

4440 SW HUBER ST. REV-01 CO07-113708

C0.07.113708REV.01.

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MAY 21 2007
MICROFILMED

45



CITY OF
PORTLAND, OREGON
BUREAU OF DEVELOPMENT SERVICES
1900 SW 4th Ave., Suite 5000
Portland, OR 97201



COMMERCIAL BUILDING PERMIT

07-113708-REV-01-CO

Site Address: 4440 SW HUBER ST
MC DONALDS

Issued: 3/20/07

PROJECT INFORMATION		Occ. Group	Const. Type
Business	Addition		
Project Description: 03-20-07 REVISE ENGINEERING			

<u>APPLICANT</u>	I-5 DESIGN & MANUFACTURE *SARA KIRK*	Phone (800) 459-2967
<u>PROPERTY OWNER</u>	TENGOLSKI PARTNERS	Phone
<u>CONTRACTOR</u>	I-5 DESIGN & MANUFACTURE *SARA KIRK*	Phone

Project Details		Project Details	
Code Edition (Year)	2003 IBC	Lot Area (Sq. Ft.)	28314
Water District	PORTLAND WATER	Zoning - Property (1)	CG

This permit expires if, at any time, 180 days pass without an approved inspection. If you are not able to obtain an inspection approval within 180 days, you may request a one-time only extension of 180 days by calling 503-823-7303.

**BEFORE
YOU DIG**

ATTENTION: Oregon law requires you to follow rules adopted by the Oregon Utility Notification Center. Those rules are set forth in OAR 952-001-0010 through OAR 952-001-0090. You may obtain copies of the rules by calling the center. (Note: the telephone number for the Oregon Utility Notification Center is 1-800-332-2344).

CITY CONTACT

PROCESS MANAGEMENT

Phone: (503) 823-7357

E-Mail:

Fax: (503) 823-4172

**INSPECTION REQUEST
PHONE NUMBERS**

Building/Trade Inspections - Call Before 6:00 AM:

(503) 823-7000

TDD: (503) 823-6868

**IVR Inspection Request
Number:**

2642067



STRUCTURAL TECHNOLOGY CONSULTANTS INC.

1821 CONVOY COURT, SUITE 406, SAN DIEGO, CA 92111

PHONE (658) 218-2400

FAX (658) 218-2424

PROJECT: MCDONALD'S, 10050 SW Barbur, Portland, OR

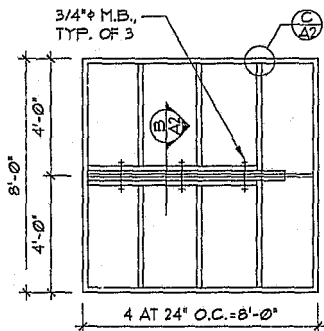
PROJ. NO.: 1110- 69

DESIGNER: LR

SHEET: A1 OF A11

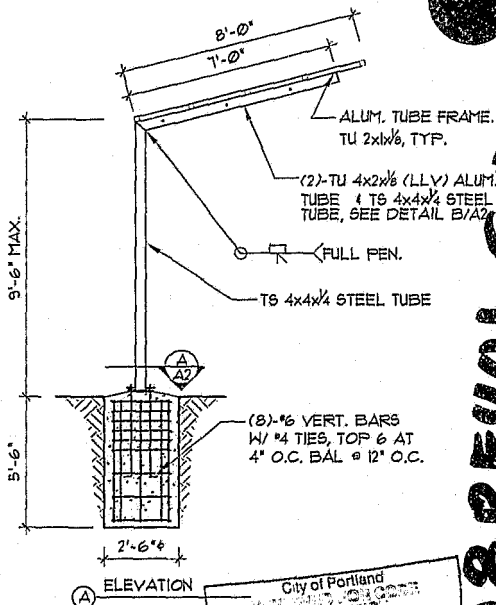
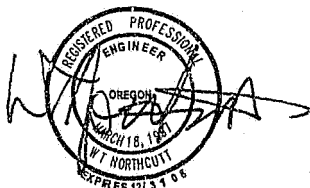
DATE: 03-07-07

CLIENT: I-5 DESIGN & MANUFACTURE



(B) CANOPY PLAN VIEW

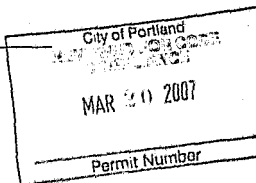
MAR 07 2007



(A) ELEVATION

GENERAL NOTES FOR POLES AND FOOTING:

1. CONCRETE $f'_c=2500$ PSI. SPECIAL INSPECTION NOT REQUIRED.
2. TUBE STEEL ASTM A500 GRADE B.
3. ALUMINUM MEMBERS 6061-T6.
4. SIGN CABINETS SHALL BE FABRICATED IN THE SHOP OF AN APPROVED FABRICATOR.
5. ALL WELDING SHALL BE PERFORMED BY QUALIFIED WELDERS. WELDS USING E70 ELECTRODES OR WIRES AND AWS APPROVED SYSTEMS AND PROCEDURES ARE ACCEPTABLE. NO FIELD WELDING SHALL BE NECESSARY.
6. ANCHOR BOLTS ASTM F1554.
7. STAINLESS STEEL (S.S.) ASTM F593, $F_y=50$ KSI, MIN.
8. REINFORCING STEEL ASTM A615, GRADE 60.
9. PROVIDE 3" MIN. CLEAR COVER ON ALL STEEL EMBEDDED IN CONCRETE.
10. PROVIDE ISOLATION OF DISSIMILAR MATERIALS.
11. PROVIDE SLOPE AWAY FROM BASE OF POLE.
12. SOIL PASSIVE PRESSURE BASED ON 2003 IBC TABLE 1804.2 CLASS 5 OR BETTER. SPECIAL INSPECTION NOT REQUIRED.



REVISED

01-31-07

07-113708 Rev 01 CO



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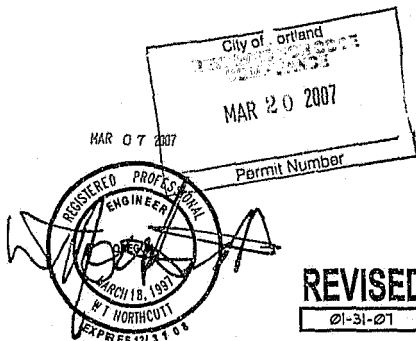
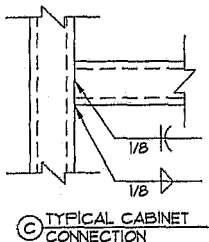
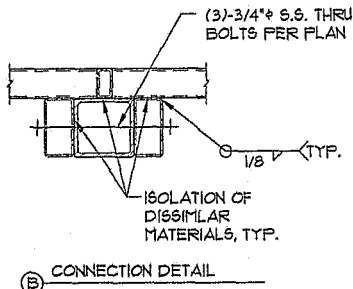
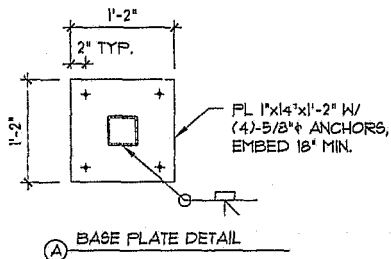
PROJ. NO.: 1110-69

DESIGNER: LR

SHEET: A2 OF A11

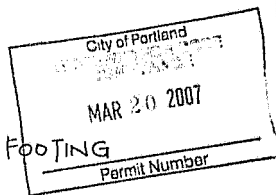
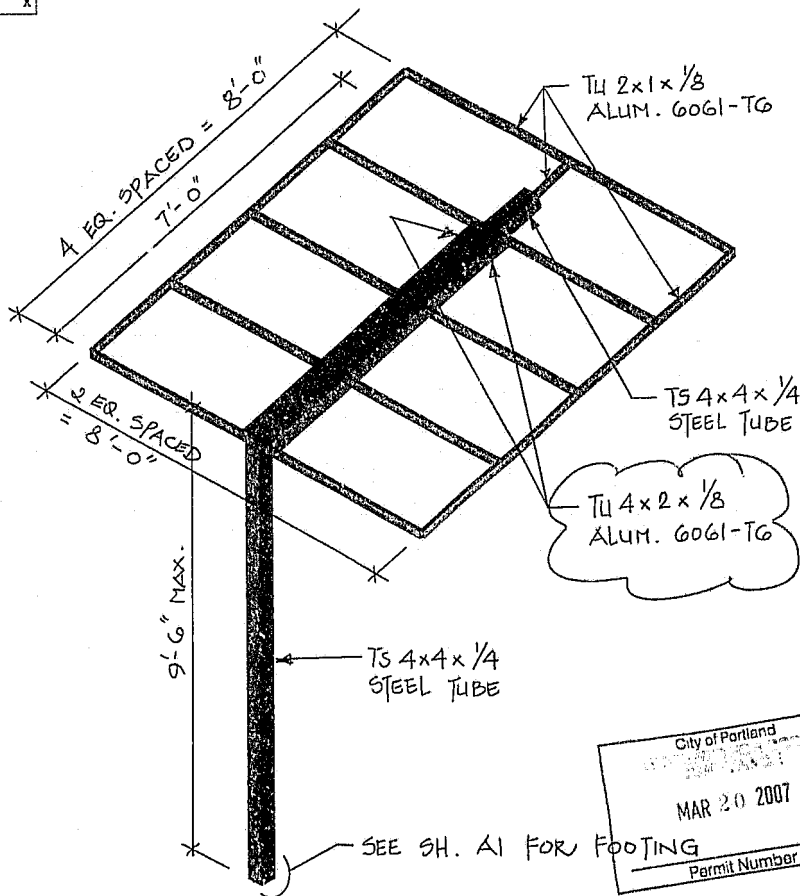
CLIENT: I-5 DESIGN & MANUFACTURE

DATE: 03-07-07





A3 OF A11



REVISED
01-31-07

Loads: BLC 1, gravity

I-5 DESIGN

LR

1110-61A.5

MCDONALD'S

Jan 18, 2007 at 7:43 AM

1110-60A.4.R3D



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PROJECT: MCDONALD'S, 10050 SW Barbur Blvd., Portland, OR

PROJ. NO.: 1110-69

DESIGNER: LR

SHEET: A4 of All

CLIENT: I-5 DESIGN & MANUFACTURE

DATE: 03-07-07

WIND-02

Wind Load Design per ASCE 7-02

Description: MCDONALD'S - PORTLAND, OR

Analysis by: LR

Structure Type	Other
Basic Wind Speed (V)	100 mph
Structural Category	II
Exposure	C
Struc Nat Frequency (n1)	1.1 Hz
Kd (Directionality Factor)	0.85
Overall Structure Ht (Ht)	15.00 ft
Width Perp. To Wind Dir (B)	11.00 ft
Width Paral. To Wind Dir (L)	8.00 ft
Damping Ratio (beta)	0.05

Red values should be changed only through "Main Menu"

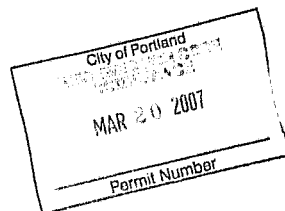
Type of Structure	
Height/Least Horizontal Dim	1.88
Flexible Structure	No

Importance Factor	1
Non-Hurricane, Hurricane (v=65-100 mph) & Alaska	
Table C6-4 Values	
Alpha =	0.500
Zg =	900.000
At =	0.105
Bt =	1.000
Am =	0.154
Bm =	0.650
Cc =	0.200
I =	500.00 ft
Epsilon =	0.200
Zmin =	15.00 ft

Gust Factor Category I: Rigid Structures - Simplified Method		
Gust1	For rigid structures (Nat Freq > 1 Hz) use 0.85	0.85
Gust Factor Category II: Rigid Structures - Complete Analysis		
Zm	Zmin	15.00 ft
Izm	$Ce * (33/2)^{0.167}$	0.2281
Lzm	$I(zm/33)^{0.5}$	427.06 ft
Q	$(1/(1+0.63*((Min(B,L)+Ht)/Lzm)^{0.63}))^{0.5}$	0.9535
Gust2	$0.925*((1+1.7*Izm^{3.4}*Q)/(1+1.7*3.4*Izm))$	0.9005
Gust Factor Summary		
G	Since this is not a flexible structure the lesser of Gust1 or Gust2 are used	0.85

Design Wind Pressure - Open Buildings and Other Structures

Elev. ft	Kz	Kzt	Qz lb/ft²	Wind Pressure based upon different Cf's (lb/ft²)					
				1.00	Cf 1.20	w 1.30	Cf*w 1.56	0.00	0.00
15	0.85	1.00	18.47	15.70	18.84	20.41	24.49	0.00	0.00





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PROJECT: MCDONALD'S, 10050 SW Barbur Blvd., Portland, OR

SHEET: AS OF A.J.I

PROJ. NO.: 1110-69

DESIGNER: LR

DATE: 03-07-07

CLIENT: I-5 DESIGN & MANUFACTURE

CODE: 2003 IBC
WIND: Basic Wind = 100
Exposure = C
Wind governs

Design Wind Pressures (psf):

0 to 15' 24.49

AREA	FORCE	ARM	MOMENT
59.6	1460	5.00	7299

60	1460	7299
----	------	------

FOOTING

SOIL PRESSURE = $100 \times 2 \times 1.33 =$

267 PSF/FT

(IBC TABLE 1804.2 VALUE 2×1.33 , PER 1804.3.1 & FOOTNOTED)

P = 1460 LBS H = 5.00 FEET

B = 2.50 FEET

$S1 = S \times D / 3 = 491.54$

$A = 2.34 \times P / (S1 \times B) =$

2.78

$D = .5 \times \Lambda (1 + (1 + 4.36 \times H / A) \wedge .5) =$

5.52 FEET

FORMULA PER IBC SECTION 1805.7.2.1

2' - 6" DIAM x 5' - 6" DEEP

COLUMN

TUBE STEEL - ASTM A500, GRADE B

S = $7299 \times 12 / (1.13 \times 24000) = 3.2 @ 0.0 \text{ FT}$

4.0" TUBE w = 9.4 #/ft t = 0.188"

ALT. USE TS 4x4x1/4" OK

CHECK MAX. WIND LOADING:

$F_{max} = 24.49 \text{ PSF IN ANY DIRECTION}$

$F_y = 24.49 \text{ PSF}$

$F_x = 3/12 \times 24.49 = 6.12 \text{ PSF}$

$M_x = 6.12 \text{ PSF} \times 2' \times 8' \times 10.5' = 1029 \text{ '}$

$M_y = 24.49 \text{ PSF} \times 8' \times 8' \times 4' = 6269 \text{ '}$

$M_{tot} = 7299 \text{ ' (USED IN CALCS)}$

CHECK MOMENT @ JOINT:

$M_x = 6.12 \text{ PSF} \times 2' \times 8' \times 1' = 98 \text{ '}$

$M_y = 6269 \text{ '}$

$M_{tot} = 7299 \text{ '}$

CHECK ARM CANTILEVER:

$M_x = 6.12 \text{ PSF} \times 5' \times 8' \times 5' = 12.24 \text{ '}$

$M_y = 24.49 \text{ PSF} \times 2' \times 8' \times 1' = 392 \text{ '}$

$S_{req} = (392 + 12.24 \times 12) / (1.33 \times 1000) = 0.332 \text{ in3}$

$S_f = 0.426 \text{ in3 (TU } 2 \times 1/2 \text{ LONG LEG VERTICAL)}$

CHECK SNOW LOADING:

$S \times 8' \times 25\% \times 1' = 1600 \text{ '}$

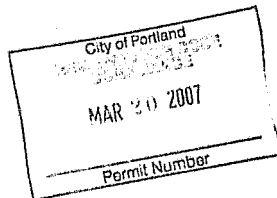
ARM = $4' \times 1600 \text{ '} = 6400 \text{ '}$ MOMENT AT CONN.

CHECK 1" BOLTS IN DOUBLE SHEAR:

MAX. SHEAR = $6400 \text{ '} \times 12' / 5' = 15360 \text{ '}$

MAX. ALLOWABLE SHEAR = 15100' EA.

ALT: FULL PEN. WELD AROUND STEEL TUBE



REVISED

01-18-07



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PROJECT: MCDONALD'S, 10050 SW Barbur Blvd., Portland, OR
PROJ. NO.: 1110- 69
CLIENT: I-5 DESIGN & MANUFACTURE

DESIGNER: LR

SHEET: A6 OF A11

DATE: 03-07-07

BASE PLATE

INPUT

M	W	EDGE
		DIST
7299	600	2
Fy	F'c	
36000	2500	

REVISED

12-21-06

BOLT TENSION

$$T = 12 \cdot M / .875d = 12 \cdot 7299 / .875 \cdot 12.00 = 8342 \text{ LB}$$

* PLATE WIDTH, MINIMUM

$$B = 2 \cdot (T + W) / .35 \cdot 1.33 \cdot F'c \cdot .375 \cdot d = 2 \cdot (8342 + 600) / .35 \cdot 1.33 \cdot 2500 \cdot .375 \cdot 12.00 = 3.41 \text{ IN. PLATE WIDTH USED} = 14.00 \text{ IN.}$$

ACTING BEARING STRESS

$$fb = 2 \cdot (T + W) / B \cdot .375 \cdot d = 2 \cdot (8342 + 600) / 14.00 \cdot .375 \cdot 12.00 = 284 \text{ PSI}$$

PLATE CANTILEVER

$$m = (\text{DEPTH} - 0.95 \cdot D) / 2 = (14.00 - 0.95 \cdot 4.00) / 2 = 5.10 \text{ IN}$$

PLATE BENDING DUE TO BOLT TENSION

$$M = T \cdot (m - \text{EDGE DIST}) / B = 8342 \cdot (5.10 - 2.00) / 14.00 = 1847 \text{ IN LB}$$

PLATE BENDING DUE TO BEARING

$$M = (T + W) \cdot (m - .125d) / B = (8342 + 600) \cdot (5.10 - .125 \cdot 12.00) / 14.00 = 2299 \text{ IN LB}$$

PLATE THICKNESS

$$t = (6 \cdot M / 1.33 \cdot F)^{.5} = (6 \cdot 2299 / 1.33 \cdot 27000)^{.5} = 0.62 \text{ IN}$$

BOLTS

$$A = T / 1.33 F_t \text{ No.} = 8342 / 1.33 \cdot 20000 \cdot 2 = 0.157 \text{ SQ IN}$$

WELD

$$\begin{aligned} & 4.00 \text{ INCH TUBE} \\ Z &= BD + D^2 / 8 = 4.00^2 \cdot 4.00 + 4.00^2 / 8 = 21.33 \text{ IN}^2 \\ f_v &= 12M / Z = 12 \cdot 7299 / 21.33 = 4105 \text{ LB/IN} \end{aligned}$$

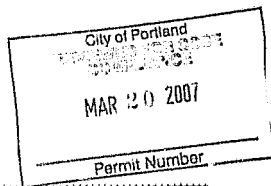
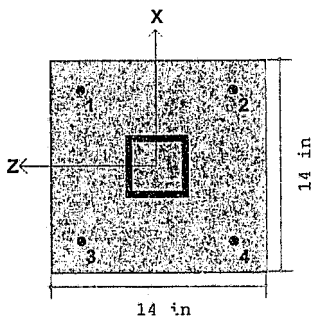


Plate 5/8" x 14.00 x 14.00
4 - 1/2" x 10" Bolts

ALT. USE 1" THICK PLATE MIN.
WITH 4-5/8" DIA. BOLTS, 18" MIN. EMBED

3/8" FILLET WELD
ALL AROUND

ALT. USE BUTT WELD ALL AROUND

**Plain Base Plate Connection**

Base Plate Thickness : 1. in
 Base Plate F_y : 36. ksi
 Bearing Surface F_p : 1.75 ksi
 Anchor Bolt Diameter : .625 in
 Anchor Bolt Material : A307
 Anchor Bolt F_u : 58. ksi
 Column Shape : TU4X4X4
 Design Code : AISC ASD 9th

Bearing Pressure

Maximum Bearing : .899 ksi
 Max/Allowable Ratio : .385 ASCE EQ.3(W)
 (ABIF = 1.333)



.899
 (ksi)
 0.

Base Plate Stress

Maximum Stress : 17.413 ksi
 Max/Allowable Ratio : .484 ASCE EQ.3(W)
 (ASIF = 1.333)



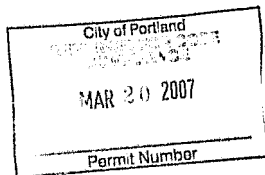
17.413
 (ksi)
 0.

Anchor Bolts

Bolt	X (in)	Z (in)	Tens.(k)	Vx (k)	Vz (k)	Ft (ksi)	Fv (ksi)	Unity	Combination
1	5.	5.	4.601	0.	.365	26.66	13.33	.563	ASCE EQ.3(W)
2	5.	-5.	4.601	0.	.365	26.66	13.33	.563	ASCE EQ.3(W)
3	-5.	5.	4.601	0.	-.365	26.66	13.33	.563	ASCE EQ.3(W)
4	-5.	-5.	4.601	0.	-.365	26.66	13.33	.563	ASCE EQ.3(W)

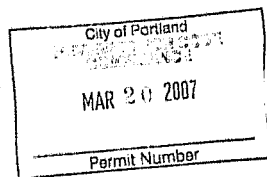
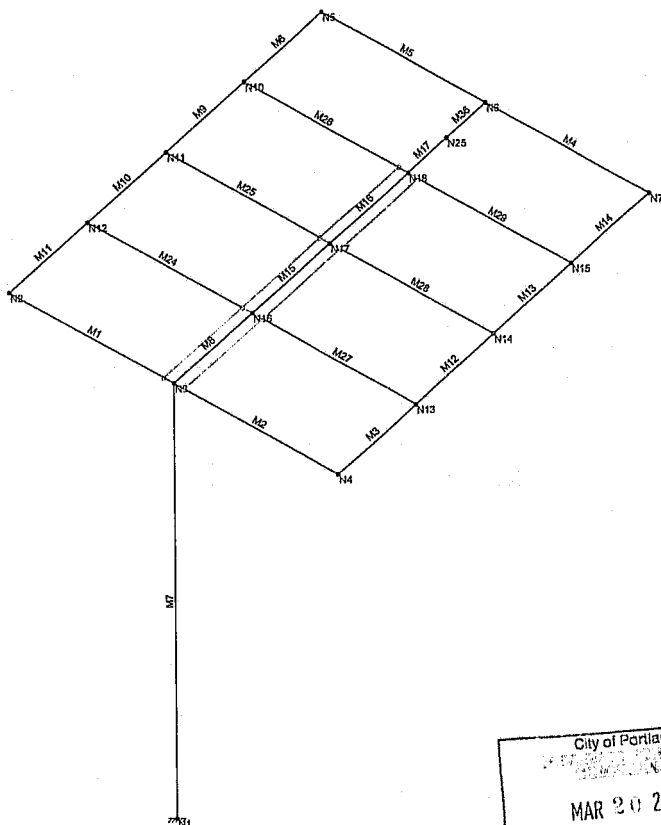
Loads

	P (k)	Vx (k)	Vz (k)	Mx (k-ft)	Mz (k-ft)	Reverse
DL	.6					No
WL			1.46		7.299	Yes





X 8 OF A11



Loads: BLC 1, gravity

I-5 DESIGN	MCDONALD'S	
LR		Jan 18, 2007 at 7:41 AM
1110-61A.5		1110-60A.4.R3D

Company : I-5 DESIGN
 Designer : LR
 Job Number : 1110-61A.5

MCDONALD'S

Jan 31, 2007
 2:26 PM
 Checked By: _____

A9 OF A11

Global

Display Sections for Member Calcs	5
Max Internal Sections for Member Calcs	97
Include Shear Deformation	Yes
Include Warping	Yes
Area Load Mesh (in^2)	144
Merge Tolerance (in)	.12
P-Delta Analysis Tolerance	0.50%
Vertical Axis	Y

Hot Rolled Steel Code	AISC: ASD 9th
Cold Formed Steel Code	AISI 99: ASD
Wood Code	NDS 91/97: ASD
Wood Temperature	< 100F
Concrete Code	ACI 2002

Number of Shear Regions	4
Region Spacing Increment (in)	4
Bi-axial Column Method	PCA Load Contour
Parma Beta Factor (PCA)	.65
Concrete Stress Block	Rectangular
Use Cracked Sections	Yes
Bad Framing Warnings	No
Unused Force Warnings	Yes

General Material Properties

	Label	E [ksi]	G [ksi]	Nu	Therm (1/E5 F)	Density (k/ft^3)
1	gen Conc3NW	3155	1372	.15	.6	.145
2	gen Conc4NW	3644	1584	.15	.6	.145
3	gen Conc3LV	2085	906	.15	.6	.11
4	gen Conc4LV	2408	1047	.15	.6	.11
5	gen Alum	10600	4077	.3	1.29	.173
6	gen Steel	29000	11154	.3	.65	.49
7	RIGID	1e+7		0	0	0

Hot Rolled Steel Properties

	Label	E [ksi]	G [ksi]	Nu	Therm (1/E5 F)	Density (k/ft^3)	Yield [ksi]
1	A36 Gr.36	29000	11154	.3	.65	.49	36
2	A572 Gr.50	29000	11154	.3	.65	.49	50
3	A992	29000	11154	.3	.65	.49	50
4	A500 Gr.42	29000	11154	.3	.65	.49	42
5	A500 Gr.46	29000	11154	.3	.65	.49	46
6	6061-T6	10600	4077	.3	1.29	.173	16

Hot Rolled Steel Section Sets

	Label	Shape	Type	Design List	Material	Design Rules	A [in2]	Iyy [in4]	Izz [in4]	J [in4]
1	TS4x4	TU4X4X4	Beam	Tube	A500 Gr.46	Typical	3.59	8.22	8.22	13.5
2	TS4x2	TU4X2X2	Beam	Tube	6061-T6	Typical	1.4	9.54	2.82	2
3	TS2x1	TS2x1x2	Beam	Tube	6061-T6	Typical	.688	.05	.332	.215

Joint Boundary Conditions

	Joint Label	X [k/in]	Y [k/in]	Z [k/in]	X Rot [k-ft/rad]	Y Rot [k-ft/rad]	Z Rot [k-ft/rad]	Forcing
1	N1	Reaction	Reaction	Reaction	Reaction	Reaction	Reaction	Forcing

City of Portland
 Permit Number

Company : I-5 DESIGN
 Designer : LR
 Job Number : 1110-61A.5

MCDONALD'S

Jan 31, 2007

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Checked By: _____

10 of 11

Member Area Loads (BLC 2 : snow)

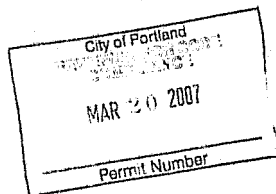
	Joint A	Joint B	Joint C	Joint D	Direction	Distribution	Magnitude[psf]
1	N2	N4	N7	N5	Y	Two Way	-25

Basic Load Cases

BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distributed Area (Member)	Surface (sq ft)
1	gravity	DL	-1					1
2	snow	SL						

Load Combinations

Description	Sol.	PD	SR	BLC Factor	BLC Factor	BLC Factor	BLC Factor	BLC Factor	BLC Factor	BLC Factor	BLC Factor	D ⁺ Factor
1	gravity+snow	Yes		1	1	2	1					



Company : I-5 DESIGN
 Designer : LR
 Job Number : 1110-61A.5

MCDONALD'S

Jan 31, 2007

2:27 PM

Checked By: _____

All of All

Joint Reactions (By Combination)

LC	Joint Label	X (lb)	Y (lb)	Z (lb)	MX (k-ft)	MY (k-ft)	MZ (k-ft)
1	1	N1	1868.826	0	6.718	0.05	0.027
2	1	Totals:	0	1868.826	0		
3	1	COG (ft):	X: 4.015	Y: 10.236	Z: -3.595		

Member AISC ASD Steel Code Checks (By Combination)

LC	Member	Shape	UC Max	Loc(ft)	Shear U ₁	Loc(ft)	Dir	F _a (ksi)	F _t (ksi)	F _{by} (ksi)	F _{bz} (ksi)	C _b	C _{my}	C _{mz}	E _{qn}
1	M31	TS2x1x2	733	25	0.82	0	z	9.6	9.6	10.56	10.56	1.003	971	85	H2-1
2	M30	TS2x1x2	718	0	0.82	0	z	9.6	9.6	10.56	10.56	1.003	972	85	H2-1
3	M7	TU4x4x4	667	9.6	0.00	0	z	27.6	27.6	30.36	30.36	1	1	1	H1-3
4	M33	TS2x1x2	515	25	0.70	0	z	9.6	9.6	10.56	10.56	1	994	85	H2-1
5	M32	TS2x1x2	508	0	0.70	0	z	9.6	9.6	10.56	10.56	1	997	85	H2-1
6	M8	TU4x4x4	414	0	0.02	0	y	27.6	27.6	30.36	30.36	1.004	533	85	H1-2
7	M26	TS2x1x2	404	0	0.25	0	y	9.6	9.6	10.56	10.56	1.011	852	85	H2-1
8	M29	TS2x1x2	401	0	0.24	0	y	9.6	9.6	10.56	10.56	1.037	864	85	H2-1
9	M36	TS2x1x2	395	0	0.55	0	y	9.6	9.6	10.56	10.56	1.046	6	85	H1-2
10	M28	TS2x1x2	384	0	0.35	0	y	9.6	9.6	10.56	10.56	1.041	821	85	H2-1
11	M25	TS2x1x2	384	0	0.36	0	y	9.6	9.6	10.56	10.56	1.015	809	85	H2-1
12	M2	TS2x1x2	371	25	0.19	25	y	9.6	9.6	10.56	10.56	1	902	85	H1-2
13	M27	TS2x1x2	366	0	0.28	0	y	9.6	9.6	10.56	10.56	1.043	808	85	H2-1
14	M24	TS2x1x2	362	0	0.33	0	y	9.6	9.6	10.56	10.56	1.015	792	85	H2-1
15	M1	TS2x1x2	339	4	1.05	3.75	y	9.6	9.6	10.56	10.56	1	912	85	H1-2
16	M19	TU4x2x2	327	2	0.38	2	y	9.6	9.6	10.56	10.56	1.003	458	85	H2-1
17	M18	TU4x2x2	325	2	0.38	0	y	9.6	9.6	10.56	10.56	1.002	487	85	H2-1
18	M4	TS2x1x2	298	0	0.30	0	y	9.6	9.6	10.56	10.56	1.003	761	85	H1-2
19	M20	TU4x2x2	291	0	0.23	0	y	9.6	9.6	10.56	10.56	1.003	399	85	H1-2
20	M22	TU4x2x2	290	2	0.24	2	y	9.6	9.6	10.56	10.56	1.002	382	85	H2-1
21	M5	TS2x1x2	289	0	0.29	0	y	9.6	9.6	10.56	10.56	1.046	788	85	H1-2
22	M35	TS2x1x2	226	0	0.90	0	z	9.6	9.6	10.56	10.56	1.02	745	85	H2-1
23	M34	TS2x1x2	222	25	0.90	0	z	9.6	9.6	10.56	10.56	1.021	729	85	H2-1
24	M15	TU4x4x4	174	0	0.01	0	y	27.6	27.6	30.36	30.36	1.001	6	85	H1-2
25	M13	TS2x1x2	114	0	0.10	0	y	9.6	9.6	10.56	10.56	1.042	201	85	H1-2
26	M12	TS2x1x2	105	0	0.10	0	y	9.6	9.6	10.56	10.56	1.048	211	85	H1-2
27	M9	TS2x1x2	101	2	0.05	0	z	9.6	9.6	10.56	10.56	1.039	203	85	H1-2
28	M10	TS2x1x2	093	2	0.09	0	z	9.6	9.6	10.56	10.56	1.043	211	85	H1-2
29	M16	TU4x4x4	092	0	0.01	0	y	27.6	27.6	30.36	30.36	1.005	6	85	H1-2
30	M3	TS2x1x2	070	0	0.35	0	y	9.6	9.6	10.56	10.56	1.109	233	85	H1-2
31	M14	TS2x1x2	067	0	0.33	0	y	9.6	9.6	10.56	10.56	1.124	248	85	H1-2
32	M6	TS2x1x2	067	0	0.30	0	y	9.6	9.6	10.56	10.56	1.144	226	85	H2-1
33	M11	TS2x1x2	061	2	0.33	0	z	9.6	9.6	10.56	10.56	1.121	23	85	H1-2
34	M17	TU4x4x4	031	0	0.08	0	y	27.6	27.6	30.36	30.36	1.407	6	85	H1-2
35	M23	TU4x2x2	011	1.354	0.31	2	y	9.6	9.6	10.56	10.56	1	438	85	H2-1
36	M21	TU4x2x2	011	1.333	0.31	0	y	9.6	9.6	10.56	10.56	1	429	85	H2-1

STRESS CHECK : Max = 0.733 < 1.0 OK

