE TO BE SEPARATED FROM DWELLING BY MINIMUM ½" GYPSUM WALLBOARD ON THE GARAGE SIDE. OPENINGS BETWEEN GARAGE AND DWELLING SHALL BE FITTED WITH SOLID CORE WOOD DOORS OR 20-MINUTE FIRE-RATED DOORS

C4 MAIN FLOOR PLAN
1/4" = 1'-0"

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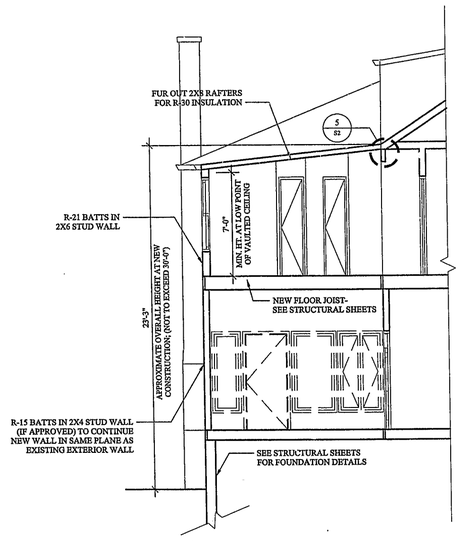
Patrick and Anne Dolan
Portland, Oregon
MAIN FLOOR PLAN / CROSS SECTION

City of Portland
Approved
OCT 25 2001
Permit Set

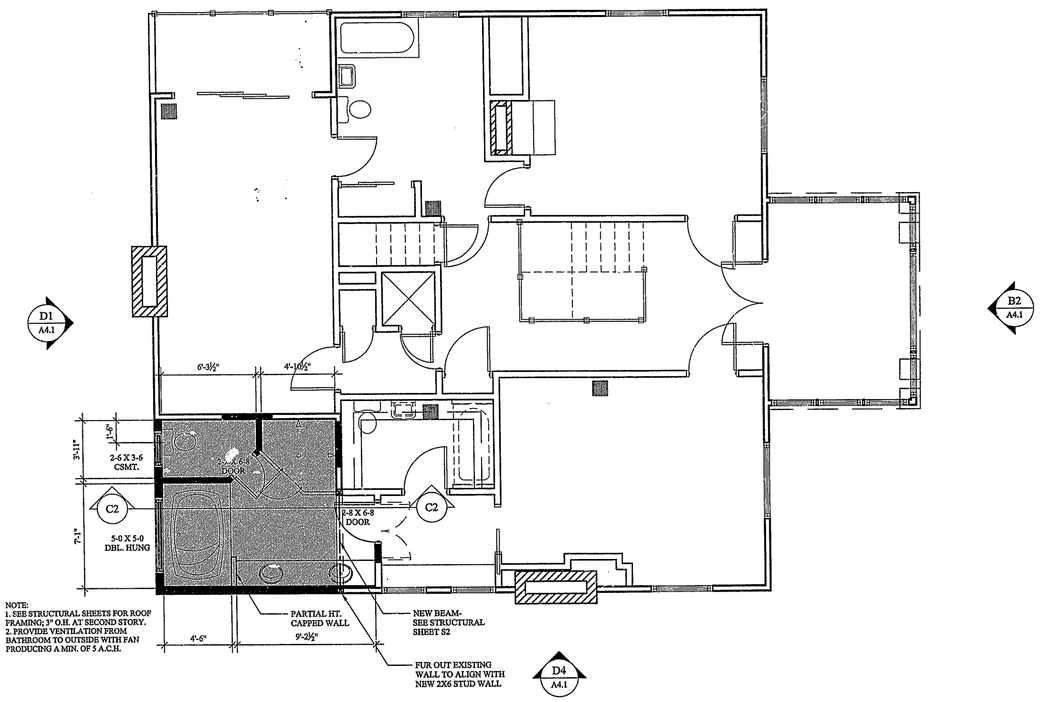
REV. DATE:	09.25.01
REV. DATE:	09.18.01

DATE:	09.12.01
CHECK:	TLA
DRAWN BY:	TLA
PROJECT #:	Dolan

SHEET:
A2.1



C2 CROSS SECTION
1/4" = 1'-0"



NOTE:
1. SEE STRUCTURAL SHEETS FOR ROOF FRAMING; 3" O.H. AT SECOND STORY.
2. PROVIDE VENTILATION FROM BATHROOM TO OUTSIDE WITH FAN PRODUCING A MIN. OF 5 A.C.H.

C4 UPPER FLOOR PLAN
1/4" = 1'-0"

City of Portland
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OCT 2 5 2001
Permit Number

LEGEND:

EXISTING WALLS TO BE REMOVED:	
EXISTING WALLS TO REMAIN:	
NEW WALLS:	
EXISTING DOORS/WINDOWS TO BE REMOVED:	
EXISTING DOORS/WINDOWS TO REMAIN:	
NEW DOORS/WINDOWS:	
AREA OF NEW CONSTRUCTION:	

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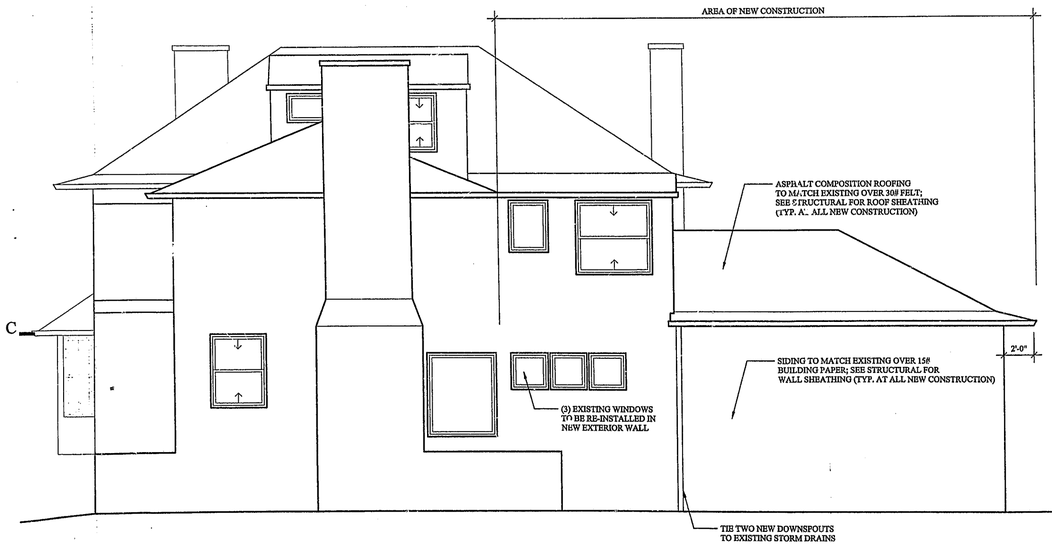
Patrick and Anne Dolan
Portland, Oregon
UPPER FLOOR PLAN / CROSS SECTION

REV. DATE:	*09.25.01*
REV. DATE:	*09.18.01*

DATE:	*09.12.01*
CHECK:	*TLA*
DRAWN BY:	*TLA*
PROJECT #:	*Dolan*

SHEET:
A2.2

B2 FRONT ELEVATION
1/4" = 1'-0"



D1 REAR ELEVATION
1/4" = 1'-0"

D4 SIDE ELEVATION
1/4" = 1'-0"

City of Portland
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OCT 2 5 2001
Permit Number

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Patrick and Anne Dolan
Portland, Oregon
EXTERIOR ELEVATIONS

REV. DATE:	*09.25.01*
REV. DATE:	*09.18.01*

DATE:	*09.12.01*
CHECK:	*TLA*
DRAWN BY:	*TLA*
PROJECT #:	*Dolan*

SHEET:
A4.1

STRUCTURAL NOTES

GENERAL:
1. THESE STRUCTURAL NOTES SUPPLEMENT THE SPECIFICATIONS. ANY DISCREPANCY FOUND AMONG THE DRAWINGS, SPECIFICATIONS, THESE NOTES, AND ANY SITE CONDITIONS SHALL BE REPORTED IN A TIMELY MANNER TO THE ENGINEER DESIGN TEAM WHO SHALL RESPOND TO ANY DISCREPANCY IN WRITING. ANY WORK DONE BY THE CONTRACTOR AFTER DISCOVERY OF SUCH DISCREPANCY SHALL BE DONE AT THE CONTRACTOR'S OWN RISK.
2. THE CONTRACTOR SHALL VERIFY AND COORDINATE THE DIMENSIONS AMONG ALL DRAWINGS PRIOR TO PROCEEDING WITH ANY WORK OR FABRICATION.
3. THE CONTRACTOR IS RESPONSIBLE FOR ALL DEMOLITION AND CONSTRUCTION METHODS, TECHNIQUES, SEQUENCING, AND SAFETY REQUIRED FOR THE WORK.
4. THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN AND CONSTRUCTION OF ALL ERECTION BRACING, FORM WORK, AND TEMPORARY SHORING REQUIRED FOR THE WORK.

DESIGN CODE:
1. CABO ONE AND TWO FAMILY DWELLING CODE 1997 EDITION, STATE OF OREGON 1998 EDITION
2. UNIFORM BUILDING CODE, 1997 EDITION, STATE OF OREGON 1998 EDITION STRUCTURAL SPECIALTY CODE.
3. ALL REFERENCE TO OTHER CODES AND STANDARDS (ACI, ASTM, ETC.) SHALL BE FOR THE LATEST OR MOST CURRENT EDITION AVAILABLE.

DESIGN LIVE LOADS:
- ROOF (SNOW LOAD).....25 PSF (PLUS DRIFT ALLOWANCE)
- FLOORS.....40 PSF
- WIND.....80 MPH (EXPOSURE B)
- SEISMIC.....ZONE 3

FOUNDATIONS:
1. MAXIMUM DESIGN SOIL BEARING PRESSURE = 1500 PSF ASSUMED.
2. FOOTING ELEVATIONS SHOWN ON DRAWINGS ARE TO BOTTOM OF FOOTING.
3. FOOTINGS SHALL BE FOUNDED ON FIRM, UNDISTURBED SOIL OR ON APPROVED STRUCTURAL FILL.
4. STRUCTURAL FILL SHALL CONSIST OF CLEAN WELL-GRADED SAND, SAND AND GRAVEL, OR CRUSHED ROCK.
5. THE STRUCTURAL FILL SHALL BE PLACED IN LOOSE LIFTS NOT EXCEEDING 8" IN THICKNESS AND THOROUGHLY COMPACTED TO A DENSE, NON-YIELDING STATE.
6. STRUCTURAL FILL SHALL BE COMPACTED TO 95% OF THE MAXIMUM DRY DENSITY AS OBTAINED BY ASTM MODIFIED PROCTOR.
7. ALL FOOTINGS SHALL BE A MINIMUM OF 1'-6" BELOW FINAL GRADES OR 1'-0" BELOW EXISTING GRADE, WHICHEVER IS LOWER.
8. BOTTOM OF FOOTINGS SHALL BE STEPPED FROM ELEVATION TO ELEVATION AT 2'-0" HORIZONTAL TO 1'-0" VERTICAL STEPS OR SHALL BE SLOPED NOT TO EXCEED 1 VERTICAL TO 10 HORIZONTAL WHEN APPROVED BY THE ENGINEER.

CONCRETE (CAST IN PLACE):
1. ALL CONCRETE SHALL BE NORMAL WEIGHT AND SHALL DEVELOP A MINIMUM 28 DAY LABORATORY CURED COMPRESSIVE-CYLINDER STRENGTH OF:
   2500 PSI FOR ALL CONCRETE
2. CONCRETE, FORMS, MIXING, PLACING AND CURING SHALL CONFORM TO ACI MANUAL OF CONCRETE PRACTICE, LATEST EDITION, AND SPECIFICATIONS.
3. CONCRETE SHALL HAVE A MAXIMUM SLUMP OF 4 INCHES.
4. CONCRETE SHALL BE PLACED IN ONE CONTINUOUS OPERATION.
5. ALL BOLTS IN CONCRETE SHALL CONFORM TO ASTM SPECIFICATION A307 AND SHALL BE OF THE SIZE INDICATED ON THE DRAWINGS.

REINFORCING STEEL:
1. REINFORCING BARS SHALL BE NEW BILLET STEEL AND SHALL CONFORM TO: ASTM A615 GRADE 60 FOR ALL REINFORCEMENT.
2. WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185.
3. REINFORCEMENT SHALL BE DETAILED, FABRICATED AND PLACED IN ACCORDANCE WITH ACI CODE 318 AND ACI MANUAL 315, UNLESS OTHERWISE NOTED. ALL REINFORCEMENT SHALL BE FREE OF LOOSE MILL AND RUST SCALE, OIL, DIRT AND COATINGS OF ANY MANNER THAT WILL REDUCE BOND. ALL REINFORCEMENT IS CONTINUOUS WITH ADEQUATE LAPS.
4. REINFORCEMENT SHALL BE SECURED IN FORMS WITH SUITABLE TIES AND ANCHORAGE TO PREVENT DISPLACEMENT. BARS ADJACENT TO EARTH SHALL BE SUPPORTED BY CEMENT MORTAR CUBES.
5. THE FOLLOWING MINIMUM CONCRETE COVER SHALL BE PROVIDED FOR REINFORCEMENT:
   - A) CONCRETE CAST AGAINST EARTH.....3"
   - B) CONCRETE EXPOSED TO EARTH OR WEATHER
     - #6 AND LARGER.....2"
     - #5 AND SMALLER.....1-1/2"
   - C) CONCRETE NOT EXPOSED TO EARTH OR WEATHER
     - SLABS, WALLS AND JOISTS, #11 AND SMALLER.....3/4"
     - BEAMS AND COLUMNS.....1-1/2"
6. PLACE 2'-0" x 2'-0" BARS AT CORNERS AND INTERSECTIONS FOR WALLS AND FOUNDATIONS EQUAL IN SIZE AND SPACING TO HORIZONTAL REINFORCING.
7. REINFORCEMENT SPLICES, SHALL BE 44 DIA. (24" MIN.) LAPS.
8. UNLESS OTHERWISE NOTED, DOWEL CONCRETE WALLS TO FOOTING WITH BARS OF SAME SIZE & SPACING AS WALL REINFORCEMENT.

TRUSS JOIST MacMILLAN:
1. ROOF AND FLOOR FRAMING DESIGNATED "LVL", SHALL BE THE TYPE AND SIZE INDICATED ON DRAWINGS, AS MANUFACTURED BY TRUS JOIST MacMILLAN CORP. WESTERN DIVISION, PORTLAND, OREGON OR APPROVED EQUAL.

METAL PLATE CONNECTED TRUSSES:
1. METAL PLATE CONNECTED TRUSSES SHALL BE DESIGNED AND MANUFACTURED IN ACCORDANCE WITH "DESIGN SPECIFICATIONS FOR LIGHT METAL PLATE CONNECTED WOOD TRUSSES", TPI-94 AS PUBLISHED BY THE TRUSS PLATE INSTITUTE AND SHALL BE DESIGNED FOR THE FOLLOWING MINIMUM LOADS:
   - TOP CHORD.....25 PSF LIVE LOAD
   - .....10 PSF DEAD LOAD
   - BOTTOM CHORD.....10 PSF
   - TOTAL LOAD.....45 PSF + SELF WEIGHT
   - NET UPLIFT (WIND).....15 PSF
2. THE TRUSS MANUFACTURER SHALL SUBMIT DESIGNS, STRESS DIAGRAMS, SHOP DRAWINGS AND CALCULATIONS BEARING THE STAMP OF A (STRUCTURAL) ENGINEER REGISTERED IN THE STATE OF OREGON TO THE ENGINEER FOR REVIEW.
3. THE TRUSS MANUFACTURER SHALL SUPPLY ALL HARDWARE, ANCHORAGE, AND METAL SEATS REQUIRED, AND SHALL DESIGN AND INDICATE BRACING REQUIRED TO BE SUPPLIED BY THE GENERAL CONTRACTOR.
4. LOWER CHORDS SHALL BE CAMBERED TO PROVIDE FOR DEAD LOAD DEFLECTION AT GYPSUM BOARD CEILING.
5. ALL CONNECTION PLATES SHALL DEVELOP THE FULL STRESS IN MEMBER WITH A MINIMUM TRANSFER AT ANY MEMBER OF 2000 LBS. AND MINIMUM SIZE OF 1-PLATES OF 3" x 5".
6. TRUSS SUPPLIER SHALL INSPECT ALL THE TRUSS, BRIDGING, HANGERS, BLOCKING WEB STIFFENERS AND ALL OTHER CONCENTRATED LOADS PRIOR TO TRUSS FABRICATION.

GLUE LAMINATED MEMBERS:
1. ALL GLUE LAMINATED TIMBER BEAMS SHALL BE DOUGLAS FIR 24F-V4 AT ALL SIMPLE BEAMS AND GRADE 24F-V8 AT ALL CANTILEVER AND CONTINUOUS BEAMS, UNLESS OTHERWISE SPECIFIED ON THE DRAWINGS.
2. ALL G.L. TIMBER SHALL BE FABRICATED IN ACCORDANCE WITH AITC 117-87 MANUFACTURING, USING D.F. LUMBER AND WATERPROOF ADHESIVE. EACH MEMBER SHALL BEAR AN AITC IDENTIFICATION MARK AND BE ACCOMPANIED BY AN AITC CERTIFICATE OF CONFORMANCE.
3. G.L. TIMBER BEAMS SHALL HAVE CAMBER AS INDICATED ON THE DRAWINGS.
4. ALL G.L. MEMBERS SHALL BE NOTCHED, SHAPED AND FINISHED AS PER PLANS AND SPECIFICATIONS.
5. SEE SPECIFICATIONS FOR FINISH AND PROTECTION.

SAWN FRAMING LUMBER:
1. ALL SAWN LUMBER SHALL BE DOUGLAS FIR, S4S, GRADED IN ACCORDANCE WITH WCLIB RULES #16, OF THE FOLLOWING GRADES:
   - STUDS, JOISTS, PLATES, HEADERS, BLOCKING.....NO. 2
   - 4x BEAMS.....NO. 2
   - 6x BEAMS, POSTS.....NO. 1
2. ALL 2x LUMBER SHALL BE S-DRY, U.N.O.
3. DOUBLE ALL JOISTS UNDER ALL PARALLEL PARTITIONS.

PLYWOOD SHEATHING:
1. ALL SHEATHING SHALL BE PLYWOOD, APA GRADE-TRADEMARKED C-D WITH EXTERIOR GLUE AND PANEL IDENTIFICATION INDEX. PANEL THICKNESS AND IDENTIFICATION INDEX SHALL BE AS FOLLOWS:
   - A) ROOF SHEATHING.....1/2" INDEX 32/16
   - B) FLOOR.....3/4" INDEX 48/24
   - C) WALLS.....1/2" INDEX 32/16
2. FLOOR AND ROOF PLYWOOD SHALL BE INSTALLED WITH FACE GRAIN PERPENDICULAR TO SUPPORTS AND END JOINTS SHALL BE STAGGERED.
3. PROVIDE 1 PLY CLIP PER SPAN @ UNSUPPORTED EDGE OF PLYWOOD ROOF SHEATHING WHERE JOIST SPACING EXCEEDS 16" O.C. U.N.O. AS BLOCKED EDGES.
4. BLOCK ALL WALL SHEATHING WITH 2 x 4 BLOCKING AT ALL EDGES.

NAILING AND FASTENERS:
1. NAILING INDICATED ON PLANS AND DETAILS ARE "COMMON" NAILS. MINIMUM FRAMING NAILING SHALL CONFORM TO UBC TABLE 23-II-B-1. SEE DETAILS FOR ADDITIONAL TYPICAL NAILING REQUIREMENTS. SUBSTITUTION OF NAILS OTHER THAN "COMMON" IS NOT PERMITTED WITHOUT PRIOR APPROVAL.
2. POWER DRIVEN NAILS OTHER THAN "COMMON" NAILS MAY BE USED IF DATA IS SUBMITTED AND APPROVED PRIOR TO USE.
3. PLYWOOD NAILING SHALL BE AS FOLLOWS:
   - A) ROOF SHEATHING:.....8d AT 6" O.C. AT ALL PANEL EDGES; 8d AT 12" O.C. AT ALL INTERMEDIATE SUPPORTS
   - B) FLOOR SHEATHING:.....8d AT 6" O.C. AT ALL PANEL EDGES; 8d AT 10" O.C. AT ALL INTERMEDIATE SUPPORTS
   - C) WALL SHEATHING:.....BLOCK ALL EDGES WITH 2 x 4 FLATS. NAILING AS INDICATED ON DRAWINGS.
4. ALL BOLTED CONNECTIONS SHALL BE MADE WITH MACHINE BOLTS (MBJ) CONFORMING TO ASTM A307. ALL BOLTS AND LAGS SHALL BE INSTALLED WITH STANDARD WROUGHT WASHERS, UNLESS NOTED OTHERWISE.
5. JOIST HANGERS, HOLD DOWNS, AND OTHER FRAMING ACCESSORIES ARE REFERRED TO ON PLANS BY PARTICULAR TYPE AS MANUFACTURED BY SIMPSON COMPANY - SAN LEANDRO, CA. ALL HARDWARE IS TO BE FASTENED PER MANUFACTURER'S SPECIFICATIONS, U.N.O.
6. EPOXY ANCHOR BOLTS INDICATED ON DRAWINGS SHALL BE AS MANUFACTURED BY HILTI CO. OR APPROVED EQUAL. DEPTH OF EMBEDMENT SHALL BE AS CALLED FOR ON THE DRAWINGS. INSTALL AS RECOMMENDED BY THE MANUFACTURER WITH C-20 GROUT.
7. SILLS AT WALLS SHALL BE BOLTED TO CONCRETE WITH 1/2" DIAMETER x 10" LONG ANCHOR BOLTS OR 1/2" DIAMETER EPOXY ANCHOR BOLTS AT 4'-0" O.C. MAXIMUM AND WITHIN 1'-0" OF SILL PLATE ENDS, CORNERS OR SPLICES, UNLESS DETAILED OTHERWISE.

SHEAR WALL SCHEDULE

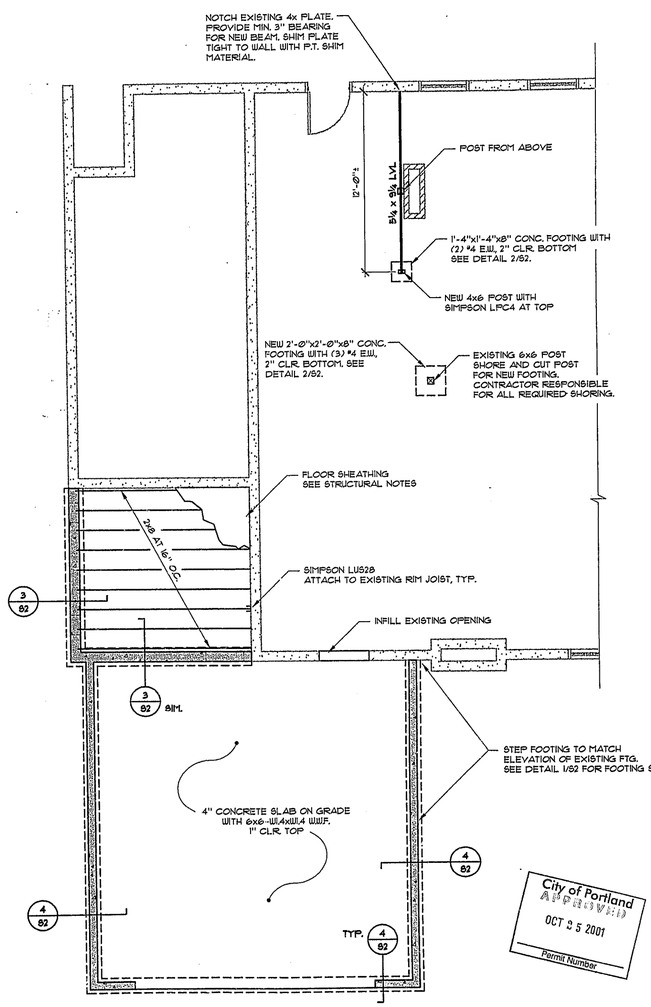
	WALL INFORMATION	P.T. SILL PLATE ANCHOR BOLTS	RIM JOIST CONDITION	COMMENTS
A	1/2" APA RATED SHT'G ONE SIDE W/ 8d NAILS @ 6" O.C. EDGES & 12" O.C. FIELD	1/2" x 10" A.B.'s @ 48" O.C. (EMBED 7")	SIMPSON A35F @ 16" O.C. RIM JOIST TO PLATE BELOW	
B	1/2" APA RATED SHT'G ONE SIDE W/ 8d NAILS @ 4" O.C. EDGES & 12" O.C. FIELD	1/2" x 10" A.B.'s @ 32" O.C. (EMBED 7")	SIMPSON A35F @ 16" O.C. RIM JOIST TO PLATE BELOW	

NOTES:
1. IF A.B. SPACING IS GREATER THAN SHEAR WALL LENGTH INSTALL (1) A.B. WITHIN 12" OF EACH END AS PER UBC 1806.6 U.N.O.

HOLDOWN SCHEDULE

	HOLDOWN	ATTACHMENT	COMMENTS
@	NON REQUIRED	NA	NA
1	SIMPSON LSTHD8 OR SIMPSON HTT16	EMBED IN CONCRETE MIN. 11/2" FROM END (CORNER) NAIL STRAP TO DBL. STUD W/ (24) 16d SINKER NAILS. OR 5/8" x 15" "J" ANCHOR BOLT TO FDN. & NAIL STRAP TO DBL. STUDS (18)-16d SINKER NAILS	PROVIDE #4 DOWELS W/ STANDARD HOOK @ EACH A.B. LOCATION
2	SIMPSON MST48	(23)-16d NAILS @ TOP INTO DBL. STUDS & (23)-16d NAILS @ BOTTOM INTO DBL. STUDS BELOW. (CENTER STRAP ON FLOOR)	WRAP & NAIL STRAP TO BEAM/HEADER BELOW IF APPLICABLE

NOTES:
1. DBL. STUDS SHALL BE LAMINATED TOGETHER W/ 16d NAILS @ 4" O.C. FULL HEIGHT (TYPICAL).
2. PROVIDE HOLDOWN MINIMUM OF 6" FROM EACH END OF EACH SHEAR WALL.



A/S1 FOUNDATION PLAN — 1/4" = 1'-0"

City of Portland APPROVED OCT 2 5 2001 Permit Number

FOUNDATION PLAN NOTES

1. REFER TO ARCHITECTURAL DRAWINGS FOR ALL DIMENSIONS.
2. REFER TO ARCHITECTURAL DRAWINGS FOR ALL FLOOR ELEVATIONS, SLOPES, AND RECESSES.
3. REFER TO GENERAL STRUCTURAL NOTES ON THIS SHEET.
4. PROVIDE 2" SAND OVER VAPOR BARRIER OVER 6" (MINIMUM) OF CRUSHED ROCK BASE BENEATH ALL SLABS ON GRADE - TYPICAL.
5. DETAILS ON THESE PLANS ARE INTENDED TO DEPICT THE GENERAL CONSTRUCTION METHODS FOR THIS STRUCTURE. CONNECTIONS, DETAILS, AND CONDITIONS NOT SPECIFICALLY SHOWN THAT ARE SIMILAR TO THOSE THAT ARE SPECIFIED SHALL BE ASSUMED ONE AND THE SAME. IF QUESTIONS REGARDING THE APPLICATION OF DETAILS ARE ENCOUNTERED, NOTIFY THE ENGINEER FOR CLARIFICATION, IN A TIMELY MANNER, PRIOR TO CONSTRUCTION.

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structural engineering
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eslinger jones & associates
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REGISTERED PROFESSIONAL ENGINEER 12,320 OREGON JULY 15, 1983 STEVEN J. ENTENMAN
EXPIRES: 12/31/01

LEI LEE ENGINEERING, INC. CONSULTING ENGINEERS
1300 John Adams Street • Oregon City, OR 97045
(503) 655-1341 fax (503) 655-1990

Patrick and Anne Dolan
Portland, Oregon
STRUCTURAL

REV. 09.25.01

DATE:	09.12.01
CHECK:	DJS
DRAWN BY:	MJG
PROJECT #:	2824

SHEET: S1

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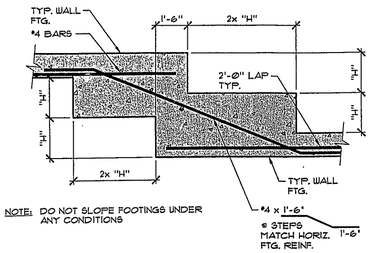
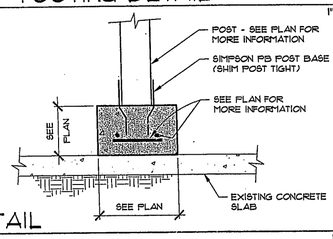


NOTE: DO NOT SLOPE FOOTINGS UNDER ANY CONDITIONS

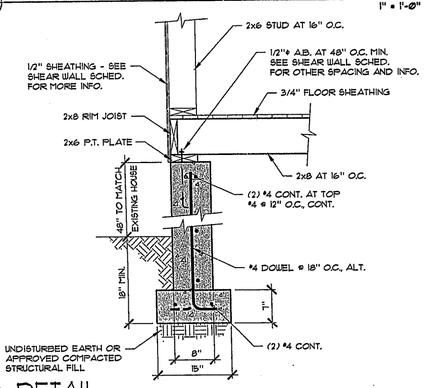
1/82 STEP FOOTING DETAIL

1" = 1'-Ø"



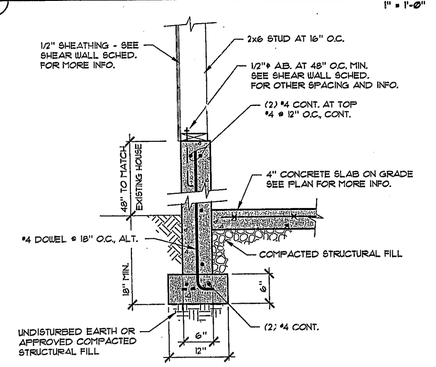
2/82 DETAIL

1" = 1'-Ø"



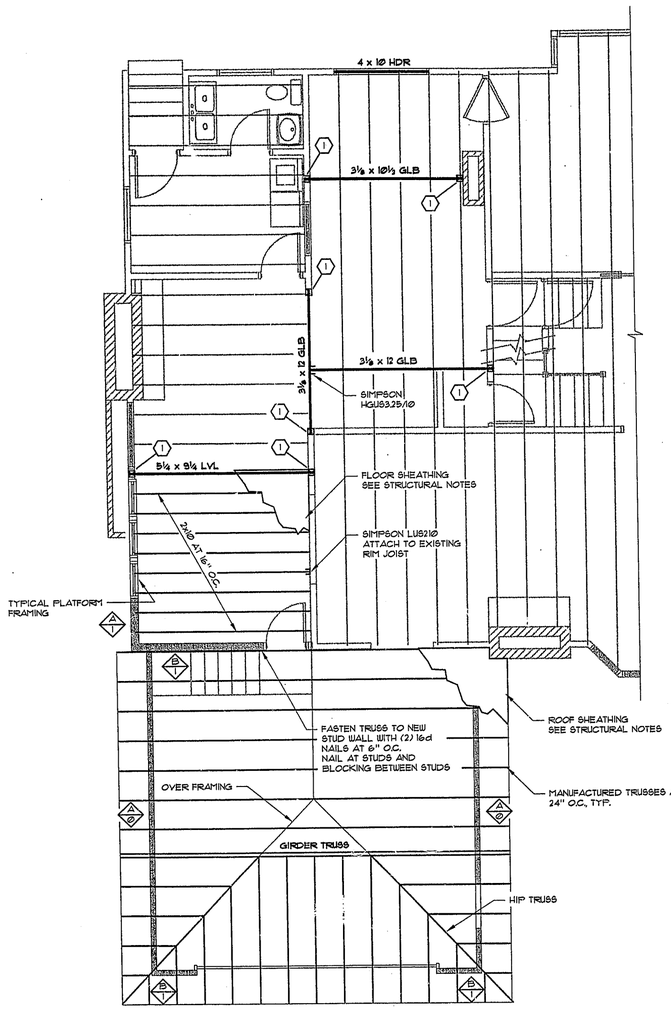
3/82 DETAIL

1" = 1'-Ø"



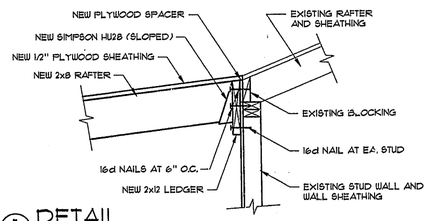
4/82 DETAIL

1" = 1'-Ø"



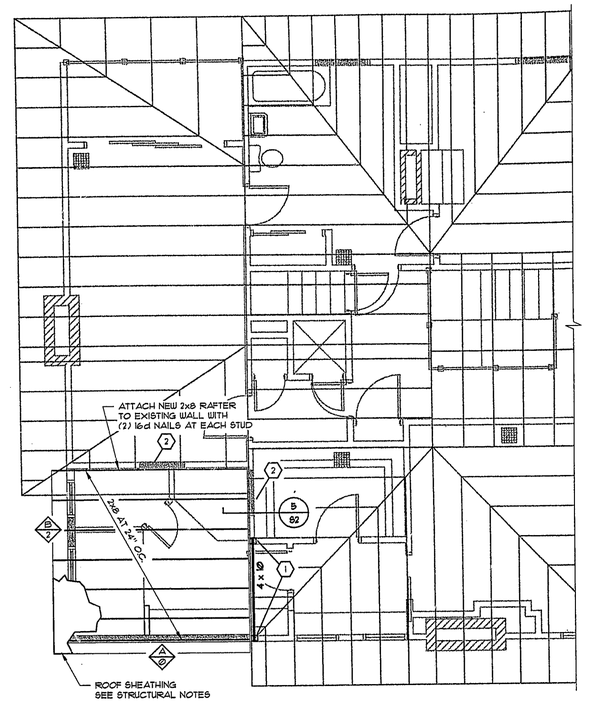
A/82 2ND FLOOR FRAMING AND GARAGE ROOF FRAMING PLAN

1/4" = 1'-Ø"



B/82 DETAIL

1" = 1'-Ø"



B/82 ROOF FRAMING PLAN

1/4" = 1'-Ø"

KEY NOTES:

1. NEW 4x4 POST WITH SIMPSON LPC4 AT TOP AND BC4Z AT BASE. PROVIDE CONTINUOUS BEARING TO FOUNDATION.
2. REMOVE ALL EXISTING SHEATHING AND REPLACE WITH 1/2" PLYWOOD SHEATHING FULL HEIGHT OF WALL. FASTEN WITH 8d NAILS AT 4" PANEL EDGES AND 12" FIELD.

FLOOR AND ROOF FRAMING PLAN NOTES

1. REFER TO ARCHITECTURAL DRAWINGS FOR ALL DIMENSIONS.
2. REFER TO ARCHITECTURAL DRAWINGS FOR ALL FLOOR ELEVATIONS, SLOPES, AND RECESSES.
3. REFER TO GENERAL STRUCTURAL NOTES ON SHEET S1.
4. ◇ INDICATES SHEAR WALL AND HOLDOWN. SEE SCHEDULE ON SHEET S1.
5. CONTRACTOR RESPONSIBLE FOR ALL REQUIRED SHORING.
6. CONTRACTOR TO FIELD INSPECT ALL EXISTING LUMBER CONDITIONS AND REPORT TO ENGINEER ANY DECAY.
7. DETAILS ON THESE PLANS ARE INTENDED TO DEPICT THE GENERAL CONSTRUCTION METHODS FOR THIS STRUCTURE. CONNECTIONS, DETAILS, AND CONDITIONS NOT SPECIFICALLY SHOWN THAT ARE SIMILAR TO THOSE THAT ARE SPECIFIED SHALL BE ASSUMED ONE AND THE SAME. IF QUESTIONS REGARDING THE APPLICATION OF DETAILS ARE ENCOUNTERED, NOTIFY THE ENGINEER FOR CLARIFICATION, IN A TIMELY MANNER, PRIOR TO CONSTRUCTION.

City of Portland APPROVED OCT 25 2001 Permit Number

design-build
eslinger jones & associates
503.655.1222
www.eslingerjones.com

STRUCTURAL REGISTERED PROFESSIONAL ENGINEER 12,320 OREGON JULY 15, 1985 STEVEN J. ENTENMAN

EXPIRES: 12/31/01

LEI Lee Engineering, Inc. CONSULTING ENGINEERS
1300 S.W. Adams Street • Oregon City, OR 97045
(503) 655-1362 fax (503) 655-1580

Patrick and Anne Dolan
Portland, Oregon
STRUCTURAL

REV. 09.25.01

DATE:	09.12.01
CHECK:	DJS
DRAWN BY:	MJG
PROJECT #:	2824

SHEET: S2

2824DES1.DWG 09/25/01 13:54:32

C.3

Rs. 01,159450

C

3

CITY OF
PORTLAND, OREGON
OFFICE OF PLANNING AND DEVELOPMENT REVIEW
1900 SW 4th Ave, Suite 5000
Portland, OR 97201

MULTNOMAH COUNTY

STATUS CHECK	Residential 1 & 2 Family Permit	Application # **01-159450-000-00-RS**

Status Date: **September 21, 2001** IVR Number: **2122915**

APPLICANT	PATRICK DOLAN	Phone: (503) 969-8000
OWNER	PATRICK DOLAN	Phone: (503) 969-8000
CONTRACTOR	PATRICK DOLAN	Phone:

PROJECT INFORMATION
Street Address: **2611 SW RAVENSVIEW DR**

Description of Work: **BUILD NEW 2 STORY ADDITION AND A 2 CAR GARAGE ATTACHED TO NEW ADDITION.**

Occupancy Group	Construction Type	Sub Type	Work Proposed
R3 U1	V-N	Single Family Dwelling	Addition

This report shows those reviews which have been assigned as of September 21, 2001 at 4:24 pm. Technical reviews may trigger additional review assignments.

Review Type/Process	Mandatory	Status	Action Date	Reviewer	Phone
Completeness Check - RS	X	Closed	9/21/01	Quillin,Bill	503-823-3444
Assign plan and file location		Open		DOCUMENT SERVICES	503-823-7357
Assign Reviews - RS		Open			
Corrections Received - RS		Open		DOCUMENT SERVICES	503-823-7357
Planning and Zoning Review	X	Open		DSC PLANNING	503-823-7526
Erosion Control Plan Review	X	Approved	9/21/01	SITE DEVELOPMENT	503-823-6892
Site Development Review	X	Open		SITE DEVELOPMENT	503-823-6892
Spec. Insp. - Site Dev.		Open		SITE DEVELOPMENT	503-823-6892
Life Safety Review	X	Open	9/26	LIFE SAFETY	
Structural Review	X	Open		STRUCTURAL	503-823-7301
Residential Plumbing Review	X	Open		Ebeling,Mike	503 823-7247
Residential Subsurface Site Evaluation	X	Open		COMMERCIAL/RESIDENTIAL PLUMB	
Urban Forestry Review	X	Open		Black,Myles	503-823-4018
Send Letter of intent to expire		Open		DOCUMENT SERVICES	503-823-7357
Plans checked out to Applicant		Open			
Pre-issuance check	X	Open		PRE-ISSUANCE RS;MI;MP	503-823-7357
Payment Received		Open			

RECEIVED
SEP 21 2001
CITY OF PORTLAND

DO NOT REMOVE

Approval criteria for Zoning must be attached to plans on construction site.

1 copy to each set of plans 1 copy to DSC file	Plancheck # 01-154450 RS

Planner: NS Hawk

ONE & TWO FAMILY RESIDENTIAL PLAN REVIEW SHEET

Date: 5/22/01 Zone: R-1 Addition and Lot:

LUR History: PLA 1238-39

Substandard Lot	Y	N	ADU	Y	N
PUD/Cluster	Y	N	Detached Access. Structure	Y	N

Lot size: 25,000 Plan District:

Minimum	*Required*	*Proposed*
Front Setback	15	NC
Side Setback	5	18
* Rear Setback	5 building eave 2½' to prop. line	5
Parking Setback	60	80
Outdoor Area	—	—
Trees (preservation, planting, or fund)	✓	✓

Maximum			
Vehicle Paving	40%		accessway easement/back yard %
Building Coverage	35%		2702 11%
Height	30'		30'
Impervious Surface	—%		— %

Base Zone Design Standards

Main Entrance No change

Street Facing Façade 1520 req'd 226 provided - OK

Length of Garage Wall OK

Street Lot Line Setbacks OK

Other Req./Notes

Form W-39

BUREAU OF BUILDINGS 9-28-20

REPORT OF PLUMBING INSPECTION

Street and No. 645 Ravensview Drive Permit No. 16591

Owner M. Cartwright

Contractor Theo Roy & Wilson

Lot 20 23 Block Addition Hillsview

Stories and class of building 2 sty old frame res.

Water Closets Sinks Urinals

Bath Tubs Laundry Tubs Slop Sinks

Basins Floor Drains 1 Connected to sewer

Rain Drains 4 Connected to

Kind of Vents Kind of Drains

No. and Size of Stacks Total No. Fixtures

Other Plumbing Fixtures connect 4 rain drains to sewer-
install floor drain

Water Permits

Date of First Inspection Date of Final Inspection 9-28-20

Inspector. Hey Inspector.

Date of First Certificate Date of Final Certificate

SEE OTHER SIDE FOR REMARKS

E
III
3" Pipe
S
W
3" Pipe

FORM W-88 (11-1-43)

BUREAU OF BUILDINGS

REPORT OF PLUMBING INSPECTION

Date 12/30/46

Address 2611 S. W. Ravensview Drive Permit 184350

Lot ... Blk ... Add ...

Owner Eric Kollpratek

Contractor F. W. Skobel

Stories and class of building 1 story new play house

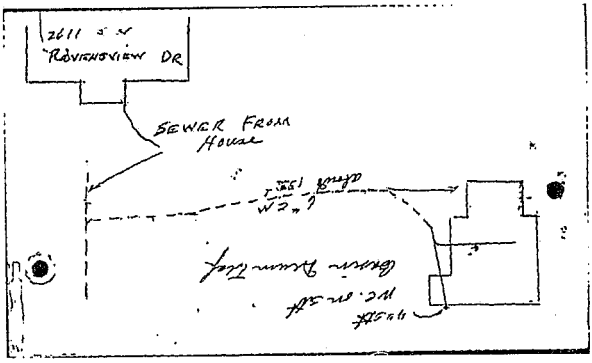
Toilets 1	Floor Drains	Beer Cab	
Bath Tubs	Rain Drains	Refr. Drains	
Bath Showers	Fountains	Urinals	
Basins	H. W. Tanks	Catch Basins	
Sinks 1	Cesspool	Water Service	
Laundry Trays	Dry Wells	Connected to Sewer	
Water Permits 177841		Sewer Permit	

Other Plumbing Fixtures Bldg. permit 287050

Date of First Inspection 12/7/46 Date of Final Inspection 12/27/46

[signature] Inspector. [signature] Inspector

Date of First Certificate ... Date of Final Certificate ...


RAVENSVIEW DR
SEWER FROM HOUSE

FORM W-58 (8-2-52)

Reinspected

BUREAU OF BUILDINGS
REPORT OF PLUMBING INSPECTION

Date 5/18/54

Address 2611 S. W. Riverview Drive Permit 41107

Lot Blk Add

Owner E. M. Kollovanitek

Contractor Service Plbg Co.

Stories and class of building

Toilets	2	Floor Drains		Beer Cab	
Bath Tubs	2	Rain Drains	1	Refr. Drains	
Bath Showers		Fountains		Urinals	
Basins	2	H. W. Tanks		Catch Basins	
Sinks	1 repl.	Cesspool		Water Service	
Laundry Trays		Dry Wells		Conn. To	
Water Permit		Bldg. Pmt.	342259	Sewer Permit	

Remarks 1 Dskw Sink repl

Date of First Inspection 5-27-54 Date of Final Inspection 10-4-54

Inspector Inspector

1ST FLOOR OLD HOUSE CHANGE SINK-D. WASH. WASTE TO NEW V.H. OF 1½ V TO 2" W.C. V. AND W. 1½ TO SINK V. TRAP IN WALL.

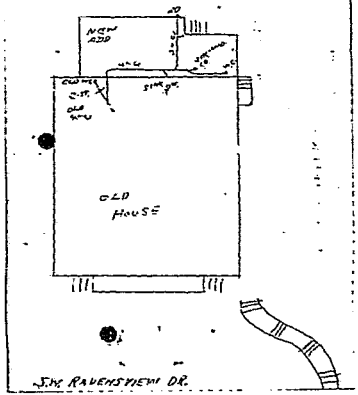
NEW ADD. 1ST FLOOR W.B. ONLY - 2" FLOOR. W.B. 1½ TO 2" J.J. 1½ TO 2" CEILING.

2ND FLOOR OLD HOUSE W.C. ON V.H. 2" TO FLOOR. TUB 1½ TO 2" W.B. 1½ TO 2".

OLD BATH ROOM W. 83' SIDE 2ND FLOOR.

REMOVED W.S. BEND AND 2" C.I. V. PLUGGED AT FLOOR AND CEILING.

W.B. PLUGGED IN WALL. SHOWER PLUGGED AT FLOOR



3227

FORM W 271

CITY OF PORTLAND, OREGON

DEPARTMENT OF PUBLIC WORKS

BUREAU OF MAINTENANCE

PERMIT NO. 5013

DATE 5-5-41

LOCATION ~~6446~~ 2611 S.W. Ravensview Dr. 4 W.S.

BETWEEN S.W. Hillcrest Dr. & S.W. Broadway Dr.

APPLICANT Port. G. & P. Co.

LOT 13 BLOCK

BOOK PAGE DATE

Structural Calculations

for

Lateral and Gravity Design
Dolan Residence Addition
Portland, Oregon

Prepared for

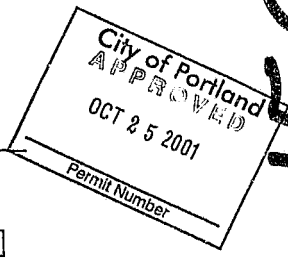
Eslinger Jones & Associates

9/20/01

LEI JOB NUMBER 2824

Limitations

Engineer was retained in limited capacity for this project. Design is based upon information provided by the client, who is solely responsible for the accuracy of same. No responsibility and/or liability is assumed by, or is to be assigned to the engineer for items beyond that shown on these sheets. 23 sheets total including this cover sheet.



This Packet of Calculations is Null and Void if Signature above is not Original

Lee Engineering, Inc.
1300 John Adams St.
Oregon City, OR 97045
(503) 655-1342 Fax (503) 655-1360

DESIGN CRITERIA

DESIGN CODE : 1997 UBC
ROOF LL : 25PSF + DRIFTING ALLOWANCE
WIND DESIGN : VELOCITY = 80 MPH, EXPOSURE B
SEISMIC ZONE : 3

SBP : 1500 PSF (ASSUMED)

ROOF DEAD LOAD

RFR := 4·PSF	FRAMING	RME := 0.5·PSF	MECH & ELECT
RPL := 2·PSF	PLYWOOD	RMS := 0.5·PSF	MISC
RRF := 5.5·PSF	ROOFING	RCG := 2.5·PSF	CEILING
		RIN := 1·PSF	INSULATION

ROOF DEAD LOAD

RDL := RFR + RPL + RRF + RME + RMS + RCG + RIN

RDL = 16.00·PSF

ROOF LIVE LOAD

RLL := 25·PSF

ROOF TOTAL LOAD

RTL := RDL + RLL RTL = 41.00·PSF

WALL WEIGHT

$WALL_{WT}$:= 15·PSF

DETERMINE WT OF STRUCTURE *LOADING DIAPHRAGMS IN TRANSVERSE DIRECTION*

RF_{DL} := RDL·(22·ft·22·ft) RF_{DL} = 7.74·K

$WALL_{DL}$:= $WALL_{WT}$·44·FT·4.75·FT $WALL_{DL}$ = 3.13·K

WT_T := RF_{DL} + $WALL_{DL}$ WT_T = 10.88·K

DETERMINE WT OF STRUCTURE LOADING DIAPHRAGMS IN LONGITUDINAL DIRECTION

RF_{DL} := RDL·(22·ft·22·ft) RF_{DL} = 7.74·K

$WALL_{DL}$:= $WALL_{WT}$·44·FT·4.75·FT $WALL_{DL}$ = 3.13·K

WT_L := RF_{DL} + $WALL_{DL}$ WT_L = 10.88·K

LEE ENGINEERING, INC.
1300 JOHN ADAMS ST.
OREGON CITY, OR 97045
(503) 655-1342 FAX (503) 655-1360

MADE BY SWE DATE: 8/30/01 SHT NO 1
CLIENT ESLINGER JONES & ASSOCIATES JOB NO 2824
PROJECT DOLAN RESIDENCE ADDITION

STATIC FORCE PROCEDURE Soil Type: S_D

SHEAR WALL BUILDING (PLYWD)

$C_v := 0.54$	1997 UBC TABLE 16-R	$R := 5.5$	1997 UBC TABLE 16-N OR 16-P
$I := 1.0$	1997 UBC TABLE 16-k	$C_t := 0.02$	1997 UBC SECTION 1630.2.2
$h_n := 18.3FT$	HEIGHT OF BUILDING	$C_a := 0.36$	1997 UBC TABLE 16-Q
		$\rho := 1.5$	

$T := C_t \cdot (h_n)^{\frac{3}{4}}$ $T = 0.18$

BASE SHEAR COEFFICIENTS

Base Shear

$$V := \frac{C_v \cdot I}{R \cdot T} \qquad V = 0.55$$

Base Shear Need Not Exceed

$$V_1 := \frac{2.5 \cdot C_a \cdot I}{R} \qquad V_1 = 0.16 \quad \textbf{Controls}$$

Base Shear Shall Not Be Less

$V_2 := 0.11 \cdot C_a \cdot I$ $V_2 = 0.04$

$$E_1 := \frac{V_1 \cdot \rho}{1.4} \qquad E_1 = 0.18$$

BASE SHEARS

TRANSVERSE DIRECTION $E_T := E_1 \cdot WT_T$ $E_T = 1907.36 \cdot lb$

LONGITUDINAL DIRECTION $E_L := E_1 \cdot WT_L$ $E_L = 1907.36 \cdot lb$

LEE ENGINEERING, INC.
1300 JOHN ADAMS ST.
OREGON CITY, OR 97045
(503) 655-1342 FAX (503) 655-1360

MADE BY SWE DATE: 9/30/01 SHT NO 2

CLIENT ESLINGER JONES & ASSOCIATES JOB NO 2824

PROJECT DOLAN RESIDENCE ADDITION

2'-6"
18'-4"
City of Portland
RECEIVED
OCT 25 2001
Permit Number

WIND ANALYSIS

Wind Speed: 80 MPH Exposure: B Method: 2 UBC

$Pw = Cq \times Ce \times Qs \times I$

$Cq := 1.3$ $Cqv := 0.7$ $Qs := 16.4 \cdot PSF$ $I := 1.0$

$Ce_{15} := 0.62$ $Ce_{20} := 0.67$ $Ce_{25} := 0.72$

WIND PRESSURE

$Pw_{15} := Cq \cdot Ce_{15} \cdot Qs \cdot I$ $Pw_{15} = 13.22 \cdot PSF$ BASIC WIND FORCE

$Pw_{20} := Cq \cdot Ce_{20} \cdot Qs \cdot I$ $Pw_{20} = 14.28 \cdot PSF$

$Pw_{25} := Cq \cdot Ce_{25} \cdot Qs \cdot I$ $Pw_{25} = 15.35 \cdot PSF$

DETERMINE WIND SAIL (LONGITUDINAL)

TRANSVERSE LENGTH $L_T := 20 \cdot FT$

$WSAIL_{15} := 8.75 \cdot FT \cdot L_T$ $WSAIL_{20} := 3.33 \cdot FT \cdot L_T$ $WSAIL_{25} := 0 \cdot FT \cdot L_T$

$WINDLD_{15} := WSAIL_{15} \cdot Pw_{15}$ $WINDLD_{20} := WSAIL_{20} \cdot Pw_{20}$ $WINDLD_{25} := WSAIL_{25} \cdot Pw_{25}$

$WFORCE_L := WINDLD_{15} + WINDLD_{20}$

$WFORCE_L = 3.59 \cdot K < E_L = 1.91 \cdot K$

DETERMINE WIND SAIL (TRANSVERSE)

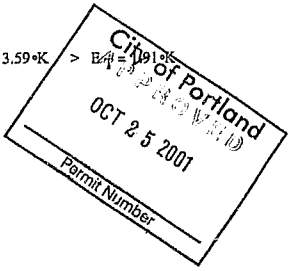
LONGITUDINAL LENGTH $L_L := 22 \cdot FT$

$WSAIL_{15} := 8.75 \cdot FT \cdot L_L$ $WSAIL_{20} := 3.33 \cdot FT \cdot L_L$ $WSAIL_{25} := 0 \cdot FT \cdot L_L$

$WINDLD_{15} := WSAIL_{15} \cdot Pw_{15}$ $WINDLD_{20} := WSAIL_{20} \cdot Pw_{20}$ $WINDLD_{25} := WSAIL_{25} \cdot Pw_{25}$

$WFORCE_T := WINDLD_{15} + WINDLD_{20}$

$WFORCE_T = 3.59 \cdot K > E_T = 1.91 \cdot K$



LEE ENGINEERING, INC.
1300 JOHN ADAMS ST.
OREGON CITY, OR 97045
(503) 655-1342 FAX (503) 655-1360

MADE BY SWE DATE: 8/30/01 SHT NO 4

CLIENT ESLINGER JONES & ASSOCIATES JOB NO 2824

PROJECT DOLAN RESIDENCE ADDITION

MASS OF EXISTING HOUSE - ROOF AND FLOOR ONLY

ASSUME FLOOR DEAD LOAD FLR := 15·PSF

$RF_{DL} := RDL \cdot (29 \cdot ft \cdot 39 \cdot ft + 12 \cdot ft \cdot 27 \cdot ft + 10 \cdot ft \cdot 14 \cdot ft)$ $RF_{DL} = 25.52 \cdot K$

$FLR_{DL} := FLR \cdot (29 \cdot ft \cdot 39 \cdot ft + 12 \cdot ft \cdot 27 \cdot ft + 10 \cdot ft \cdot 14 \cdot ft)$ $FLR_{DL} = 23.93 \cdot K$

$WT_{TOTE} := RF_{DL} + FLR_{DL}$ $WT_{TOTE} = 49145.00\ lb$

MASS OF HOUSE ADDITION - ROOF AND FLOOR ONLY

ASSUME FLOOR DEAD LOAD FLR := 15·PSF

$RF_{DL} := RDL \cdot (12 \cdot ft \cdot 12 \cdot ft)$ $RF_{DL} = 2.30 \cdot K$

$FLR_{DL} := FLR \cdot (12 \cdot ft \cdot 12 \cdot ft)$ $FLR_{DL} = 2.16 \cdot K$

$WT_{TOTA} := RF_{DL} + FLR_{DL}$ $WT_{TOTA} = 4464.00\ lb$

MASS TRIBUTARY TO EXISTING BUILDING

$TRIB_{WTA} := WT_{TOTA} \frac{1}{2}$ $TRIB_{WTA} = 2232.00\ lb$

$\frac{TRIB_{WTA}}{WT_{TOTE}} = 0.045$ LESS THAN 5%

SEISMIC LOAD AT BUILDING ADDITION

TOTAL MASS OF ADDITION

$WALL_{DL} := WALL_{WT} \cdot 24 \cdot FT \cdot 4 \cdot FT$ $WALL_{DL} = 1.44 \cdot K$

$MASS_{ADDITION} := TRIB_{WTA} + WALL_{DL}$

BASE SHEARS

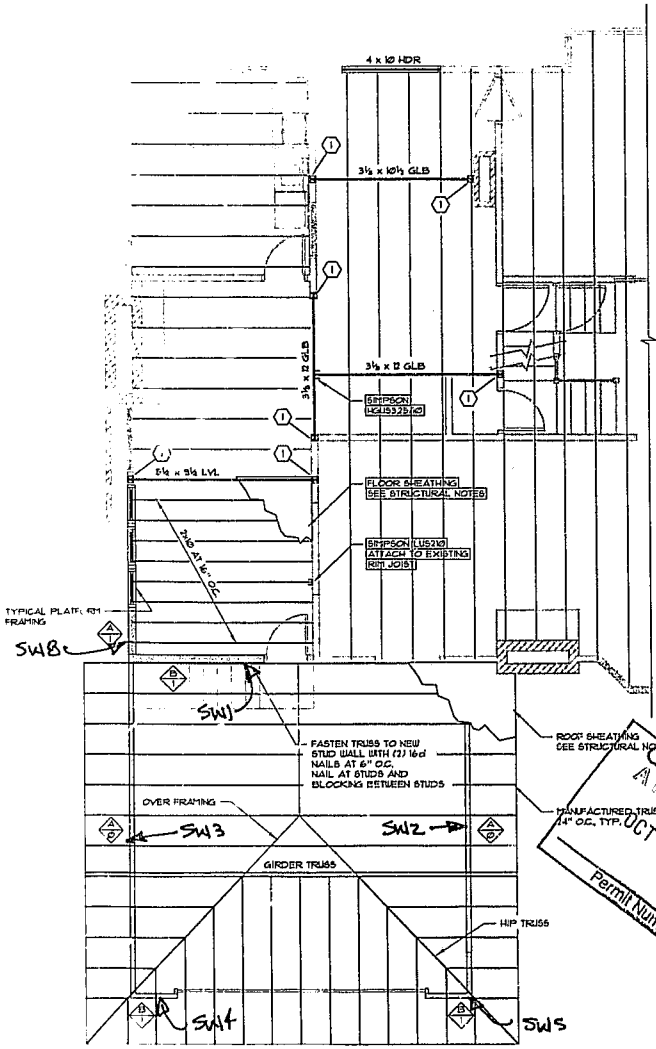
TRANSVERSE DIRECTION $E_T := E_I \cdot MASS_{ADDITION}$ $E_T = 643.79 \cdot lb$

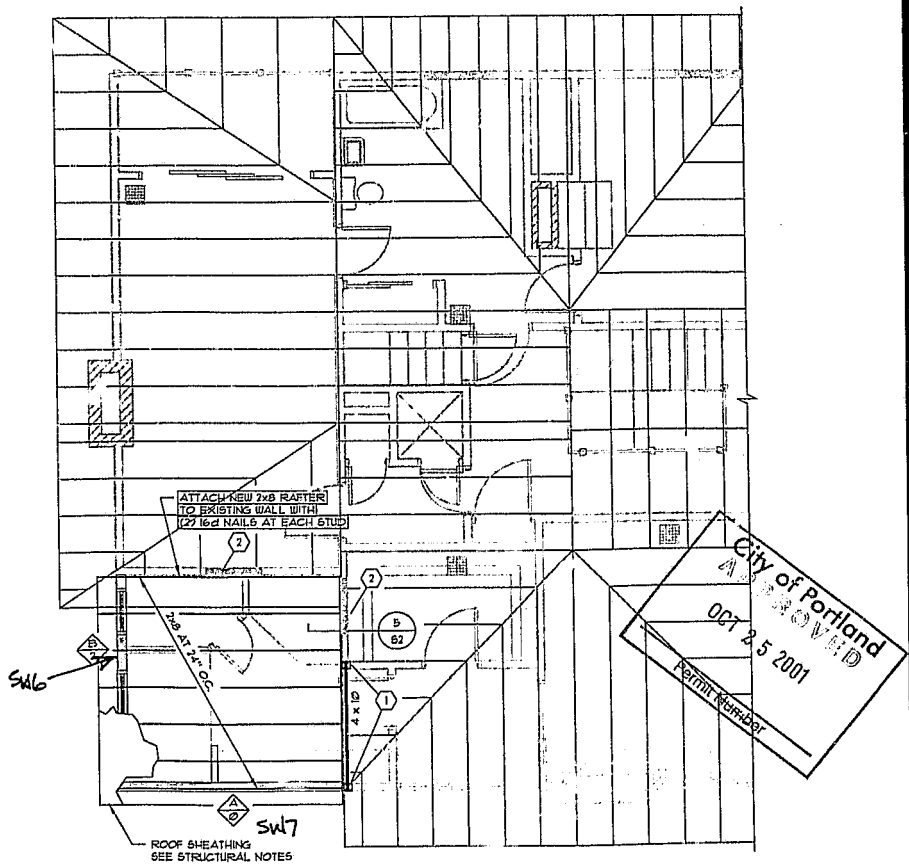
LONGITUDINAL DIRECTION $E_L := E_I \cdot MASS_{ADDITION}$ $E_L = 643.79 \cdot lb$

City of Portland
APPROVED
OCT 25 2001
Permit Number

LEE ENGINEERING, INC.
1300 JOHN ADAMS ST.
OREGON CITY, OR 97045
(503) 655-1342 FAX (503) 655-1360

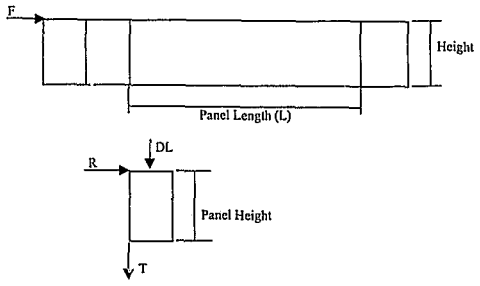
MADE BY SWE DATE: 8/30/01 SHT NO 4A
CLIENT ESLINGER JONES & ASSOCIATES JOB NO 2824
PROJECT DOLAN RESIDENCE ADDITION




ATTACH NEW 2x8 RAFTER TO EXISTING WALL WITH (2) 16d NAILS AT EACH STUD
2x8 AT 24" O.C.
4 x 12
SW6
SW7
ROOF SHEATHING SEE STRUCTURAL NOTES
City of Portland
APPROVED
OCT 25 2001
Permit Number

5A.

SHEAR PANELS



L = Panel Length

H = Panel Height

$V = F / \Sigma L$

F = Line Load

$R = V \times L$

$M_O = R \times H + R(\text{above}) \times H(\text{above})$

$M_R = 0.67 \times DL \times L^2/2$

Uplift (T) = $(M_O - M_R) / (L - 6\text{ in})$

Shear Panel	H (ft)	L (ft)	Wall Lgth. (ft)	H/L		Line Load ht	Line Load k	Line Load From Above ht	Line Load From Above k	Dead Load (klf)	V (plf)	Panel Sides	Panel Type	M_O (ft-k)	M_R (ft-k)	Uplift T (k)
1	8	8.00	8.00	1.00	OK	8.00	2.44			0.27	305	Single	II	19.54	8.64	1.83
2	8	16.00	16.00	0.50	OK	8.00	1.80			0.29	113	Single	I	14.40	36.48	-0.65
3	8	22.00	22.00	0.36	OK	8.00	1.80			0.20	82	Single	I	14.40	47.19	-0.80
4	8	3.00	6.00	2.67	OK	8.00	1.80			0.28	300	Single	II	7.20	1.26	2.54
5	8	3.00	6.00	2.67	OK	8.00	1.80			0.28	300	Single	II	7.20	1.26	2.54
6	7	2.25	2.25	3.11	OK	7.00	0.64			0.17	286	Single	II	4.50	0.42	2.41
7	8	12.00	12.00	0.67	OK	8.00	0.64			0.17	54	Single	I	5.14	11.88	-0.24
8	8	3.00	3.00	2.67	OK	8.00	0.64			0.17	214	Single	I	5.14	0.74	1.86

LEE ENGINEERING, INC
1300 JOHN ADAMS ST
OREGON CITY, OR 97045

DONE BY:_____

SHTNO: 10

JOB NO:_____

SHEAR WALL SUMMARY

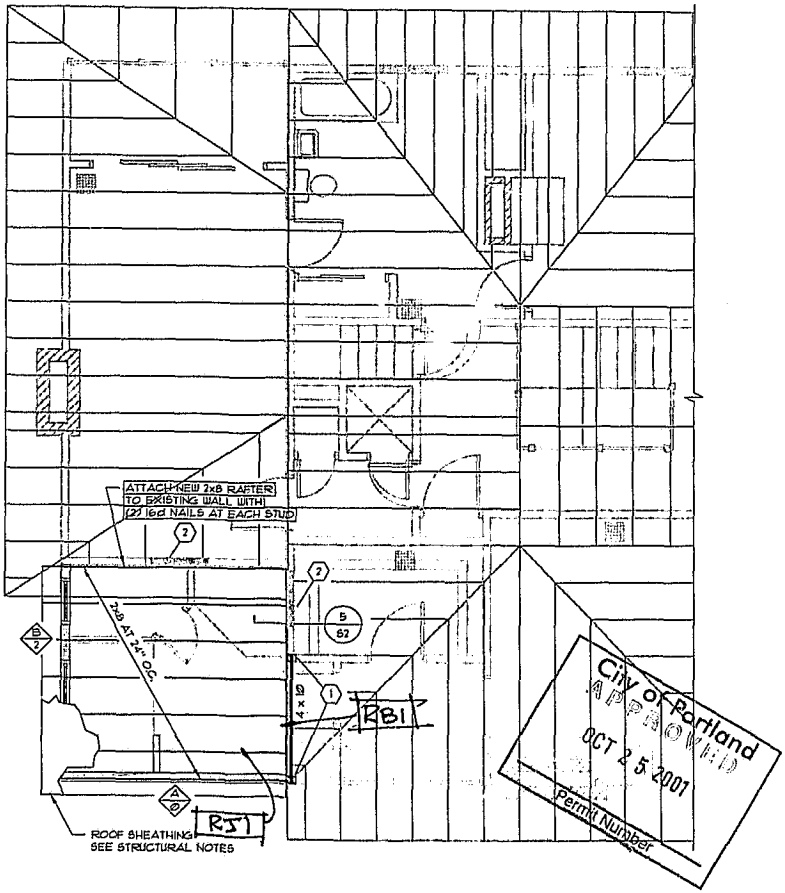
Panel	Wall Shear V (plf)	Wall Type	Good For (plf)	Uplift (lb)	Simpson Holdown	Good For (lb)	Shear Check	Holdown Check
1	305	1/2" APA Rated Plyw'd w/ 8d Nails @ 4/12	380	1834.0	Simpson HTT16	3480	OK	OK
2	113	1/2" APA Rated Plyw'd w/ 8d Nails @ 6/12	260	647.8	Simpson None	0	OK	OK
3	82	1/2" APA Rated Plyw'd w/ 8d Nails @ 6/12	260	800.8	Simpson None	0	OK	OK
4	300	1/2" APA Rated Plyw'd w/ 8d Nails @ 4/12	380	2542.3	Simpson HTT16	3480	OK	OK
5	300	1/2" APA Rated Plyw'd w/ 8d Nails @ 4/12	380	2542.3	Simpson HTT16	3480	OK	OK

LEE ENGINEERING, INC
1300 JOHN ADAMS ST
OREGON CITY, OR 97045

DONE BY:_____

SHT NO: 7

JOB NO:_____


ATTACH NEW 2x8 RAFTER TO EXISTING WALL WITH (2) 16d NAILS AT EACH STUD
2x8 AT 24" O.C.
RB1
RJ1
ROOF SHEATHING SEE STRUCTURAL NOTES
City of Portland
APPROVED
OCT 25 2001
Permit Number

LEE ENGINEERING, INC
1300 JOHN ADAMS ST
OREGON CITY, OR
(503) 655-1342
(503) 655-1360 (FAX)

Title : DOLAN RESIDENCE ADDITION Job # 2824
Dsgnr: SWE Date: 4:19PM, 31 AUG 01
Description :

Scope :

Timber Beam & Joist

Description Roof Joist RJ1

Timber Member Information

Calculations are designed to 1997 NDS and 1997 UBC Requirements

Timber Section		2x8
Beam Width	in	1.500
Beam Depth	in	7.250
Le: Unbraced Length	ft	0.00
Timber Grade		ouglas Fir - Larch,
Fb - Basic Allow	psi	675.0
Fv - Basic Allow	psi	95.0
Elastic Modulus	ksi	1,600.0
Load Duration Factor		1.150
Member Type		Sawn
Repetitive Status		Repetitive

Center Span Data

Span	ft	11.50
Dead Load	#/ft	30.00
Live Load	#/ft	50.00

Results

Ratio = 0.8697

Mmax @ Center	in-k	15.87
@ X =	ft	5.75
fb : Actual	psi	1,207.7
Fb : Allowable	psi	1,388.6
		Bending OK
fv : Actual	psi	56.8
Fv : Allowable	psi	109.3
		Shear OK

Reactions

@ Left End	DL	lbs	172.50
	LL	lbs	287.50
	Max. DL+LL	lbs	460.00
@ Right End	DL	lbs	172.50
	LL	lbs	287.50
	Max. DL+LL	lbs	460.00

Deflections

Ratio OK

Center DL Defl	in	-0.155
L/Defl Ratio		890.9
Center LL Defl	in	-0.258
L/Defl Ratio		534.6
Center Total Defl	in	-0.413
Location	ft	5.750
L/Defl Ratio		334.1

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Title : DOLAN RESIDENCE ADDITION Job # 2824
Dsgnr: SWE Date: 4:19PM, 31 AUG 01
Description :

Scope :

Rev: 510304
User: KW-0603240, Ver 5.1.3, 22-Jun-1999, Win32
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Timber Beam & Joist

Description Roof Beam RB1

Timber Member Information

Calculations are designed to 1997 NDS and 1997 UBC Requirements

Timber Section		4x10
Beam Width	in	3.500
Beam Depth	in	9.250
Le: Unbraced Length	ft	0.00
Timber Grade		ouglas Fir - Larch,
Fb - Basic Allow	psi	875.0
Fv - Basic Allow	psi	95.0
Elastic Modulus	ksi	1,600.0
Load Duration Factor		1.150
Member Type		Sawn
Repetitive Status		Repetitive

Center Span Data

Span	ft	6.50
Dead Load	#/ft	315.00
Live Load	#/ft	525.00

Results

Ratio = 0.8892

Mmax @ Center	ln-k	53.23
@ X =	ft	3.25
fb : Actual	psi	1,066.6
Fb : Allowable	psi	1,388.6
		Bending OK
fv : Actual	psi	97.1
Fv : Allowable	psi	109.3
		Shear OK

Reactions

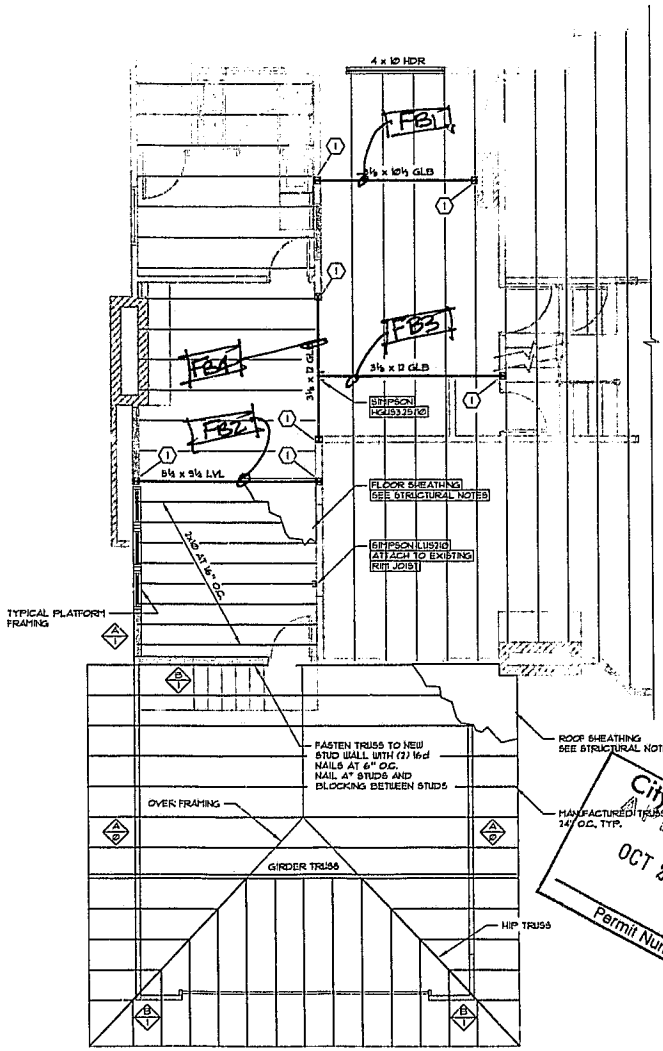
@ Left End	DL	lbs	1,023.75
	LL	lbs	1,706.25
	Max. DL+LL	lbs	2,730.00
@ Right End	DL	lbs	1,023.75
	LL	lbs	1,706.25
	Max. DL+LL	lbs	2,730.00

Deflections

Ratio OK

Center DL Defl	in	-0.034
L/Defl Ratio		2,277.1
Center LL Defl	in	-0.057
L/Defl Ratio		1,366.3
Center Total Defl	in	-0.091
Location	ft	3.250
L/Defl Ratio		853.9

10



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APPROVED
OCT 25 2001
Permit Number

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1300 JOHN ADAMS ST
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(503) 655-1342
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Title : DOLAN RESIDENCE ADDITION Job # 2824
Dsgnr: SWE Date: 3:33PM, 7 SEP 01
Description :

Scope :

Timber Beam & Joist

Description Floor Beam FB1

Timber Member Information

Calculations are designed to 1997 NDS and 1997 UBC Requirements

Timber Section		3.125x10.5i
Beam Width	in	3.125
Beam Depth	in	10.500
Le: Unbraced Length	ft	0.00
Timber Grade		ouglas Fir, 24F-V4
Fb - Basic Allow	psi	2,400.0
Fv - Basic Allow	psi	165.0
Elastic Modulus	ksi	1,800.0
Load Duration Factor		1.150
Member Type		GlcLam
Repetitive Status		No

Center Span Data

Span	ft	10.25
Dead Load	#/ft	225.00
Live Load	#/ft	575.00

Results

Ratio = 0.8218

Mmax @ Center	in-k	126.07
@ X =	ft	5.12
fb : Actual	psi	2,195.6
Fb : Allowable	psi	2,760.0
		Bending OK
fv : Actual	psi	155.9
Fv : Allowable	psi	189.8
		Shear OK

Reactions

@ Left End	DL	lbs	1,153.12
	LL	lbs	2,946.87
	Max. DL+LL	lbs	4,100.00
@ Right End	DL	lbs	1,153.12
	LL	lbs	2,946.87
	Max. DL+LL	lbs	4,100.00

Deflections

Ratio OK

Center DL Defl	in	-0.103
L/Defl Ratio		1,194.4
Center LL Defl	in	-0.263
L/Defl Ratio		467.4
Center Total Defl	in	-0.366
Location	ft	5.125
L/Defl Ratio		335.9

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Title : DOLAN RESIDENCE ADDITION Job # 2824
Dsgnr: SWE Date: 4:19PM, 31 AUG 01
Description :

Scope :

Timber Beam & Joist

Description Floor Beam FB2

Timber Member Information

Calculations are designed to 1997 NDS and 1997 UBC Requirements

Timber Section		MicroLam:
Beam Width	in	5.250
Beam Depth	in	9.250
Le: Unbraced Length	ft	0.00
Timber Grade		uss Joist - MacMil
Fb - Basic Allow	psi	2,600.0
Fv - Basic Allow	psi	285.0
Elastic Modulus	ksi	1,900.0
Load Duration Factor		1.150
Member Type		Manuf/Pine
Repetitive Status		Repetitive

Center Span Data

Span	ft	11.75
Dead Load	#/ft	315.00
Live Load	#/ft	400.00

Results

Ratio = 0.5752

Mmax @ Center	ln-k	148.07
@ X =	ft	5.87
fb : Actual	psi	1,977.8
Fb : Allowable	psi	3,438.5
		Bending OK
fv : Actual	psi	113.1
Fv : Allowable	psi	327.8
		Shear OK

Reactions

@ Left End	DL	lbs	1,850.62
	LL	lbs	2,350.00
	Max. DL+LL	lbs	4,200.62
@ Right End	DL	lbs	1,850.62
	LL	lbs	2,350.00
	Max. DL+LL	lbs	4,200.62

Deflections

Ratio OK

Center DL Defl	in	-0.205
L/Defl Ratio		686.7
Center LL Defl	in	-0.261
L/Defl Ratio		540.7
Center Total Defl	in	-0.466
Location	ft	5.875
L/Defl Ratio		302.5

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Title : DOLAN RESIDENCE ADDITION Job # 2824
Dsgnr: SWE Date: 3:33PM, 7 SEP 01
Description :

Scope :

Timber Beam & Joist

Description Floor Beam FB3

Timber Member Information

Calculations are designed to 1997 NDS and 1997 UBC Requirements

Timber Section		3.125x12.0
Beam Width	in	3.125
Beam Depth	in	12.000
Le: Unbraced Length	ft	0.00
Timber Grade		Douglas Fir, 24F-V4
Fb - Basic Allow	psi	2,400.0
Fv - Basic Allow	psi	165.0
Elastic Modulus	ksi	1,800.0
Load Duration Factor		1.150
Member Type		GluLam
Repetitive Status		No

Center Span Data

Span	ft	12.00
Dead Load	#/ft	233.00
Live Load	#/ft	515.00

Results

Ratio = 0.7947

Mmax @ Center	in-k	161.57
@ X =	ft	6.00
fb : Actual	psi	2,154.2
Fb : Allowable	psi	2,760.0
		Bending OK
fv : Actual	psi	150.8
Fv : Allowable	psi	189.8
		Shear OK

Reactions

@ Left End	DL	lbs	1,398.00
	LL	lbs	3,090.00
	Max. DL+LL	lbs	4,488.00
@ Right End	DL	lbs	1,398.00
	LL	lbs	3,090.00
	Max. DL+LL	lbs	4,488.00

Deflections

Ratio OK

Center DL Defl	in	-0.134
L/Defl Ratio		1,073.0
Center LL Defl	in	-0.297
L/Defl Ratio		485.4
Center Total Defl	in	-0.431
Location	ft	6.000
L/Defl Ratio		334.2

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Title : DOLAN RESIDENCE ADDITION Job # 2824
Dsgnr: SWE Date: 3:33PM, 7 SEP 01
Description :

Scope :

Description Floor Beam FB4

Timber Member Information

Calculations are designed to 1997 NDS and 1997 UBC Requirements

Timber Section		**3.125x12.0**
Beam Width	in	3.125
Beam Depth	in	12.000
Le: Unbraced Length	ft	0.00
Timber Grade		ouglas Fir, 24F-V4
Fb - Basic Allow	psi	2,400.0
Fv - Basic Allow	psi	165.0
Elastic Modulus	ksi	1,800.0
Load Duration Factor		1.150
Member Type		GluLam
Repetitive Status		No

Center Span Data

Span	ft	9.00
Dead Load	#/ft	180.00
Live Load	#/ft	390.00
Point #2 DL	lbs	1,400.00
LL	lbs	3,090.00
@ X	ft	4.000

Results

Ratio = 0.9498

Mmax @ Center	in-k	188.00
@ X =	ft	4.00
fb : Actual	psi	2,506.7
Fb : Allowable	psi	2,760.0
		Bending OK
fv : Actual	psi	180.2
Fv : Allowable	psi	189.8
		Shear OK

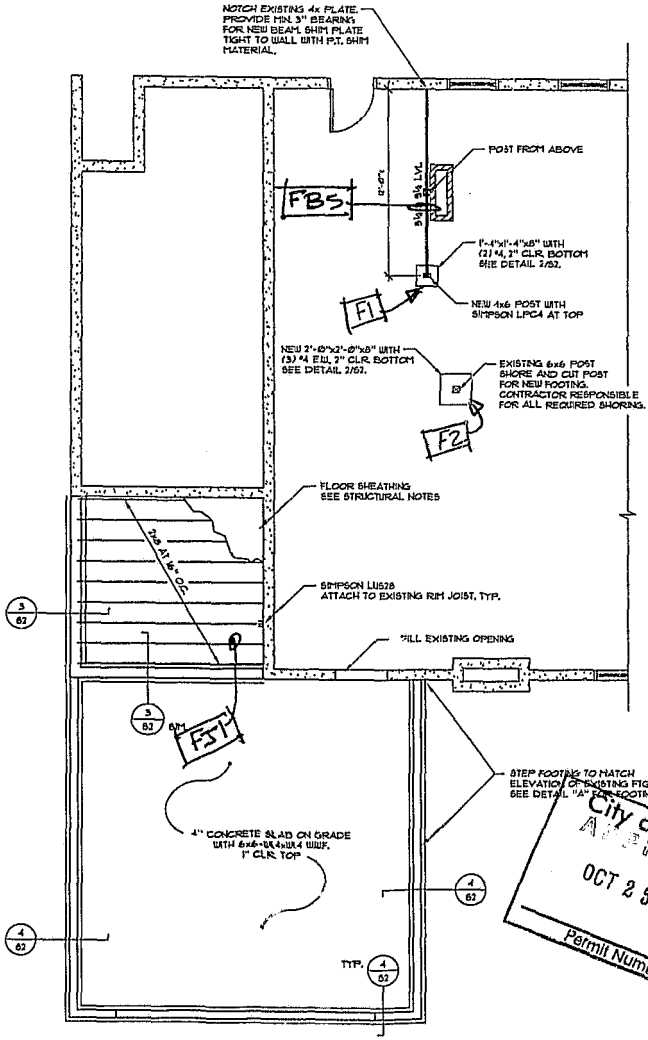
Reactions

@ Left End DL	lbs	1,587.78
LL	lbs	3,471.67
Max. DL+LL	lbs	5,059.44
@ Right End DL	lbs	1,432.22
LL	lbs	3,128.33
Max. DL+LL	lbs	4,560.55

Deflections

Ratio OK

Center DL Defl	in	-0.077
L/Defl Ratio		1,395.4
Center LL Defl	in	-0.170
L/Defl Ratio		637.2
Center Total Defl	in	-0.247
Location	ft	4.392
L/Defl Ratio		437.4



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1300 JOHN ADAMS ST
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Title : DOLAN RESIDENCE ADDITION Job # 2824
Dsgnr: SWE Date: 4:19PM, 31 AUG 01
Description :

Scope :

Rev: 510304
User: KW-0603240, Ver 5.1.3, 22-Jun-1999, Win32
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Timber Beam & Joist

Description Floor Joist FJ1

Timber Member Information

Calculations are designed to 1997 NDS and 1997 UBC Requirements

Timber Section		2x8
Beam Width	in	1.500
Beam Depth	in	7.250
Le: Unbraced Length	ft	0.00
Timber Grade		ouglas Fir - Larch,
Fb - Basic Allow	psi	875.0
Fv - Basic Allow	psi	95.0
Elastic Modulus	ksi	1,600.0
Load Duration Factor		1.000
Member Type		Sawn
Repetitive Status		Repetitive

Center Span Data

Span	ft	11.75
Dead Load	#/ft	16.00
Live Load	#/ft	56.30

Results

Ratio = 0.9436

Mmax @ Center	in-k	14.97
@ X =	ft	5.87
fb : Actual	psi	1,139.4
Fb : Allowable	psi	1,207.5
		Bending OK
fv : Actual	psi	53.0
Fv : Allowable	psi	95.0
		Shear OK

Reactions

@ Left End	DL	lbs	94.00
	LL	lbs	330.76
	Max. DL+LL	lbs	424.76
@ Right End	DL	lbs	94.00
	LL	lbs	330.76
	Max. DL+LL	lbs	424.76

Deflections

Ratio OK

Center DL Defl	in	-0.090
L/Defl Ratio		1,566.1
Center LL Defl	in	-0.317
L/Defl Ratio		445.1
Center Total Defl	in	-0.407
Location	ft	5.875
L/Defl Ratio		346.6

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Title : DOLAN RESIDENCE ADDITION Job # 2824
Dsgnr: SWE Date: 2:01PM, 11 SEP 01
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Timber Beam & Joist

Description Floor Beam FB5

Timber Member Information

Calculations are designed to 1997 NDS and 1997 UBC Requirements

Timber Section		MicroLam:
Beam Width	in	5.250
Beam Depth	in	9.250
Le: Unbraced Length	ft	0.00
Timber Grade		uss Joist - MacMil
Fb - Basic Allow	psi	2,600.0
Fv - Basic Allow	psi	285.0
Elastic Modulus	ksi	1,900.0
Load Duration Factor		1.000
Member Type		Manuf/Pine
Repetitive Status		No

Center Span Data

Span	ft	12.00
Dead Load	#/ft	15.00
Live Load	#/ft	40.00
Point #1 DL	lbs	1,153.00
LL	lbs	2,947.00
@ X	ft	6.000

Results

Ratio = 0.8193

Mmax @ Center	in-k	159.48
@ X =	ft	6.00
fb : Actual	psi	2,130.2
Fb : Allowable	psi	2,600.0
		Bending OK
fv : Actual	psi	72.2
Fv : Allowable	psi	285.0
		Shear OK

Reactions

@ Left End	DL	lbs	666.50
	LL	lbs	1,713.50
	Max. DL+LL	lbs	2,380.00
@ Right End	DL	lbs	666.50
	LL	lbs	1,713.50
	Max. DL+LL	lbs	2,380.00

Deflections

Ratio OK

Center DL Defl	in	-0.120
L/Defl Ratio		1,203.4
Center LL Defl	in	-0.307
L/Defl Ratio		469.0
Center Total Defl	in	-0.427
Location	ft	6.000
L/Defl Ratio		337.5

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Title : DOLAN RESIDENCE ADDITION Job # 2824
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Description :

Scope :

Rev: 510301
User: KW-0603240, Ver 5.1.3, 22-Jun-1999, Win32
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Square Footing Design

Description FOOTING F1

General Information

Calculations are designed to ACI 318-95 and 1997 UBC Requirements

Dead Load	0.670 k	Footing Dimension	1.330 ft
Live Load	1.720 k	Thickness	8.00 in
Short Term Load	0.000 k	# of Bars	2
Seismic Zone	4	Bar Size	4
Overburden Weight	0.000 psf	Rebar Cover	2.000
Concrete Weight	150.00 pcf	f'c	2,500.0 psi
LL & ST Act Separately		Fy	60,000.0 psi
Load Duration Factor	1.000		
Column Dimension	0.00 in	Allowable Soil Bearing	1,500.00 psf

REACTION FBS

Reinforcing

Rebar Requirement

Actual Rebar "d" depth used	5.750 in	As to USE per foot of Width	0.097 in2
200/Fy	0.0033	Total As Req'd	0.128 in2
As Req'd by Analysis	0.0003 in2	Min Allow % Reinf	0.0014
Min. Reinf % to Req'd	0.0014 %		

Summary

Footing OK

1.33ft square x 8.0in thick with 2- #4 bars

Max. Static Soil Pressure	1,451.12 psf	Vu : Actual One-Way	6.26 psi
Allow Static Soil Pressure	1,500.00 psf	Vn*Phi : Allow One-Way	85.00 psi
Max. Short Term Soil Pressure	478.77 psf	Vu : Actual Two-Way	27.04 psi
Allow Short Term Soil Pressure	1,500.00 psf	Vn*Phi : Allow Two-Way	170.00 psi
Mu : Actual	0.51 k-ft / ft	Alternate Rebar Selections...	
Mn * Phi : Capacity	7.30 k-ft / ft	1 # 4's 1 # 5's 1 # 6's	
		1 # 7's 1 # 8's 1 # 9's 1 # 10's	

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Scope :

Rev: 510301
User: KW-0603240, Ver 5.1.3, 22-Jun-1999, Win32

Square Footing Design

Description FOOTING F2

General Information

Calculations are designed to ACI 318-95 and 1997 UBC Requirements

Dead Load	1.500 k	Footing Dimension	2.000 ft
Live Load	4.000 k	Thickness	8.00 in
Short Term Load	0.000 k	# of Bars	2
Seismic Zone	3	Bar Size	4
Overburden Weight	0.000 psf	Rebar Cover	2.000
Concrete Weight	150.00 pcf	f'c	2,500.0 psi
LL & ST Act Separately		Fy	60,000.0 psi
Load Duration Factor	1.000		
Column Dimension	0.00 in	Allowable Soil Bearing	1,500.00 psf

REACTION FB3 + 2400# FOR ADDITIONAL LOAD

Reinforcing

Rebar Requirement

Actual Rebar "d" depth used	5.750 in	As to USE per foot of Width	0.097 in2
200/Fy	0.0033	Total As Req'd	0.193 in2
As Req'd by Analysis	0.0007 in2	Min Allow % Reinf	0.0014
Min. Reinf % to Req'd	0.0014 %		

Summary

Footing OK

2.00ft square x 8.0in thick with 2- #4 bars

Max. Static Soil Pressure	1,475.00 psf	Vu : Actual One-Way	17.85 psi
Allow Static Soil Pressure	1,500.00 psf	Vn*Phi : Allow One-Way	85.00 psi
Max. Short Term Soil Pressure	475.00 psf	Vu : Actual Two-Way	67.43 psi
Allow Short Term Soil Pressure	1,500.00 psf	Vn*Phi : Allow Two-Way	170.00 psi
Mu : Actual	1.18 k-ft / ft	Alternate Rebar Selections...	
Mn * Phi : Capacity	4.96 k-ft / ft	1 #4's 1 #5's 1 #6's	
		1 #7's 1 #8's 1 #9's 1 #10's	