DFS01 FA11-103548

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CITY OF PORTLAND, OREGON – BUREAU OF DEVELOPMENT SERVICES

1900 SW Fourth Avenue, Suite 5000 • Portland, Oregon 97201 • www.portlandonline.com/bds • Fax 503-823-7425

# Facility Permit Plan Intake Form

| FOR INTAKE, STAFF USE ONLY | | Building/Mechanical | Done |
|---|---|---|---|
| Date Received | | Electrical | |
| Building Registration # | | Plumbing | |
| Fixed Bid | | Fire | |
| Bin # | B2 | Planning | |
| Building Permit # | 11-103548 FS-01 | BES | |
| Mechanical # | | PDOT | |
| Plumbing Permit # | | Structural | Brc |
| Electrical Permit # | | Other | |

**APPLICANT: Complete all sections below that apply to the project. Please print legibly.**

Print Name Glenn Whitefield Sign Name

Street Address 600 SE 10th Ave, Suite 310

City Portland State OR Zip Code 97205

Day Phone 503-753-3538 FAX 503-283-7874 email glenn.whitefield@lewisbuilds.com

**Plans / permits available for pick up at 1900 SW 4th Avenue, 2nd floor between 8:00 am to 5:00 pm**

Contact Name for plan/permit pick up Glenn Whitefield

Day Phone 503-753-3538 email glenn.whitefield@lewisbuilds.com

Project Building Name / # _____

Project Address or Location 336 SW Morrison

Project Name and Description Remove & existing concrete beam that was cut

Total Project Value $4,500.00 Project Reference #/Billing ID # _____

Building Contractor Lease Crutcher Lewis CCB # 92219

Mechanical Contractor _____ CCB # _____

Electrical Contractor _____ CCB# _____ License # _____

Plumbing Contractor _____ CCB# _____ License # _____

☐ **Building Permit**

No. of Stories _____

Const. Type _____

[Y] [N] Alarms Required

[Y] [N] Smoke Det. Req'd

[Y] [N] Sprinklers Req'd

[Y] [N] Struct. Eng / Calcs Submitted

☐ **Mechanical Permit**

Mechanical Valuation _____

Description _____

City of Portland
APR 15 2011
11-103548-
Permit Number
FS-01-FA

☐ **Electrical Permit**

Please provide a completed standard electrical permit application form. You may mail or deliver it to 1900 SW 4th Avenue, Portland, Oregon 97201 or FAX to 503-623-7425.

☐ **Plumbing Permit**

Number of Fixtures _____

Back Flow Devices _____

Water Service (# of Feet) _____

Medical Gas _____

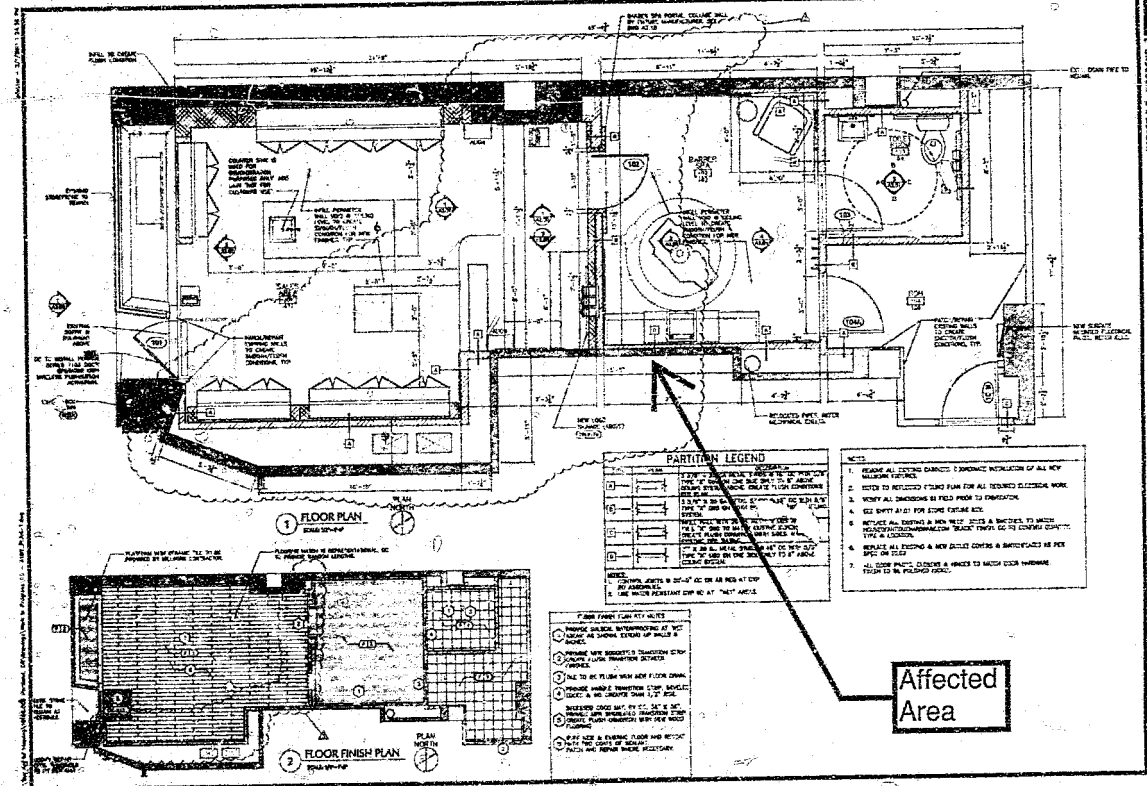
Other _____

B2

MICRO

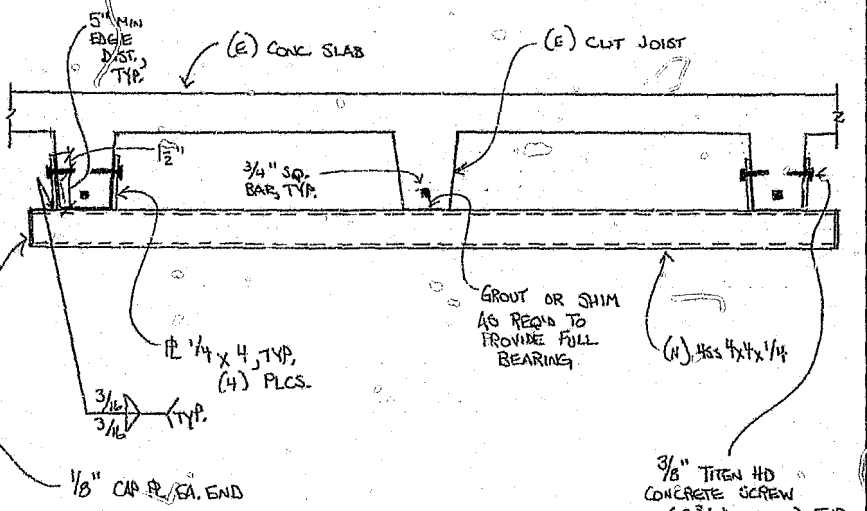
11-103548-0FS-01-FA

MICRO


1 FLOOR PLAN
2 FLOOR FINISH PLAN
PARTITION LEGEND
Affected Area
"ART" SHAVING
A1.00

| | | | |
|---|---|---|---|
| Project: PIONEER PARK | By: AC | Sheet No. | |
| Location: PDX | Date: 3/10/11 | | |
| Client: LCL | Revised | Job No. | |
| | Date | | |

KPFF Consulting Engineers
Portland, Oregon




Note: Protect steel with 2hr. protection.

Level 3/12 S7.01

JOIST HEADER DETAIL

kpff Consulting Engineers

March 29, 2011

Mr. Glenn Whitefield
**Lease Crutcher Lewis**
600 SW 10th Avenue, Suite 310
Portland, OR 97205

RE: Pioneer Park - Joist Header

Dear Glenn:

Attached please find calculation sheets 1 through 9, dated March 10, 2011, which verify the structural adequacy of the joist header detail, as shown on drawing SK-1, dated March 10, 2011. Design is based on the requirements of the 2010 Oregon Structural Specialty Code, based on the 2009 International Building Code.

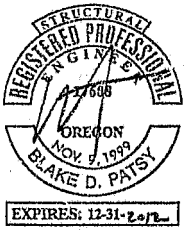
If you have any questions or need further information, please call me.

Sincerely,

Andi Camp

Andi Camp, P.E.

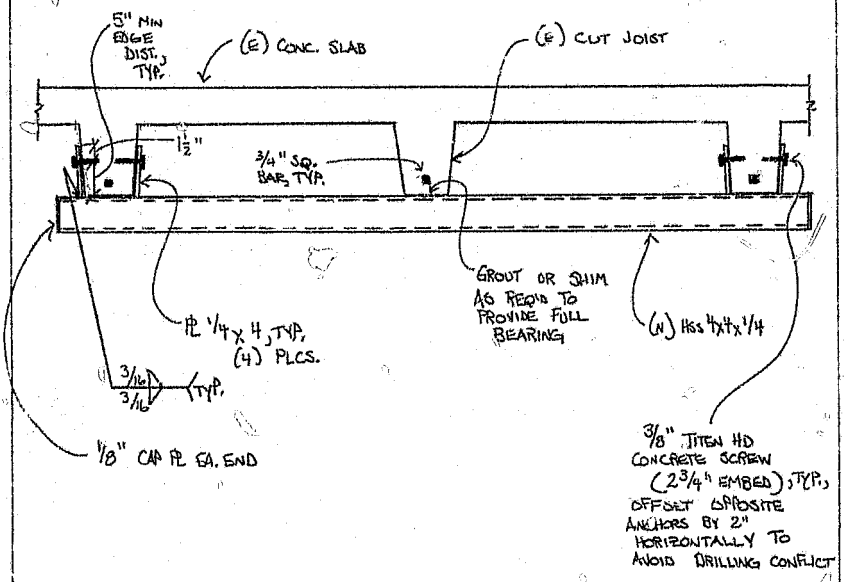
Attachments




[P:\...2\100-calc-3-29-11.docx]

111 SW Fifth Avenue, Suite 2500 Portland, Oregon 97204-3628 (503) 227-3251 Fax (503) 227-7980
Seattle Tacoma Portland Eugene San Francisco Sacramento Los Angeles Irvine San Diego Phoenix Denver St. Louis New York

| | | | |
|---|---|---|---|
| kpff Consulting Engineers<br>Portland, Oregon | Project: PIONEER PARK | By: AC | Sheet No. |
| | Location: PDX | Date: 3/10/11 | |
| | Client: LCL | Revised | Job No. |
| | | Date | |

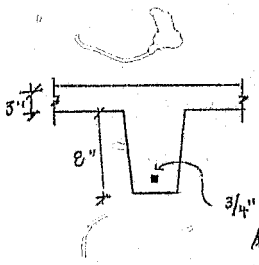



SK-1 JOIST HEADER DETAIL

| kpff Consulting Engineers<br>Portland, Oregon | Project: PIONEER PARK | By: AC | Sheet No.: 1 |
|---|---|---|---|
| | Location: PDX | Date: 3/10/11 | |
| | Client: LCL | Revised: | Job No.: |
| | | Date: | |

(E) JOIST CAPACITY

$$a = \frac{A_s f_y}{.85 f'_c b} = \frac{.5625\,in^2 \cdot 40\,ksi}{.85(2.5\,ksi)(41.75)} < .25\,in$$




$$\phi M_n = .9 \cdot 40\,ksi \left(9.5\,in - \frac{.25}{2}\right) = 337.5\ k\cdot in$$

SELF WT = $\left(\frac{3}{12} \cdot 150\,pcf\right) + \left(8 \cdot 5.5/144 \cdot 150\,pcf\right)/2.5' = 59.8\,psf$

$b = 5.5 + 2 \cdot 8 \cdot \frac{3}{4} = 53.5$

$26.75 + 15 = 41.75$

$1.2(55.8 + 10) + 1.6(100) = 239\,psf \cdot 2.5' = 598\,plf$

SPAN = 15'

$M_u = 598\,plf\,(15')^2/8 = 201.8\ k\cdot in \cdot 1.5 = 302.7\ k\cdot in$

EA. NEIGHBORING JOIST TAKES 50% MORE LOAD DUE TO CUT JOIST

$302.7\ k\cdot in < 337.5\ k\cdot in$ OK.

CHECK SHEAR

$\phi V_c = .75 \cdot 2\sqrt{2500} \cdot (8 \cdot 6 + 3 \cdot 41.75) = 13.0\ k$

$V_u = 598\,plf \cdot 15'/2 + \frac{1}{2} \cdot 598 \cdot 15'/2 = 6.7\ k$

$6.7/13 = .51$ ← 1% OVER 50% NEGLECTING SHEAR REINF. OK

HSS 4x4 TO SPAN 5'

$598\,plf \cdot 15'/2 = 4485\,lbs$

$4485\,lbs \cdot 5'/4 = 5.61\ k\cdot ft = 67.3\ k\cdot in$

REQ'D $Z_x = 67.3\ k\cdot in / .9 \cdot 46\,ksi = 1.63\ in^3$

HSS 4x4x1/4 $Z = 4.67\ in^3$ OK

kpff Consulting Engineers
Portland, Oregon

| | | | |
|---|---|---|---|
| Project | PIONEER PARK | By | AC |
| Location | PDX | Date | 3/10/11 |
| Client | LCL | Revised | |
| | | Date | |

Sheet No. 2
Job No.

ANCHORS TO (E) JOIST

598 lb/ft = 4485 lbs / 2 = 2242.5 lbs
↑ FACTORED REACTION AT CONNECTION

2242.5 / 2 ANCHORS = 1121.25 lbs SHEAR PER ANCHOR

3/8" TITEN HD, 2 3/4" EMBED

SEE ATTACHED SIMPSON OUTPUT

# Anchor Calculations

## Anchor Selector (Version 4.3.0.0)

Job Name : Date/Time : 3/29/2011 9:36:38 AM

**1) Input**

**Calculation Method : ACI 318 Appendix D For Cracked Concrete**

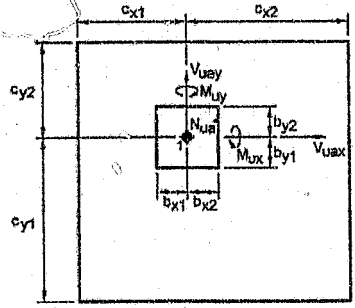
Calculation Type : Analysis

**a) Layout**

Anchor : 3/8" Titen HD Number of Anchors : 1

Embedment Depth : 2.75 in

Built-up Grout Pads : No




1 ANCHOR

*Nua IS POSITIVE FOR TENSION AND NEGATIVE FOR COMPRESSION.

+ INDICATES CENTER OF THE ANCHOR

Anchor Layout Dimensions :

$c_{x1}$ : 18 in

$c_{x2}$ : 18 in

$c_{y1}$ : 5.5 in

$c_{y2}$ : 5 in

$b_{x1}$ : 1.5 in

$b_{x2}$ : 1.5 in

$b_{y1}$ : 1.5 in

$b_{y2}$ : 1.5 in

**b) Base Material**

Concrete : Normal weight — $f'_c$ : 2500.0 psi

Cracked Concrete : Yes — $\Psi_{c,V}$ : 1.00

Condition : B tension and shear — $\phi F_p$ : 1381.3 psi

Thickness, h : 6.5 in

Supplementary edge reinforcement : No

**c) Factored Loads**

Load factor source : ACI 318 Section 9.2

$N_{ua}$ : 0 lb — $V_{uax}$ : 0 lb

$V_{uay}$ : 1121.25 lb — $M_{ux}$ : 0 lb*ft

$M_{uy}$ : 0 lb*ft

$e_x$ : 0 in

$e_y$ : 0 in

Moderate/high seismic risk or intermediate/high design category : Yes

Apply entire shear load at front row for breakout : No

**d) Anchor Parameters**

From [C-SAS-2009] :

Anchor Model = THD37 $d_o$ = 0.375 in

Category = 1 $h_{ef}$ = 1.98 in

$h_{min}$ = 4.167 in $c_{ac}$ = 3 in

$c_{min}$ = 1.75 in $s_{min}$ = 3 in

Ductile = No

**2) Tension Force on Each Individual Anchor**

Anchor #1 $N_{ua1}$ = 0.00 lb

Sum of Anchor Tension $\Sigma N_{ua}$ = 0.00 lb

$a_x$ = 0.00 in

$a_y$ = 0.00 in

$e'_{Nx}$ = 0.00 in

$e'_{Ny}$ = 0.00 in

**3) Shear Force on Each Individual Anchor**

Resultant shear forces in each anchor:

Anchor #1 $V_{ua1}$ = 1121.25 lb ($V_{ua1x}$ = 0.00 lb , $V_{ua1y}$ = 1121.25 lb )

Sum of Anchor Shear $\Sigma V_{uax}$ = 0.00 lb, $\Sigma V_{uay}$ = 1121.25 lb

$e'_{Vx}$ = 0.00 in

$e'_{Vy}$ = 0.00 in

**4) Steel Strength of Anchor in Tension [Sec. D.5.1]**

$N_{sa} = nA_{se}f_{uta}$ [Eq. D-3]

Number of anchors acting in tension, n = 0

$N_{sa}$ = 10890 lb (for a single anchor) [C-SAS-2009]

$\phi$ = 0.65 [D.4.4]

$\phi N_{sa}$ = 7078.50 lb (for a single anchor)

**5) Concrete Breakout Strength of Anchor in Tension [Sec. D.5.2]**

$N_{cb} = A_{Nc}/A_{Nco}\Psi_{ed,N}\Psi_{c,N}\Psi_{cp,N}N_b$ [Eq. D-4]

Number of influencing edges = 0

$h_{ef}$ = 1.98 in

$A_{Nco}$ = 35.28 in$^2$ [Eq. D-6]

$A_{Nc}$ = 35.28 in$^2$

Smallest edge distance, $c_{a,min}$ = 5.00 in

$\Psi_{ed,N}$ = 1.0000 [Eq. D-10 or D-11]

Note: Cracking shall be controlled per D.5.2.6

$\Psi_{c,N}$ = 1.0000 [Sec. D.5.2.6]

$\Psi_{cp,N}$ = 1.0000 [Eq. D-12 or D-13]

$N_b = k_c\sqrt{f'_c}\,h_{ef}^{1.5}$ = 2368.19 lb [Eq. D-7]

$k_c$ = 17 [Sec. D.5.2.6]

$N_{cb}$ = 2368.19 lb [Eq. D-4]

$\phi$ = 0.65 [D.4.4]

$\phi_{seis}$ = 0.75

$\phi N_{cb}$ = 1154.49 lb (for a single anchor)

**6) Pullout Strength of Anchor in Tension [Sec. D.5.3]**

$N_{eq}$ = 1723lb ($f'_c$/2,500 psi)$^{0.5}$ = 1723.00 lb

$\phi$ = 0.65

$\phi_{seis}$ = 0.75

$\phi N_{eq}$ = 839.96 lb (for a single anchor)

**7) Side Face Blowout of Anchor in Tension [Sec. D.5.4]**

Concrete side face blowout strength is only calculated for headed anchors in tension close to an edge, $c_{a1} < 0.4h_{ef}$. Not applicable in this case.

**8) Steel Strength of Anchor in Shear [Sec D.6.1]**

$V_{eq}$ = 2855.00 lb (for a single anchor) [C-SAS-2009]

$\phi$ = 0.60 [D.4.4]

$\phi V_{eq}$ = 1713.00 lb (for a single anchor)

**9) Concrete Breakout Strength of Anchor in Shear [Sec D.6.2]**

Case 1: Anchor checked against total shear load

In x-direction...

$V_{cbx} = A_{vcx}/A_{vcox}\Psi_{ed,V}\Psi_{c,V}V_{bx}$ [Eq. D-21]

$c_{a1}$ = 4.33 in (adjusted for edges per D.6.2.4)

$A_{vcx}$ = 68.25 in$^2$

$A_{vcox}$ = 84.50 in$^2$ [Eq. D-23]

$\Psi_{ed,V}$ = 0.9308 [Eq. D-27 or D-28]

$\Psi_{c,V}$ = 1.0000 [Sec. D.6.2.7]

$V_{bx} = 7(l_e/d_o)^{0.2}\sqrt{}\ d_o\sqrt{}\ f'_c(c_{a1})^{1.5}$ [Eq. D-24]

$l_e$ = 1.98 in

$V_{bx}$ = 2696.77 lb

$V_{cbx}$ = 2027.36 lb [Eq. D-21]

$\phi$ = 0.70

$\phi_{seis}$ = 0.75

$\phi V_{cbx}$ = 1064.37 lb (for a single anchor)

In y-direction...

$V_{cby} = A_{vcy}/A_{vcoy}\Psi_{ed,V}\Psi_{c,V}V_{by}$ [Eq. D-21]

$c_{a1}$ = 5.00 in

$A_{vcy}$ = 97.50 in$^2$

$A_{vcoy}$ = 112.50 in$^2$ [Eq. D-23]

$\Psi_{ed,V}$ = 1.0000 [Eq. D-27 or D-28]

$\Psi_{c,V}$ = 1.0000 [Sec. D.6.2.7]

$V_{by} = 7(l_e/d_o)^{0.2}\sqrt{}\ d_o\sqrt{}\ f'_c(c_{a1})^{1.5}$ [Eq. D-24]

$l_e$ = 1.98 in

$V_{by}$ = 3342.45 lb

$V_{cby}$ = 2896.79 lb [Eq. D-21]

$\phi = 0.70$

$\phi_{seis} = 0.75$

$\phi V_{cby} = 1520.82$ lb (for a single anchor)

Case 2: This case does not apply to single anchor layout

Case 3: Anchor checked for parallel to edge condition

Check anchors at $c_{x1}$ edge

$V_{cbx} = A_{vcx}/A_{vcox} \Psi_{ed,V} \Psi_{c,V} V_{bx}$ [Eq. D-21]

$c_{a1} = 4.33$ in (adjusted for edges per D.6.2.4)

$A_{vcx} = 66.25$ in$^2$

$A_{vcox} = 84.50$ in$^2$ [Eq. D-23]

$\Psi_{ed,V} = 1.0000$ [Sec. D.6.2.1(c)]

$\Psi_{c,V} = 1.0000$ [Sec. D.6.2.7]

$V_{bx} = 7(l_e/d_o)^{0.2}\sqrt{d_o}\sqrt{f'_c}(c_{a1})^{1.5}$ [Eq. D-24]

$l_e = 1.98$ in

$V_{bx} = 2696.77$ lb

$V_{cbx} = 2178.16$ lb [Eq. D-21]

$V_{cby} = 2 * V_{cbx}$ [Sec. D.6.2.1(c)]

$V_{cby} = 4356.32$ lb

$\phi = 0.70$

$\phi_{seis} = 0.75$

$\phi V_{cby} = 2287.07$ lb (for a single anchor)

Check anchors at $c_{y1}$ edge

$V_{cby} = A_{vcy}/A_{vcoy} \Psi_{ed,V} \Psi_{c,V} V_{by}$ [Eq. D-21]

$c_{a1} = 5.50$ in

$A_{vcy} = 107.25$ in$^2$

$A_{vcoy} = 136.13$ in$^2$ [Eq. D-23]

$\Psi_{ed,V} = 1.0000$ [Sec. D.6.2.1(c)]

$\Psi_{c,V} = 1.0000$ [Sec. D.6.2.7]

$V_{by} = 7(l_e/d_o)^{0.2}\sqrt{d_o}\sqrt{f'_c}(c_{a1})^{1.5}$ [Eq. D-24]

$l_e = 1.98$ in

$V_{by} = 3856.16$ lb

$V_{cby}$ = 3038.18 lb [Eq. D-21]

$V_{cbx}$ = 2 * $V_{cby}$ [Sec. D.6.2.1(c)]

$V_{cbx}$ = 6076.37 lb

$\phi$ = 0.70

$\phi_{sels}$ = 0.75

$\phi V_{cbx}$ = 3190.09 lb (for a single anchor)

Check anchors at $c_{x2}$ edge

$V_{cbx} = A_{vcx}/A_{vcox}\Psi_{ed,V}\Psi_{c,V}V_{bx}$ [Eq. D-21]

$c_{a1}$ = 4.33 in (adjusted for edges per D.6.2.4)

$A_{vcx}$ = 68.25 in$^2$

$A_{vcox}$ = 84.50 in$^2$ [Eq. D-23]

$\Psi_{ed,V}$ = 1.0000 [Eq. D-27 or D-28] [Sec. D.6.2.1(c)]

$\Psi_{c,V}$ = 1.0000 [Sec. D.6.2.7]

$V_{bx} = 7(l_e/d_o)^{0.2}\sqrt{d_o}\sqrt{f'_c}(c_{a1})^{1.5}$ [Eq. D-24]

$l_e$ = 1.98 in

$V_{bx}$ = 2696.77 lb

$V_{cbx}$ = 2178.16 lb [Eq. D-21]

$V_{cby}$ = 2 * $V_{cbx}$ [Sec. D.6.2.1(c)]

$V_{cby}$ = 4356.32 lb

$\phi$ = 0.70

$\phi_{sels}$ = 0.75

$\phi V_{cby}$ = 2287.07 lb (for a single anchor)

Check anchors at $c_{y2}$ edge

$V_{cby} = A_{vcy}/A_{vcoy}\Psi_{ed,V}\Psi_{c,V}V_{by}$ [Eq. D-21]

$c_{a1}$ = 5.00 in

$A_{vcy}$ = 97.50 in$^2$

$A_{vcoy}$ = 112.50 in$^2$ [Eq. D-23]

$\Psi_{ed,V}$ = 1.0000 [Sec. D.6.2.1(c)]

$\Psi_{c,V}$ = 1.0000 [Sec. D.6.2.7]

$V_{by} = 7(l_e/d_o)^{0.2}\sqrt{d_o}\sqrt{f'_c}(c_{a1})^{1.5}$ [Eq. D-24]

$l_e$ = 1.98 in

$V_{by}$ = 3342.45 lb

$V_{cby}$ = 2896.79 lb [Eq. D-21]

$V_{cbx}$ = 2 * $V_{cby}$ [Sec. D.6.2.1(c)]

$V_{cbx}$ = 5793.59 lb

$\phi$ = 0.70

$\phi_{seis}$ = 0.75

$\phi V_{cbx}$ = 3041.63 lb (for a single anchor)

**10) Concrete Pryout Strength of Anchor in Shear [Sec. D.6.3]**

$V_{cp}$ = $k_{cp}N_{cb}$ [Eq. D-29]

$k_{cp}$ = 1 [Sec. D.6.3.1]

$N_{cb}$ = 2368.19 lb (from Section (5) of calculations)

$V_{cp}$ = 2368.19 lb

$\phi$ = 0.70 [D.4.4]

$\phi_{seis}$ = 0.75

$\phi V_{cp}$ = 1243.30 lb (for a single anchor)

**11) Check Demand/Capacity Ratios [Sec. D.7]**

Tension

- Steel : 0.0000
- Breakout : 0.0000
- Pullout : 0.0000
- Sideface Blowout : N/A

Shear

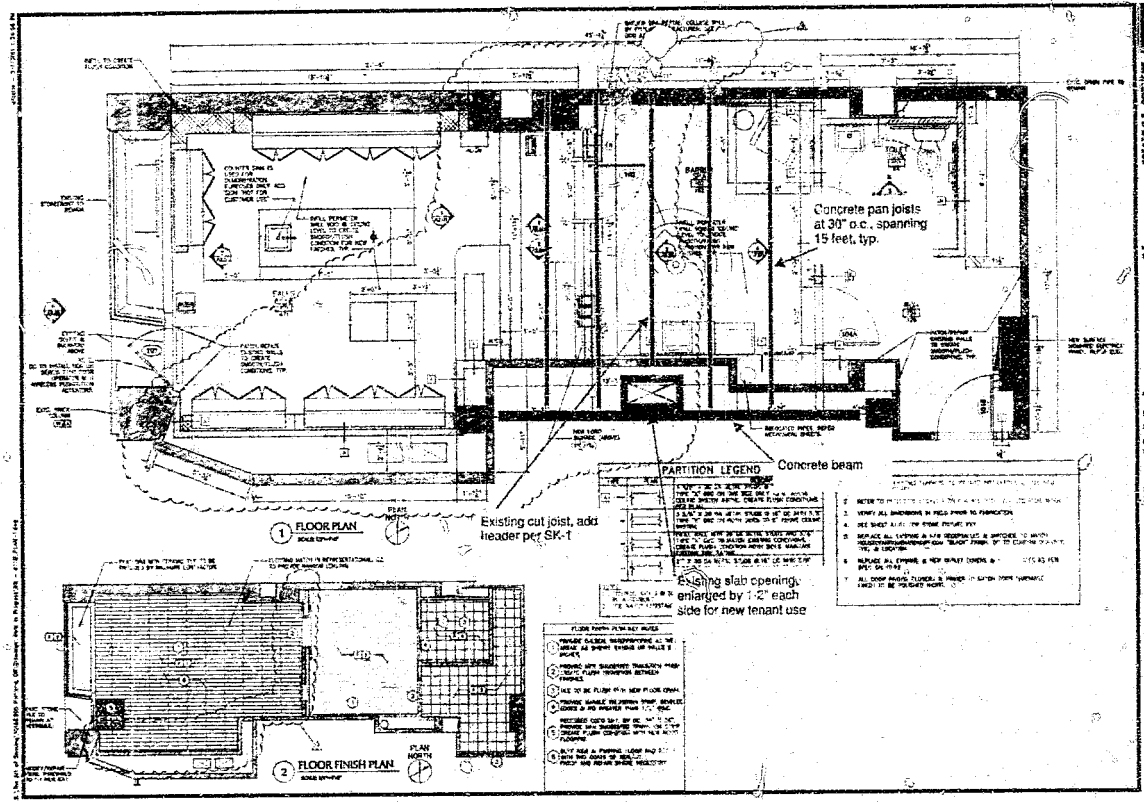
- Steel : 0.6546
- Breakout (case 1) : 0.7373
- Breakout (case 2) : N/A
- Breakout (case 3) : 0.4903
- Pryout : 0.9018

T.Max(0) <= 0.2 and V.Max(0.9) <= 1.0 [Sec D.7.2]

Interaction check: PASS

**Use 3/8" diameter Titen HD anchor(s) with 2.75 in. embedment**

BRITTLE FAILURE GOVERNS: Governing anchor failure mode is brittle failure. Per 2006 IBC Section 1908.1.16, anchors shall be governed by a ductile steel element in structures assigned to Seismic Design Category C, D, E, or F. Alternatively the minimum design strength of the anchor(s) shall be at least 2.5 times the factored forces or the anchor attachment to the structure shall undergo ductile yielding at a load level less than the design strength of the anchor(s). Designer must exercise own judgement to determine if this design is suitable.





ART OF SHAVING

719 SW MORRISON ST.
PORTLAND, OR 97205


A1.00

11-103548 DFS 01 FA

CITY OF PORTLAND, OREGON – BUREAU OF DEVELOPMENT SERVICES

1900 SW Fourth Avenue, Suite 5000 • Portland, Oregon 97201 • www.portlandonline.com/bds

# Structural Special Inspection and Observation Program Checksheet

The registered design professional in responsible charge shall prepare and submit a special inspection and structural observation program in accordance with IBC Sections 1704.1.1, 1705.2, and 106.3.4.1, and confirm that the special inspection and structural observations noted below are indicated on the drawings

*~ Please Note that separate Soils and Life Safety Inspection Checksheets may also be required ~*

**Instructions – Part D and E of this Checksheet must be fully completed by the Owner (or Architect or Engineer acting as the owner's agent) in order to obtain your permit.** When complete, return this form to BDS Permitting Services. You may return it in person at 1900 SW 4th Ave, by fax to (503) 823-7425, or by email to fppspecialinspectionschecksheets@portlandoregon.gov

| | | | |
|---|---|---|---|
| **Application #** | 11-103548-DFS-01-FA | **Date:** | April 4, 2011 |
| **Project Name:** | BW-DFS-01 SHAVING 3035228:Pioneer Pk :Pioneer PK | | |
| **Site Address:** | 715 SW MORRISON ST | | |
| **Architect of Record (Firm)** | None | **Phone #** | N/A |
| **Engineer of Record (Firm)** | KPFF | **Phone #** | 503-227-3251 |

The following special inspections and structural observations shall be performed according to the State Building Code and City of Portland Special Inspection Program Administrative Rules unless a program of inspections is submitted by the Engineer of Record and approved by Bureau of Development Services.

**A. REQUIRED SPECIAL INSPECTIONS FOR ALL BUILDING TYPES**

| | | | |
|---|---|---|---|
| [X] Steel Construction | [ ] Concrete Construction | [ ] Anchors – Adhesive | [ ] Wood Construction |
| [ ] Curtainwall | [ ] Prestressed Concrete | [ ] Anchors – Cast-in-place | [ ] Masonry |
| [ ] Structural Silicone Glazing | [ ] Shotcrete | [ ] Anchors – Expansion | [ ] Cold Formed Steel Framing |
| Special Cases: | [X] Screw Anchors | [ ] | [ ] |

**B. REQUIRED SPECIAL INSPECTIONS FOR CATEGORY III AND IV BUILDINGS (In addition to those noted above.)**

| | | | |
|---|---|---|---|
| [ ] Seismic Force Resist. System | [ ] Storage Racks | [ ] Access Floors | [ ] Suspended Ceilings |
| [ ] Mechanical Components | [ ] Electrical Components | [ ] Cladding | [ ] Veneer |
| [ ] Nonbearing Walls | [ ] Seismic Isolation System | [ ] | [ ] |
| Special Cases: | [ ] | [ ] | [ ] |

**C. STRUCTURAL OBSERVATION**

[ ] Required (The stages of construction at which structural observation is to occur shall be indicated on the drawings.)

**D. APPROVED SPECIAL INSPECTOR OR INSPECTION AGENCY (To be completed by the applicant.)**

Indicate the City approved special inspector or special inspection agency to perform the required special inspections noted in parts A. and B. above:

| | | |
|---|---|---|
| [✓] Carlson Testing (503) 684-3460 | [ ] Northwest Geotech (503) 682-1880 | [ ] PSI (503) 289-1778 |
| [ ] Mayes Testing, Inc. (503) 281-7515 | [ ] Clair Company (800) 383-8855 | [ ] Krazan & Assoc. (503) 665-3574 |
| [ ] ACS (503) 443-3799 | [ ] | [ ] Other: |

**E. To be completed by the applicant.**

By completing Part E the project owner (or the Architect or Engineer acting as the Owner's Agent) hereby agrees to employ the special inspector or inspection agency and/or engineer of record for the above noted special inspections and/or structural observations. ***(Contractors are NOT authorized to agree for the Owner.)***

Print Name Christopher M Kopca (Project Owner or the Architect or Engineer acting as the Owner's Agent) Date 4/7/11

Firm Portland Public Schools/Construction Management Phone 503-819-4480

The project owner shall provide a copy of this checksheet to the special inspections agency and engineer of record.

Plans Examiner: Eric Thomas

APPLICANT – COMPLETE PARTS D & E