

4335 SW CHESAPEAKE AVE

DFS-01 RS06-178878

RS.06.178878 DFS.01.

N

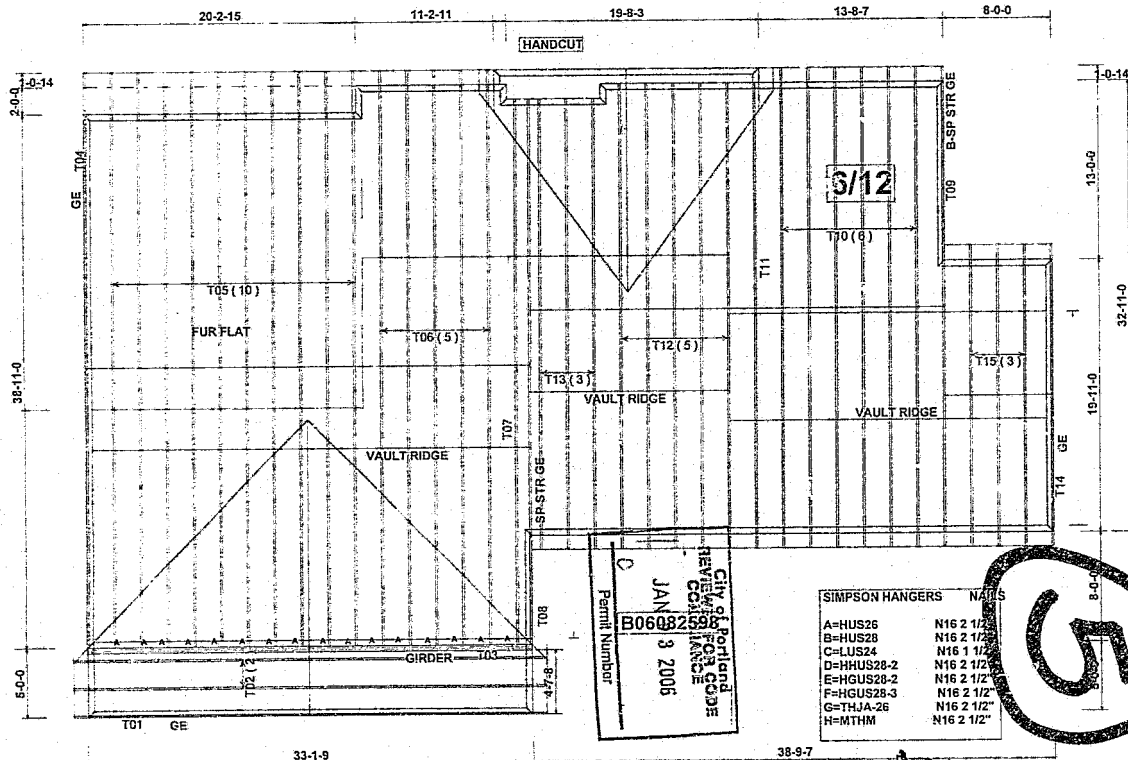
MAR 09 2007
MICROFILMED

5

06 178878

4341 S.W. Chesapeake

Needs to be a
Deferred Submittal
after permits are issued



06-178878 DFS 01 RS

MICRO $\rightarrow$

Job
B06082598

Truss
T01

Truss Type
CABLE

Qty
1

Ply
1

RICK ZIMMERMAN

R2375623

Truss-Way Inc., Vancouver, WA 98688, Scott Cowles

Job Reference (optional)
6200-2 Jul 13 2005 Mitak Industries, Inc. Fri Dec 15 16:57:58 2006 Page 1

1-0-14
1-0-14

16-0-12
16-0-12

33-1-9
16-0-12

34-2-7
1-0-14

Scale = 1:50.9

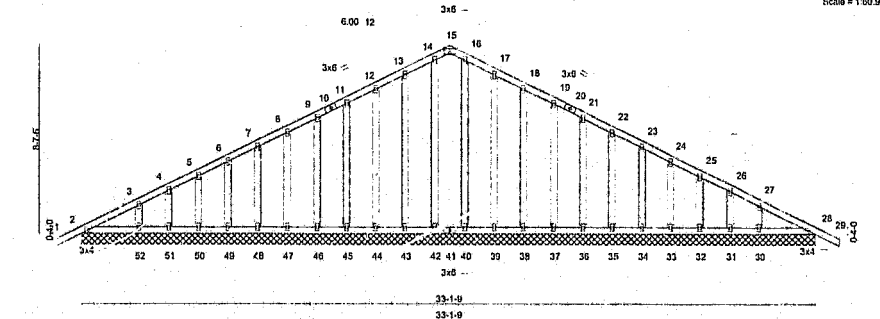


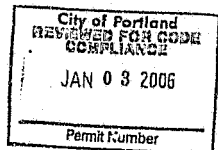
Plate Offsets (X,Y): [15:0-3:0,Edge]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	Udof	L/d	PLATES	GRIP
TCLL 25.0	Plates Increase	1.15	TC 0.15	Ver(LL) -0.00	29	n/r	120	MT20	220/195
TCOL 7.0	Lumber Increase	1.15	BC 0.04	Ver(TL) -0.00	28	n/r	90		
BCLL 0.0	Rep Stress Incr	NO	WB 0.09	Horz(TL) 0.01	28	n/a	n/a		
BCDL 10.0	Code IRC2003/TP12002		(Matrix)						
								Weight: 230 lb	

LUMBER
TOP CHORD 2 X 4 DF No.15Btr G
BOT CHORD 2 X 4 DF No.15Btr G
OTHERS 2 X 4 DF Sld G

BRACING
TOP CHORD Sheathed or 6-0-0 cc purfins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 cc bracing

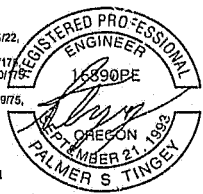
REACTIONS (lb/size) 26=181/33-1-9, 42=106/73-1-9, 43=113/33-1-9, 44=112/33-1-9, 45=112/33-1-9, 46=112/33-1-9, 47=112/33-1-9, 48=112/33-1-9, 49=113/33-1-9, 50=118/33-1-9, 51=84/33-1-9, 52=187/33-1-9, 49=108/33-1-9, 39=113/33-1-9, 38=112/33-1-9, 37=112/33-1-9, 36=112/33-1-9, 35=112/33-1-9, 34=112/33-1-9, 33=111/33-1-9, 32=116/33-1-9, 31=84/33-1-9, 30=181/33-1-9, 29=181/33-1-9, 28=181/33-1-9
Max Up12=63(load case 3), 43=65(load case 3), 44=62(load case 3), 45=57(load case 3), 46=58(load case 3), 47=58(load case 3), 48=57(load case 3), 49=58(load case 3), 50=57(load case 3), 51=57(load case 3), 52=59(load case 3), 39=59(load case 3), 38=62(load case 3), 37=57(load case 3), 36=58(load case 3), 35=58(load case 3), 34=57(load case 3), 33=58(load case 3), 32=57(load case 3), 31=57(load case 3), 30=69(load case 3), 29=73(load case 3)
Max Grw26=18 (load case 1), 42=106(load case 1), 43=115(load case 4), 44=112(load case 4), 45=112(load case 4), 46=112(load case 4), 47=112(load case 4), 48=112(load case 4), 49=111(load case 1), 50=118(load case 4), 51=94(load case 4), 52=100(load case 1), 39=115(load case 5), 38=112(load case 5), 37=112(load case 1), 36=112(load case 5), 35=112(load case 5), 34=112(load case 1), 33=111(load case 5), 32=116(load case 5), 31=84(load case 5), 30=187(load case 5), 29=181(load case 1)



FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD
1-2=0/30, 2-3=112/29, 3-4=55/5, 4-5=29/13, 5-6=31/22, 6-7=31/60, 7-8=31/68, 8-9=31/136, 9-10=31/171, 10-11=12/174, 11-12=31/122, 12-13=31/252, 13-14=31/293, 14-15=28/259, 15-16=28/259, 16-17=31/293, 17-18=31/252, 18-19=31/212, 19-20=12/174, 20-21=31/171, 21-22=31/136, 22-23=31/136, 23-24=31/60, 24-25=31/22, 25-26=28/13, 26-27=55/5, 27-28=112/29, 28-29=0/30
BOT CHORD
2-52=0/175, 51-52=0/175, 50-51=0/175, 49-50=0/175, 48-49=0/175, 47-48=0/175, 46-47=0/175, 45-46=0/175, 44-45=0/175, 43-44=0/175, 42-43=0/175, 41-42=0/175, 40-41=0/175, 39-40=0/175, 38-39=0/175, 37-38=0/175, 36-37=0/175, 35-36=0/175, 34-35=0/175, 33-34=0/175, 32-33=0/175, 31-32=0/175, 30-31=0/175, 29-30=0/175
WEBS
4-12=87/30, 13-43=88/81, 12-44=88/78, 11-45=85/73, 9-46=85/74, 6-47=85/74, 7-48=85/74, 6-49=85/73, 5-50=89/75, 4-51=87/64, 3-52=138/105, 16-108=83/0, 17-39=89/81, 18-30=80/78, 19-37=89/73, 21-39=85/74, 22-35=85/74, 23-34=85/74, 24-33=85/73, 25-32 89/76, 26-31=87/64, 27-30=138/105

NOTES
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02, 110mph; Int. Hgt. TCOL=6.0psf; BCDL=6.0psf; Category II; Exp B; enclosed; C-O Exterior(2); cont/ever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.

Continued on page 2



EXPIRATION DATE: 06/30/08
December 18, 2006

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITAK REFERENCE PAGE M7-7473 BEFORE USE.
Design valid for use only with Mitak connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Basing upon a for fabric support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANYTITE Quality Criteria, B44 and B431 Building Component. Safety information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

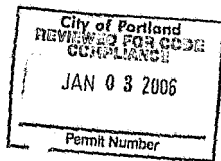


Job	Truss	Truss Type	Qty	Ply	RICK ZIMMERMAN	R2378022
006082598	T01	CABLE	1	1	Job Reference (optional)	
Troy-Way Inc., Vancouver, WA 98666 Scott Curwell					© 2004 Jul 13 2005 All Rights Reserved, Inc. P1 Dec 15 10 27 08 2006 Page 2	

NOTES

- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Cable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Plates checked for a plus or minus 5 degree rotation about its center.
- 7) Cable requires continuous bottom chord bearing.
- 8) Cable studs spaced 1" - 0 pc.
- 9) A plate rating reduction of 20% has been applied for the green lumber members.
- 10) This truss is designed in accordance with the 2003 International Residential Code sections R502.11.1 and R602.10.2 and referenced standard ANSI/TPI 1.
- 11) This truss does not include any time dependent deformation for long term loading (creep) in the total load deflection. The building designer shall verify that this parameter fits with the intended use of this component.

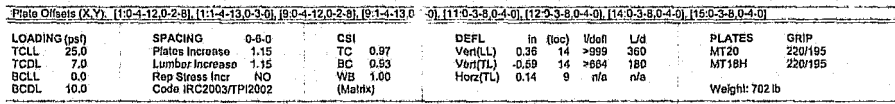
LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE M17-7473 BEFORE USE.

Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Mittek has designed bracing system for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding installation, consult product literature, design selection and erection manual. ANSI/TPI 1 Quality Criteria, B3A-17 and B3A-18 building component. Safety information available from Truss Plate Institute, 503 D Orofino Drive, Madison, WI 53719.

Mittek
 7777 Greenback Lane, Suite 100
 Citrus Heights, CA 95610



REACTIOnS (lb/size) 1=14327/0-5-8, 9=14327/0-5-8
Max Uplift=5204/4 (load case 3), 9=5204/4 (load case 3)

FORCES (lb) -Maximum Compression/Maximum Tension
TOP CHORD 1-2=27346/9533, 2-3=24926/9065, 3-4=20927/7323, 4-5=16351/5581, 5-6=16351/5581, 6-7=20092/7323,
7-8=24422/9065, 8-9=27346/9533
BOT CHORD 1-2=24926/9065, 2-3=27346/9533, 3-4=20927/7323, 4-5=16351/5581, 5-6=16351/5581, 6-7=20092/7323,
7-8=24422/9065, 8-9=27346/9533
WEBS 10-11=89922/2508, 11-12=89922/2508, 12-13=89922/2508, 14-15=8080/22306, 15-14=-6461/117946, 12-13=-6461/117946, 11-12=-8080/22306,
10-11=-89922/2508, 9-10=-89922/2508
2-10=7252052, 2-15=2460/9068, 3-15=2012/5570, 3-14=-6020/2035, 4-14=-2449/5770, 4-13=-6046/2218,
5-13=5248/14327, 6-13=5040/2218, 6-12=-2445/5770, 7-11=25629/2505, 7-11=2012/5570, 8-11=2460/9068,
9-11=7252052

REVIEWED FOR COMPLIANCE
JAN 03 2006
Permit Number

Continued on page 2

EXPIRATION DATE: 06/30/08
December 18, 2006

EXPIRATION DATE: 06/30/08
December 18, 2006

WARNING: Verify design parameters and READ THOROUGHLY THE DATA SHEET AND INCLUDED LITERATURE PRIOR TO INSTALLATION. This design is based on use only with Miltek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper installation of component is responsibility of building designer, not Miltek designer. Breaching strain is for historic purpose of individual wire members only. Additional termination bracketing to house strains during construction is the responsibility of the user. Additional parameter loading at the end of the cable is the responsibility of the building designer. For general questions regarding (technical data, quality control, storage, delivery, erection and breaking, consult: Miltek's Quality Center, 414-81 and 81411 Building Component Safety Information, 4540000, 4540001, 4540002, 4540003, 4540004, 4540005, 4540006, 4540007, 4540008, 4540009, 4540010, 4540011, 4540012, 4540013, 4540014, 4540015, 4540016, 4540017, 4540018, 4540019, 4540020, 4540021, 4540022, 4540023, 4540024, 4540025, 4540026, 4540027, 4540028, 4540029, 4540030, 4540031, 4540032, 4540033, 4540034, 4540035, 4540036, 4540037, 4540038, 4540039, 4540040, 4540041, 4540042, 4540043, 4540044, 4540045, 4540046, 4540047, 4540048, 4540049, 4540050, 4540051, 4540052, 4540053, 4540054, 4540055, 4540056, 4540057, 4540058, 4540059, 4540060, 4540061, 4540062, 4540063, 4540064, 4540065, 4540066, 4540067, 4540068, 4540069, 4540070, 4540071, 4540072, 4540073, 4540074, 4540075, 4540076, 4540077, 4540078, 4540079, 4540080, 4540081, 4540082, 4540083, 4540084, 4540085, 4540086, 4540087, 4540088, 4540089, 4540090, 4540091, 4540092, 4540093, 4540094, 4540095, 4540096, 4540097, 4540098, 4540099, 4540100, 4540101, 4540102, 4540103, 4540104, 4540105, 4540106, 4540107, 4540108, 4540109, 4540110, 4540111, 4540112, 4540113, 4540114, 4540115, 4540116, 4540117, 4540118, 4540119, 4540120, 4540121, 4540122, 4540123, 4540124, 4540125, 4540126, 4540127, 4540128, 4540129, 4540130, 4540131, 4540132, 4540133, 4540134, 4540135, 4540136, 4540137, 4540138, 4540139, 4540140, 4540141, 4540142, 4540143, 4540144, 4540145, 4540146, 4540147, 4540148, 4540149, 4540150, 4540151, 4540152, 4540153, 4540154, 4540155, 4540156, 4540157, 4540158, 4540159, 4540160, 4540161, 4540162, 4540163, 4540164, 4540165, 4540166, 4540167, 4540168, 4540169, 4540170, 4540171, 4540172, 4540173, 4540174, 4540175, 4540176, 4540177, 4540178, 4540179, 4540180, 4540181, 4540182, 4540183, 4540184, 4540185, 4540186, 4540187, 4540188, 4540189, 4540190, 4540191, 4540192, 4540193, 4540194, 4540195, 4540196, 4540197, 4540198, 4540199, 4540200, 4540201, 4540202, 4540203, 4540204, 4540205, 4540206, 4540207, 4540208, 4540209, 4540210, 4540211, 4540212, 4540213, 4540214, 4540215, 4540216, 4540217, 4540218, 4540219, 4540220, 4540221, 4540222, 4540223, 4540224, 4540225, 4540226, 4540227, 4540228, 4540229, 4540230, 4540231, 4540232, 4540233, 4540234, 4540235, 4540236, 4540237, 4540238, 4540239, 4540240, 4540241, 4540242, 4540243, 4540244, 4540245, 4540246, 4540247, 4540248, 4540249, 4540250, 4540251, 4540252, 4540253, 4540254, 4540255, 4540256, 4540257, 4540258, 4540259, 4540260, 4540261, 4540262, 4540263, 4540264, 4540265, 4540266, 4540267, 4540268, 4540269, 4540270, 4540271, 4540272, 4540273, 4540274, 4540275, 4540276, 4540277, 4540278, 4540279, 4540280, 4540281, 4540282, 4540283, 4540284, 4540285, 4540286, 4540287, 4540288, 4540289, 4540290, 4540291, 4540292, 4540293, 4540294, 4540295, 4540296, 4540297, 4540298, 4540299, 4540300, 4540301, 4540302, 4540303, 4540304, 4540305, 4540306, 4540307, 4540308, 4540309, 4540310, 4540311, 4540312, 4540313, 4540314, 4540315, 4540316, 4540317, 4540318, 4540319, 4540320, 4540321, 4540322, 4540323, 4540324, 4540325, 4540326, 4540327, 4540328, 4540329, 4540330, 4540331, 4540332, 4540333, 4540334, 4540335, 4540336, 4540337, 4540338, 4540339, 4540340, 4540341, 4540342, 4540343, 4540344, 4540345, 4540346, 4540347, 4540348, 4540349, 4540350, 4540351, 4540352, 4540353, 4540354, 4540355, 4540356, 4540357, 4540358, 4540359, 4540360, 4540361, 4540362, 4540363, 4540364, 4540365, 4540366, 4540367, 4540368, 4540369, 4540370, 4540371, 4540372, 4540373, 4540374, 4540375, 4540376, 4540377, 4540378, 4540379, 4540380, 4540381, 4540382, 4540383, 4540384, 4540385, 4540386, 4540387, 4540388, 4540389, 4540390, 4540391, 4540392, 4540393, 4540394, 4540395, 4540396, 4540397, 4540398, 4540399, 4540400, 4540401, 4540402, 4540403, 4540404, 4540405, 4540406, 4540407, 4540408, 4540409, 4540410, 4540411, 4540412, 4540413, 4540414, 4540415, 4540416, 4540417, 4540418, 4540419, 4540420, 4540421, 4540422, 4540423, 4540424, 4540425, 4540426, 4540427, 4540428, 4540429, 4540430, 4540431, 4540432, 4540433, 4540434, 454043

Job	Truss	Truss Type	City	Ply	RICK ZIMMERMAN	RZ3786J25
GC0002598	T03	TRIPLE FINK	1	3	Job Reference (optional)	
Truss-Way Inc., Vancouver, WA 98668, Scott Cornwell						© 2005 Jul 13 2006 Mitek Industries, Inc. Fm Dec 15 10 57 59 2006 Page 2

NOTES

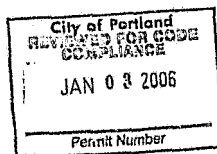
10) This truss does not include any time dependant deformation for long term loading (creep) in the total load deflection. The building designer shall verify that this parameter fits with the intended use of this component.

LOAD CASE(S) Standard

1) Regular Lumber Increase=1.15, Plate Increase=1.15

Uniform Loads (pl)

Vert: 1-S=-16, 5-S=-16, 1-9=-861(F=-850)



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGES BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon conditions shown, and is for an individual building component.
 Applicability of design parameters and proper incorporation of component is responsibility of building designer. Not for design or tracing shown
 is for support of individual with members only. Additional temporary bracing to insure stability during construction is the responsibility of the
 erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding
 fabrication, quality control, storage, delivery, erection and bracing, consult AIA/ITPA Quality Criteria, D34-87 and ECIF Building Component
 Safety Information, available from Truss Plate Institute, 503 D'Oroville Drive, Madison, WI 53719.

Mitek
 INDUSTRIES, INC.
 7777 Greenback Lane, Suite 109
 Cedar Rapids, IA 52410

Job Reference (Optional)
© 2005 Jun 13 2005 M/Tek Industries Inc. Ex Dec 15 16 58 00 2005 Page 1

-1-0-14
1044

20-5-8

41-2-0

SCALE = 1.741

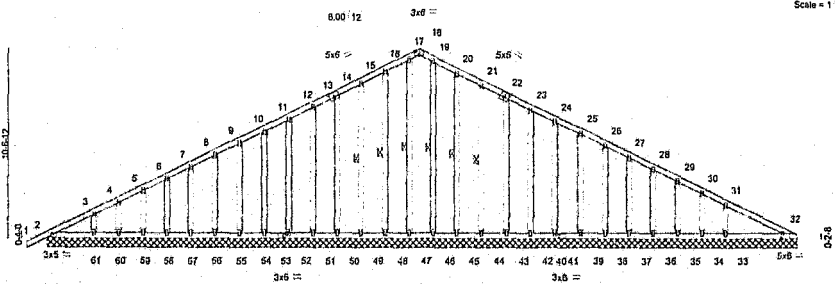


Plate Offsets (X,Y): [13:0-3-0,0-3-4], [18:0-3-0,Edge], [22:0-3-0,0-3-0], [32:0-1-3,Edge], [40:0-2-0,0-1-8], [53:0-2-0,0-1-5]

[illegible]

LUMBER
TOP CHORD 2 X 4 DF No.1&Btr G
BOT CHORD 2 X 4 DF No.1&Btr G
OTHERS 2 X 4 DF Sld G

BRACING
TOP CHORD Sheathed or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midspan 17.47 18.48 15.48 19.46 20.45 21.44

REACTIONS (lb/size) 2=182/41-2-0, 47=105/41-2-0, 48=112/41-2-0, 49=112/41-2-0, 50=112/41-2-0, 51=112/41-2-0, 52=112/41-2-0, 54=112/41-2-0, 55=112/41-2-0, 56=112/41-2-0, 57=112/41-2-0, 58=111/41-2-0, 59=118/41-2-0, 60=63/41-2-0, 61=189/41-2-0, 48=107/41-2-0, 45=113/41-2-0, 44=111/41-2-0, 43=112/41-2-0, 42=113/41-2-0, 41=112/41-2-0, 39=112/41-2-0, 38=112/41-2-0, 37=113/41-2-0, 36=108/41-2-0, 35=134/41-2-0, 34=4/41-2-0, 33=332/41-2-0, 32=121/41-2-0

[illegible]

City of Portland
REVIEWED FOR CODE
COMPLIANCE
JAN 03 2006
Permit Number

FORCES (lb) - Maximum Compression/Maximum Tension

TOP CHORD
1-2-0/30, 2-3-107/20, 4-5-100/5, 4-5-72/13, 5-6-37/12, 6-7-31/12, 7-8-31/4, 8-9-31/2, 9-10-31/20, 10-11-31/16, 11-12-31/12, 12-13-31/8, 13-14-31/4, 14-15-31/2, 15-16-31/3, 16-17-31/3, 17-18-31/2, 18-19-31/2, 19-20-31/2, 20-21-31/2, 21-22-31/2, 22-23-31/2, 23-24-31/2, 24-25-31/2, 25-26-31/2, 26-27-31/2, 27-28-31/2, 28-29-31/2, 29-30-31/2, 30-31-31/2, 31-32-31/2, 32-33-31/2, 33-34-31/2, 34-35-31/2, 35-36-31/2, 36-37-31/2, 37-38-31/2, 38-39-31/2, 39-40-31/2, 40-41-31/2, 41-42-31/2, 42-43-31/2, 43-44-31/2, 44-45-31/2, 45-46-31/2, 46-47-31/2, 47-48-31/2, 48-49-31/2, 49-50-31/2, 50-51-31/2, 51-52-31/2, 52-53-31/2, 53-54-31/2, 54-55-31/2, 55-56-31/2, 56-57-31/2, 57-58-31/2, 58-59-31/2, 59-60-31/2, 60-61-31/2, 61-62-31/2, 62-63-31/2, 63-64-31/2, 64-65-31/2, 65-66-31/2, 66-67-31/2, 67-68-31/2, 68-69-31/2, 69-70-31/2, 70-71-31/2, 71-72-31/2, 72-73-31/2, 73-74-31/2, 74-75-31/2, 75-76-31/2, 76-77-31/2, 77-78-31/2, 78-79-31/2, 79-80-31/2, 80-81-31/2, 81-82-31/2, 82-83-31/2, 83-84-31/2, 84-85-31/2, 85-86-31/2, 86-87-31/2, 87-88-31/2, 88-89-31/2, 89-90-31/2, 90-91-31/2, 91-92-31/2, 92-93-31/2, 93-94-31/2, 94-95-31/2, 95-96-31/2, 96-97-31/2, 97-98-31/2, 98-99-31/2, 99-100-31/2, 100-101-31/2, 101-102-31/2, 102-103-31/2, 103-104-31/2, 104-105-31/2, 105-106-31/2, 106-107-31/2, 107-108-31/2, 108-109-31/2, 109-110-31/2, 110-111-31/2, 111-112-31/2, 112-113-31/2, 113-114-31/2, 114-115-31/2, 115-116-31/2, 116-117-31/2, 117-118-31/2, 118-119-31/2, 119-120-31/2, 120-121-31/2, 121-122-31/2, 122-123-31/2, 123-124-31/2, 124-125-31/2, 125-126-31/2, 126-127-31/2, 127-128-31/2, 128-129-31/2, 129-130-31/2, 130-131-31/2, 131-132-31/2, 132-133-31/2, 133-134-31/2, 134-135-31/2, 135-136-31/2, 136-137-31/2, 137-138-31/2, 138-139-31/2, 139-140-31/2, 140-141-31/2, 141-142-31/2, 142-143-31/2, 143-144-31/2, 144-145-31/2, 145-146-31/2, 146-147-31/2, 147-148-31/2, 148-149-31/2, 149-150-31/2, 150-151-31/2, 151-152-31/2, 152-153-31/2, 153-154-31/2, 154-155-31/2, 155-156-31/2, 156-157-31/2, 157-158-31/2, 158-159-31/2, 159-160-31/2, 160-161-31/2, 161-162-31/2, 162-163-31/2, 163-164-31/2, 164-165-31/2, 165-166-31/2, 166-167-31/2, 167-168-31/2, 168-169-31/2, 169-170-31/2, 170-171-31/2, 171-172-31/2, 172-173-31/2, 173-174-31/2, 174-175-31/2, 175-176-31/2, 176-177-31/2, 177-178-31/2, 178-179-31/2, 179-180-31/2, 180-181-31/2, 181-182-31/2, 182-183-31/2, 183-184-31/2, 184-185-31/2, 185-186-31/2, 186-187-31/2, 187-188-31/2, 188-189-31/2, 189-190-31/2, 190-191-31/2, 191-192-31/2, 192-193-31/2, 193-194-31/2, 194-195-31/2, 195-196-31/2, 196-197-31/2, 197-198-31/2, 198-199-31/2, 199-200-31/2, 200-201-31/2, 201-202-31/2, 202-203-31/2, 203-204-31/2, 204-205-31/2, 205-206-31/2, 206-207-31/2, 207-208-31/2, 208-209-31/2, 209-210-31/2, 210-211-31/2, 211-212-31/2, 212-213-31/2, 213-214-31/2, 214-215-31/2, 215-216-31/2, 216-217-31/2, 217-218-31/2, 218-219-31/2, 219-220-31/2, 220-221-31/2, 221-222-31/2, 222-223-31/2, 223-224-31/2, 224-225-31/2, 225-226-31/2, 226-227-31/2, 227-228-31/2, 228-229-31/2, 229-230-31/2, 230-231-31/2, 231-232-31/2, 232-233-31/2, 233-234-31/2, 234-235-31/2, 235-236-31/2, 236-237-31/2, 237-238-31/2, 238-239-31/2, 239-240-31/2, 240-241-31/2, 241-242-31/2, 242-243-31/2, 243-244-31/2, 244-245-31/2, 245-246-31/2, 246-247-31/2, 247-248-31/2, 248-249-31/2, 249-250-31/2, 250-251-31/2, 251-252-31/2, 252-253-31/2, 253-254-31/2, 254-255-31/2, 255-256-31/2, 256-257-31/2, 257-258-31/2, 258-259-31/2, 259-260-31/2, 260-261-31/2, 261-262-31/2, 262-263-31/2, 263-264-31/2, 264-265-31/2, 265-266-31/2, 266-267-31/2, 267-268-31/2, 268-269-31/2, 269-270-31/2, 270-271-31/2, 271-272-31/2, 272-273-31/2, 273-274-31/2, 274-275-31/2, 275-276-31/2, 276-277-31/2, 277-278-31/2, 278-279-31/2, 279-280-31/2, 280-281-31/2, 281-282-31/2, 282-283-31/2, 283-284-31/2, 284-285-31/2, 285-286-31/2, 286-287-31/2, 287-288-31/2, 288-289-31/2, 289-290-31/2, 290-291-31/2, 291-292-31/2, 292-293-31/2, 293-294-31/2, 294-295-31/2, 295-296-31/2, 296-297-31/2, 297-298-31/2, 298-299-31/2, 299-300-31/2, 300-301-31/2, 301-302-31/2, 302-303-31/2, 303-304-31/2, 304-305-31/2, 305-306-31/2, 306-307-31/2, 307-308-31

Continued on page 2

EXPIRATION DATE: 06/30/08
December 18, 2006

WARNING: Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER REFERENCE PAGE K11-7473 BEFORE USE. Design void to be used with Mitel connection. This design is based only upon personnel having, and/or in the individual building component. Applicability of design is limited to the use of the component in the manner shown. The design is not intended to be used for a full tenant suite of individual work member only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing consult: AISC(1989) Quality Criteria, D38-88 and ICC(1990) Building Component

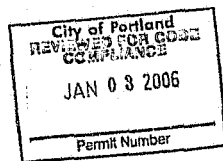
MiTek
POWER TO PERFORM[®]
7777 Greenback Lane, Suite 100

Job	Truss	Truss Type	City	Plf	RICK ZIMMERMAN	R23785628
800082509	T04	GABLE				
Truss-Way Inc., Vancouver, WA 98668, Scott Cornwell					Job Reference (optional) 8 200 x Jul 13 2005 Mittek Industries, Inc. Fri Dec 15 16:58:00 2006 Page 2	

NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph; h=18ft; TCFL=4.2psf; BCLD=6.0psf; Category II; Exp B; enclosed; C-C Exterior(2); cantilever left and right exposed ; Lumber DCL=1.33 plate grip DCL=1.33.
- 3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
- 4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
- 5) All plates are 2x4 MT20 unless otherwise indicated.
- 6) Plates checked for a plus or minus 5 degree rotation about its center.
- 7) Gable requires continuous bottom chord bracing.
- 8) Gable studs spaced at 1-4-0 oc.
- 9) A plate rating reduction of 20% has been applied for the green lumber members.
- 10) This truss is designed in accordance with the 2003 International Residential Code sections R502.11.1 and R502.10.2 and referenced standard ANSI/TPI 1.
- 11) This truss does not include any time dependent deformation for long term loading (creep) in the total load deflection. The building designer shall verify that this parameter fits with the intended use of this component.

LOAD CASE(S) Standard



WARNING: Verify design parameters and READ NOTES ON THIS AND INCLUDED MITTEK REFERENCE PAGE MIT-743 BEFORE USE.
Design valid for use only with Mittek connectors. This design is based only upon parameters shown, and is for an individual bracing component.
Applicable design parameters and proper incorporation of component is responsibility of building designer, not truss designer. Bracing shown is for lateral support of individual walls/roofs only. Additional temporary bracing to insure stability during construction is the responsibility of the engineer. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI 1 Quality Criteria, 938-N and ICBI Building Component Safety Information available from Truss Plate Institute, 593 D Anclote Drive, Alachua, FL 32319.

Mittek
POWER TO THE STRUCTURE
2777 Grinnell Lane, Suite 109
Chico, CA 95910

1-0-14 8-3-2 15-11-5 20-7-10 25-4-0 31-0-0 35-7-7 41-2-0
 1-0-14 8-3-2 7-4-4 4-8-5 4-8-5 5-8-1 5-7-7 4-6-9
 6 00 12 5x8 5
 Scale = 1.852

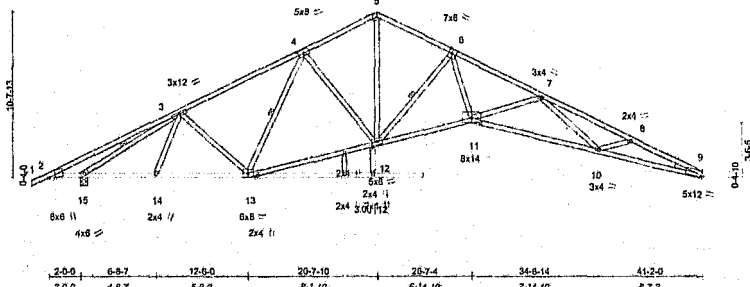


Plate Offsets (X,Y):		[2-0-1-4, Edge], [4-0-4-0-0-3-0], [6-0-2-4, Edge], [8-0-1-7, Edge], [11-0-7-0-0-3-10], [12-2-0-0-0-2]							
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (ft)	ldoff	L/d	PLATES	CRIP
TCLL 25.0	Plates Increase	1.15	TC 0.78	Vert(TL)	-0.44	10-11	>999	M720	220/195
TCLL 7.0	Lumber Increase	1.15	BC 0.82	Vert(TL)	-0.79	10-11	>999		
BCLL 0.0	Rep Stress Incr	YES	WB 0.83	Horz(TL)	0.43	9	n/a		
BCLL 10.0	Code IRC2003/TP12002		(Matrix)						
									Weight: 229 lb

LUMBER
 TOP CHORD 2 X 4 DF No.1&Btr G
 BOT CHORD 2 X 4 DF No.1&Btr G
 WEBS 2 X 4 DF Sld G "Except"
 6-11 2 X 4 DF No.1&Btr G
 WEDGE
 Right: 2 X 4 EYP No.3

BRACING
 TOP CHORD Sheathed or 2-2-0 cc purtins.
 BOT CHORD Rigid ceiling directly applied or 4-11-15 cc bracing.
 WEBS 1 Row at midpt 4-13, 6-12, 3-15

REACTIONS (lb/size) 15=1684/D-5-8, 9=1630/Mechanical
 Max Horz 15=38(load case 3)
 Max Uplift 15=825(load case 3), 9=588(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/30, 2-3=401/353, 3-4=2300/901, 4-5=2234/936, 5-6=2219/925, 6-7=4404/1502, 7-8=5315/1836, 8-9=5401/1932
 BOT CHORD 2-15=214/380, 14-15=620/2054, 13-14=642/2056, 12-13=553/2141, 11-12=859/3353, 10-11=1459/1623, 9-10=1721/1625
 WEBS 3-14=0/164, 3-13=211/198, 4-13=286/102, 4-12=355/287, 5-12=657/1656, 6-12=2102/760, 6-11=651/2371, 7-11=664/438, 7-10=594/448, 8-10=118/237, 3-15=2731/1178

NOTES
 1) Unbalanced roof live loads have been considered for this design.
 2) Wind, ASCE 7-02, 110mph; h=181, TCDL=4.2psf, C-CDL=6.0psf; Category II; Exp B; enclosed; C-C Exterior(2); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 4) Plates checked for a plus or minus 5 degree rotation about its center.
 5) A plate rating reduction of 20% has been applied for the green lumber members.
 6) Refer to girder(s) for truss to truss connections.
 7) This truss is designed in accordance with the 2003 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 8) This truss does not include any time dependent deformation for long term loading (creep) in the total load deflection. The building designer shall verify that this parameter fits with the intended use of this component.

LOAD CASE(S) Standard

City of Portland
 REVIEWED FOR CODE
 COMPLIANCE
 JAN 03 2006
 Permit Number

REGISTERED PROFESSIONAL
 ENGINEER
 168802E
 SEPTEMBER 21, 1993
 PALMER S. TINGEY

EXPIRATION DATE: 06/30/08
 December 18, 2006

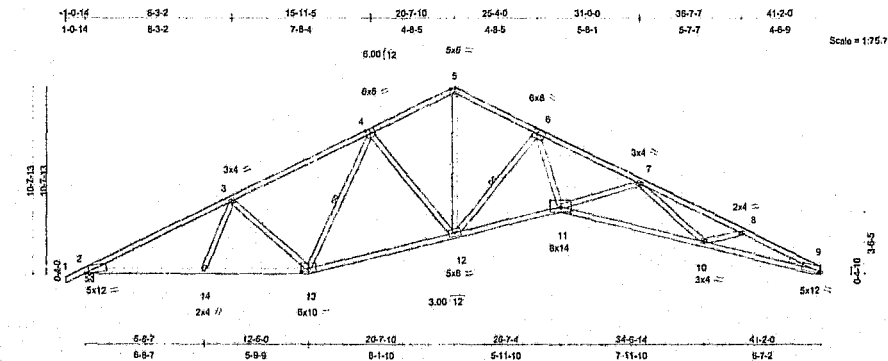


Plate Offsets (X,Y): [2-0-2-10, Edge], [4-0-3-0, Edge], [6-0-3-0, 0-3-0], [8-0-1-7, Edge], [11-0-7-0, 0-3-10], [13-0-5-0, 0-2-4]									
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	in (loc)	ldell	L/d	PLATES	GRIP
TCLL 25.0	Plates Increase	1.15	TC 0.85	Vert(LL)	-0.49	10-11	>999	MT20	220/195
TCOL 7.1	Lumber Increase	1.15	BC 0.63	Vert(TL)	-0.87	10-11	>992		
BCLL 0.0	Rep Stress Incr	YES	WB 0.89	Horz(TL)	0.47	9	n/a		
BCOL 10.0	Code IRC2003/TP2002		(Matrix)						
								Weight: 201 lb	

LUMBER
 TOP CHORD 2 X 4 DF No.1&Btr G
 BOT CHORD 2 X 4 DF No.1&Btr G
 WEBS 2 X 4 DF Sld G "Except"
 6-11 2 X 4 DF No.1&Btr G

BRACING
 TOP CHORD Sheathed or 2-1-9 cc purlins.
 BOT CHORD Rigid ceiling directly applied or 4-10-0 cc bracing.
 WEBS 1 Row at midpt 4-13, 6-12

WEDGE
 Right: 2 X 4 SYP No.3

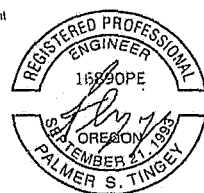
REACTIONS (lb/size) 2=1798/0-5-8, 9=1712/Mechanical
 Max Horz 2=38/(load case 3)
 Max Uplift 2=719/(load case 3), 9=821/(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/30, 2-3=3260/1153, 3-4=-2715/1092, 4-5=-2453/1026, 5-6=-2445/1019, 6-7=-4747/1645, 7-8=-5631/1589, 8-9=-5805/2108
 BOT CHORD 2-14=-905/2801, 13-14=-922/2742, 12-13=-870/2426, 11-12=-971/3629, 10-11=-1588/4932, 9-10=-1844/5195
 WEBS 3-14=0/264, 3-15=-595/386, 4-12=-104/130, 4-12=-452/326, 5-12=-725/1835, 6-12=-220/560, 6-11=-711/2514, 7-11=-080/435, 7-10=-504/32, 8-10=-108/227

- NOTES**
- 1) Unbalanced roof live loads have been considered for this design.
 - 2) Wind: ASCE 7-02, 110mph; h=18ft, TCD=1.2psf; BCDL=6.0psf; Category II; Exp B; enclosed, C-C Exterior(2); cantilever left and right exposed ; Lumber DOL=1.33 plate grip DOL=1.33.
 - 3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 4) Plates checked for a plus or minus 5 degree rotation about its center.
 - 5) A plate railing reduction of 20% has been applied for the green lumber members.
 - 6) Refer to girder(s) for truss to truss connections.
 - 7) This truss is designed in accordance with the 2003 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/TPI 1.
 - 8) This truss does not include any time dependant deformation for long term loading (creep) in the total load deflection. The building designer shall verify that this parameter fits with the intended use of this component.

LOAD CASE(S) Standard

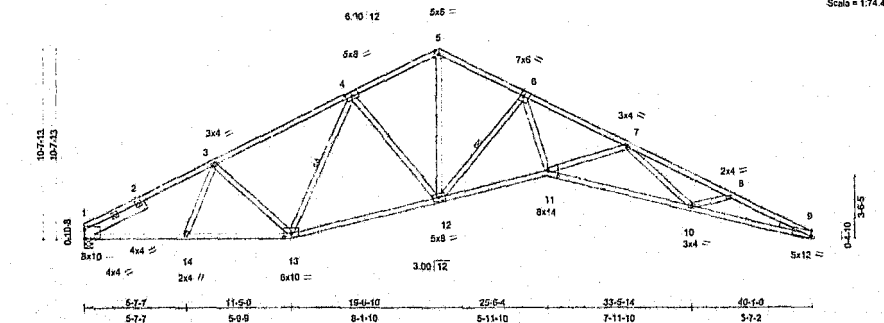
City of Portland
 REVIEWED FOR CODE
 COMPLIANCE
 JAN 03 2006
 Permit Number



EXPIRATION DATE: 06/30/08
 December 18, 2006

Job Reference (optional)

Scale 1:74.4



Plain Offsets (X,Y): [1:0-0-3-0-5-13], [4:0-4-0-0-3-0], [6:0-2-4-Edge], [9:0-1-7-Edge], [11:0-7-0-0-3-10], [13:0-5-0-0-2-4]

LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (in)	f'df'df	U/d	PLATES	GRIP
TCLL 25.0	Plates Increase	1.15	TC 0.82	Vert(LL)	-0.46	10-11	>999	MT20	220/195
BCDL	Lumber Increase	1.16	BC 0.81	Vert(TL)	-0.84	10-11	>73		
BCLL	Pop Strips Incr		VB 0.95	Horz(TL)	0.45	9	n/a		
TCOL 10.0	Col's IR22933/TP12932		(Weight)						

Weight: 204 lb

LUMBER

TOP CHORD 2 X 4 DF No.1&Btr.G

BOT CHORD 2 X 4 DF No.1&B1/ G

WEBS 2 X 4 DF Sld G *Excp1*

Уведомит

WEDGE
Dish 2-Y 4.5YB No 2

SLIDER 1 of 2 X 6 DE SS-G 3-B-12

BRACING

TOP CHORD Sheathed or 2-2-0 on purlins

BOT CHORD Rigid ceiling directly applied or 4-10-9 oc bracing.

WEBS

REACTIONS (lb/size) 1=1677/0-5-8, 9=1877/Mechanical
Max Horz 1=-4(load case 3)
Max Uplift 1=-609(load case 3), 9=-609(load case 3)

FORCES

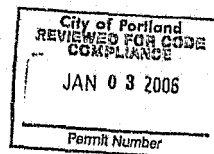
TOP CHORD
1-2=2883/1016, 2-3=2774/1040, 3-4=2515/1024, 4-5=2364/998, 5-6=2348/988, 6-7=4603/1508, 7-8=5498/1925,
8-9=5578/2067

BOT CHORD
1-14=780/2114, 13-14=.806/2396, 12-13=.833/2314, 11-12=.933/3513, 10-11=.1548/1802, 9-10=.1807/5082
3-14=.671/313, 1-13=.378/301, 4-13=.178/46, 4-12=.423/319, 5-12=.707/1773, 6-12=.2166/789, 6-11=.601/1254,
7-11=.661/336, 7-10=.534/338, 6-10=.112/231

NOTES

- 1) Unbalanced rock live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 110mph; $b=16ft$; $TCDL=4.2psf$; $BCDL=6.0psf$; Category II; Exp B; enclosed; C-C Exterior(2); cantilever loft and right exposed - Lumber DOL = 1.33 plates C-C
- 3) This truss has been designed for a 100 psf bottom chord live load nonconcurrent with any other live loads.
- 4) Plates connected for a plus or minus 5 degree rotation about its center.
- 5) A plate rafter reduction of 20% has been applied for the green lumber members.
- 6) All connections for truss to truss connections.
- 7) This truss is designed in accordance with the 2003 International Residential Code sections R502.11.4 and R802.10.2 and referenced standard ANSI/TPI 1.
- 8) This truss does not include any time dependent deformation for long term loading (creep) in the total load deflection. The building foundation and floor joists are assumed to be rigid.

LOAD CASE(S) Standard

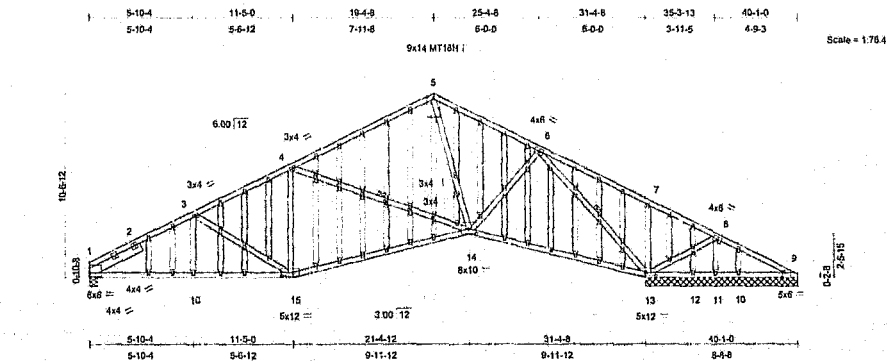


EXPIRATION DATE: 06/30/08

December 18, 2006

WARNING - Verify design parameters and READ NOTES ON TYPEN AND INCLUDE TYPEN REFERENCE PAGE MW-7473 BEFORE USE. Design valid for use only with the connections. This design is based only upon parameters shown, use for an individual building component is not intended. Proper installation, proper component and proper incorporation of component is responsibility of building designer. Not listed design. ERM, as shown is for lateral support of individual web members only. Additional temporary bracing to ensure stability during construction is the responsibility of the architect. Additional permanent bracing of the entire truss is the responsibility of the building designer. For general guidance regarding fabrication quality control, please refer to the American Institute of Steel Construction, Inc. (AISC) Specification for Structural Steel Buildings, 13th Edition, 2005, and AISC 360 Building Code of Practice for Steel Buildings and Bridges, 2005.

Mi
MiTek
A TOTAL SOLUTION FOR YOUR BUSINESS
7777 Greenback Lane, Suite 109
Culver City, CA 90230



LOADING (psf)	SPACING	2-0-0	CSI	DEFL.	in (lcc)	Modf	Lid	PLATES	GRIP
TCCL	Plates Increase	1.15	TC 0.53	Vert(LL)	-0.19	14-15	>999	360	MT20
TCOL	Lumber Increase	1.15	BC 0.59	Vert(TL)	-0.34	14-15	>999	180	MT18H
BCCL	Rep Stress Incr	NO	WB 0.86	Horz(TL)	0.08	13	n/a	n/a	220/195
ECOL	Code IRC2003/ITP2002		(Matrix)						220/195
									Weight: 350 lb

LUMBER
 TOP CHORD 2 X 4 DF No.18B/G
 BOT CHORD 2 X 4 DF No.18B/G
 WEBS 2 X 4 DF Sld G
 OTHERS 2 X 4 DF Sld G
 SLIDER Lst 2 X 6 DF SS G-3-3-11

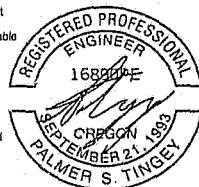
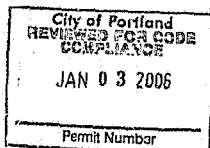
BRACING
 TOP CHORD Sheathed or 4-3-10 cc purlins.
 BOT CHORD Rigid ceiling directly applied or 6-0-0 cc bracing.
 WEBS 1 Row at midpt 4-14, 6-13

REACTIONS (lb/inch) 1=1244/0-5-6, 9=9/8-8-8, 13=2049/8-8-8, 12=75/8-8-8, 11=32/8-8-8, 10=10/8-8-8
 Max Uplift(1)=449(load case 3), 9=120(load case 4), 13=760(load case 3), 12=136(load case 2), 10=10(load case 3)
 Max Grav(1)=1244(load case 1), 9=85(load case 5), 13=2049(load case 1), 11=110(load case 2), 10=113(load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=2043/722, 2-3=1942/739, 3-4=1677/689, 4-5=1203/490, 5-6=1329/555, 6-7=58/665, 7-8=206/648, 8-9=93/939
 BOT CHORD 1-10=529/1677, 15-16=529/1677, 14-15=398/1536, 13-14=88/852, 12-13=344/132, 11-12=344/132, 10-11=344/132, 9-10=344/132
 WEBS 3-10=0/118, 3-15=268/183, 4-15=62/180, 4-14=602/383, 5-14=1477/91, 6-14=0/535, 6-13=2043/627, 7-13=355/288, 8-13=272/229

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-02; 110mph; 1st Bldg; TCOL=4.2psf; BCCL=6.0psf; Category II; Exp B; enclosed; C-C Exterior(2); cantilever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/ITP 1-2002.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - All plates are MT20 plates unless otherwise indicated.
 - All plates are 2x4 MT20 unless otherwise indicated.
 - Plates checked for a plus or minus 5 degree rotation about its center.
 - Gable slats spaced at 1-4-0 cc.
 - A plate rafter reduction of 20% has been applied for the green lumber members.
 - This truss is designed in accordance with the 2003 International Residential Code sections R502.11.1 and R502.10.2 and referenced standard ANSI/ITP 1.
 - This truss does not include any time dependent deformation for long term loading (creep) in the total load deflection. The building designer shall verify that this parameter fits with the intended use of this component.

LOAD CASE(S) Standard



EXPIRATION DATE: 06/30/08
 December 18, 2006

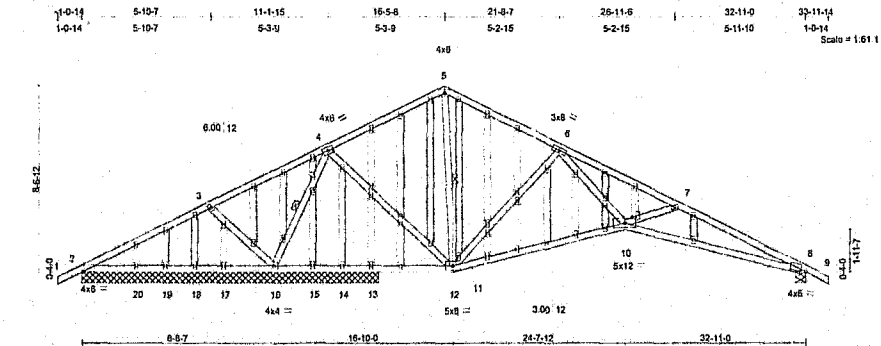


Plate Offsets (X,Y): [2-0-0-11, 8-6-7] [8-0-2-12, 0-0-11] [53-0-0-15, 0-2-4]									
LOADING (psf)	SPACING	2-0-0	CSI	DEFL	In (ft)	1/8th	L/d	PLATES	GRIP
TCCL 25.0	Plates Increase	1.15	TC 0.42	Ver(TL)	0.12	10-11	>999	MT20	220/195
TCBL 7.0	Lumber Increase	1.15	BC 0.51	Ver(TL)	-0.25	10-11	>947		
BCLL 0.0	Rep Stress Incr	NO	WB 0.84	Horz(TL)	0.09	8	n/a		
BCCL 10.0	Code IRC2003/TP12602		(Matrix)						
									Weight: 253 lb

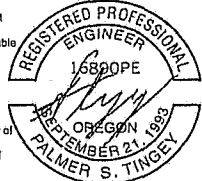
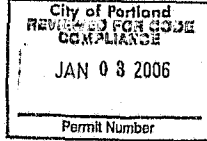
LUMBER	BRACING
TOP CHORD 2 X 4 DF No.18Bir G	TOP CHORD Sheathed or 4-1-3 oc purlins.
BOT CHORD 2 X 4 DF No.18Bir G	BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing. Except:
OTHERS 2 X 4 DF Sid G	10-0-0 oc bracing: 11-12, 10-11
	8-3-15 oc bracing: 8-10.
	WEBS 1 Row at midpt 4-16, 5-12

REACTIONS (lb/size) 2=204/13-5-8, 18=1930/13-5-8, 13=133/13-5-8, 14=80/13-5-8, 15=54/13-5-8, 17=37/13-5-8, 18=34/13-5-8, 19=20/13-5-8, 20=132/13-5-8, 8=880/0-8
 Max Horiz 2=20 (load case 3), 18=1930 (load case 3), 13=133 (load case 3), 14=80 (load case 1), 19=23 (load case 4), 20=39 (load case 3), 8=880 (load case 3)
 Max Uplift 2=377 (load case 6), 18=816 (load case 3), 13=17 (load case 3), 14=80 (load case 1), 19=23 (load case 4), 20=39 (load case 3), 8=880 (load case 3)
 Max Grav 2=54 (load case 4), 18=1930 (load case 1), 13=133 (load case 1), 14=53 (load case 3), 15=72 (load case 2), 17=77 (load case 2), 18=50 (load case 2), 19=42 (load case 3), 20=138 (load case 4), 8=880 (load case 2)

FORCES (lb) - Maximum Compression/Maximum Tension
 TOP CHORD 1-2=0/30, 2-3=248/1008, 3-4=323/1279, 4-5=386/308, 5-6=382/311, 6-7=194/1626, 7-8=235/831, 8-9=0/28
 BOT CHORD 2-20=873/323, 19-20=873/323, 18-10=873/323, 17-16=873/323, 16-17=873/323, 15-16=406/262, 14-15=406/262, 13-14=406/262, 12-13=406/262, 11-12=0/170, 10-11=141/890, 8-10=840/2091
 WEBS 5-16=360/302, 4-16=1790/889, 4-12=101/895, 5-12=53/87, 6-11=891/388, 6-10=269/1316, 7-10=365/337

- NOTES**
- Unbalanced roof live loads have been considered for this design.
 - Wind: ASCE 7-02; 110mph; Hs=18ft; TCCL=4.2psf; BCCL=6.0psf; Category II; Exp B; enclosed; C-C Exterior(2); can't/ever left and right exposed; Lumber DOL=1.33 plate grip DOL=1.33.
 - Truss designed for wind loads in the plane of the truss only. For studs exposed to Wind (normal to the face), see Standard Industry Gable End Details at applicable, or consult qualified building designer as per ANSI/PTP 1-2002.
 - This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - All plates are 2x4 MT20 unless otherwise indicated.
 - Gable studs spaced at 1-4-0 oc.
 - A plate rafter reduction of 20% has been applied for the green lumber members.
 - Boasting at joint(s) B considers parallel to grain value using ANSI/PTP 1 angle to grain formula. Building designer should verify capacity of boasting surface.
 - This truss is designed in accordance with the 2003 International Residential Code sections R502.11.1 and R802.10.2 and referenced standard ANSI/PTP 1.
 - This truss does not include any time dependent deformation for long term loading (creep) in the total load deflection. The building designer shall verify that this parameter fits with the intended use of this component.

Continued on page 2



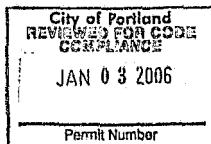
EXPIRATION DATE: 06/30/08
 December 18, 2006

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED LITER REFERENCE PAGE #1-7474 BEFORE USE.
 Design void for use only with Mittek connectors. This design is based on the parameters shown, and is for individual building component. Acceptability of design and proper installation of component is responsibility of building designer. Not for use in other applications. If for internal bracing of individual web members only. Additional temporary bracing is required during construction and erection of the structure. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, shipping, erection and bracing, consult ANSI/PTP Quality Criteria, P88-87 and ECP1 Building Component Safety Information available from Truss Plate Institute, 333 D'Onofrio Drive, Madison, WI 53719.

Mittek
 1777 Greenback Lane, Suite 100
 Citrus Heights, CA 95610

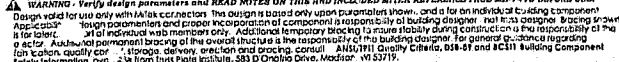
Job	Truss	Truss Type	Qty	Ply	RICK ZIMMERMAN	R2378631
800022599	709	GABLE	1	1	Job Reference (optional)	
Trux-Way Inc., Vancouver, WA 98668, Scott Cornwell					6/200 & Jul 13 2006 / Lafax Industries, Inc. Fri Dec 15 10:58:03 2006 Page 2	

LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE BEFORE USE.
 Design valid for use only with Mittek connectors. This design is based on the parameters shown, and is for an individual building component.
 Acceptability of design parameters and proper installation of component is responsibility of building designer, not Mittek designer. Bracing shown
 is for lateral support of individual wall members only. Additional temporary bracing to insure stability during construction is the responsibility of the
 erector. Additional permanent bracing of the erect structure is the responsibility of the building designer. For general guidance regarding
 fabrication, quality control, storage, delivery, erection and bracing, consult ANSMITH Quality Criteria, D14-01 and EC111 Building Component
 Safety Information available from Trus Plate Institute, 383 D Ordway Drive, Madison, WI 53719.

Mittek
 "TRUSS TO TRUSS"
 7177 Greenback Lane, Suite 109
 Citrus Heights, CA 95610



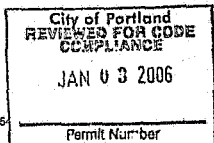
Job	Truss	Truss Type	Qty	Plf	RICK ZIMMERMAN	
B0602595	T12	SPECIAL	5	1		R23766534
Truss-Way Inc., Vancouver, WA 98008, Scott Connell					Job Reference (6-30-04) 6/30 x Jul 13 2005 Arflex Industries, Inc. Fri Dec 15 16:30:04 2006 Page 1	
	5-6-1	10-5-5	16-5-8	23-8-2	27-7-8	33-11-14
	5-6-1	4-11-3	6-0-3	7-3-10	3-10-3	5-3-11
						1-0-14
						Scale = 1/60.0

Plate Offsets (X,Y): [1:0-1-4,Edgn], [7:0-5-0,Edgn], [10:0-5-0-0-3-10], [11:0-7-0-0-2-4]									
LOADING (psf)	SPACING	2-0-0	C81	DEFL	In (in)	Udefl	L/d	PLATES	GRIP
TCLL 25.0	Plates Increase	1-15	TC 0.68	Ver(LL)	-0.32	10	>999	M720	220/195
TCR 7.1	Lumber Increase	0.72	BC 0.72	Ver(UL)	-0.68	10-1	>930		
BCLL 0.0	Rep Stress Incr	YES	WB 0.76	Horz(TL)	0.28	7	n/a		
BCDL 0.0	Code [RC2003/TP12002		(Metric)					Weight: 150 lb	

REACTIONS (lb/size) 1=1362/0.5-8, 7=1448/0.5-8
Max Horiz 1=-40(load case 3)
Max Uplift 1=-493(load case 3), 7=-592(load case 3)

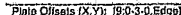
NOTES

- 1) Unbalanced roof live loads have been considered for this design.
- 2) Wind: ASCE 7-02; 10mph; H=18ft; TC=4; Z=20ft; BCDL=5.0psf; Category II; Exp B; enclosed; C-C Exterior(2); can/wave left and right exposed; Lumber DCL=1.33 plus 6% DCL=1.33.
- 3) This truss has been designed for a 100.0 psf bottom chord live load nonconcurrent with any other live loads.
- 4) Plates checked for a plus or minus 5 degree rotation: about all center.
- 5) A plate rating reduction of 20% has been applied for the green lumber members.
- 6) Beaming at joints(6) 7 considerers parallel to grain value using ANSI/FPI 1 angle to grain formula. Building designer should verify capacity of beaming surface.
- 7) This truss is designed in accordance with the 2003 International Residential Code sections R502.11.1, and R502.10.2 and referenced standard ANSI/FPI 1.
- 8) This design does not include any time dependent deformation for long term loading (creep) in the total load deflection. The building designer shall verify that this parameter fits with the intended use of this component.



EXPIRATION DATE: 06/30/08
December 18, 2006

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITER KNEE SHEET, PAGE III-7473 BEFORE USE. Design valid for use only with MITER connector. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. **NOT** truss design. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to allow stability during construction is the responsibility of the erector. Additional permanent bracing of the overall truss is the responsibility of the building designer. For general guidance regarding design of any truss, consult the American Institute of Steel Construction, Inc. (AISC) Manual of Steel Construction, 9th Edition, Part 5, Design of Trusses, 5.1-1 through 5.1-10. For more information, contact Philip H. Miller, SPS Building Group, Madison, WI 53717.



LUMBER	BRACING
TOP CHORD 2 X 4 DF No.1&Btr G	TOP CHORD Sheathed or 6-0-0 cc purlins.
BOT CHORD 2 X 4 DF No.1&Btr G	BOT CHORD Rigid ceiling directly applied or 10-0-0 cc bracing.
OTHERS 2 X 4 DF Sld G	

City of Portland
REVIEWED FOR CODE
COMPLIANCE
JAN 03 2006
Permit Number

NOTES

-

December 18, 2006

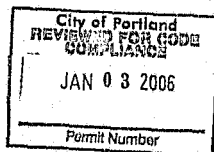
WARNING: Verify design parameters and READ WITH CARE THIS AND INCLUDED METER REFERENCE PAGE 101-7473 BEFORE USE. This design voids for use with other connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Instruct designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing is to be added during construction is the responsibility of the erector. Additional permanent bracing of the diaphragm structure is the responsibility of the building designer. For general guidance regarding design of bracing systems, see the American Institute of Steel Construction, Inc. (AISC) Manual of Steel Construction, 13th Edition, 2009, and AISC Building Companion. All information copyright by Inco Steel Institute, 55133 Oroville Drive, Modesto, CA 95370.

Job	Truss	Truss Type	City	Ply	RICK ZIMMERMAN	H23766036
00002568	T14	GABLE				
Truss-Way Inc., Vancouver, WA 98668, Scott Cornwell					Job Reference (optional) © 2003 & Jul 13 2005 Mitek Industries, Inc. Fri Dec 15 16:56:06 2006 Page 2	

NOTES

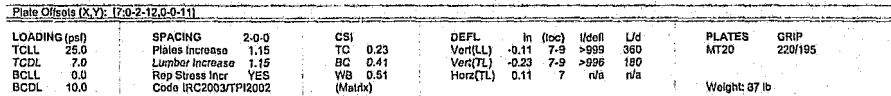
1) This truss does not include any time dependent deformation for long term loading (creep) in the total load deflection. The building designer shall verify that this parameter fits with the intended use of this component.

LOAD CASE(S) Standard



WARNING - Verify design parameters and READ NOTES ON STEEL AND INCLUDES MITEK REFERENCE PAGE M17473 BEFORE USE.
 Design valid for use only with Mitek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer. Not that designer, bracing shown is for lateral support of individual web member only. Additional temporary bracing to ensure stability during construction is the responsibility of the inspector. Additional permanent bracing of the overall structure is the responsibility of the building designer for general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSII/TPI Quality Criteria, D15-87 and ICC311 Building Component Safety Information available from Mitek Plate Institute, 5513 D'Oroville Drive, Madison, WI 53719.

MITEK
Mitek
 MADE IN THE USA
 7777 Greenback Lane, Suite 109
 Clous Heights, CA 95019

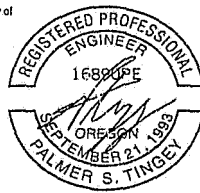
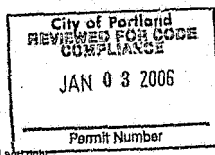


REACTIONS (lb/size) 2=901/0-5-8, 7=901/0-5-8.
Max Ho:2=2(load case 3)
Max Up/1/2=-392(load case 3), 7=-392(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/31, 2-3=-1463/515, 3-4=-1369/477, 4-5=-1468/548, 5-6=-2004/578, 6-7=-2417/826, 7-8=0/28
BOT CHORD 2-11=-370/1240, 10-11=-369/1386, 9-10=-188/1215, 7-9=-641/2167
WEBS 3-11=-76/100, 4-11=-274/121, 4-10=-168/148, 5-10=-135/274, 5-9=-201/1165, 6-9=-405/339

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-02; 110mph; $h=181'$; TCDL=4.2psf; BDDL=8.0psf; Category II; Exp B; enclosed; C-2 Exterior/2; cantilever left and right exposed ; Lumber DDL=1.33 plate grill DDL=1.33.
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) Details checked for a plus or minus 5 degree rotation about its center.
5) A plate rating reduction of 20% has been applied for the green lumber members.
6) Boring at joint(s) 7 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) This truss is designed in accordance with the 2003 International Residential Code sections R602.11.1 and R602.10.2 and referenced standard ANSI/TPI 1.
8) This truss does not include any time dependent deformation for long term loading (creep) in the total load deflection. The building designer shall verify that this parameter fits with the intended use of this component.

LOAD CASE(S) Standard



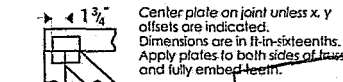
December 18, 2006

WARNING - Verify design parameters and READ NOTES ON THIS and INCLUDED MITEK REFERENCE PAGES M17-7473 BEFORE USE.
Design used for use only with Mitek connections. This design is based only upon parameters shown, and is not an individual building component. Applicability of design parameters and proper installation of all component is responsibility of building designer, not Mitek designer. Proper support of all components is required. Design is for use only with Mitek connections. Design is for use only with Mitek connections. All components are the property of the building designer. For general guidance, regarding fabrication, quality control, storage, delivery, erection and bracing, consult AHS/ITM Quality Criteria, Q55-87 and Q56-87 Building Component Installation, Maintenance and Repair.

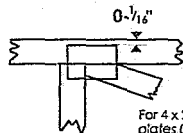


Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-1/4\" from outer edge of truss.



This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section and/or the output. Use T, I or Eliminator bracing if indicated.

BEARING

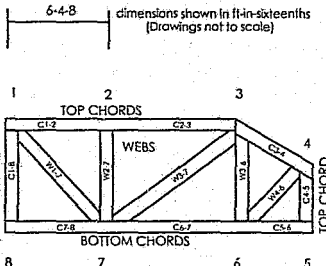


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCS11: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS ICC-ES Reports:

ESR-1311, ESR-1352, ER-5243, 9604B, 9730, 95-43, 96-31, 9667A
NER-487, NER-551
95110, 84-32, 96-67, ER-3907, 9432A

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MiTek
POWER TO PERFORM.™

MiTek Engineering Reference Sheet: MIL-7473

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

- Additional stability bracing for truss system, e.g., diagonal or X-bracing, is always required. See BCS11.
- Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative T, I, or Eliminator bracing should be considered.
- Never exceed the design loading shown and never stack materials on inadequately braced trusses.
- Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
- Cut members to bear tightly against each other.
- Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
- Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
- Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
- Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
- Cambar is a non-structural consideration and is the responsibility of truss fabricator. General practice is to cambar for dead load deflection.
- Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
- Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
- Top chords must be sheathed or purlins provided at spacing indicated on design.
- Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
- Connections not shown are the responsibility of others.
- Do not cut or alter truss member or plate without prior approval of an engineer.
- Install and load vertically unless indicated otherwise.
- Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
- Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
- Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.