

6248 SW HAMILTON ST

REV-01 RS06-170153

RS.06.170153 REV.01

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MICROFILMED

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CITY OF
PORTLAND, OREGON
BUREAU OF DEVELOPMENT SERVICES
1900 SW 4th Ave., Suite 5000
Portland, OR 97201



RESIDENTIAL 1 & 2 FAMILY PERMIT

06-170153-REV-01-RS

Site Address: 6248 SW HAMILTON ST

Issued: 6/20/07

ADULT CARE HOME 6248 HAMILTON ST

PROJECT INFORMATION		Occ. Group	Const. Type
Single Family Dwelling	Addition		V-N
Project Description: 6-20-07 REVISE TRUSSES PER INSPECTOR			
APPLICANT IONUT PISICA *IONUT PISICA*		Phone (503) 819-1105	
PROPERTY OWNER MAGDALENA PISICA		Phone	
CONTRACTOR IONUT PISICA *IONUT PISICA*		Phone	

Project Details		Project Details	
# of Fixtures	31	Clothes Dryer Exhaust	1
Code Edition	IRC - 2003	Construction Type 4	
Final adj to ICC Value reason - Display	Unapplied	Furnace/Burner (inc ductwork/vent/liner)	1
GIS Update Flag	10/16/06	New Square Footage Living Area	4670
Number of Stories	2	On-Site Stormwater Facility	None
Pavement Date Paved	9/15/2003	Pavement Moratorium End Date	9/15/2008
Pavement Proposed Season	SUMMER	Pavement Proposed Year	2003
Pavement Status	ACTIVE	Rain Drain # of Feet	99
Res Wir: 1,000 sf or less	1	Res Wir: Each Add 500 sf or part	7
SI-Anchors - Adhesive	Yes	Single-Duct Exhaust (Bath/Toilet/Util)	10
Valuation at Issuance	431508	Water District	Portland Water Bureau

This permit expires if, at any time, 180 days pass without an approved inspection. If you are not able to obtain an inspection approval within 180 days, you may request a one-time only extension of 180 days by calling 503-823-7388.

BEFORE YOU DIG	ATTENTION: Oregon law requires you to follow rules adopted by the Oregon Utility Notification Center. Those rules are set forth in OAR 932-001-0010 through OAR 932-001-0090. You may obtain copies of the rules by calling the center. (Note: the telephone number for the Oregon Utility Notification Center is 1-800-332-2344).	
CITY CONTACT	E-Mail:	Phone: Fax: (503) 823-4172
INSPECTION REQUEST PHONE NUMBERS	Building/Trade Inspections - Call Before 6:00 AM:	(503) 823-7000
TDD: (503) 823-6868		
IVR Inspection Request Number:	2611215	

Precision Roof Trusses Inc.

11550 SE Jennifer St.
Clackamas, OR 97015
Phone: 503/656-2983 Fax: 503/656-2647

Project: Block No:
Model: Lot No:

Contact: Site: Office:

Name: Phone: Fax:

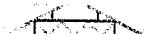
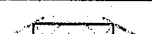
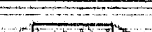
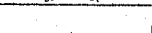
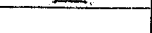
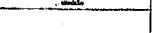
Tentative Delivery Date: 5/9/07

To:
JOHN PISICA
503-819-1105

Deliver To:
6248 SW HAMILTON
PDX

Delivery List

Job Number: 3913
Page: 1
Date: 05-03-2007 4:53
Project ID: none
Account No:
Designer: SS
Salesperson: TOM MARTIN
Quote Number:

Profile:	Qty:	Span:	Pitch	Truss Type:	Truss Id:	LOH	ROH	LABEL EACH TRUSS	Load By:
	2	33 - 11 - 10	4 - 11 - 7 6.00 0.00	CAL HIP	A2 (1) 2-Ply	16 - 0	16 - 0		
	1	33 - 11 - 10	5 - 11 - 7 6.00 0.00	CAL HIP	A4-1	16 - 0	16 - 0		
	1	33 - 11 - 10	6 - 11 - 7 6.00 0.00	CAL HIP	A4-2	16 - 0	16 - 0		
	1	33 - 11 - 10	7 - 11 - 7 6.00 0.00	CAL HIP	A4-3	16 - 0	16 - 0		
	1	33 - 11 - 10	8 - 11 - 7 6.00 0.00	CAL HIP	A4-4	16 - 0	16 - 0		
	16	33 - 11 - 10	9 - 5 - 14 6.00 0.00	MOD. QUEEN	A1	16 - 0	16 - 0		
	1	33 - 11 - 10	8 - 11 - 7 6.00 0.00	CAL HIP	A4-4	16 - 0	16 - 0		
	1	33 - 11 - 10	7 - 11 - 7 6.00 0.00	CAL HIP	A4-3	16 - 0	16 - 0		
	1	33 - 11 - 10	6 - 11 - 7 6.00 0.00	CAL HIP	A4-2	16 - 0	16 - 0		
	1	33 - 11 - 10	5 - 11 - 7 6.00 0.00	CAL HIP	A4-1	16 - 0	16 - 0		
	2	33 - 11 - 10	4 - 11 - 7 6.00 0.00	CAL HIP	A2 (1) 2-Ply	16 - 0	16 - 0		
	2	9 - 7 - 0 2X4/2X10	3 - 0 - 15 6.00 0.00	CAL HIP	B1 (1) 2-Ply	16 - 0	16 - 0		
	1	7 - 8 - 0	3 - 7 - 3 8.00 0.00	KINGPOST	C1	12 - 6	12 - 6		
	4	7 - 8 - 0	3 - 7 - 3 8.00 0.00	KINGPOST	C2	12 - 6	12 - 6		
	4	5 - 0 - 0	3 - 6 - 0 6.00 0.00	JACK	D1	16 - 0	0 - 0		
	15	5 - 0 - 0	3 - 6 - 0 6.00 0.00	JACK	D2	16 - 0	0 - 0		

City of Portland
UN 20 2007
Permit Number

STEVE PIERCE
6/19/07 THESE MATCH
ON SITE TRUSSES

06-170153 REV 01#5

Precision Roof Trusses Inc.
11550 SE Jennifer St.
Clackamas, OR 97015
Phone: 503/656-2983 Fax: 503/556-2647

To:
JOHN PISICA
503-819-1105

Delivery List

Job Number: 3913
Page: 2
Date: 05-03-2007 - 4:53:51 PM
Project ID: none

Project: Block No:
Model: Lot No:

Contact: Site: Office:
Name:
Phone:
Fax:

Deliver To:
6248 SW HAMILTON
PDX

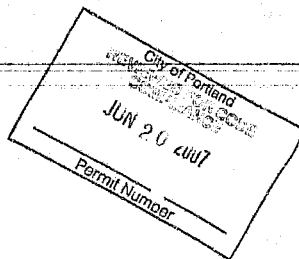
Account No:
Designer: SS
Salesperson: TOM MARTIN
Quote Number:

Tentative Delivery Date: 5/9/07

MISC. ITEMS

Quantity:	Description:
160	16D COMMON
306	N16
82	2X4 SOLID BLOCK
41	2X4 VENT BLOCK
142	TIES
52	JUS24
4	JUS210
1	THDH210-2
2	KTS24

Trusses - 54
Hips - 6
Stacks - 54



Delivery Date: _____ Delivered by: _____ Received by: _____
Unloading Time: 25 mins. C.O.D. Amt. Due: _____ Plus Additional Crane Time _____
Flag Cars: _____

Time In: _____ Time Out: _____ Standby Time: _____ Hrs. _____ Mins.

Roof Print _____ Bracing Info _____ Total No. Of Trusses: 54

FOR ADDITIONAL CRANE TIME, BAD ENTRIES, OR POOR SITE CONDITIONS:

Top plate delivery is at the discretion of our driver. He must be able to enter and exit without assistance.

Contractor must assume responsibility and cost (of tow truck and crane standby time) if assistance is needed, and of any damage to property.

If truck is unable to enter, trusses will be ground dropped as close to job site as possible.

Additional crane time will be charged at \$120.00 per hour. Contractor or representative understands and agrees to these terms.

Signature: _____

2/2

Sort: JACKLIST

MiTek Industries, Inc. Thu May 03 17:24:45 2007

*Qty	TQty	Label	Size	Grade	Species	Overall	Center	Longside	Job	Truss
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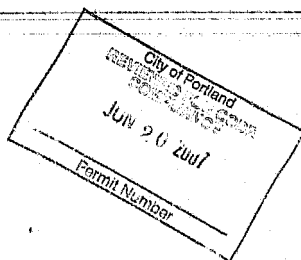
63.4

TO Maett

Jack

0-1-12-1 63.4

2	4	T1	2 X 4	No.1&Btr	DF-G	19-5-2	19-3-6	19-3-6	3913	A4-4
2		T1	2 X 4	No.1&Btr	DF-G	19-5-2	19-3-6	19-3-6	3913	A4-4
2	4	T1	2 X 4	No.1&Btr	DF-G	17-2-5	17-0-8	17-0-8	3913	A4-3
2		T1	2 X 4	No.1&Btr	DF-G	17-2-5	17-0-8	17-0-8	3913	A4-3
2	4	T1	2 X 4	No.1&Btr	DF-G	14-11-8	14-9-11	14-9-11	3913	A4-2
2		T1	2 X 4	No.1&Btr	DF-G	14-11-8	14-9-11	14-9-11	3913	A4-2
2	4	T1	2 X 4	No.1&Btr	DF-G	12-8-10	12-6-14	12-6-14	3913	A4-1
2		T1	2 X 4	No.1&Btr	DF-G	12-8-10	12-6-14	12-6-14	3913	A4-1
4	8	T1	2 X 4	No.1&Btr	DF-G	10-5-13	10-4-1	10-4-1	3913	A2
4		T1	2 X 4	No.1&Btr	DF-G	10-5-13	10-4-1	10-4-1	3913	A2
2	4	T1	2 X 4	No.1&Btr	DF-G	6-3-8	6-1-12	6-1-12	3913	B1
2		T1	2 X 4	No.1&Btr	DF-G	6-3-8	6-1-12	6-1-12	3913	B1

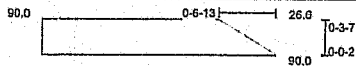


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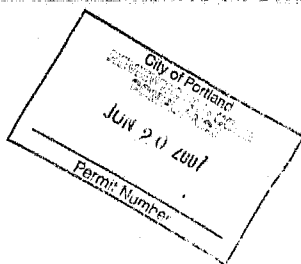
Sort: JAC/K-J

MITek Industries, Inc. Thu May 03 17:36:38 2007

Qty	TQty	Label	Size	Grade	Species	Overall	Center	Longside	Job	Truss
2	2	T1	2 X 4	No.1&Btr	DF-G	20-6-11	20-4-15	20-4-15	3913	A2J8-10 - Comp/Plaint
4	4	T1	2 X 4	No.1&Btr	DF-G	19-5-2	19-3-6	19-3-6	3913	A2J8-8
4	4	T1	2 X 4	No.1&Btr	DF-G	17-2-5	17-0-8	17-0-8	3913	A2J8-7
4	4	T1	2 X 4	No.1&Btr	DF-G	14-11-8	14-9-11	14-9-11	3913	A2J8-6
4	4	T1	2 X 4	No.1&Btr	DF-G	12-8-10	12-6-14	12-6-14	3913	A2J8-5
4	4	T1	2 X 4	No.1&Btr	DF-G	10-5-13	10-4-1	10-4-1	3913	A2J8-4
4	8	T1	2 X 4	No.1&Btr	DF-G	8-3-0	8-1-3	8-1-3	3913	A2J2-3
4	8	T1	2 X 4	No.1&Btr	DF-G	8-3-0	8-1-3	8-1-3	3913	A2J8-3
4	8	T1	2 X 4	No.1&Btr	DF-G	6-0-2	5-10-6	5-10-6	3913	A2J2-2
4	8	T1	2 X 4	No.1&Btr	DF-G	6-0-2	5-10-6	5-10-6	3913	A2J8-2
4	8	T1	2 X 4	No.1&Btr	DF-G	3-9-5	3-7-9	3-7-9	3913	A2J2-1
4	8	T1	2 X 4	No.1&Btr	DF-G	3-9-5	3-7-9	3-7-9	3913	A2J8-1
6	8	T1	2 X 4	No.1&Btr	DF-G	1-7-11	1-5-14	1-5-14	3913	A2-END



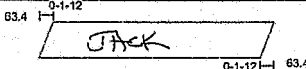
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2		B1	2 X 4	No.1&Btr	DF-G	7-11-4	7-7-15	7-11-4	3913	A2J8-10
4		B1	2 X 4	No.1&Btr	DF-G	7-11-4	7-8-0	7-11-4	3913	A2J3-2
4		B1	2 X 4	No.1&Btr	DF-G	7-11-4	7-8-0	7-11-4	3913	A2J8-3
4		B1	2 X 4	No.1&Btr	DF-G	7-11-4	7-8-0	7-11-4	3913	A2J8-4
4		B1	2 X 4	No.1&Btr	DF-G	7-11-4	7-8-0	7-11-4	3913	A2J8-5
4		B1	2 X 4	No.1&Btr	DF-G	7-11-4	7-8-0	7-11-4	3913	A2J8-6
4		B1	2 X 4	No.1&Btr	DF-G	7-11-4	7-8-0	7-11-4	3913	A2J8-7
4		B1	2 X 4	No.1&Btr	DF-G	7-11-4	7-8-0	7-11-4	3913	A2J8-8
4	12	B1	2 X 4	No.1&Btr	DF-G	1-11-4	1-8-0	1-11-4	3913	A2J2-1
4		B1	2 X 4	No.1&Btr	DF-G	1-11-4	1-8-0	1-11-4	3913	A2J2-2
4		B1	2 X 4	No.1&Btr	DF-G	1-11-4	1-7-15	1-11-4	3913	A2J2-3

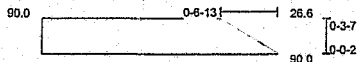


2/3

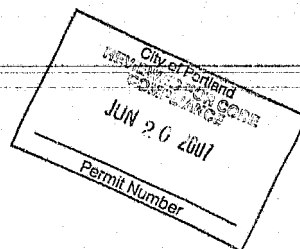
Sprt: JACK-J

MITek Industries, Inc. Thu May 03 17:39:27 2007

Qty	TQty	Label	Size	Grade	Species	Overall	Center	Longside	Job	Truss
										
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2		T1	2 X 4	No.1&Btr	DF-G	6-0-2	5-10-6	5-10-6	3913	B1J4-2
2	4	T1	2 X 4	No.1&Btr	DF-G	3-9-5	3-7-9	3-7-9	3913	B1J2-1
2		T1	2 X 4	No.1&Btr	DF-G	3-9-5	3-7-9	3-7-9	3913	B1J4-1
4	4	T1	2 X 4	No.1&Btr	DF-G	1-7-11	1-5-14	1-5-14	3913	B1-END



2	4	B1	2 X 4	No.1&Btr	DF-G	4-2-4	3-11-0	4-2-4	3913	B1J4-1
2		B1	2 X 4	No.1&Btr	DF-G	4-2-4	3-10-15	4-2-4	3913	B1J4-2
2	4	B1	2 X 4	No.1&Btr	DF-G	1-11-4	1-8-0	1-11-4	3913	B1J2-1
2		B1	2 X 4	No.1&Btr	DF-G	1-11-4	1-8-0	1-11-4	3913	B1J2-2



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Job 3913	Truss A1	Truss Type MOD. QUEEN	Qty 10	Ply 1	JOHN FISICA TMR/SS	R25203989
PRECISION ROOF TRUSSES, CLACKAMAS, OR. 97015						Job Reference (optional) 6.500 s Apr 2 2007 MTek Industries, Inc. F4 May 04 08:28:03 2007 Page 1

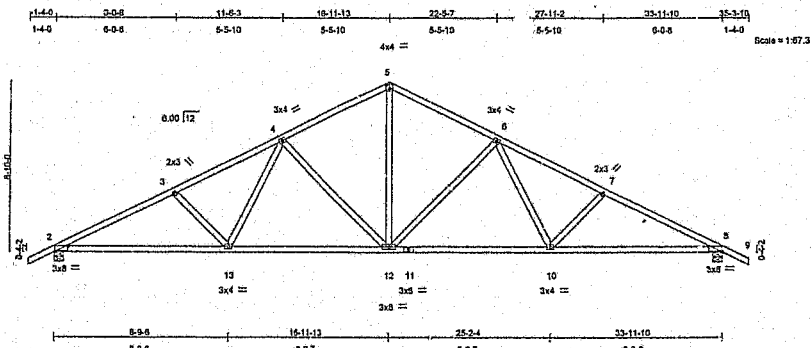
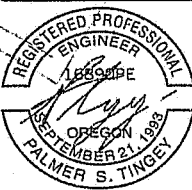
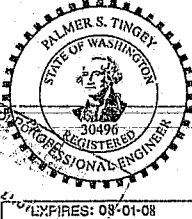


Plate Offsets (K, Y): 12-0-9-3 Edge, 15-0-2-0-2-4, 18-0-9-3 Edge							
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	Udefl	L/d
TCLL 25.0	Plates Increase	1.15	TC 0.27	Var(L)	-0.12	12	>999
TCLL 7.0	Lumber Increase	1.15	BC 0.45	Var(TL)	-0.36	2-13	>999
BCLL 0.0	Rep Stress Incr	YES	WB 0.70	Horz(TL)	0.12	0	N/A
BCDL 10.0	Code IRC2003/TP12002		(Matrix)				
						Weight: 163 lb	

LUMBER	BRACING
TOP CHORD 2 X 4 DF No.14B/G	TOP CHORD Structural wood sheathing directly applied or 3-10-9 oc purlins.
BOT CHORD 2 X 4 DF No.14B/G	BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 2 X 4 YF S1d G	
REACTIONS (lb/ft)	
2=1507/0-5-8, 6=1507/0-5-8	
Max Horiz 2=138/0 (C 8)	
Max Uplift 2=265/LC 5, 6=265/LC 5)	
FORCES (lb) - Maximum Compression/Maximum Tension	
TOP CHORD 1-2=033, 2-3=2599/345, 3-4=2341/316, 4-5=1849/271, 5-6=1849/271, 6-7=2341/316, 7-8=2599/345, 8-9=033	
BOT CHORD 2-13=342/2235, 12-13=208/1827, 11-12=114/1827, 10-11=114/1827, 8-10=204/2235	
WEBS 3-13=318/477, 4-13=264/57, 4-12=832/211, 5-12=112/1046, 6-12=632/211, 6-10=274/57, 7-10=318/178	

- NOTES
- Unbalanced roof live loads have been considered for this design.
 - Wind 72; 100mph; h=25ft; TCDL=4.2psf, BCDL=0.6psf; Category II; Exp C; enclosed; MWFRS; cantilever left and right exposed; and vertical w. id right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - This truss requires plate inspection per the Tenth Count Method when this truss is chosen for quality assurance inspection.
 - A plate rating reduction of 20% has been applied for the green lumber members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 205 lb uplift at joint 2 and 205 lb uplift at joint 1.
 - This truss is designed in accordance with the 2003 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



EXPIRATION DATE: 06/30/08

May 2007

WARNING: Verify design parameters and READ NOTES OF THIS AND INCLUDED REFERENCE PAGE SET PRIOR TO USE.
 Design valid for use only with metal connections. This design is based only upon parameters shown, and is for an individual building component.
 Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Tracing shown is for general layout of individual web members only. Additional temporary bracing to truss is necessary during construction is the responsibility of the user. For additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding installation, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, B19-87 and B201 Building Component Safety Information available from Truss Plate Institute, 363 D'Onofrio Drive, Madison, WI 53719.

MTek
 7777 Greenback Lane, Suite 109
 Citrus Heights, CA, 95610

Job	Truss	Truss Type	Qty	Ply	JOHN FISICA TM/KSS	R25203990
3013	A2	CAL HIP	2	2	Job Reference (optional)	
PRECISION ROOF TRUSSES, CLACKAMAS, OR. 97015						6,500 s Apr 2, 2007 Mitek Industries, Inc. Fri May 04 08:28:04 2007 Page 1

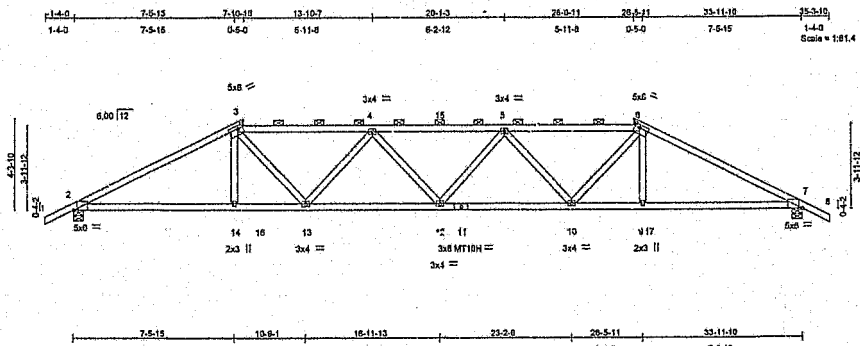


Plate Offsets (X,Y): 12-0-12 Edges, 13-0-2-12-0-1-12, 16-0-2-12-0-1-12, 17-0-1-12 Edges

LOADING (psf)	SPACING	2-0-0	CSI	DEPL	In (loc)	Udoff	L/d	PLATES	GRIP
TCOL	25.0	Plates Increase	1.15	TC	0.50			MT20	220/105
BDCL	7.0	Lumber Increase	1.15	BC	0.57			MT18H	220/105
BCOL	0.0	Rep Stress Inv	N/D	WB	0.42				
BCDL	10.0	Code IRC2003/TP2002							

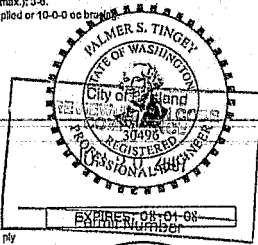
LUMBER			BRACING	
TOP CHORD	2 X 4 DF No.18Btr G		TOP CHORD	Structural wood sheathing directly applied or 5-1-5 oc purlins, except 2-0-0 oc purlins (4-0-4 max.); 3-6.
BOT CHORD	2 X 4 DF No.18Btr G		BOT CHORD	
WEBS	2 X 4 DF Sld G			

REACTIONS (lb/line)	2-34380-5-8, 7-34380-5-8				
Max Horiz 2-75(L.C.G.)					
Max Uplift 2-713(L.C.G.)					

FORCES (lb) - Maximum Compression/Maximum Tension					
TOP CHORD	1-2=0/38, 2-3=8775/1653, 3-4=7148/1773, 4-15=8280/2016, 5-15=8280/2016, 5-6=7148/1773, 6-7=8775/1653, 7-8=0/38				
BOT CHORD	2-14=1456/0011, 1-14=116/5999, 13-16=1462/5999, 12-13=2052/0141, 11-12=2032/0141, 10-11=2032/0141, 10-17=1416/5999, 9-17=1416/5999, 7-9=1411/0011				
WEBS	3-14=0/424, 6-9=0/424, 4-13=1027/555, 4-12=0/317, 5-12=0/317, 5-10=1627/555, 5-10=451/1650, 3-13=451/1650				

- NOTES**
- 1) 2-ply truss to be connected together with 10d (0.131"x3") nails as follows:
Top chords connected as follows: 2 X 4 + 2 rows at 0-9-0 on.
Bottom chords connected as follows: 2 X 4 + 2 rows at 0-9-0 on.
Webs connected as follows: 2 X 4 + 1 row at 0-9-0 on.
 - 2) All loads are considered equally applied to all piers, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
 - 3) Unbalanced roof live loads have been considered for this design.
 - 4) Wind: ASCE 7-02; 100mph; h=26ft; TCOL=4.2psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS; cantilever left and right exposed; end vertical left and right exposed; Lumber DOL=1.23 plate grip NCL=1.23.
 - 5) Provide adequate drainage to prevent water ponding.
 - 6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 7) All plates are MT20 plates unless otherwise indicated.
 - 8) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
 - 9) A plate rolling reduction of 20% has been applied for the greater lumber members.
 - 10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 713 lb uplift at joint 2 and 713 lb uplift at joint 7.
 - 11) This truss is designed in accordance with the 2003 International Residential Code sections R502.11.1 and R502.10.2 end referenced standard ANSI/TPI 1.
 - 12) Gilder carries hip end with 7-11-4 end seabolt.
 - 13) Design assumes 4x2 (flat orientation) purlins at oc spacing indicated, fastened to truss TC w/ 2-10d nails.

Continued on page 2



EXPIRATION DATE: 06/30/08

Major 200707

Job #	Truss	Truss Type	Qty	Ply	JOHN PISICA TMKSS	R25203902
3913	B1	CAL HIP	1	2	Job Reference (optional)	

PRECISION ROOF TRUSSES, CLACKAMAS, OR. 97015

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LOAD CASE(S) Standard

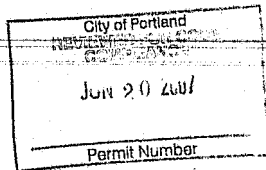
1) Regular: Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (pl)

Vert: 1-3=-64, 3-4=-64, 4-5=-64, 2-6=-20, 6-9=-692(F=-672)

Concentrated Loads (lb)

Vert: 0=-3335(F)



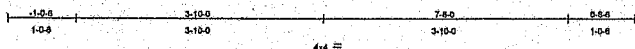
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED METER REFERENCE PAGE NO. 1473 BEFORE USE.

Design valid for use only with AMTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper interpretation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing is to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall truss is the responsibility of the building designer, for general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult AMT/THI Quality Criteria, B3-47 and B3-11 Building Component Safety Information available from Truss Plate Institute, 363 O'Connell Drive, Madison, WI 53717.



7777 Greenback Lane, Suite 100
Grand Rapids, MI 49519

JOB NO.	Truss	Truss Type	Qty	Ply	JOHN FISICA TMRSS	R2502093
0813	C1	RINGPOST	1	1	Job Reference (optional)	
PRECISION ROOF TRUSSES, CLACKAMAS, OR, 97015						6.500 s Apr 2 2007 MITek Industries, Inc. Fri May 04 08:28:07 2007 Page 1



Scale = 1/20.5

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	Defl	L/d	PLATES	GRIP
TCLL 25.0	Plates Increase	1.15	TC 0.13	Vert(LL)	-0.00	2-8	>909	MT20	220/195
TDDL 7.0	Lumber Increase	1.15	BC 0.11	Vert(TL)	-0.01	2-8	>909		
BCLL 0.0	Rep Stress Incr	YES	WB 0.07	Horz(TL)	0.00	4'	n/a		
BCDL 10.0	Code IRC2003/TP12002		(Multifx)						
Weight: 34 lb									

LUMBER
TOP CHORD 2 X 4 DF No.1&Btr G
BOT CHORD 2 X 4 DF No.1&Btr G
WEBS 2 X 4 DF Sld G
OTHERS 2 X 4 DF Sld G

BRACING
TOP CHORD
BOT CHORD

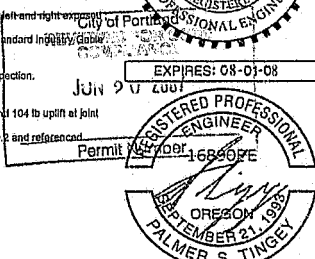
Structural wood shanking directly applied or 5-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (k/ft) 2=383/0-5-6, 4=383/0-5-6
Max Horz 2=78 (LC 3)
Max Uplift 2=104 (LC 5), 4=104 (LC 6)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/37, 2-3=323/23, 3-4=323/23, 4-5=0/37
BOT CHORD 2=0/201, 4=0/201
WEBS 3=0/0/17

- NOTES:
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-02: 100mph h=25ft; TDD=4.3psf; BCDL=6.0psf; Category II; Exp C; enclosed; MWFRS; cantilever left and right exposed and vertical left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Inquiry/Detail. End Details as applicable, or consult qualified building designer as per ANSI/TP1 1-2002.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - This truss requires plate inspection per the Teeth Count Method when this truss is chosen for quality assurance inspection.
 - Gable studs spaced at 14-0 oc.
 - A plate rating reduction of 20% has been applied for the green lumber members.
 - Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 104 lb uplift at joint 2 and 104 lb uplift at joint 4.
 - This truss is designed in accordance with the 2003 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TP1 1.

LOAD CASE(S) Standard



EXPIRATION DATE: 06/30/08

May 04, 2007

WARNING: Verify design parameters and READ NOTES ON THIS AND INCLUDED REFERENCED PARTS BEFORE USE.
Design valid for use only with MITek connection. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer, for general guidance regarding bracing, quality control, storage, delivery, erection and bracing, consult ANSI/TP1 Quality Criteria, 518-517 and USDI Building Component Safety Information available from Truss Plate Institute, 563 D'Orazio Drive, Madison, WI 53719.

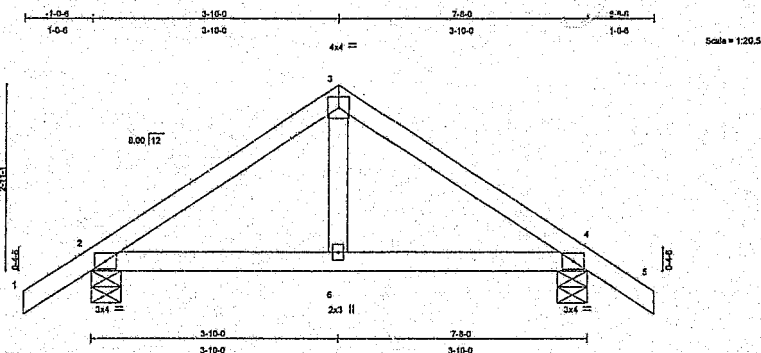


177 Greenback Lane, Suite 100
San Jose, CA 95131

App 3913	Truss C2	Truss Type KINGPOST	Qty 4	Ply 1	JOHN PISICA TMS/SS	R25203994
Job Reference (optional)						

PRECISION ROOF TRUSSES, CLACKAMAS, OR, 97015

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LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (loc)	Idelt	L/d	PLATES	GRIP
TCCL 25.0	Plates Increase	1.15	TC 0.13	Vert(TL)	-0.01	2-0	>999	MT20	220/195
TCCL 7.0	Lumber Increase	1.15	BC 0.11	Vert(TL)	-0.01	2-0	>999		
BCCL 0.0	Rep Stress Incr	YES	WB 0.07	Horz(TL)	0.00	4	n/a		
BCCL 10.0	Code IRC2003/TP12002		(Matrix)						
Weight: 30 lb									

LUMBER

TOP CHORD 2 X 4 DF No.16Btr G
BOT CHORD 2 X 4 DF No.16Btr G
WEBS 2 X 4 DF Std G

BRACING

TOP CHORD
BOT CHORD

Structural wood sheathing directly applied or 6-0-0 oc purlins.
Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS

(lb/size) 2=353/0-5-8, 4=353/0-5-8
Max Horz 2=70(LC 3)
Max Uplift 2=104(LC 5), 4=104(LC 6)

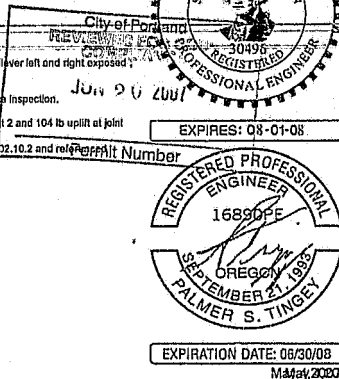
FORCES

(lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/57, 2-3=323/23, 3-4=323/23, 4-5=0/37
BOT CHORD 2-6=0/201, 4-6=0/201
WEBS 3-6=0/178

NOTES

- Unbalanced roof live loads have been considered for this design.
- Wind: ASCE 7-02; 100mph; h=28ft; TCCL=4.2psf; BCCL=0.0psf; Category II; Exp C; enclosed; MWFRG; cantilever left and right exposed.
- This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
- A plate rating reduction of 20% has been applied for live green lumber members.
- Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 104 lb uplift at Joint 2 and 104 lb uplift at joint 4.
- This truss is designed in accordance with the 2003 International Residential Code sections R502.11.1 and R502.10.2 and related provisions.

LOAD CASE(S) Standard



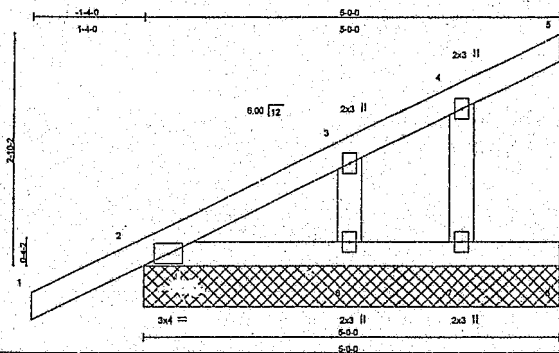
May 20, 2007

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE M17473 BEFORE USE.
Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component.
Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for better transport of individual web member only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer, for general guidance regarding bracing, quality control, storage, delivery, erection and bracing, consult ANSI/APRI Quality Criteria, 038-01 and ECSI Building Component Safety Information available from Trus Joist Institute, 933 O'Connell Drive, Madison, WI 53719.



7777 Greenback Lane, Suite 109
Ellen Harris, CA 95615

Job #	Truss	Truss Type	Qty	Ply	JOHN PISICA TM/KSS	R25203995
3913	D1	JACK	4	1	Job Reference (optional)	
PRECISION ROOF TRUSSES, CLACKAMAS, OR. 97015					6,500 s Apr 2 2007 44/Tek Industries, Inc. Fri May 04 08:28:08 2007 Page 1	



LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 25.0	2'-0"	TC 0.10	In (loc) 1/defl 1/2	MIT20	220/195
TCOL 7.0	Lumber Increase 1.15	BC 0.04	Vert(TL) -0.00 1 n/r 90		
BCOL 0.0	Rep Stress Incr YES	WB 0.02	Horz(TL) -0.00 5 n/a n/a		
BCOL 10.0	Code IRC2003/TP12002	(Matrix)			Weight: 21 lb

LUMBER
TOP CHORD 2 X 4 DF No.18B/G
BOT CHORD 2 X 4 DF No.18B/D
OTHERS 2 X 4 DF SH G

BRACING
TOP CHORD Structural wood sheathing directly applied or 5'-0" oc purlins.
BOT CHORD Rigid ceiling directly applied or 10'-0" oc bracing.

REACTIONS (k/lin) 5-32(5'-0"), 2-20(15'-0"), 6-11(5'-0"), 8-16(5'-0"), 7-8(5'-0")
Max Horz 2-12(LC 6)
Max Uplift 5-17(LC 8), 2-72(LC 5), 8-20(LC 4), 7-41(LC 5)
Max Grav 5-32(LC 1), 2-20(LC 1), 8-22(LC 2), 8-16(LC 1), 7-8(LC 1)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/37, 2-3=71/20, 3-4=40/15, 4-5=19/11
BOT CHORD 2-5=0/0, 7-8=0/0, 6-7=0/0
WEB 3-8=14/51, 4-7=7/52

NOTES
1) Wind: ASCE 7-02; 100mph; I=25; TCCL=4.2psf; BCCL=6.0psf; Category II; Exp C; enclosed; MWFRS; cantilevered left and right exposed;
and vertical left and right exposed; Lumber DOL=1.33; plate grip DOL=1.33.

2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Code of Portland.

3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.

4) This truss requires plate inspection per the Truss Count Method when this truss is chosen for quality assurance inspection.

5) Gable requires continuous bottom chord bracing.

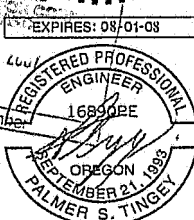
6) Gable studs spaced at 1'-0" oc.

7) A plate rating reduction of 20% has been applied for the green lumber members.

8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 17 lb uplift at joint 5, 72 lb uplift at joint 2, 20 lb uplift at joint 8 and 41 lb uplift at joint 7.

9) This truss is designed in accordance with the 2003 International Residential Code sections R502.11.1 and R502.10.2 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



EXPIRATION DATE: 06/30/08

M&J/200707

MITK
7777 Greenback Lane, Suite 109
Clackamas, OR 97015

