Co.04.006836 REV.01.

W

32

CITY OF

PORTLAND, OREGON

BUREAU OF DEVELOPMENT SERVICES
1900 SW 4th Ave, Suite 5000
Portland, OR 97201

MULTNOMAH COUNTY

COMMERCIAL BUILDING PERMIT

04-006836-REV-01-CO

Site Address: 1616 SW HARBOR WAY - Unit 604
1616 SW HARBOR WAY 604

Issued: 5/25/04

PROJECT INFORMATION		Occ. Group	Const. Type
Apartments/Condos (3 or more units)	Alteration	R1	V-1HR

Project Description: REVISION 4/27/04 - STRUCTURAL REVISIONS

APPLICANT	COLLIER & COMPANY CONSTRUCTION INC	Phone (503) 222-9274
PROPERTY OWNER	PATRICK N DE LA GRANGE	Phone
CONTRACTOR	COLLIER & COMPANY CONSTRUCTION INC	Phone

Project Details		Project Details	
Code Edition (Year)	1997	Final Permit Valuation	2000
SI-Structural Steel	Yes	Zoning - Property (1)	TBD

PAID
MAY 25 2004
CITY OF PORTLAND

This permit expires if, at any time, 180 days pass without an approved inspection. If you are not able to obtain an *inspection approval within 180 days, you may request a one-time only extension of 180 days by calling* 503-823-7303.

BEFORE YOU DIG ATTENTION: Oregon law requires you to follow rules adopted by the Oregon Utility Notification Center. Those rules are set forth in OAR 952-001-0010 through OAR 952-001-0090. You may obtain copies of the rules by calling the center. (Note: the telephone number for the Oregon Utility Notification Center is 1-503-232-1987).

CITY CONTACT	PROCESS MANAGEMENT	Phone: 503-823-7357
Mail:		Fax: (503) 823-4172

INSPECTION REQUEST PHONE NUMBERS Building/Trade Inspections - Call Before 6:00 AM (503) 823-7000

TDD: (503) 823-6868

IVR Inspection Request Number: 2344599

FACSIMILE TRANSMITTAL

James G. Pierson, Inc.
Consulting Structural Engineers
320 S.W. STARK SUITE 535 PORTLAND, OR. 97204
(503) 226-1286 FAX 226-3130 Peder@jgpierson.com

To	**Michael Walkiewicz**
Company	**City of Portland**
Fax No.	**823-~~8696~~ 4224**
Number of Pages Including this Sheet	**12**

From	**Peder Golberg, P.E., S.E.**
Project Title	**De La Grange**
Project no.	
Date	**April 27, 2004**

MESSAGE:

Structural Calculations for the Harvey Collier project (he is in Customer Service #4) as requested.

City of Portland
APPROVED
APR 27 2004
Permit Number

City of Port
APPRO
APR 27 20
Permit N

COPY TO: file

SIGNED: Peder

If enclosures are not as noted, kindly notify us at once.

STRUCTURAL CALCULATIONS

New Header

De La Grange Remodel

River Place Condominium #604A/605

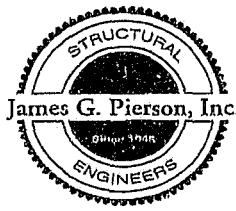
Portland, Oregon

for

Collier and Company

1507 NW 24th Ave. #200.

Portland, Oregon 97210



EXPIRES: JUNE. 30, 05

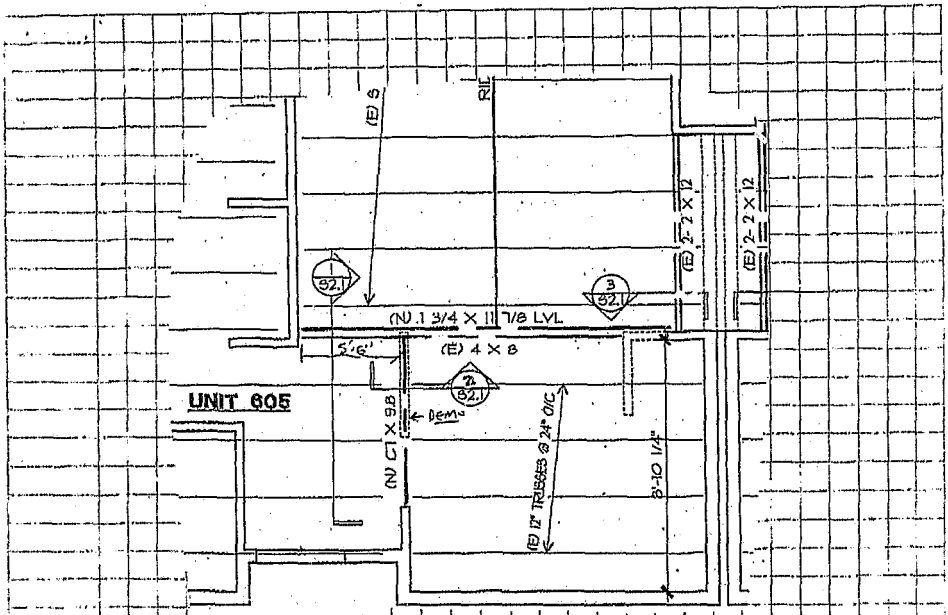
James G. Pierson, Inc.

Consulting Structural Engineers

320 S.W. STARK SUITE 535 PORTLAND, OR. 97204

(503) 226-1286 FAX 226-3130

April 27, 2004



(E) 4x8 Header - 11' span to be increased to 11'-9" (unit 604)
Valley - see checklist or 14' (unit 605)

DL= 15psf LL= 25psf + Gabled wall
Tributary load = 24" -Use 30 plf DL, 50 plf LL

- (E) Bearing wall @ Kitchen To be demo'd - new Beam R= 420 DL, 700 LL

-use 13/4 x 11 7/8" LVL as Header - see checklic attached

James G. Pierson, Inc.	Project	Job no.
Consulting Structural Engineers 320 S.W. Stark, Suite 535 Portland, Oregon 97204 Tel: (503) 226-1286 Fax: (503) 226-3130	Location: Riverplace Client: Collier	Date: 4/27/04 Sheet no.: 1 of 10

James G. Pierson, Inc.
Pedar Golberg, P.E./S.E.
James G. Pierson, Inc.
320 SW Stark #536
Portland, Oregon 97204
503-226-1286 fax 503-226-3130

Title :
Dsgnr:
Description :
Scope :

Job #
Date: 2:13PM, 27 APR 04

Rev: 5801CD
User: KW-0601815, Ver 5.6.1, 25-Oct-2002
(c)1983-2003 ENERCALC Engineering Software

General Timber Beam

Description Riverplace kitchen beam existing

General Information

Calculations are designed to 1997 NDS and 1997 UBC Requirements

Section Name	4x8	Center Span	11.00 ft	....Lu	0.00 ft
Beam Width	3.500 in	Left Cantilever	ft	....Lu	0.00 ft
Beam Depth	7.250 in	Right Cantilever	ft	....Lu	0.00 ft
Member Type	Sawn	Douglas Fir - Larch, No.2			
Bm Wt, Added to Loads		Fb Base Allow	900.0 psi		
Load Dur. Factor	1.150	Fv Allow	95.0 psi		
Beam End Fixity	Pin-Pin	Fc Allow	625.0 psi		
Wood Density	35.000 pcf	E	1,600.0 ksi		

Full Length Uniform Loads

Center	DL	100.00 #/ft	LL	50.00 #/ft	
Left Cantilever	DL	#/ft	LL	#/ft	
Right Cantilever	DL	#/ft	LL	#/ft	

Summary

Beam Design OK

Span= 11.00ft, Beam Width = 3.500in x Depth = 7.25in, Ends are Pin-Pin

Max Stress Ratio	0.687 : 1				
Maximum Moment	2.4 k-ft	Maximum Shear * 1.6			1.2 k
Allowable	3.4 k-ft	Allowable			2.8 k
Max. Positive Moment	2.36 k-ft at 5.500 ft	Shear:	@ Left		0.86k
Max. Negative Moment	0.00 k-ft at 11.000 ft		@ Right		0.86k
Max @ Left Support	0.00 k-ft	Camber:	@ Left		0.000in
Max @ Right Support	0.00 k-ft		@ Center		0.295in
Max. M allow	3.44		@ Right		0.000in
fb 914.43 psi	fv 45.49 psi	Reactions... Left DL	0.58 k	Max	0.86k
Fb 1,345.50 psi	Fv 109.25 psi	Right DL	0.58 k	Max	0.86k

Deflections

Center Span...	Dead Load	Total Load	Left Cantilever...	Dead Load	Total Load
Deflection	-0.197 in	-0.289 in	Deflection	0.000 in	0.000 in
...Location	5.500 ft	5.500 ft	...Length/Defl	0.0	0.0
...Length/Defl	671.2	456.31	Right Cantilever...		
Camber (using 1.5 * D.L. Defl) ...			Deflection	0.000 in	0.000 in
@ Center	0.295 in		...Length/Defl	0.0	0.0
@ Left	0.000 in				
@ Right	0.000 in				

Stress Calcs

Bending Analysis

Ck	31.887	Le	0.000 ft	Sxx	30.661 in3	Area	25.375 in2
Cf	1.300	Rb	0.000	Ci	0.000		

	Max Moment	Sxx Req'd	Allowable fb
@ Center	2.36 k-ft	21.07 in3	1,345.50 psi
@ Left Support	0.00 k-ft	0.00 in3	1,345.50 psi
@ Right Support	0.00 k-ft	0.00 in3	1,345.50 psi

Shear Analysis

	@ Left Support	@ Right Support
Design Shear	1.15 k	1.15 k
Area Required	10.566 in2	10.566 in2
Fv: Allowable	109.25 psi	109.25 psi

Bearing @ Supports

Max. Left Reaction	0.86 k	Bearing Length Req'd	0.393 in
Max. Right Reaction	0.86 k	Bearing Length Req'd	0.393 in

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320 SW Stark #535
Portland, Oregon 97204
503-226-1286 fax 503-226-3130

Title :
Dsgnr:
Description :
Scope :

Job #
Date: 2:13PM, 27 APR 04

Rev: 560100
User: KW-0601015, Ver 5.6.1, 25-Oct-2002
(c)1983-2002 ENERCALC Engineering Software

General Timber Beam

Description Riverplace kitchen beam existing

Query Values

M, V, & D @ Specified Locations		Moment	Shear	Deflection
@ Center Span Location =	0.00 ft	0.00 k-ft	0.86 k	0.0000 in
@ Right Cant. Location =	0.00 ft	0.00 k-ft	0.00 k	0.0000 in
@ Left Cant. Location =	0.00 ft	0.00 k-ft	0.00 k	0.0000 in

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Portland, Oregon 97204
503-226-1286 fax 503-226-3130

Title :
Dsgnr:
Description :

Scope :

Job #
Date: 2:13PM, 27 APR 04

General Timber Beam

Description Riverplace kitchen beam new 13/4x11 7/8" Microlam LVL

General Information

Calculations are designed to 1997 NDS and 1997 UBC Requirements

Section Name	MicroLam: 1.75x11.25	Center Span	14.00 ft	.....Lu	0.00 ft
Beam Width	1.750 in	Left Cantilever	ft	.....Lu	0.00 ft
Beam Depth	11.250 in	Right Cantilever	ft	.....Lu	0.00 ft
Member Type		Truss Joist - MacMillan, MicroLam 1.9 E			
Bm Wt. Added to Loads		Fb Base Allow	2,600.0 psi		
Load Dur. Factor	1.150	Fv Allow	285.0 psi		
Beam End Fixity	Pin-Pin	Fc Allow	750.0 psi		
Wood Density	35.000 pcf	E	1,900.0 ksi		

Full Length Uniform Loads

Center	DL	100.00 #/ft	LL	50.00 #/ft	
Left Cantilever	DL	#/ft	LL	#/ft	
Right Cantilever	DL	#/ft	LL	#/ft	

Point Loads

Dead Load	420.0 lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs
Live Load	700.0 lbs	lbs	lbs	lbs	lbs	lbs	lbs	lbs
...distance	5.500 ft	0.000 ft	0.000 ft	0.000 ft	0.000 ft	0.000 ft	0.000 ft	0.000 ft

Summary

Beam Design OK

Span= 14.00ft, Beam Width = 1.750in x Depth = 11.25in, Ends are Pin-Pin

Max Stress Ratio	0.799 : 1	Maximum Shear * 1.5			2.4 k
Maximum Moment	7.3 k-ft	Allowable			6.5 k
Allowable	9.2 k-ft				
Max. Positive Moment	7.35 k-ft at 5.544 ft	Shear:	@ Left		1.76 k
Max. Negative Moment	0.00 k-ft at 0.000 ft		@ Right		1.52 k
Max @ Left Support	0.00 k-ft	Camber:	@ Left		0.000 in
Max @ Right Support	0.00 k-ft		@ Center		0.492 in
			@ Right		0.000 in
Max. M allow	9.20	Reactions...			
fb 2,388.95 psi	fv 123.79 psi	Left DL	0.99 k	Max	1.76 k
Fb 2,990.00 psi	Fv 327.75 psi	Right DL	0.99 k	Max	1.52 k

Deflections

Center Span...	Dead Load	Total Load	Left Cantilever...	Dead Load	Total Load
Deflection	-0.323 in	-0.602 in	Deflection	0.000 in	0.000 in
...Location	6.888 ft	6.778 ft	...Length/Defl	0.0	0.0
...Length/Defl	512.0	278.98	Right Cantilever...		
Camber (using 1.5 * D.L. Defl) ...			Deflection	0.000 in	0.000 in
@ Center	0.492 in		...Length/Defl	0.0	0.0
@ Left	0.000 in				
@ Right	0.000 in				

Roof

4

Peder Golberg, P.E./S.E.
James G. Pierson, Inc.
320 SW Stark #535
Portland, Oregon 97204
503-226-1286 fax 503-226-3130

Title :
Dsgnr:
Description :

Scope :

Job #
Date: 2:13PM, 27 APR 04

General Timber Beam

Description Riverplace kitchen beam new

Stress Calcs

Bending Analysis

Ck	20.444	Le	0.000 ft	Sxx	36.914 in3	Area	19.688 in2
	1.000	Rb	0.000	Cl	0.000		

	Max Moment	Sxx Req'd	Allowable fb
@ Center	7.35 k-ft	29.49 in3	2,990.00 psi
@ Left Support	0.00 k-ft	0.00 in3	2,990.00 psi
@ Right Support	0.00 k-ft	0.00 in3	2,990.00 psi

Shear Analysis	@ Left Support	@ Right Support
Design Shear	2.44 k	2.08 k
Area Required	7.436 in2	6.338 in2
Fv; Allowable	327.75 psi	327.75 psi

Bearing @ Supports

Max. Left Reaction	1.76 k	Bearing Length Req'd	1.344 in
Max. Right Reaction	1.52 k	Bearing Length Req'd	1.161 in

Query Values

M, V, & D @ Specified Locations		Moment	Shear	Deflection
@ Center Span Location =	0.00 ft	0.00 k-ft	1.76 k	0.0000 in
@ Right Cant. Location =	0.00 ft	0.00 k-ft	0.00 k	0.0000 in
@ Left Cant. Location =	0.00 ft	0.00 k-ft	0.00 k	0.0000 in

James G. Pierson, Inc. STRUCTURAL ENGINEERS
Peder Golberg, P.E./S.E.
James G. Pierson, Inc.
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Portland, Oregon 97204
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Title :
Dsgnr:
Description :
Scope :

Job #
Date: 2:13PM, 27 APR 04

Rev: 550100
User: KW-0601613, Ver 5.6.1, 23-Oct-2002
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General Timber Beam

Description Kitchen Bearing Wall replacement

Non USEO

General Information

Calculations are designed to 1997 NDS and 1997 UBC Requirements

Section Name	4x8	Center Span	7.00 ft	....Lu	0.00 ft
Beam Width	3.500 in	Left Cantilever	ft	....Lu	0.00 ft
Beam Depth	7.250 in	Right Cantilever	ft	....Lu	0.00 ft
Member Type	Sawn	Douglas Fir - Larch, No.2			
Bm Wt Added to Loads		Fb Base Allow	875.0 psi		
Load Dur. Factor	1.150	Fv Allow	95.0 psi		
Beam End Fixity	Pin-Pin	Fc Allow	625.0 psi		
Wood Density	35.000 pcf	E	1,600.0 ksi		

Full Length Uniform Loads

Center	DL	120.00 #/ft	LL	200.00 #/ft	
Left Cantilever	DL	#/ft	LL	#/ft	
Right Cantilever	DL	#/ft	LL	#/ft	

Summary

Beam Design OK

Span= 7.00ft, Beam Width = 3.500in x Depth = 7.25in, Ends are Pin-Pin

Max Stress Ratio	0.598 : 1				
Maximum Moment	2.0 k-ft		Maximum Shear * 1.5		1.4 k
Allowable	3.3 k-ft		Allowable		2.8 k
Max. Positive Moment	2.00 k-ft	at 3.500 ft	Shear:	@ Left	1.14 k
Max. Negative Moment	0.00 k-ft	at 0.000 ft		@ Right	1.14 k
Max @ Left Support	0.00 k-ft		Camber:	@ Left	0.000 in
Max @ Right Support	0.00 k-ft			@ Center	0.057 in
Max. M allow	3.34			@ Right	0.000 in

Reactions...

fb	781.87 psi	fv	56.15 psi	Left DL	0.44 k	Max	1.14 k
Fb	1,308.13 psi	Fv	109.25 psi	Right DL	0.44 k	Max	1.14 k

Deflections

Center Span...	Dead Load	Total Load	Left Cantilever...	Dead Load	Total Load
Deflection	-0.036 in	-0.099 in	Deflection	0.000 in	0.000 in
...Location	3.500 ft	3.500 ft	...Length/Defl	0.0	0.0
...Length/Defl	2,191.7	847.80	Right Cantilever...		
Camber (using 1.5 * D.L. Defl) ...			Deflection	0.000 in	0.000 in
@ Center	0.057 in		...Length/Defl	0.0	0.0
@ Left	0.000 in				
@ Right	0.000 in				

Stress Calcs

Bending Analysis

Ck	32.339	Le	0.000 ft	Sxx	30.661 in3	Area	25.375 in2
Cf	1.300	Rb	0.000	Cf	0.000		

	Max Moment	Sxx Req'd	Allowable fb
@ Center	2.00 k-ft	18.33 in3	1,308.13 psi
@ Left Support	0.00 k-ft	0.00 in3	1,308.13 psi
@ Right Support	0.00 k-ft	0.00 in3	1,308.13 psi

Shear Analysis	@ Left Support	@ Right Support
Design Shear	1.42 k	1.42 k
Area Required	13.041 in2	13.041 in2
Fv: Allowable	109.25 psi	109.25 psi

Bearing @ Supports

Max. Left Reaction	1.14 k	Bearing Length Req'd	0.522 in
Max. Right Reaction	1.14 k	Bearing Length Req'd	0.522 in

6

Peder Golberg, P.E./S.E.
James G. Pierson, Inc.
320 SW Stark #535
Portland, Oregon 97204
503-226-1286 fax 503-225-3130

Title :
Dsgnr:
Description :

Scope :

Job #
Date: 2:13PM, 27 APR 04

General Timber Beam

Description Kitchen Bearing Wall replacement

Query Values

M, V, & D @ Specified Locations		Moment	Shear	Deflection
@ Center Span Location =	0.00 ft	0.00k-ft	1.14 k	0.0000 in
@ Right Cant. Location =	0.00 ft	0.00k-ft	0.00 k	0.0000 in
@ Left Cant. Location =	0.00 ft	0.00k-ft	0.00 k	0.0000 in

Peder Golberg, P.E./S.E.
James G. Pierson, Inc.
320 SW Clark #535
Portland, Oregon 97204
503-226-1286 fax 503-226-3130

Title :
Dsgnr:
Description :

Scope :

Job #
Date: 2:13PM, 27 APR 04

Rev: 560100
User: KW-0601015, Ver 5.5.1, 25-Oct-2003
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Steel Beam Design

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Description Kitchen Beam - Notch @ Sprinkler

General Information

Calculations are designed to AISC 9th Edition ASD and 1997 UBC Requirements

Steel Section : C7X9.8

			Fy	36.00 ksi	
Center Span	7.00 ft	Pinned-Pinned	Load Duration Factor	1.00	
Left Cant.	0.00 ft	Brn Wt. Added to Loads	Elastic Modulus	29,000.0 ksi	
Right Cant	0.00 ft	LL & ST Act Together			
Lu : Unbraced Length	0.00 ft				

Distributed Loads

	#1	#2	#3	#4	#5	#6	#7	
DL	0.120							k/ft
LL	0.200							k/ft
ST								k/ft
Start Location								ft
End Location								ft

Summary

Beam OK
Static Load Case Governs Stress

Using: C7X9.8 section, Span = 7.0', y = 36.0ksi
End Fixity = Pinned-Pinned, Lu = 0.0 ft, LDF = 1.000

	Actual	Allowable		
Moment	2.020 k-ft	10.954 k-ft	Max. Deflection	-0.029 in
fb : Bending Stress	3.983 ksi	21.600 ksi	Length/DL Defl	7,402.7 : 1
fb / Fb	0.184 : 1		Length/(DL+LL Defl)	2,912.6 : 1
Shear	1.154 k	21.168 k		
fv : Shear Streat	0.785 ksi	14.400 ksi		
fv / Fv	0.055 : 1			

Force & Stress Summary

	Maximum	DL Only	LL @ Center	LL+ST @ Center	LL @ Cants	LL+ST @ Cants	
			<<-- These columns are Dead + Live Load placed as noted -->>				
Max. M +	2.02 k-ft	0.79	2.02				k-ft
Max. M -							k-ft
Max. M @ Left							k-ft
Max. M @ Right							k-ft
Shear @ Left	1.15 k	0.45	1.15				k
Shear @ Right	1.15 k	0.45	1.15				k
Center Defl.	-0.029 in	-0.011	-0.029	-0.029	0.000	0.000	in
Left Cant Defl	0.000 in	0.000	0.000	0.000	0.000	0.000	in
Right Cant Defl	0.000 in	0.000	0.000	0.000	0.000	0.000	in
...Quary Defl @	0.000 ft	0.000	0.000	0.000	0.000	0.000	in
Reaction @ Left	1.15	0.45	1.15	1.15			k
Reaction @ Rt	1.15	0.45	1.15	1.15			k

Fa calc'd per Eq. E2-1, K*Lr < Cc
I Beam Passes Table B5.1, Fb per Eq. F1-1, Fb = 0.66 Fy

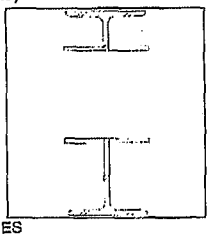
Section Properties C7X9.8

Depth	7.000 in	Weight	9.75 #/ft	r-xx	2.724 in
Width	2.090 in	I-xx	21.30 in4	r-yy	0.581 in
Web Thick	0.210 in	I-yy	0.97 in4		
Flange Thickness	0.366 in	S-xx	6.086 in3		
Area	2.87 in2	S-yy	0.625 in3		

James G. Pierson, Inc. beam-penetration.xls Page 1

BEAM PENETRATION DESIGN

(Example Steel-28)



INPUT PARAMETERS

LOADS	DL	LL	TL
SHEAR	0.6	0.6	1.2
MOMENT	1.1	1	2.1

BEAM PROPERTIES

Size	C7x9.8
Fy	36
Area	2.87
Depth	7
Web thickness	0.21
Flange Width	2
Flange Thickness	0.343

HOLE PROPERTIES

Height	3
Width	3
dist. to top	2.5

REINFORCING PROPERTIES

Fy	36
Thickness	0.1875
Width	2
Length	12
Side(s)	1

Vu = 1.68 kips
Mu= 2.92 kip-ft

Mp= 5.84 kip-ft

A tension 1.495 in^2
dm= 5 in

Mm= 22.425 kip-ft Mm = 5.84 kip-ft

Vm=Vmtop + Vmbot

Vm, top		Vm, bot	
Vpt	10.91192 kips	Vpt	6.547152 kips
ao	3 in	ao	3 in
st	2.40625 in	st	1.40625 in
v	1.246753	v	2.133333
Pr	13.5 kips	Pr	13.5 kips
dr	2.40625 in	dr	1.40625 in
mu	2.38157	mu	6.615472

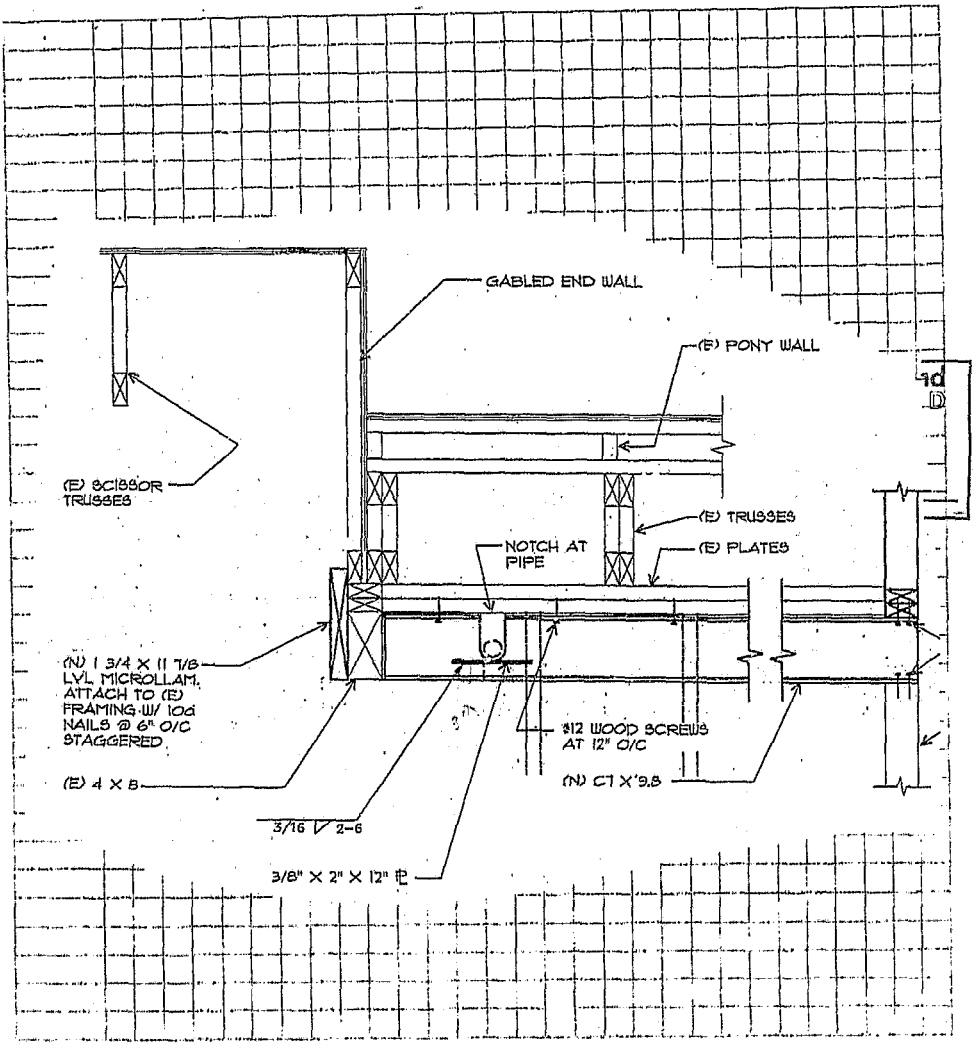
Vm, top= 17.69708 10.91192 Vm, bot= 15.35415 6.547152

Vm = 17.45907 kips

m ratio 0.555556
v ratio 0.106917

comb= 0.17269 OKAY

$$\left(\frac{M_u}{\Phi M_m}\right)^3 + \left(\frac{V_u}{\Phi V_m}\right)^3 \le 1.0$$



James G. Pierson, Inc.

Consulting Structural Engineers

320 S.W. Stark, Suite 535 Portland, Oregon 97204

Tel: (503) 226-1286 Fax: (503) 226-3130

Project: De La Grange	Job no.
Location: River place	Date: 4/27/04
Client: Cutler	Sheet no.: 10 of 10

April 27, 2004

John Matteson
Matteson Architects
800 NW Sixth Ave Suite 331
Portland, Oregon 97209

James G. Pierson, Inc.
STRUCTURAL ENGINEERS
Since 1945

Remodel of Units A604/A605
River Place Condominiums

Dear Mr. Matteson:

I have reviewed the remodel and required structural modifications to the roof framing of units A604/605 for the De La Grange Condominium Remodel that Collier and Company is doing.

There are existing architectural and structural framing plans for the units. However, the existing conditions were found to deviate from the original design drawings. Instead of 2x12 joists, the roof framing used was pre-manufactured scissor trusses for the vaulted areas (12:12 pitch top chord, 9:12 pitch at the ceiling). Over the kitchen, the actual framing was found to be 12" deep pre-manufactured trusses with a small pony walls on top for drainage. The bearing walls per the original design drawings were to be the party walls and also the plumbing walls by the bathtubs, however, the partition walls between the entrance corridor and the kitchen was found to be the actual bearing wall instead of the plumbing wall.

The remodel extended the existing opening between the kitchen and living room from 11 feet to about 14 feet. This opening carries about two feet of roof area along with the gabled end wall of the vaulted roof (plus snow drift). A new 14 foot header (1 3/4" x 11 7/8" Microlam LVL) will also carry a new perpendicular header required based on the partial removal of the bearing wall in the kitchen (that was thought to be non-structural). That new header is a steel channel (a 4x8 sawn beam would of been adequate) to allow for a large notch around an existing fire sprinkler pipe.

The vertical load path is being minimally modified by this remodel as the loads now get closer to the main load bearing party walls. These changes should have no net effect on the adjacent units. The lateral load path has not been modified at all by this remodel.

If you have any questions on my observations or analysis, please feel free to call and discuss.

Sincerely yours,

Peder Golberg, P.E., S.E.
Principal

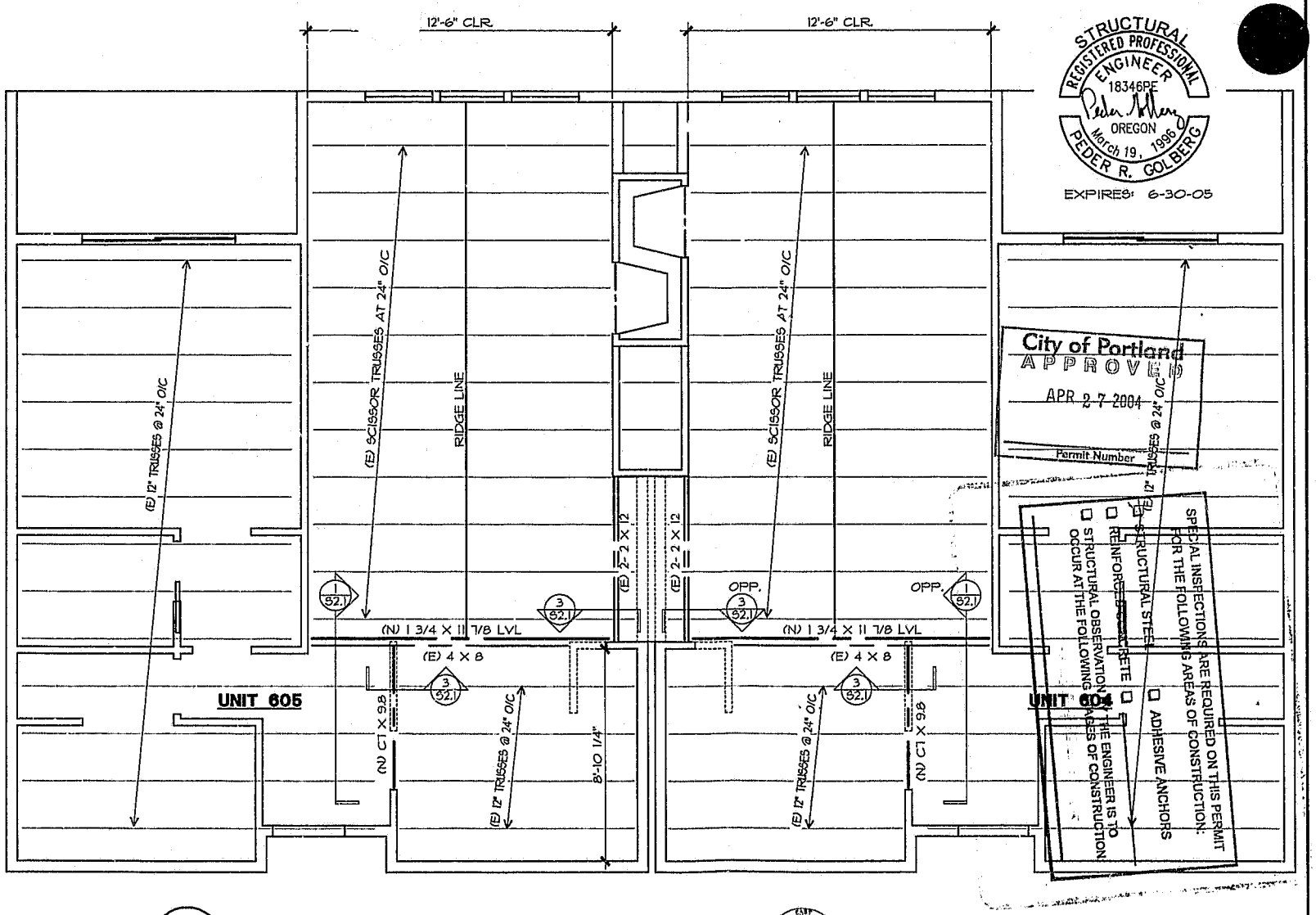
cc: Harvey Collier, Jr.

City of Portland
APPROVED
APR 27 2004
Permit Number

REGISTERED PROFESSIONAL ENGINEER
18340PE
OREGON
MARCH 19, 1993
PEDER R. GOLBERG

EXPIRES: JUNE. 30, 05

Consulting Structural Engineers
320 S.W. Stark, Suite 535 Portland, Oregon 97204 Tel: (503) 226-1286 Fax: (503) 226-3130



STRUCTURAL
REGISTERED PROFESSIONAL ENGINEER
18346PE
Peder R. Golberg
OREGON
March 19, 1996
PEDER R. GOLBERG
EXPIRES: 6-30-05

City of Portland
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APR 2 7 2004
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SPECIAL INSPECTIONS ARE REQUIRED ON THIS PERMIT FOR THE FOLLOWING AREAS OF CONSTRUCTION:
☐ STRUCTURAL OBSERVATION BY THE ENGINEER IS TO OCCUR AT THE FOLLOWING STAGES OF CONSTRUCTION
☐ REINFORCED CONCRETE
☐ STRUCTURAL STEEL
☐ ADHESIVE ANCHORS

1 S1.1 **DEMO/ROOF FRAMING PLAN**
1/4" = 1'-0"

WALLS TO BE DEMOLISHED

James G. Pierson, P.C.
Consulting Structural Engineers
320 S.W. Stark, Suite 535
Portland, Oregon 97204
Tel: (503) 226-1286
Fax: (503) 226-3130
Web: www.jgpierson.com

CLIENT NAME
HARVEY COLLIER & CO.
1507 NW 24th Avenue, #200
PORTLAND, OREGON 97210

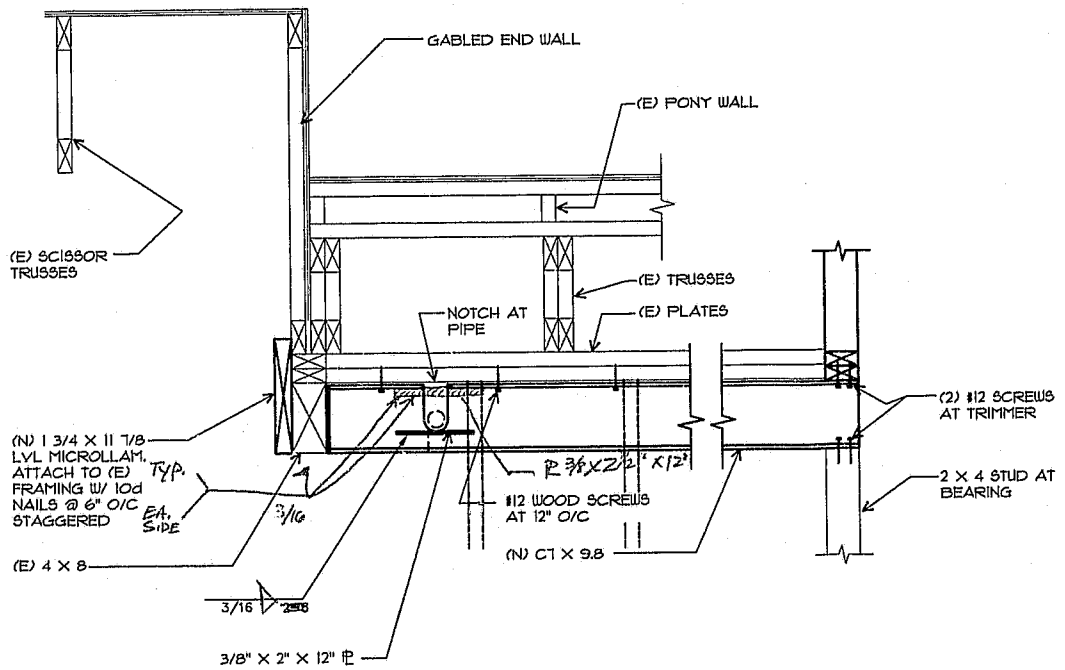
Patrick De La Grange
Condominium Remodel
Riverplace Condominiums
Portland, Oregon

DATE 04/07/2004
DRAWN BY KMW
CHECKED BY PRG
REVISIONS

SHEET TITLE
FLOOR PLAN

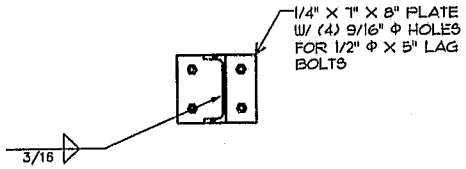
SHEET NUMBER
S1.1

01-006836-RFU-01-CO



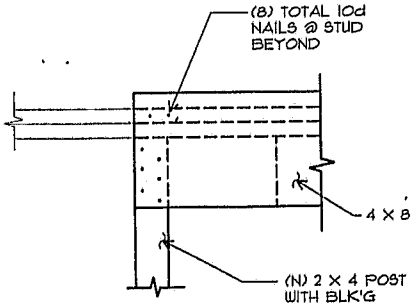
1 / S2.1

1" = 1'-0"



2 / S2.1

1/4" = 1'-0"



3 / S2.1

1" = 1'-0"

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Patrick De La Grange
Condominium Remodel
Riverplace Condominiums
Portland, Oregon

DATE 02/04/2004
DRAWN BY KAW
CHECKED BY PRG
REVISIONS

SHEET TITLE
DETAILS

SHEET NUMBER
S2.1