

3730 SW CHEHALEM AVE
REV-01 RS08-177529

RS.08.177529 REV.01.

P

DEC 19 2008
MICROFILMED

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CITY OF
PORTLAND, OREGON
BUREAU OF DEVELOPMENT SERVICES
1900 SW 4th Ave., Suite 5000
Portland, OR 97201



RESIDENTIAL 1 & 2 FAMILY PERMIT

08-177529-REV-01-RS

Site Address: 3730 SW CHEHALEM AVE

Issued: 12/16/08

PROJECT INFORMATION		Occ. Group	Const. Type
Single Family Dwelling	Alteration	R-3	V-B
Project Description: ADJUST LOCATION OF DOOR			
APPLICANT		Phone (503) 684-5611	
PROPERTY OWNER		Phone	
CONTRACTOR		Phone	
INTERSTATE ROOFING INC			
C'IRISTINE L RUBENSTEIN			
INTERSTATE ROOFING INC			

Project Details		Project Details	
Code Edition	2008 ORSC	Construction Type 4	
Final adj to ICC Value reason - Display	Calculated via Tables	GIS Update Flag	11/13/08
Valuation at Issuance	5109	Water District	Portland Water Bure
Zoning Enforcement Agency	Portland		

PAID
DEC 16 2008
CITY OF PORTLAND

This permit expires if, at any time, 180 days pass without an approved inspection. If you are not able to obtain an inspection approval within 180 days, you may request a one-time only extension of 180 days by calling 503-823-7388.

**BEFORE
YOU DIG**

ATTENTION: Oregon law requires you to follow rules adopted by the Oregon Utility Notification Center. These rules are set forth in OAR 922-001-0010 through OAR 922-001-0090. You may obtain copies of the rules by calling the center. (Note: the telephone number for the Oregon Utility Notification Center is 1-800-332-2344).

CITY CONTACT

E-Mail:

Phone:

Fax: (503) 823-4172

**INSPECTION REQUEST
PHONE NUMBERS**

Building/Trade Inspections - Call Before 6:00 AM:

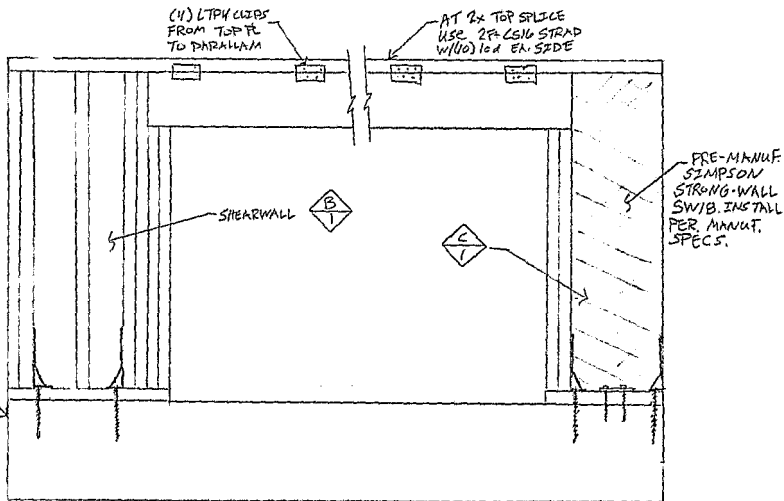
(503) 823-7000

TDD: (503) 823-6868

**IVR Inspection Request
Number:**

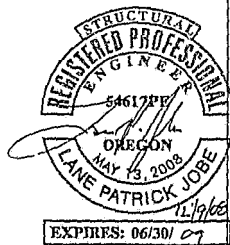
2809373

(C) 13'6" x 17'6" FIG.



ELEVATION

NTS



MILLER
CONSULTING
ENGINEERS

9576 SW Barber Blvd
Suite One Hundred
Portland, OR 97219-5412

(503) 246-1250
Fax: 246-1305

Project Name Rubenstein Door Replacement Project # 080781
Location 3730 SW Chehalis Ave., Portland
Client Interstate Roofing
By EMH CK'd LPT Date 12/8/08 Page 1 of 1

STRUCTURAL NOTES

1. THE CONTRACTOR IS RESPONSIBLE FOR VERIFICATION AND CORRELATION OF ALL ITEMS AND WORK NECESSARY FOR COMPLETION OF THE PROJECT AS REQUIRED BY THE CONTRACT DOCUMENTS. THE CONTRACTOR SHOULD ANTICIPATE ANY QUESTIONS ARISING FROM THE CONTRACT DOCUMENTS OR SITE CONDITIONS. THE CONTRACTOR SHALL REQUEST INTERPRETATION AND CLARIFICATION FROM THE ENGINEER BEFORE BEGINNING THE PROJECT. THE ENGINEER'S RESPONSE TO SUCH REQUEST SHALL BE IN WRITING. THE CONTRACTOR HAS REVIEWED AND FAMILIARIZED HIMSELF WITH ALL ASPECTS OF THE PROJECT AND HAS COMPLETED CONSTRUCTION UNDER THE ASPECTS OF THE WORK SHALL CONFORM TO THE STATE OF OREGON 2007 EDITION STRUCTURAL SPECIALTY CODE, EFFECTIVE DATE APRIL 1, 2007, BASED ON THE 2005 INTERNATIONAL BUILDING CODE. THE CONTRACTOR SHALL BE RESPONSIBLE FOR CONFORMANCE TO ALL SAFETY REGULATIONS DURING CONSTRUCTION.

2. THE CONTRACT DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE, UNLESS OTHERWISE SPECIFICALLY NOTED. THEY DO NOT INDICATE THE METHOD OF CONSTRUCTION OR CONSTRUCTION LOADS. ONLY THE CONTRACTOR SHALL PROVIDE ALL METHODS, DIRECTION AND RELATED EQUIPMENT NECESSARY TO PROTECT THE STRUCTURE, WORKMEN AND OTHER PERSONS AND PROPERTY DURING CONSTRUCTION. THE CONTRACTOR SHALL AT HIS OWN EXPENSE, EMPLOY PROPERLY QUALIFIED PERSONS TO DETERMINE WHERE AND HOW TEMPORARY PRECAUTIONARY MEASURES SHALL BE USED AND INSPECT SAME IN THE FIELD. ANY MATERIALS NOT AS SPECIFIED OR IMPROPER MATERIAL, INSTALLATION OR WORKMANSHIP SHALL BE REMOVED AND REPLACED WITH SPECIFIED MATERIAL IN A WORKMANLIKE MANNER AT THE CONTRACTOR'S EXPENSE.

3. THESE PLANS, SPECIFICATIONS, ENGINEERING AND DESIGN WORK ARE INTENDED SOLELY FOR THE PROJECT SPECIFIED HEREIN. MILLER CONSULTING ENGINEERS DISCLAIMS ALL LIABILITY FOR THE DESIGN AND SPECIFICATIONS OR THE DESIGN, ADVISE AND INSTRUCTIONS ATTENDANT THEREON ARE USED ON ANY PROJECT OR AT ANY LOCATION OTHER THAN THE PROJECT AND LOCATION SPECIFIED HEREIN. OBSERVATION VISITS TO THE JOB SITE AND SPECIAL INSPECTIONS ARE NOT PART OF THE STRUCTURAL ENGINEER'S RESPONSIBILITY UNLESS THE CONTRACT DOCUMENTS SPECIFY OTHERWISE.

4. CONTRACTOR SHALL VERIFY SOIL CONDITIONS AT THE FOOTINGS AND MAKE ANY NECESSARY CORRECTIONS TO PLACE THEM ON FIRM NATIVE SOIL OR STRUCTURAL FILL COMPACTED TO 95% OF MAXIMUM DENSITY AT OPTIMUM MOISTURE CONTENT. AMERICAN SOCIETY FOR TESTING AND MATERIALS, ASTM D698 (STANDARD PROCTOR), OR ASTM D1557 (MODIFIED PROCTOR). THE COMPACTOR SHALL BE VERIFIED BY A QUALIFIED INSPECTOR APPROVED BY THE BUILDING OFFICIAL. COMPACTED STRUCTURAL FILL FOR DEPTHS GREATER THAN 12 INCHES SHALL COMPLY WITH PROVISIONS OF AN APPROVED GEOTECHNICAL REPORT ASSUMED SOIL BEARING PRESSURE 1500 POUNDS PER SQUARE FOOT (PSF).

5. DESIGN LOADS: LIVE LOAD REDUCTION FOR BEAMS AND COLUMNS WAS NOT USED. THE FOLLOWING ARE THE DESIGN REQUIREMENTS:

- A) DEAD LOADS: ROOF = 15 PSF
- WALL = 8 PSF
- B) ROOF LIVE LOAD: SNOW LOAD CONTROLS DESIGN
ROOF SNOW LOAD: PG = 20 PSF
RF = 17 PSF (MINIMUM 25 PSF ROOF SNOW)
SE = 1.0
IS = 1.0
CT = 1.0
- C) WIND DESIGN: BASIC WIND SPEED (FASTEST MILE) = 80 MPH
W = 1.0
EXPOSURE = B
PER IRC 1609.11.2
- D) SEISMIC DESIGN: IE = 1.0
SEISMIC USE GROUP = I
SS = 0.085, SI = 0.348
SDS = 0.73
SEISMIC DESIGN CATEGORY = D
RESISTING SYSTEM = LIGHT FRAME SHEARWALLS
CS = 1.0
PROCEDURE = ASCE 7 EQUIVALENT LATERAL FORCE

6. ALL STRUCTURAL WOOD COLUMNS AND BEAMS TO BE DOUGLAS FIR/LARCH (DF/L), #1 UNLESS NOTED OTHERWISE. ALL JOISTS, PURLINS, AND GIRTS TO BE DF/L #2 AND BETTER UNLESS NOTED OTHERWISE. ALL BLOCKING AND NON-STRUCTURAL FRAMING TO BE CONSTRUCTION GRADE AND BETTER. ALL WOOD PLATES IN CONTACT WITH CONCRETE OR MASONRY SHALL BE HEU-FIR #2 PRESURE TREATED UNLESS NOTED OTHERWISE. METAL TUBER CONNECTORS AND HANGERS SHALL BE FULLY BOLTED AND/OR NAILED. ALL CONNECTORS, HANGERS AND FASTENERS IN CONTACT WITH PRESURE TREATED WOOD SHALL BE GALVANIZED OR EQUIVALENTLY PROTECTED PER IRC 2304.9.5. ALL PLATE TO BE ANCHORED TO FOUNDATION WITH 1/2" DIA. TIE ANCHOR BOLTS AT 4'-0" O.C. MAXIMUM UNLESS NOTED OTHERWISE. SWISSON PRE-FABRICATED METAL TUBER CONNECTORS NOTED. OTHER TYPES OF METAL CONNECTORS REQUIRE PRIOR REVIEW. ALL BOLT HEADS OR NUTS BEARING ON WOOD TO HAVE STANDARD WASHERS. BOLT HOLES IN WOOD SHALL BE 1/32" TO 1/16" LARGER THAN THE BOLT, DEPENDING ON BOLT SIZE. ALL LAG BOLTS SHALL HAVE 1/16" HOLES AS PER SECTION 11.1.3 OF THE NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION. ALL SHEATHING SHALL BE APA RATED GROUP 1, [STRUCTURAL], EXPOSURE 1 IN COMPLIANCE WITH VOLUNTARY PRODUCT STANDARDS DOCUMENT P51 AND DOCUMENT P52.

- 15/32" THICK SHEATHING TO BE C-D WITH SPAN RATING OF 32/16
- 19/32" SHEATHING TO BE C-D WITH SPAN RATING OF 40/20
- 3/4" SHEATHING TO BE C-D WITH SPAN RATING OF 40/24
- 3/4" TONGUE AND GROOVE (TAG) STUD-FLOOR WITH SUPPORTS AT 24" O.C. EXTERIOR GRADE UNDER SLABS, INTERIOR GRADE WHEN PROTECTED FROM MOISTURE AND TO BE GROUTED AND NAILED
- 1 1/8" TAG STUD-FLOOR WITH SUPPORTS AT 48" O.C. INTERIOR AND EXTERIOR G.L. GLUE ALL FLOORS WITH STRUCTURAL ADHESIVE, 3M-5200 OR EQUIVALENT, AT ALL SUPPORTS

ALL SHEATHING WALLS AND/OR SHEAR WALLS TO HAVE 2X BLOCKING AT PANEL EDGES UNLESS NOTED OTHERWISE. SEE NAILING SCHEDULE.

7. ALL STRUCTURAL WOOD MEMBERS EXPOSED TO WEATHER OR AS NOTED ON DRAWINGS, OR AS REQUIRED BY IRC SECTION 2303.1.8, SHALL BE PRESURE TREATED IN ACCORDANCE WITH AMERICAN WOOD PRESERVATIVE ASSOCIATION (AWPA) STANDARDS FOR WOOD PRESERVATIVE (ACQ, ACA, ACQ-A, CCA, AOC, ETC.) OR BORON PRESERVATIVE (CUPROX NAPHENATE) WITH A RETENTION NOT LESS THAN 0.4 LBS./CU. FT. PER AWPA STANDARD OR PRESERVATIVE TREATMENT SHALL CONFORM TO THE REQUIREMENTS AND APPLICATION STANDARDS LISTED AND MA FOR THE SPECIES, PRODUCT, PRESERVATIVE AND END USE. ALL GLUED MEMBERS THAT ARE TO BE TREATED SHALL BE CONSTRUCTED OF FREELY AVAILABLE LVL OR LVL AND STANDARD C24 ALL TRIMMED SECTIONS, CUTS, ENDS, OR HOLES IN PRESURE TREATED MATERIALS SHALL BE TREATED WITH COPPER NAPHENATE, IN ACCORDANCE WITH AWPA STANDARD OR PER ADDITIONAL REQUIREMENTS, SEE IRC SECTION 2303.1.8 FOR PRECAUTIONARY AND TREATMENT.

8. ALL STRUCTURAL MEMBERS EXPOSED TO WEATHER AND SURFACED TO DECAY, WHICH ARE MANUFACTURED BY THIS FIRM, INC., SHALL BE TREATED WITH PERMA-SPR 8-B WOOD TREATMENT. THE PRESERVATIVE CARRIER SHALL CONFORM TO AWPA STANDARD OR THE ACTIVE INGREDIENT CARRIER 8-B QUINACRYLATES SHALL CONFORM TO AWPA STANDARD OR THE TREATING PROCEDURES AND QUALITY CONTROL PRACTICES SHALL BE AS SPECIFIED IN THE AWPA STANDARDS M2 AND M3. HOWEVER, PERMA-SPR 8-B IS NOT RECOMMENDED TO BE USED FOR GROUND CONTACT APPLICATIONS.

10. ALL WOOD TRUSSES SHALL COMPLY WITH IRC SECTION 2303.4, EXCEPT AS OTHERWISE NOTED. TRUSSES SHALL NOT EXCEED A TOTAL LOAD DEFLECTION OF 1/360 TRUSS MANUFACTURER SHALL FURNISH COMPLETE ENGINEERING SHOP DRAWINGS WITH AN OREGON REGISTERED PROFESSIONAL ENGINEER'S SEAL. SHOP DRAWINGS SHALL BE PROVIDED WITH THE SHIPMENT OF TRUSSES DELIVERED TO THE JOB SITE, AND SHALL INCLUDE, AT A MINIMUM, THE INFORMATION SPECIFIED IN IRC SECTION 2303.4.1.1. EACH MANUFACTURER OF TRUSSES USING METAL PLATE CONNECTORS SHALL RETAIN AN APPROVED AGENCY TO MAKE UNSCHEDULED INSPECTIONS OF ALL TRUSS MANUFACTURING AND DELIVERY OPERATIONS. SHOW CLEARLY ALL BRACING FOR TRUSS COMPRESSION MEMBERS AND REQUIRED CONNECTIONS. ALL CONNECTION PLATES SHALL DEVELOP THE FULL STRESS OF THE WOOD MEMBER. THE TRUSS MANUFACTURER SHALL OBTAIN CERTIFICATES FROM AN INDEPENDENT TESTING COMPANY THAT ALL TRUSSES DELIVERED TO THE JOB SITE CONFORM TO APPROVED SHOP DRAWINGS. COST OF ALL TEST CERTIFICATES SHALL BE BORNE BY THE TRUSS MANUFACTURER.

11. ALL NAILS SHALL BE COMMON AND NAILING SHALL BE PER THE NAILING SCHEDULE UNLESS OTHERWISE NOTED ON THE DRAWINGS. THE FOLLOWING NAIL SIZES SHALL BE USED UNLESS NOTED OTHERWISE:

- 6D NAIL: 0.113 INCH DIA. X 2 INCHES LONG WITH MIN. HEAD DIA. 17/64 IN.
8D NAIL: 0.131 INCH DIA. X 2 1/2 INCHES LONG WITH MIN. HEAD DIA. 9/32 IN.
10D NAIL: 0.148 INCH DIA. X 3 INCHES LONG WITH MIN. HEAD DIA. 5/16 IN.
12D NAIL: 0.148 INCH DIA. X 3 1/2 INCHES LONG WITH MIN. HEAD DIA. 5/16 IN.
12D NAIL: 0.162 INCH DIA. X 3 1/2 INCHES LONG WITH MIN. HEAD DIA. 11/32 IN.

STAPLE OF EQUIVALENT VALUE MAY BE SUBSTITUTED AFTER REVIEW BY ENGINEER. NAILS AND STAPLES SHALL NOT BE OVERDRIVEN.

NAILING SCHEDULE

- LUMBER
- EDITION (DOT) PLATE (PL) TO DECKING/JOISTS, 16D AT 8" O.C. FACE NAIL
- DOT PL/ TOP PL TO STUDS, END NAIL (2) 16D AT 2X4, (3) 10D AT 2X6
- BUILT-UP/DOUBLE (DBL) STUDS, FACE NAIL 10D AT 8" O.C. STAGGERED
- DBL TOP PL, FACE NAIL 10D AT 12" O.C. STAGGERED
- DBL TOP PL SPLICES, FACE NAIL (14) 10D EA. SIDE OF JOINT
- DBL TOP PL CORNERS/INTERSECTIONS, (4) 10D STAGGERED FACE NAIL
- CONTINUOUS HEADER(S) (HRS.) TO STUDS, (4) 10D TOE NAIL
- TOP PL TO HRS., FACE NAIL 16D AT 16" O.C.
- JOIST TO DBL TOP PL/SILL PL, (4) 10D TOE NAIL
- BLOCKING TO STUDS, TOE NAIL (2) 10D
- STUDS TO BLOCKING, END NAIL (3) 16D
- JOISTS TO HRS. END NAIL (6) 16D AT 4X6/4X8 (8) 16D AT 4X10/4X12
- HRS. TO TRIPPLE STUDS, TOE NAIL (4) 10D
- ROOF TRUSS BOT CHORD TO DBL TOP PL/SIMPSON H2-5
- NAIL-LAMINATED BEAMS, FACE NAIL 16D AT 8" O.C. STAGGERED W/ (3) 16D AT EA. END
- BLOCKING TO TRUSSES/JOISTS, TOE NAIL (4) 10D
- JOISTS LAP SPLICES OVER DBL TOP PL (4) 10D FACE NAIL
- END/DIAG JOIST TO JOISTS, END NAIL (3) 16D

- SAW LUMBER DECKING
- 2 X 6 TONGUE AND GROOVE (2) 16D GALV. FACE NAILS AND (1) 10D GALV. TOE NAIL EA. COURSE AT EA. SUPPORT

CLADDING

- ROOF
- 8D AT 6" O.C. AT ALL SUPPORTED PANEL EDGES AND ROOF EDGES AND 12" O.C. AT ALL INTERMEDIATE SUPPORTS.
- FLOORS
- 10D GALV. AT 6" O.C. AT ALL EDGES AND INTERMEDIATE SUPPORTS.
- WALLS
- 8D AT 6" O.C. AT ALL PANEL EDGES AND AT 12 O.C. AT ALL INTERMEDIATE FRAMING MEMBERS.

GYPSUM WALLBOARD

- TOP AT 1/2" GYP - 50 COOLER OR WALLBOARD NAILS AT 7" O.C. AT ALL FRAMING MEMBERS.
- TOP AT 5/8" GYP - 60 COOLER OR WALLBOARD NAILS AT 7" O.C. AT ALL FRAMING MEMBERS.
- TOP AT (2) LAYS 5/8" GYP - 60 COOLER OR WALLBOARD NAILS AT 9" O.C. BASE PLY AND 8D COOLER AT 7" O.C. FACE PLY.

12. PLUMBING, FIRE SUPPRESSION, ELECTRICAL, MECHANICAL, LAND USE, SITE PLANNING AND EROSION CONTROL, ARE BEYOND THE SCOPE OF THESE DRAWINGS AND ARE PROVIDED BY OTHERS.

13. SPECIAL INSPECTION IS REQUIRED IN ACCORDANCE WITH IRC SECTION 1105 ON THE ITEMS AS NOTED IN THE MATRIX ON SHEET 50.01. THE CONTRACTOR SHALL PROVIDE SUFFICIENT NOTICE TO ALLOW SCHEDULING OF SPECIAL INSPECTION. IT IS THE OWNER'S RESPONSIBILITY TO PROVIDE PRIOR INSPECTION BY A QUALIFIED THIRD PARTY, SUCH AS A TESTING AGENCY REVIEWED BY THE ENGINEER.

14. WHEREVER SHORING IS REQUIRED, LOCATE THE SYSTEM CLEAR WITHOUT OBSTRUCTION OF THE PERMANENT STRUCTURE AND TO PERMIT CONSTRUCTION TO PROCEED. PROVIDE SHORING SYSTEM ADEQUATELY ANCHORED AND BRACED TO RESIST EARTH AND HYDROSTATIC PRESSURES.

TABLE 2 REQUIRED STRUCTURAL SPECIAL INSPECTIONS				
SYSTEM OF MATERIAL	IRC CODE REFERENCE	INSPECTION		REMARKS
		CODE IN STANDARD	INSPECTION	
			Continual Periodic	
POST-INSTALLATION CONCISE AND ANCHORS				
INSTALLATION OF HARDBOARD CONCRETE AND COMPLETED MASONRY	1703.42 1704.13	ICC EVALUATION REPORT	X	SPECIAL INSPECTIONS APPLY TO ANCHOR PRODUCT NAME, TYPE AND ANCHORING HOLE DIMENSIONS. COMPLIANCE WITH DIAL, BT REQUIREMENTS. EVIDENCE OF PROPER EMBEDMENT AND ANCHORING. EXPIRATION DATE, ANCHOR/ANCHORING INSTALLATION, MOISTURE EMBELEMMENT AND TIGHTENING.



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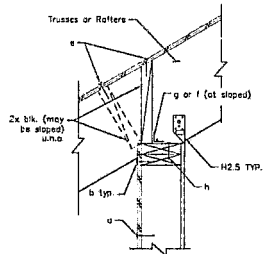
Rubenstein Shear Wall
Interstate Roofing

3730 SW Chehalam Ave
Portland, OR

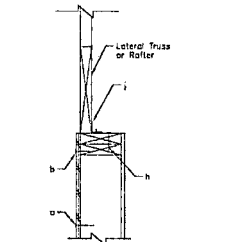
STRUCTURAL NOTES

SHEET CONT'D
JOB No. 080781
DRAWN EMH
DATE 12-08-08
REVISIONS
SHEET 50.01

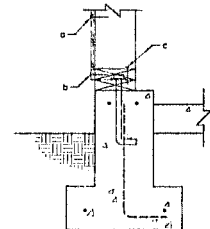
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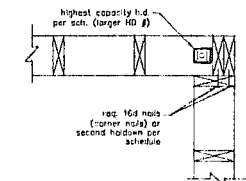
1 ROOF to WALL
joists perp. NTS



2 ROOF to WALL
joists para. NTS

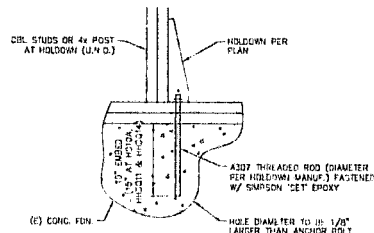


3 SHEAR WALL
NTS



4 CORNER HOLDOWN
REQUIREMENT NTS

NOTES:
1. All plywood edges to be blocked. Fed not at 12" o.c.
2. Use (1) Pressure Treated 2x or 3x mem. for No. 2 for bottom plate to concrete foundation.
3. Anchor bolt length based upon minimum embedment of 2"
4. Plywood must nail to both double top plates.
5. All new exterior walls to be type A, unless noted otherwise.
6. Framing at adjoining panel edges sh. be 3 in nominal or wider, and nails shall be staggered where nails are spaced 2 inches - center. Plywood joint and s. plate nailing shall be staggered.
7. Where panels are applied on both faces of a wall and nailing is less than 6 inches o.c. on either side, panel joints shall be offset to fall on different framing members OR framing shall be 3 in inches nominal or thicker and no. 4 on each side shall be staggered. Plywood joint and sill plate nailing shall be staggered.
8. Any fastener exposed to weather shall be galvanized.
9. Holdown occurs at end of ea. shearwall and fasten to min (2) 2x studs or 4x stud unless noted otherwise. Wall sheathing shall be edge nailed to holdown studs.
10. Top wall plates minimum 4'-0" between splices w/ (14) 10d ea. side.
11. Simpson products shall be installed as per manuf. specs.
12. Equivalent holdown, straps, bolts, etc., by other manuf. to be reviewed by engineer of record prior to substitution.
13. Sheathing on shearwalls shall not be interrupted by any wall butting into shearwall.
14. n. Seismic Design Category D, E or F, all framing members receiving edge nailing from abutting panels shall not be less than a single 3-inch nominal member. Plywood joint and sill plate nailing shall be staggered in all cases.
15. Use min. 6x stud ea. end of shear panel for holdown.
16. (2) Anchor bolts min. per plate.
17. Fasteners for preservative-treated and fire-retardant-treated wood shall be of hot dipped zinc-coated galvanized steel and stainless steel.
18. ALL NAILS TO BE COMMON OR GALVANIZED BOX NAILS unless alternate approved by engineer of record.
19. Assume 3/4" thick diaphragm.
20. At anchor bolts, typ. 1/4 x 3 x 0'-3" plate washer between sill plate and nut.
21. At anchor bolts, typ. 1/4 x 3 x 0'-3" plate washer between sill plate and nut.



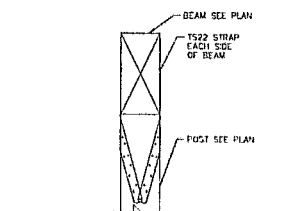
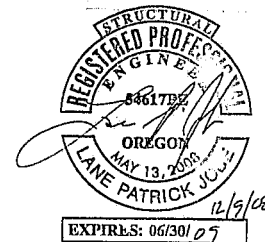
5 RETROFIT HOLDOWN ANCHOR
NTS

WALL TYPE	WALL MATERIAL OR WOOD STUDS	PANEL EDGE NAILING	SOIL PL TO NEW JOIST
15/32" APA Rated Sheathing	15/32" APA Rated Sheathing	6d at 6" o.c.	16d at 6" o.c.
DIAPHRAGM PRE-ENGINEERED SHEAR WALL SYSTEM		N/A	N/A

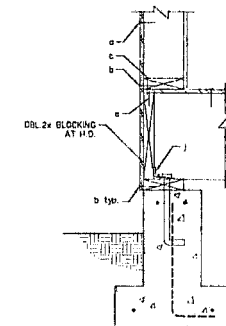
LEGEND
DISCONTINUED SHEAR PANEL AND HOLDOWN WHERE "SP" IS THE PANEL TYPE AND "HD" IS THE HOLDOWN TYPE

COMMON NAIL SIZES			
Nail Type	Min. Dia.	Min. Length	Min. Head Dia.
6d	0.113"	2"	17/32 in.
6d	0.131"	2 1/2"	7/32 in.
10d	0.148"	3"	5/16 in.
12d	0.148"	3 1/2"	5/16 in.
16d	0.162"	3 1/2"	11/32 in.

HOLDOWN SCHEDULE # 1, 2			
Holdown Type	Holdown	Fasteners	Corner Note, see corner holdown det (see schedule)
	PH02	(10) 50s 1/4" x 3" Screws and 1/4" x 3" W/Compression on Ecopy Bolted 10" Min. core	(17) 10d Nails



6 TYP. BEAM TO COL. CONN.
NTS



7 EXT. WALL - FLOOR - FDN
NTS

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Rubenstein Shear Wall
Interstate Roofing

3730 SW Chisholm Ave
Portland, OR

SHEET CONTENT
SHEAR WALL
SCHEDULE

JOB No.
080781

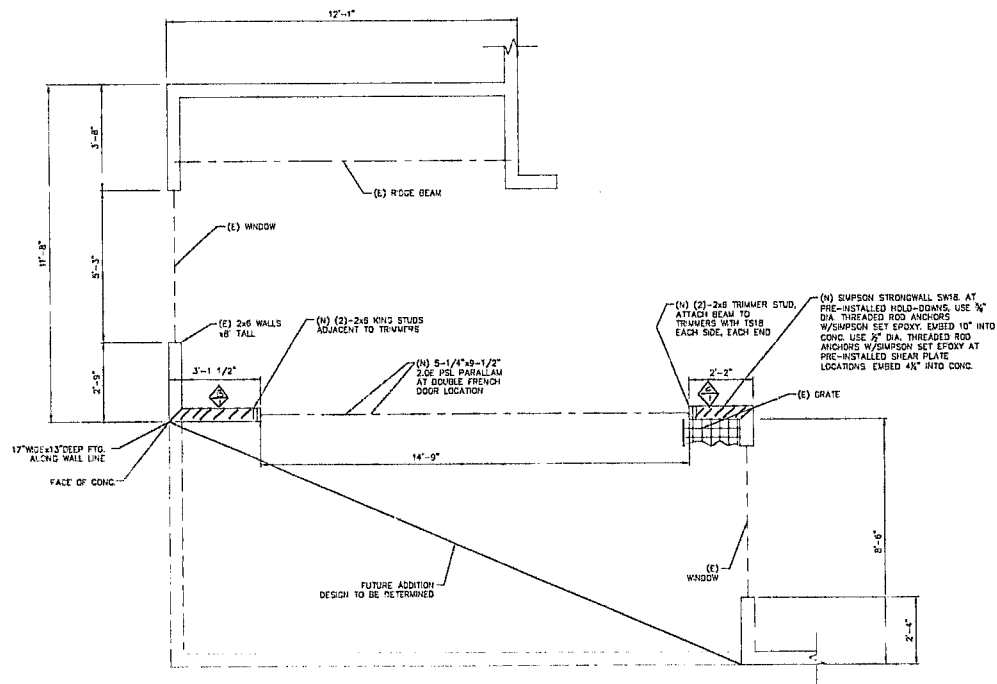
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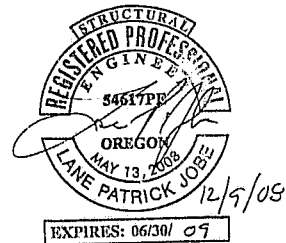
DATE
12-08-08

REVISIONS

SHEET
S1.01



1 PARTIAL PLAN
SCALE: 1/2" = 1'-0"



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Rubenstein Shear Wall
Interstate Roofing

3730 SW Chehalis Ave
Portland, OR

SHEET CONTENT
SHEAR WALL
SCHEDULE

JOB NO. 080781

DRAWN EMH CHECKED

DATE 12-08-08
REVISIONS

SHEET S2.01