1532 SW MORRISON ST. DFS-01 CO07-168389

Co.07.168389 DFS.01.

H

6

07-168389 RDFS 01 CO

CITY OF PORTLAND, OREGON - BUREAU OF DEVELOPMENT SERVICES

1900 SW Fourth Avenue, Suite 5000 • Portland, Oregon 97201 • www.portlandonline.com/bds

Deferred Submittal Requirements and Application

Applicants will provide:

- ☐ Three (3) sets of plans
- ☐ One (1) set of calculations
- ☐ Two (2) sets of product information
- ☐ Drawings and calculations must be stamped and signed by an Engineer registered in Oregon and approved by the Architect/Engineer of record for the building.
- ☐ A copy of this application
- ☐ One (1) copy of your main building permit approved plans (NOTE: Approved plans do not need to be submitted if your project has a process manager assigned)
- ☐ Permit fee (paid at time of submittal)

Contractor submittal information:

Contact name KRAIG O'CONNOR

Address 1530 SW TAYLOR ST.

City PORTLAND State OR Zip Code 97205

Phone 503-228-7177 E-mail KOCONNOR@RHCONST.COM

Value of deferred submittal $55,000 Issued main building permit # 07-168389-000-00-CO

Description/Scope of work RAISED FLOOR

Fees

Deferred submittal (DFS) fees are collected in addition to the standard building review fee paid on the main building permit. DFS fees cover the cost of the additional processing and review time associated with the design build element.

The DFS fee for processing and reviewing deferred plan submittals is 10 percent of the building permit fee calculated using the value of the particular deferred portion of the project.,

Minimum fee: Residential, one and two family dwelling ...$100 for DFS with valuation of less than or equal to $222,000

Commercial and all other projects .....$250 for DFS with valuation of less than or equal to $680,000

The Bureau of Development Services (BDS) fee schedule is also available on the BDS web site at ***www.portlandonline.com/bds*** | select the Fees tab.

Helpful Information

Bureau of Development Services
1900 SW 4th Avenue, Portland, OR 97201

Submit your plans to:
Development Services Center (DSC), First Floor,
Monday through Friday, 7:30 am - 3:00 pm
Thursday Evenings, 5:00 pm - 7:30 pm

Important Telephone Numbers

BDS main number	503-823-7300
DSC automated information line	503-823-7310
Building code information	503-823-1456
BDS 24 hour inspection request line	503-823-7000
Residential information for one and two family dwellings	503-823-7388
City of Portland TTY	503-823-6868

R&H CONSTRUCTION

SUBMITTAL COVER SHEET

January 08, 2008

To: Rashmi Vasavada
Allied Works Architecture
910 NW Hoyt St #200
Portland, OR 97209

From: Kraig O'Connor
R&H Construction Co.
1530 SW Taylor Street
Portland, OR 97205

City of Portland
Permit Number

Project: 56623 - Allied Works Remodel

Submittal No.: 10100-03 -- Access Floor (Deffered)

Specification Section:
Subcontractor/Supplier: C S I Interior Contractors
Quantity Sent: 4
Review Cycle: 3
Request Return to R&H by: 1/14/08

R&H Review Status: **No Exceptions Taken**

CHECKING IS ONLY TO VERIFY GENERAL CONFORMANCE AND COMPLIANCE WITH THE DESIGN CONCEPT AND CONTRACT DOCUMENTS. SUBCONTRACTOR OR SUPPLIER IS RESPONSIBLE FOR DIMENSIONS, ERRORS AND OMISSIONS IN THESE DRAWINGS OR LISTS WHICH HE SHALL CONFIRM AND CORRELATE AT THE JOBSITE WITH HIS WORK OR FABRICATION AND WITH THAT OF OTHER TRADES AFFECTED BY HIS WORK. HE SHALL BE RESPONSIBLE FOR ANY DEVIATION FROM THE CONTRACT DOCUMENTS OR THESE DRAWINGS OR LISTS UNLESS HE HAS OBTAINED WRITTEN APPROVAL TO DO SO, AND SHALL BE RESPONSIBLE FOR THE SATISFACTORY COMPLETION OF HIS WORK.

R&H CONSTRUCTION CO. Date: 1/8/08 By:

allied works architecture inc.
910 nw hoyt | portland, oregon 97209
503 227 3737 | f 503 227 6509

☐ Approved
☒ Approved as Corrected
Resubmittal
☐ Not Required
☐ Required

Fabrication/installation may be undertaken. Approval does not authorize changes in the Contract Sum or Contract Time unless stated by Change Order or Construction Change Directive.

☐ Revise & Resubmit
☐ Rejected

Fabrication/installation MAY NOT be undertaken. Revise/submit as noted. Resubmittal is required.

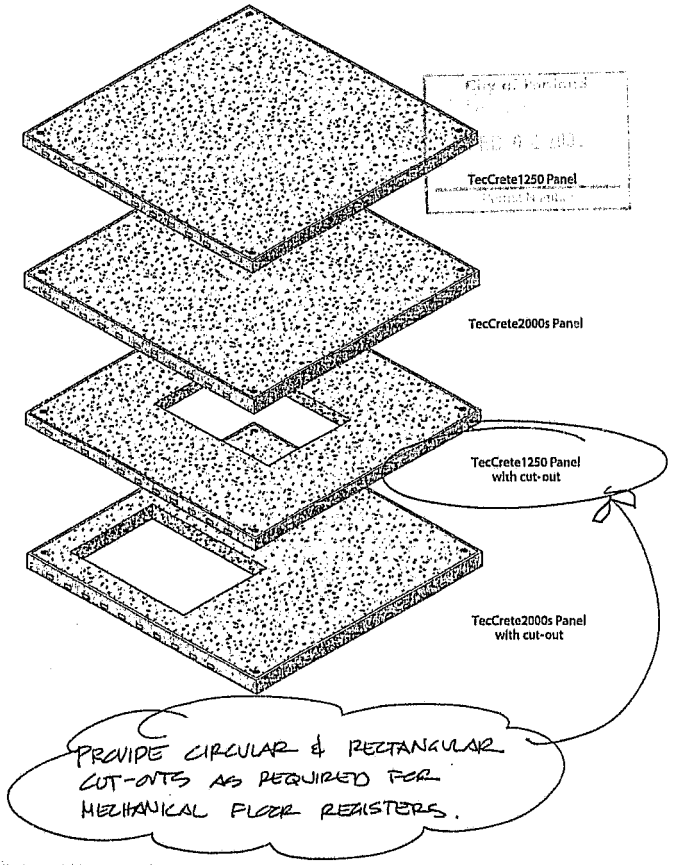
This submittal review is only for the purpose of determining general conformance with information given and the design intent expressed in the Contract Documents. Approval of a specific item does not constitute approval of an assembly of which that item is a component. Review of Submittals does not relieve the Contractor of its sole responsibility for: (1) determining the accuracy and completeness of details such as dimensions and quantities; (2) substantiating instructions for installation or performance of equipment or systems designed by the Contractor; (3) selecting fabrication processes and all construction means and methods; (4) coordinating the Work with the work of other trades; and (5) performing the Work in accordance with the Contract Documents.

By: Date: 1/14/2008

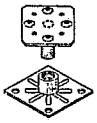
MI 020

07-166389 DFS 01 CO 2

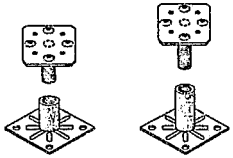
TecCrete Panels



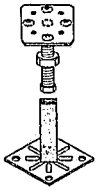
Note See price list for available cutout options.



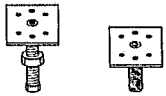
TecCrete Understructure,
Low Profile 3" (76mm) FFH



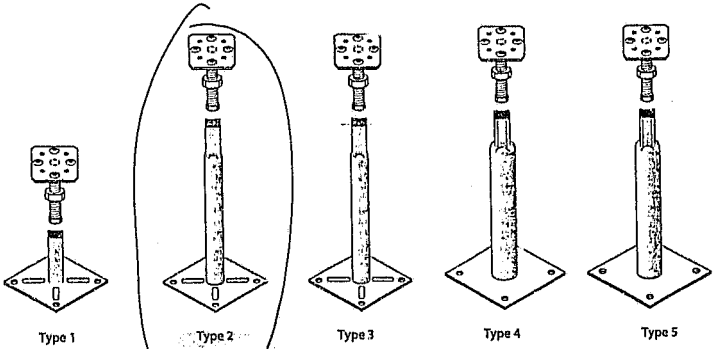
TecCrete Understructure,
Low Profile 4" (102mm), and 5" (127mm) FFH



TecCrete1250 Understructure,
Standard Profile 6" (152mm) minimum
to 30" (762mm) maximum FFH

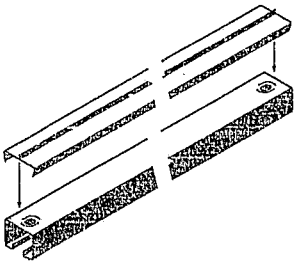


All TecCrete bases are available in
Cornerlock Field Heads or Perimeter Heads
shown above in standard
and low profile versions.

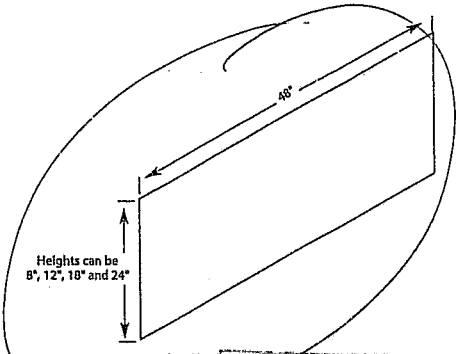


COORDINATE HEIGHT TO ALLOW FOR MECHANICAL UNIT CLEARANCES & UNISTRUT SUPPORTS WHERE REQUIRED

Note For additional seismic base options, please see page 18.

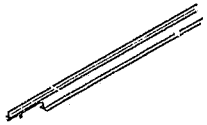


Steel Stringer with Gasket – 2' and 4'

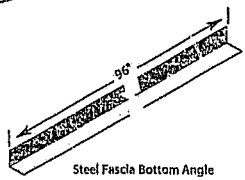


Steel Fascia Plates

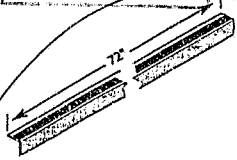
SUBMIT STEEL & ALUMINUM FASCIA PLATES FOR REVIEW.



TecSeal Air Seal

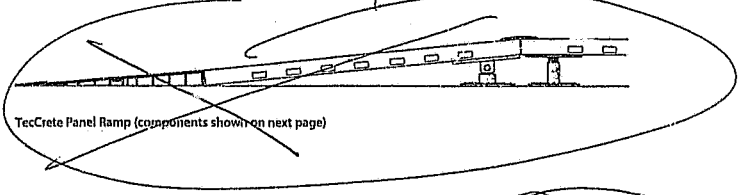


Steel Fascia Bottom Angle



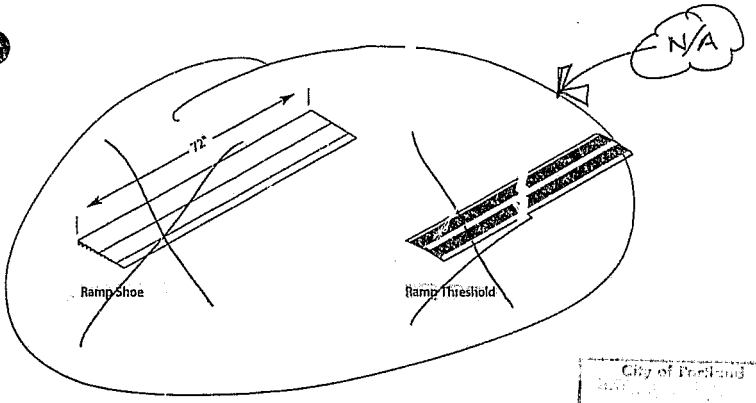
Aluminum Fascia Top Angle

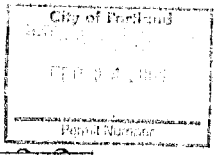
N/A

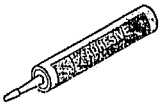


TecCrete Panel Ramp (components shown on next page)

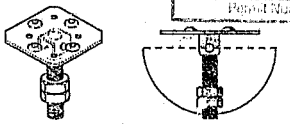
SUBMIT FOR REVIEW AS PERTAINS TO DETAIL (2) ON HAWORTH ACCESS FLOOR PLAN DIAGRAM





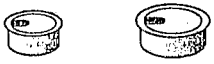


Adhesives



Swivel-head Pedestal

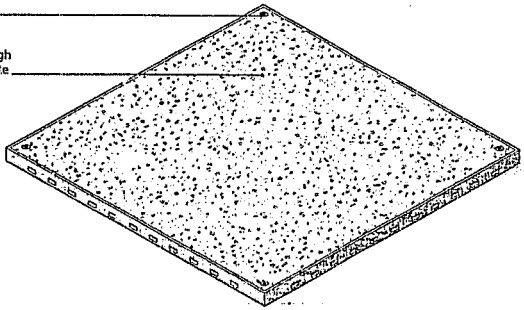
Lifting Bar



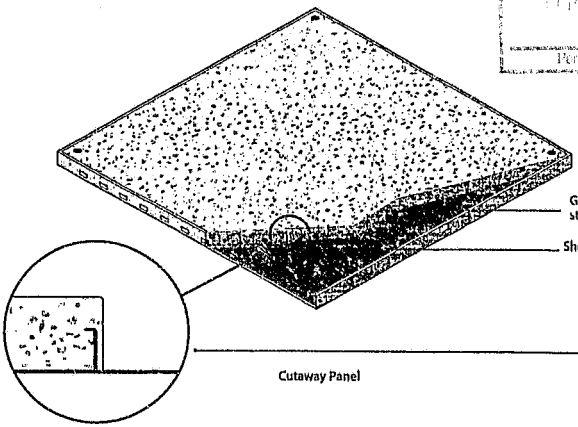
Grommet – 1 ¼" & 3"

Grommet – 4", 5"

TecCrete Cornerlock Panels



Full Panel



Cutaway Panel

General Information

Construction:	Lightweight high-strength concrete in a hot-dipped galvanized steel pan. Non-combustible.
Panel size (nominal):	24" x 24" (610mm x 610mm)
Panel thickness:	TecCrete1250 – 1⅛" (29mm) high bare finish TecCrete2000s – 1½" (38mm) high bare finish
Panel weight:	TecCrete1250 – 11.2 lbs./ft² (55.68 kgs/m²) bare finish TecCrete2000s – 13.2 lbs./ft² (6.0 kgs/m²) bare finish
Finished floor height:	3" to 30" (76mm x 762mm) – Other heights available
Panel finish:	Standard bare concrete
Fire resistance:	Non-combustible

Panel Options

- Factory supplied water jet cutouts for diffusers, grommets and electrical boxes.

PROVIDE

Understructure Options

- Cornerlock pedestal, field, and perimeter understructure is available for low 3" (76mm), 4" (102mm), and 5" (127mm), and standard, 6" to 30" (152mm to 762mm) finished floor heights.
- Seismic options are also available.
- For non-standard pedestal options, please contact your Haworth representative.
- Stringers are available in 24" (610mm) and 48" (1219mm) lengths.
- TecCrete's flat underside allows placement of pedestals anywhere under the panel, making support of partial panels at walls and columns easier and more secure.

Portland

OCT 0 1 2002

Permit Number

Underfloor Air Applications

- ~~TecSeal™ air seal strip provides a plenum seal when TecCrete is used with a bare finish in an underfloor air application.~~

COORDINATE AND PROVIDE AS REQUIRED FOR MECHANICAL DESIGN

NO. PER E-MAIL DATED 1/14/03

Companion Products and Systems

- Haworth Power Web.
- Haworth Pre-Terminated Zone Voice and Data.
- Haworth furniture and wall systems.

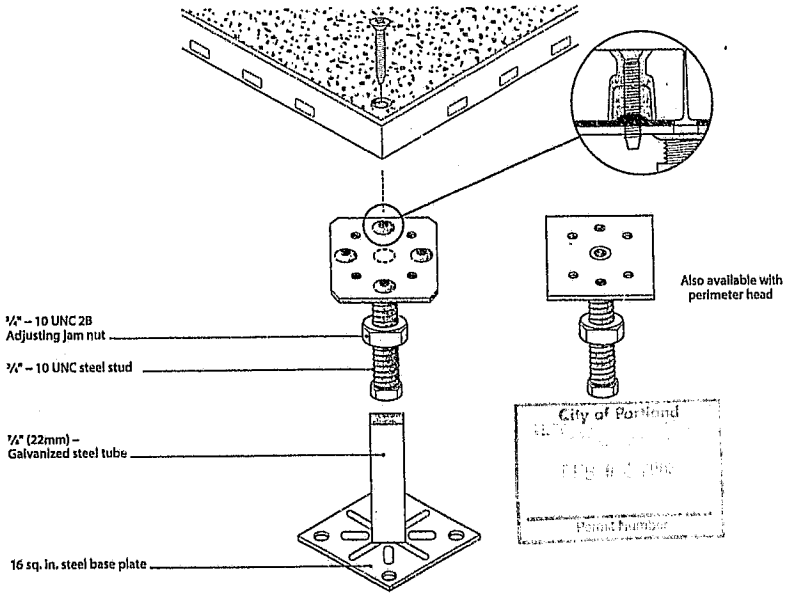
TecCrete Panel Performance Ratings

Access Floor Performance Ratings	Dynamic Load Rating			Static Load Rating		
	Rolling 10 Passes	Rolling 10,000 Passes	Impact 36" drop	Concentrated PSI	Uniform PSF	Ultimate PSI
TecCrete1250 Without Stringers	1,200 lbs.<.020 (554 kg)	800 lbs.<.040 (363 kg)	150 lbs. (68 kg)	1,250 lbs.<.10 (567 kg)	371 PSF	1,800 lbs. (816 kg)
TecCrete1250 With Bolted Stringers	1,300 lbs.<.020 (590 kg)	900 lbs.<.040 (408 kg)	150 lbs. (68 kg)	1,600 lbs.<.10 (726 kg)	392 PSF	2,800 lbs. (1,270 kg)
TecCrete2000s Bolted Stringer	2,000 lbs.<.020 (907 kg)	1,750 lbs.<.040 (794 kg)	150 lbs. (68 kg)	2,000 lbs.<.10 (907 kg)	440 PSF	3,000 lbs. (1,361 kg)

Link For detailed information on testing, please see page 64.

Standard Cornerlock Understructure

Standard cornerlock pedestals are used to support floor panels for 6" to 30" (152mm to 762mm) finished floor heights. Standard cornerlock pedestals are used to support four panels by their corners. Perimeter heads support TecCrete panels that are used against a perimeter wall, or where two different types of floors meet.



Note Standard pedestal bases are specified separately, for use with your choice of cornerlock field pedestal heads, or perimeter/transition pedestal heads.

Type 2 Pedestal Base

Stability and Load Rating

- To obtain overturning moment and axial load test values please contact your Haworth representative.

Finished Floor Height

- Standard finished floor heights are 12" to 30" (305mm to 762mm).

Pedestal Base

- Pedestal bases have a 4"x 4" (102mm x 102mm), 7/64" (3mm) thick square base plate.
- Pedestal tubes have a 1.163" (29.54mm) outside diameter, and are available from 9" to 27" (229mm to 686mm) in length.
- Bases and tubes are made of hot-dipped galvanized steel.
- Bases are fillet welded.

Pedestal Adjustment

- Pedestal assemblies provide an adjustment range of ± 1" (25mm) when finished floor height is 12" (305mm) or more, adjustable at 1/64" (0.4mm) increments.
- Minimum finished floor height is 12" (305mm) for TecCrete without stringers, or 13" (330mm) for TecCrete 2000s, or TecCrete1250 with stringers.

Pedestal Head (specified separately)

- Bases are for use with standard pedestal heads with 3/4" (19mm) diameter steel studs.

Type 3 Pedestal Base

Stability and Load Rating

- To obtain overturning moment and axial load test values please contact your Haworth representative.

Finished Floor Height

- Standard finished floor heights are 12" to 30" (305mm to 762mm).

Pedestal Base

- Pedestal bases have a 5"x 5" (127mm x 127mm), 3/16" (5mm) thick square base plate.
- Pedestal tubes have a 1.163" (29.54mm) outside diameter, and are available from 9" to 27" (229mm to 686mm) inches in length.
- Bases and tubes are made of hot-dipped galvanized steel.
- Bases are fillet welded.

Pedestal Adjustment

- Pedestal assemblies provide an adjustment range of ± 1" (25mm) when finished floor height is 12" (305mm) or more, adjustable at 1/64" (0.4mm) increments.
- Minimum finished floor height is 6" (152mm) for TecCrete without stringers, or 13" (330mm) for TecCrete 2000s, or TecCrete1250 with stringers.

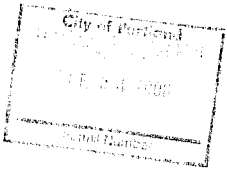
Pedestal Head (specified separately)

- Bases are for use with standard pedestal heads with 3/4" (19mm) diameter steel studs.

kpff Consulting Engineers

January 3, 2008

Ms. Rashmi Vasavada
Allied Works Architecture, Inc
910 NW Hoyt Street
Portland, OR 97209



RE: Morrison Street - Tenant Improvement
1532 SW Morrison Street, Portland, OR 97205
Access Floor Calculations

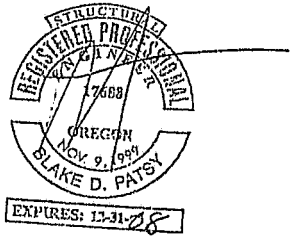
Dear Rashmi:

As requested, we have reviewed the access floor calculations for the referenced project. The calculations meet the requirements of the 2007 Oregon Structural Specialty Code based on the 2006 International Building Code. In addition, we have included a calculation for the anchorage of the system to the existing wood framed floor.

If you have any questions or need further information, please call.

Sincerely,

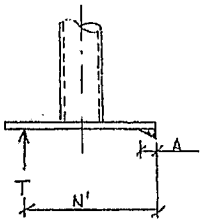
Ugo G. Costa, P.E., S.E.



Attachments

111 SW Fifth Avenue, Suite 2500 Portland, Oregon 97204-3628 (503) 227-3251 Fax (503) 227-7980
Seattle Tacoma Portland Eugene San Francisco Sacramento Los Angeles Irvine San Diego Phoenix Denver St Louis New York

KPFF Consulting Engineers Portland, Oregon	Project MORRISON STREET	By UGC	Sheet No.
	Location	Date 1/2/2007	
	Client ALLIED WORKS ARCHITECTURE	Revised	Job No.
		Date	



ANCHORAGE OF PEDESTAL.

M_{Pi} = 841 #-ft (SEE SHT 1)

$M_{PASD} = M_{PU}/1.4$

$= 601$ #-ft

$P_D = 0.9(11.2\,psi)(1') = 40.3$#

$f_p = 625\,psi$

$f' = f_p(B)N'/2$ $\quad B = 4"$, $N' = 3.5"$

$= 4375\,psi$

$$A = f'\frac{\sqrt{f'^2 - \frac{1}{6}(f_p B)(PA' + M)}}{f_p B/3} \qquad A = 1.5"$$

$= 0.153"$

$\Rightarrow T = \frac{f_p A B}{2} - P_D$

$= 157$#

$\Rightarrow T/SCREW = T/2 = 74$#

$\Rightarrow$ USE 3/8" x 1" LAG SCREWS

T-E = 17/32"

~~$\Rightarrow W' = C_D W$~~ $\quad C_D = 1.33$

$W = 305^{\#}/_{in}$

$= 216^{\#} > T/SCREW$ (OK)

HAWORTH® Access Flooring

Evaluated by:	David Benner		Date:	November 30, 2007
Showroom & Dealer:	Not Provided	LOCATION	PHONE	
Requested by:	Rashmi Vasavada		508-227-1737	
Client:	Allied Works			
Project:	Not Provided			
Location:	Not Provided		Unknown description	
		OR	No Zip Code provided	

Input Page

Input type: Metric SI or Inch-Pound? Inch-Pound — All values must be entered in Inch-Pound units.

Intended use of floor space: Office
Further explanation >>>>>>>>>>>>>>>

Access Floor Panel type: TecCrete 1250

Loads:

Dead Load:	DL =	11.20 psf	0.54 kPa	Based on TecCrete 1250 floor panel
Enter the expected Live Load for the floor space.				
Live Load:	LL =	50 psf	2.39 kPa	
Partition Load:	PL =	15 psf	0.72 kPa	
Panel Area:	A_p =	4.00 sq.ft	0.37 sq.m	

Heights:

Finished Floor Height:	FFH =	13.0 inches	330.2 mm
Height of Slab within building above grade:	z =	13 ft	3.962
Height of Roof of building above grade:	h =	45 ft	13.716 m

Building Code:

Applicable Building Code: 2003 International Building Code

Seismic evaluation of pedestals will be based on the 2003 International Building Code.

Disclaimer:
The calculations in this document follow the prescribed methods of the noted edition of the model code specified, and may not reflect local amendments in force at time of review. The calculations are based on site-specific information provided to Haworth by others; Haworth has not reviewed this data for accuracy or correctness. Calculations assume that the conditions of use noted in Haworth's installation recommendations will be observed. Calculations must not be considered valid for plan review or construction purposes unless bearing the seal of the engineer-of-record registered in the jurisdiction.

Engineer's stamp

HAWORTH® Access Flooring

Evaluated by:	David Benner	Date:	November 30, 2007
Showroom & Dealer:	Not Provided LOCATION	PHONE	
Requested by:	Rashmi Vasavada		503-227-1737
Client:	Allied Works		
Project:	Not Provided		
Location:	Not Provided		Unknown description
	0 OR		No Zip Code provided

2003 International Building Code

The following values need to be provided for the evaluation:

Site Class, Table 1615.1.1 = D — Section 1615.1.1
- If Site Class unknown, use D

Seismic Use Group, Table 1604.5 = I — Section 1616.2

Component Response Modification Factor, R_p, Table 9.6.2.2 = 1.5 — ASCE 7-02 Section 9.6.2.2

Component Importance Factor, I_p, ASCE 7-02 Section 9.6.1.5
All other components = I_p = 1.5

Mapped Spectral Acceleration for short periods, S_s = 0.99 — From Figures 1615(1) thru (10), Section 1615.1
Mapped Spectral Acceleration for 1-second period, S_1 = 0.35 — From Figures 1615(1) thru (10), Section 1615.1

$F_p = \frac{0.4\, a_p S_{DS} W_p (1 + 2\,z/h)}{R_p / I_p}$ (Eq. 9 6 1 3-1)

= Seismic Design Force, ASCE 7-02 Section 9.6.1.3

Limits:

$F_{pU} = 1.6\, S_{DS} I_p W_p$ (Eq. 9 6 1 3-2) Maximum where: $\frac{0.4\, a_p (1 + 2\,z/h)}{R_p}$ = 1.6 = constant

$F_{pm} = 0.3\, S_{DS} I_p W_p$ (Eq. 9 6 1 3-3) Minimum where: $\frac{0.4\, a_p (1 + 2\,z/h)}{R_p}$ = 0.3 = constant

a_p = Component Amplification Factor, ASCE 7-02 Table 9 6 2 2
= 1 0 for Access Floors

S_{DS} = Design Spectral Response Acceleration at short periods, Section 1615 1 3
= 2/3 S_{MS}

S_{MS} = Maximum considered earthquake Spectral Response Acceleration for short periods, Section 1615.1 2
= $F_a S_s$
= 1.1

Where: F_a = Site Coefficient corresponding to S_s for short periods, Table 1615 1.2(1)
S_s = Mapped Spectral Acceleration for short periods, Figures 1615(1) thru (10)
S_s = 0 99

Site Class = D

Table 1615.1.2(1), Site Coefficient, F_a, for short periods

Site Class	Mapped Spectral Acceleration for Short Periods, S_s				
	S_s <= 0.25	S_s = 0.50	S_s = 0.75	S_s = 1.00	S_s >= 1.25
A	0 8	0.8	0 8	0 8	0.8
B	1 0	1.0	1.0	1.0	1 0
C	1.2	1.2	1.1	1.0	1.0
D	1 6	1.4	1.2	1.1	1.0
E	2.5	1.7	1.2	0.9	
F	see note				

Note: site specific geotechnical investigation and dynamic site response shall be performed to determine appropriate values

F_a = 1.1 straight line interpolation

S_{D1} = Design Spectral Response Acceleration at 1-second period, Section 1615 1.3
= 2/3 S_{M1}

S_{M1} = Maximum considered earthquake Spectral Response Acceleration for a 1-second period, Section 1615 1 2
= $F_v S_1$
= 0.0

Where: F_v = Site Coefficient corresponding to S_1 for a 1-second period, Table 1615 1.2(2)
S_1 = Mapped Spectral Acceleration for short periods, Figures 1615(1) thru (10)
S_1 = 0.35

Site Class = D

Table 1615.1.2(2), Site Coefficient, F_v, for 1-second period

Site Class	Mapped Spectral Acceleration for a 1-Second Period, S_1				
	S_1 <= 0 1	S_1 = 0.2	S_1 = 0.3	S_1 = 0.4	S_1 >= 0.5
A	0 8	0.8	0 8	0 8	0 8
B	1 0	1.0	1.0	1 0	1 0
C	1 7	1 5	1 5	1 4	1 3
D	2 4	2 0	1 8	1 6	1 5
E	3 5	3 2	2 8	2 4	
F	see note				

Note: Site specific geotechnical investigation and dynamic site response shall be performed to determine appropriate values

F_v = 1 7 straight line interpolation

Continued next page

HAWORTH® Access Flooring

Evaluated by:	David Benner		Date:	My	2007
Showroom & Dealer:	Not Provided	LOCATION	PHONE		
Requested by:	Reshmi Vasavada		508-227-1737		
Client:	Allied Works				
Project:	Not Provided				
Location:	Not Provided		Unknown description		
	0	OR	No Zip Code provided		

2003 International Building Code

S_{DS} = 0.725 g — These values are used to determine the Seismic Design Category, Tables 1616.3(1) & 1616.3(2)
S_{D1} = 0.392 g — The most severe design category will be assigned

Seismic Design Category based on S_{DS} for engineering use.
Seismic Use Group = III
Table 1616.3(1), Seismic Design Category, for short periods

Value of S_{DS}	Seismic Use Group I	II	III
S_{DS} < 0.167g	A	A	A
0.167g <= S_{DS} < 0.33g	B	B	C
0.33g <= S_{DS} < 0.50g	C	C	D
0.50g <= S_{DS}	D*	D*	D*

Seismic Design Category: D* See * Note

Seismic Design Category based on S_{D1} for engineering use.
Seismic Use Group = III
Table 1616.3(2), Seismic Design Category, for a 1-second period

Value of S_{D1}	Seismic Use Group I	II	III
S_{D1} < 0.067g	A	A	A
0.067g <= S_{D1} < 0.133g	B	B	C
0.133g <= S_{D1} < 0.20g	C	C	D
0.20g <= S_{D1}	D*	D*	D*

Seismic Design Category: D* See * Note

* Note: Seismic Use Groups I and II structures located on sites with mapped maximum considered earthquake spectral response acceleration at 1-second period, S1, equal to or greater than 0.75g_1 shall be assigned to Seismic Design Category E, and Seismic Use Group III structures located on such sites shall be assigned to Seismic Design Group Category F.

z = Height of slab, above grade, where Access Floor will be installed
= 13.0 ft 3.96 m from input page
h = Average height of roof above grade
= 45.0 ft 13.72 m from input page

R_p = Component Response Modification Factor, ASCE 7-02 Table 9.6.2.2
= 1.5

I_p = Component Importance Factor, ASCE 7-02 Section 9.6.1.5
= 1.5

W_p = Component Operating Weight, ASCE 7-02 Section 9.0.2.7.1
= (DL + 25%LL + PL) * A

where,	DL =	11.2 psf	0.54 kPa	from input page
	LL =	50.0 psf	2.39 kPa	from input page
	PL =	15.0 psf	0.72 kPa	from input page
	Area of panel =	4.0 sq ft	0.37 sq.m	from input page
W_p =	154.5 lbs.	689 N		

F_p = 70.6 lbs. 316 N

Limits
F_{pa} = 269.3 lbs. 1,198 N
F_{pr} = 50.5 lbs 225 N

See Results Summary page for Component details

City of Portland
FEB 0 1 2008
Permit Number

Disclaimer:
The calculations in this document follow the prescribed methods of the noted edition of the model code specified, and may not reflect local amendments in force at time of review. The calculations are based on site-specific information provided to Haworth by others. Haworth has not reviewed this data for accuracy or correctness. Calculations assume that the conditions of use noted in Haworth's installation recommendations will be observed. Calculations must not be considered valid for plan review or construction purposes unless bearing the seal of the engineer-of-record registered in the jurisdiction

Engineer's stamp

HAWORTH® Access Flooring

Evaluated by:	David Benner		Date: November 30, 2007
Showroom & Dealer:	Not Provided	LOCATION	PHONE
Requested by:	Rashmi Vasavada		508-227-1737
Client:	Allied Works		
Project:	Not Provided		
Location:	Not Provided		Unknown description
	0	OR	No Zip Code provided

Building Code / Jurisdiction: 2003 International Building Code

Results Summary:

Seismic Design Force as calculated according to the 2003 International Building Code.

Seismic Force	F_p = 70.8 lbs.	315 N	
Finished Floor Height	FFH = 13.0 in	330.2 mm	
Panel Type:	TecCrete 1250		
Panel thickness	t_p = 1.125 in	28.6 mm	
Pedestal height	h_{ped} = FFH - t_p =	11.88 in	301.6 mm
Axial Load on pedestal	P = (DL + LL + PL)*A		
	= 304.8 lbs.		1,356 N

City of [illegible]
FEB 0 4 2008
Permit Number

Moment applied to pedestal due to Seismic Loading

M_p = Seismic Force x Pedestal height

M_p =	841 in-lbs	95.0 N-m
=	70.1 ft-lbs	

Pedestals

Pedestal type: 12 | Connection type: Mechanical Anchor | Anchor to be specified by others.

Axial Load — Mode of failure: Buckling of pedestal shaft

Tested capacity of Pedestal	P_{test} =	8,000 lbs	35,142 N
Reference test - Intertek 3075039-02			
Safety Factor for Axial Load on pedestals	SF =	3 - reference ICC-ES AC300, Section 3.4.2.6.	
Capacity of pedestal	P_a =	2,933 lbs	13,047 N

Allowable Axial Load from test loads: P / P_a = 0.104 < 1.0 is OK

Bending / Overturning — Mode of failure: Pedestal shaft to baseplate

Tested capacity of Pedestal	M_{test} =	402.6 ft-lbs	545.9 N-m
Reference test - Intertek 3075039-02			
Safety Factor for connection type	SF =	3	
Capacity of pedestal	M_a =	134.2 ft-lbs	182.0 N-m

Allowable Bending / overturning from test loads: M_p / M_a = 0.532 < 1.0 is OK

Disclaimer:
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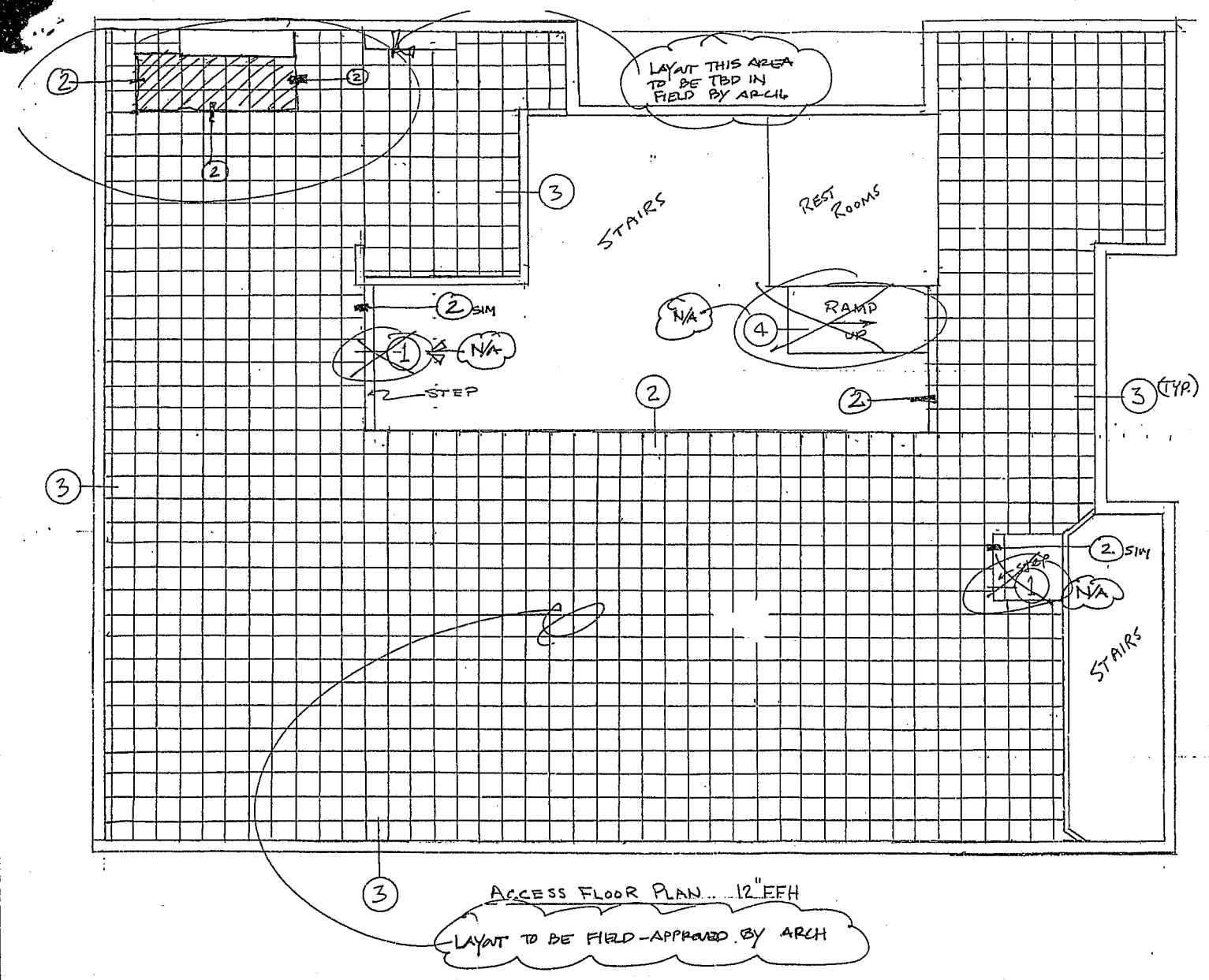
Engineer's stamp



BILL OF MATERIAL.

- TEC-CRETE CORNERLOCK

City of Portland
FEB 04 2008
Permit Number

allied works architecture inc.
910 nw hoyt | portland, oregon 97209
v 503 227 1737 | f 503 227 6509

☐ Approved
☒ Approved as Corrected
Resubmittal:
☐ Not Required
☒ Required

Fabrication/Installation may be undertaken. Approval does not authorize changes in the Contract Sum or Contract Time unless stated by Change Order or Construction Change Directive.

for items noted

☐ Revise & Resubmit
☐ Rejected

Fabrication/Installation MAY NOT be undertaken. In resubmitting limit corrections to the items marked.

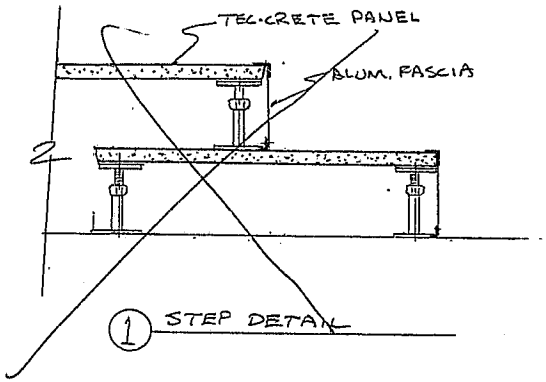
This submittal review is only for the purpose of determining general conformance with information given and the design intent expressed in the Contract Documents. Approval of a specific item does not constitute approval of an assembly of which that item is a component. Review of Submittals does not relieve the Contractor of its sole responsibility for: (1) determining the accuracy and completeness of details such as dimensions and quantities; (2) substantiating instructions for installation or performance of equipment or systems designed by the Contractor; (3) selecting fabrication processes and construction means and methods; (4) coordinating the Work with the work of other trades; and (5) performing the Work in accordance with the Contract Documents.

By: Date: 1/14/2008

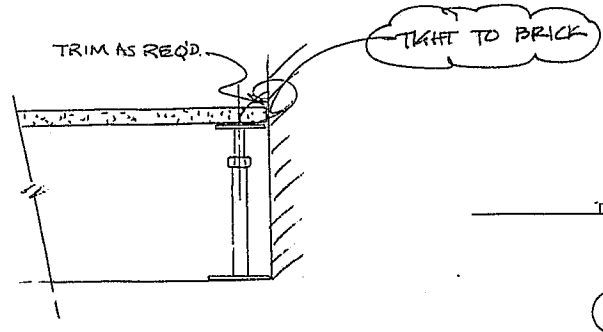
HAWORTH ACCESS FLOOR
ALLIED WORKS ARCHITECTURE

CONT: R&H CONSTRUCTION
BY: CSI INTERIOR CONTRACTORS

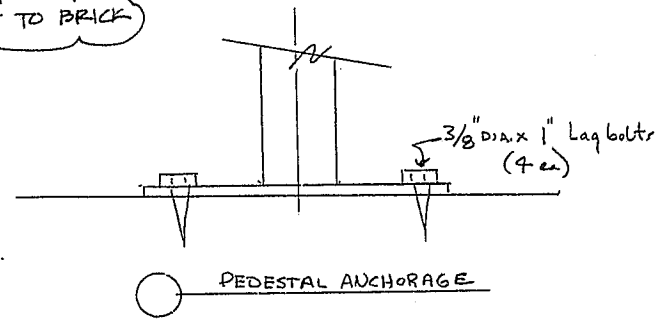
12/27/07



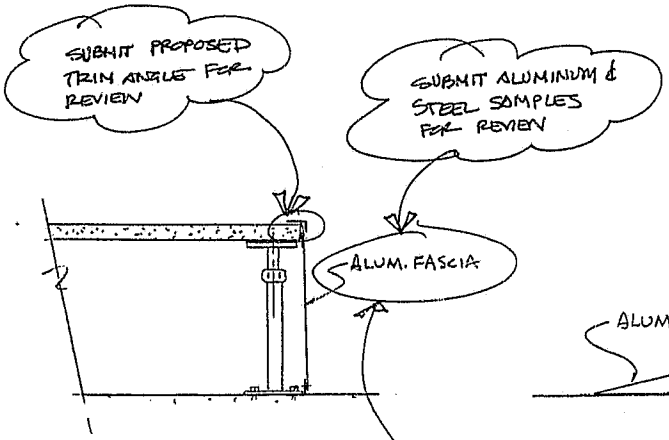
1 STEP DETAIL



3 DETAIL @ BORDER

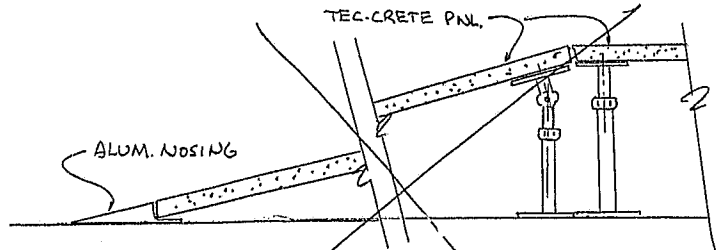


PEDESTAL ANCHORAGE



2 DETAIL @ FASCIA

2 SIM. REFERS TO TRIM ANGLE @ PANEL EDGE / TRANSITION



4 RAMP DETAIL

COORDINATE W/ ARCHITECT'S DETAIL & MATERIAL SELECTION. MATERIAL TO ALLOW FOR REQUIRED RETURN AIR TO UNDER-FLOOR UNIT.

- HAWORTH ACCESS FLOOR.
ALLIED WORKS ARCHITECTURE.

CONT. : R & H CONSTRUCTION
... BY : CSI INTERIOR CONTRACTORS

— 12/27/07