

330 SW 6TH AVE

CO-01-150532

CO. 01,150532

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AUG 08 2001
MICROFILMED

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CITY OF
PORTLAND, OREGON
OFFICE OF PLANNING AND DEVELOPMENT REVIEW
1900 SW 4th Ave, Suite 5000
Portland, OR 97201



COMMERCIAL BUILDING PERMIT

01-150532-000-00-CO

Site Address: 330 SW 6TH AVE
330 SW 6TH-BIDWELL BLDG.

Issued: 8/3/01

PROJECT INFORMATION		Occ. Group	Const. Type
Business	Alteration	B	II-1HR
Project Description: REPAIR BUILT IN COPPER GUTTER SYSTEM AT ROOF'S EDGE. PEDESTRIAN PROTECTION ON SIDEWALK.			

APPLICANT CC&L ROOFING CO. *DAN CORNWELL*
OWNER H GERALD BIDWELL PROPERTIES LLC
CONTRACTOR CC&L ROOFING CO. *DAN CORNWELL*

Phone (503) 774-0928
Phone
Phone

Project Details		Project Details	
Alarm System Required?	Yes	Code Edition (Year)	1997
Ground Disturbance Area (Sq. Ft.)	102611	Lot Area (Sq. Ft.)	5000
Number of Stories	4	Smoke Detectors Required?	Yes
Sprinkler System Required?	Yes	Water District	City of Portland
Zoning - Property (1)	CXdkCC		

PAID
AUG - 3 2001
CITY OF PORTLAND

**BEFORE
YOU DIG**

ATTENTION: Oregon law requires you to follow rules adopted by the Oregon Utility Notification Center. These rules are set forth in OAR 922-001-0010 through OAR 922-001-0090. You may obtain copies of the rules by calling the center. (Note: the telephone number for the Oregon Utility Notification Center is 1-800-332-2344).

CITY CONTACT

Hansen, Kelth

Phone: 503-823-0655

E-Mail:

Fax: (503) 823-4172

**INSPECTION REQUEST
PHONE NUMBERS**

Building/Trade Inspections - Call Before 6:00 AM:

(503) 823-7000

TDD: (503) 823-6868

**IVR Inspection Request
Number:**

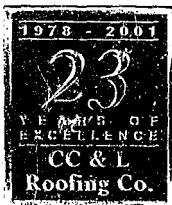
2113588

Proposal and Contract

CC&L Roofing Co.

3319 SE 92nd Avenue Portland, Oregon 97266-1924
(503) 774-0928 (360) 695-3719 FAX (503) 774-1835
(OR) 46625 (WA) CCLROC*163R9

EMAIL dan@ccandlroofing.com
www.ccandlroofing.com



RECEIVED
7-19-2001

Name Bidwell and Company Date June 22, 2001
Address 320 SW 6th Avenue City Portland State OR Zip 97204-1702
Job Address Same as above City State Zip

Member/Director

WSRCA
Western States
Roofing Contractors
Association

Member

NRCA
National Roofing
Contractors Association

ARC
Associated Roofing
Contractors of
Oregon and SW
Washington

HBA
Home Builders
Association of
Metropolitan Portland

CAI
Community
Associations
Institute

BBB
Better Business
Bureau

ASTM
American Society
for Testing and
Materials

NFIB
National Federation
of Independent
Business

ORA
Oregon Remodelers
Association

We propose to furnish all materials, equipment, and labor necessary to complete the following tile roofing repair and gutter replacement for the Bidwell Building at the above address, which includes:

- 1) Purchase building permits, sidewalk permits for Stark and 6th Streets and street use parking permits along Stark Street adjacent building for hoists and equipment. Set scaffolding on the sidewalk to extend up to the building eave line. Scaffolding to have overhead protection walk through canopy frames at the street level on Stark Street and walk-through tunnels to the building entries on 6th Street. Scaffolding will have a fully planked top deck with toe boards and mesh screen at the handrails to prevent debris from falling on pedestrians below. Scaffolding will also have plywood sides 8 ft. high at street level to prevent pedestrians from climbing up scaffolding or accessing area between scaffold and building. Scaffolding will be set upon Alumna Beams blocked up over structural beams under sidewalk. Note: Engineering will be required to verify location and capacity of structural sidewalk beams. Scaffold load is 2,400 lbs. every 10 ft. Engineering costs are not included in the following price quotations.
- 2) At a point approximately 7 ft. up roof slope from exterior eave line and 3 ft. 6 inches above the built-in gutter, remove roof tile (approximately the 6 bottom courses) and stack upon upper tile resting against 2x6 toe boards set in brackets and attached to vertical 2x3 roof tile nailer strips. At a point approximately 6 ft. 6 inches up the roof slope and 3 ft. above the built-in gutters, cut off and remove existing vertical 2x3 nailer strips which span over built-in gutters and haul away.
- 3) Remove existing built-in copper gutter and haul away. Inspect, remove and replace any deteriorated existing 3/4" Fir Flooring gutter liner with new 3/4" CDX plywood. (excludes replacement of any wood nailer strips embedded in concrete) Install Ice & Water shield self-sealing underlayment at built-in gutter over wood sheathing. At sloped roof area between eave and built in gutter install modified bitumen single ply roofing membrane over existing substrate.
- 4) Fabricate and install new 20 ounce copper gutters in shop formed profile to match existing. Install gutters in 26'0" lengths with riveted soldered joints and new soldered outlets to drain into existing downspouts. At roof slope immediately above built-in gutter, install modified bitumen underlayment over gutter flange and under existing underlayment if it can be cut and lifted. If not, install roofers mastic and fiberglass mesh to seal new underlayment to existing as effectively as possible, and/or torch apply new membrane over existing as possible.

- 5) Install new pressure-treated 2x3 nailer strips over built-in gutters and new underlayment to replace previously removed nailer strips. Fasten 2x3's with nails into existing wood roof sheathing to match existing installation.
- 6) Reinstall existing roof tile over new 2x3's and underlayment, spanning over new built-in gutters. Wire bottom pan tile courses at eave and above gutter in place with stainless steel tie wire fastened to nailer strips. Fasten remaining cap tile in place with 11 gauge copper nails and copper wire storm hooks fastened into 2x3 nailer strips with pan tiles locked in place between cap tiles and nailer strips. At final replacement cap tile course, drill new nail holes in tile just below the head lap of the upper tile course. Install a one-inch wide strip of sheet lead over upslope nose of cap tile and extending from under upper course to cover new nail hole. Seal around head of nail with caulking and fold lead tab over nail head and sealant. Provide and install new Gladding McBean #8 Mix tile to replace breakage as required.
- 7) Remortar roof hips as necessary.
- 8) Material and labor complete. Clean up and haul away all debris.

Alternate. Remove existing exterior gutters and install new exterior gutters in lieu of new built-in gutters. Cover over existing gutters with 22 gauge steel, modified bitumen underlayment, 2x3 nailer strips, and roof tile. Install new 26 foot length custom formed 20 ounce copper gutters with soldered seams set in 3/16"x1" copper flat bar brackets at approximately 3'0" on center. Install new exterior 3"x4" 16 ounce copper downspouts to replace existing. Add \$2,421.00 to the submitted price below.

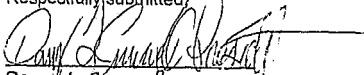
Acceptance of Alternate _____

All the above work to be completed in a workmanlike manner according to standard practices for the sum of ONE HUNDRED TWO THOUSAND SIX HUNDRED ELEVEN AND 00/100 DOLLARS (102,611.00).
Payable net 10th for work completed by the 30th of the previous month

We may withdraw this proposal if not accepted within thirty (30) days. Any deviation from the above specifications will be executed only upon written orders, and may become an extra charge over the sum as set forth in the contract.

CC&L Roofing Company agrees to carry Workman's Compensation and Public Liability Insurance, also to pay all Sales Taxes, Old Age benefit and Unemployment Compensation Taxes upon the material and labor furnished under this contract, as required by the United States Government and the State in which this work is performed.

Respectfully submitted,



Danny L. Cornwell

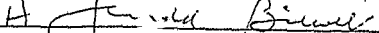
President

ACCEPTANCE

You are hereby authorized to furnish all materials and labor required to complete the work specified in the above proposal, for which the undersigned agrees to pay the amount as shown above and in accordance with the terms therein.

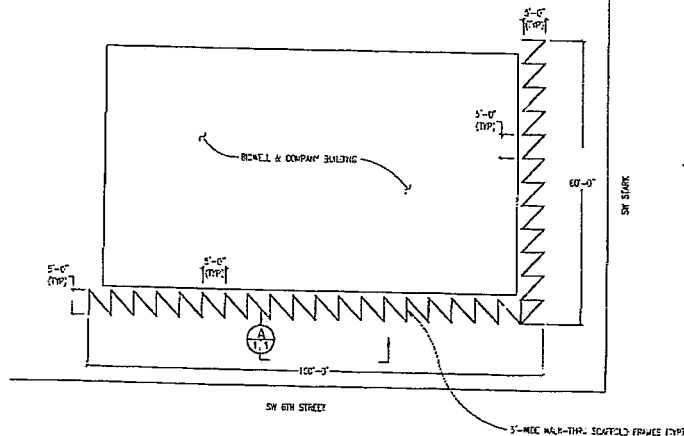
I acknowledge having received a copy of the State Construction Contractors Board Information Notice to Owner about Construction Liens.

It is further agreed that if the charges made shall remain unpaid for a period of 30 days beyond the pay period above checked, that any balance so remaining unpaid shall be subject to a monthly late payment charge and shall bear interest at the highest legal rate permitted by law. In the event that any suit or action is instituted to collect any amount due under my account, whether principle or interest or both, I agree to pay in addition to the costs provided by law, a reasonable sum as attorney's fees.

Accepted by,  Date 7-18 2001

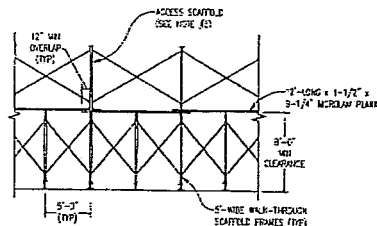
Proposal and contract to Bidwell and Company dated June 22, 2001

PEDESTRIAN PROTECTION CANOPY
BIDWELL & COMPANY BUILDING
CORNER OF S.W. 6TH STREET AND S.W. STARK
PORTLAND, OR
SUNBELT RENTALS



NOTES:

1. CANOPY IS DESIGNED FOR PEDESTRIAN PROTECTION ONLY AND SHALL NOT BE USED AS ACCESS SCAFFOLDING.
2. CANOPY SCAFFOLD SHALL BE 20' x 5'-WIDE WALK-THRU SCAFFOLD FRAMES.
3. CANOPY FRAMING SHALL BE 1-1/2" x 5-1/2" MICRO-LAM SCAFFOLD FRAMES. OVERLAP PLATING A MINIMUM OF 12" IN BOTH DIRECTIONS.
4. ACCESS SCAFFOLD ABOVE CANOPY FRAMES SHALL BE LIMITED TO 7 FRAMES HIGH WITH A MAXIMUM LIVE LOAD OF 25 PSF ON 1 LEVEL AT ANY ONE TIME.
5. ACCESS SCAFFOLD NOT CHECKED BY D.H. CHARLES ENGINEERING, INC.



REVISIONS
NO. DATE

PEDESTRIAN PROTECTION CANOPY
BIDWELL & COMPANY BUILDING
CORNER OF S.W. 6TH STREET AND
S.W. STARK
PORTLAND, OR
SUNBELT RENTALS

SUNBELT
RENTALS
A Sunbelt Industries Company
1000 N. 10th Street
Portland, OR 97227
Phone: (503) 244-4000
Fax: (503) 244-5112

AMERICAN
INSTITUTION
OF STEEL CONSTRUCTION
INC.
1015 N. 17th Street
Portland, OR 97227
Phone: (503) 244-4000
Fax: (503) 244-5112

DETC
DESIGN ENGINEERING
1015 N. 17th Street
Portland, OR 97227
Phone: (503) 244-4000
Fax: (503) 244-5112

DATE: 7-30-01
DRAWN BY: CPS
CHECKED BY: RJS
DRAWING NO: 01-580

NOTES:

1. CANOPY IS DESIGNED FOR PEDESTRIAN PROTECTION ONLY AND SHALL NOT BE USED AS ACCESS SCAFFOLDING.
2. CANOPY SCAFFOLD SHALL BE ABLE 5'-WIDE WALK-THRU SCAFFOLD FRAMES.
3. CANOPY PLANKING SHALL BE 1-1/2" x 9-1/2" MICRO-LAM SCAFFOLD PLANKS. OVERLAP PLANKING A MINIMUM OF 12" IN BOTH DIRECTIONS.
4. ACCESS SCAFFOLD ABOVE CANOPY FRAMES SHALL BE LIMITED TO 7 FRAMES HIGH WITH A MAXIMUM LIVE LOAD OF 25 PSF ON 1 LEVEL AT ANY ONE TIME.
5. ACCESS SCAFFOLD NOT CHECKED BY D.H. CHARLES ENGINEERING, INC.

REVISIONS

NO.	DATE

PEDESTRIAN PROTECTION CANOPY
BIDWELL & COMPANY BUILDING
CORNER OF S.W. 6TH STREET AND
S.W. STARK
PORTLAND, OR
SUNBELT RENTALS

SUNBELT
RENTALS
Access Specialists

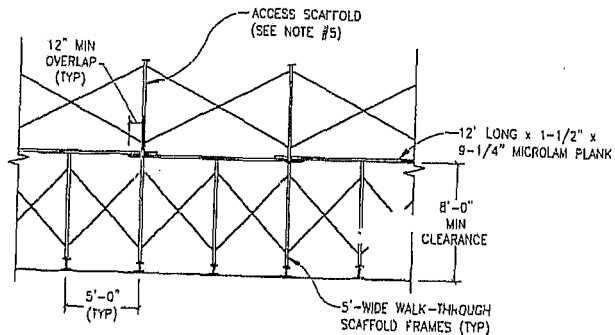


7626 NE KELINGSMORR
PORTLAND, OR 97218
PHONE (503) 256-9800
FAX: (503) 256-0762



D.H. CHARLES ENGINEERING, INC.
4327 Montgomery Drive, Suite M
Santa Rosa, CA 95409
PL (707) 537-8382 Fax: (707) 537-4318

DATE: 7-30-01
DRAWN BY: CPS
CHECKED BY: RJG
DRAWING NO.: 01-580



ELEVATION
SCALE: 1/8"=1'-0"

SW STARK

SCAFFOLD FRAMES (TYP)

NOTES:

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5. ACCESS SCAFFOLD NOT CHECKED BY D.H. CHARLES ENGINEERING, INC.

REVISIONS

NO.	DATE

PEDESTRIAN PROTECTION CANOPY
BIDWELL & COMPANY BUILDING
CORNER OF S.W. 6TH STREET AND
S.W. STARK
PORTLAND, OR
SUNBELT RENTALS

SUNBELT
RENTALS
Access Specialties



7626 NE KELINGSMITH
PORTLAND, OR 97218
PHONE: (503) 256-4900
FAX: (503) 256-0762



DH
D.H. CHARLES ENGINEERING, INC.
4027 Montgomery Drive, Suite M
South Korea, CA 94409
Ph: (707) 574-8338 Fax: (707) 574-8338

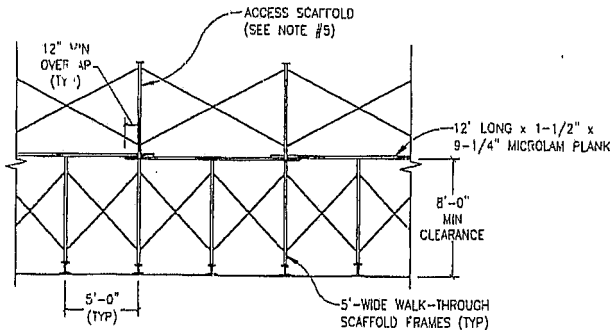
DATE: 7-30-01

DRAWN BY: CPS

CHECKED BY: RJG

DRAWING NO.:
01-580

SW STARK



A
ELEVATION
SCALE: 1/8"=1'-0"

SCAFFOLD FRAMES (TYP)



4527 Montgomery Dr., Suite M
Santa Rosa, CA 95409
(707) 537-8282 (Office)
(707) 537-8338 (Fax)

Pedestrian Canopy Check

Bidwell & Company Bldg. Portland OR

Sunbelt Rentals

Sheet 1 of 2

Date: 7/30/01

Revision: _____

Pedestrian Canopy Check:

Contractor will erect pedestrian canopies under access scaffolding around the perimeter of the building facing the street sidewalks.

Canopy consists of timber scaffold planks supported by 5'-wide walk-thru scaffold frames 5' o.c. Check canopy of 150 psf live load with a 2.5:1 factor of safety.

Check Timber Scaffold planks:

Use 1-1/2" x 9-1/2" Micro-Lam scaffold planks.

$$F_b = 2900 \text{ psi}$$

$$F_v = 145 \text{ psi}$$

$$W = (150 \text{ psf})(9.5"/12) = 119 \text{ plf}$$

$$M = (119 \text{ plf})(5')^2/(8) = 372 \text{ ft-lb}$$

$$V = (119 \text{ plf})(5')(1/2) = 300 \text{ lb}$$

$$f_b = (372 \text{ ft-lb})(12)/(3.56 \text{ in}^3) = 1253 \text{ psi} < 2900 \text{ psi} \quad \text{OK}$$

$$f_v = (300 \text{ lb})(1.5)/(14.3 \text{ in}^3) = 31.5 \text{ psi} < 145 \text{ psi} \quad \text{OK}$$

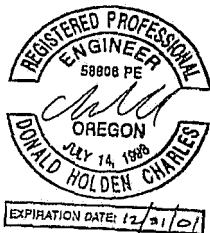
$$\text{Bending factor of safety} = (2900 \text{ psi})/(1253 \text{ psi}) = 2.3$$

Note, the allowable bending stress includes at least a 1.5 Factor of Safety also; therefore the combined factor of safety is at least $(2.3)(1.5) = 3.5$ OK

$$\text{Shear factor of safety} = (145 \text{ psi})/(31.5 \text{ psi}) = 4.6 > 2.5 \quad \text{OK}$$

Check 5'-Wide Scaffold frames:

Use 5'-wide Able scaffold frames.





4527 Montgomery Dr., Suite M
 Santa Rosa, CA 95409
 (707) 537-8282 (Office)
 (707) 537-8338 (Fax)

Pedestrian Canopy Check

Bidwell & Company Bldg. Portland OR

Sunbelt Rentals

Sheet 2 of 2

Date: 7/30/01

Revision: _____

Distributed load on the frame ledger:

$$P = (150 \text{ psf})(5')(5') = 3750 \text{ lb}$$

$$P_{\text{allow}} = 5100 \text{ lb} > 3750 \text{ lb} \quad \text{OK}$$



Includes a 4:1 factor of safety

Leg loads:

Include the single level of live load on the access scaffold above.

$$\text{Access scaffold live load} = (25 \text{ psf})(10')(5')(1/2) = 625 \text{ lb}$$

Access dead load:

$$\text{Frames: } (7 \text{ levels})(60 \text{ lb})(1/2) = 210 \text{ lb}$$

$$\text{Planking: } (6 \text{ psf})(1 \text{ level})(25 \text{ ft}^2) = 150 \text{ lb}$$

$$\text{X-bracing } (25 \text{ lb})(7 \text{ levels})(1/2) = 88 \text{ lb}$$

$$\text{Total dead load} = 448 \text{ lb}$$

$$\text{Canopy live load} = (150 \text{ psf})(5')(5')(1/2) = 1875 \text{ lb}$$

Canopy dead load:

$$\text{Frames: } (1 \text{ levels})(60 \text{ lb})(1/2) = 30 \text{ lb}$$

$$\text{Planking: } (6 \text{ psf})(1 \text{ level})(13 \text{ ft}^2) = 78 \text{ lb}$$

$$\text{X-bracing } (25 \text{ lb})(1 \text{ levels})(1/2) = 13 \text{ lb}$$

$$\text{Total dead load} = 121 \text{ lb}$$

$$\text{Total load} = 625 \text{ lb} + 1875 \text{ lb} + 448 \text{ lb} + 121 \text{ lb} = 3069 \text{ lb}$$

$$P_{\text{allow}} = 5000 \text{ lb} > 3069 \text{ lb} \quad \text{OK}$$



Includes a 4:1 factor of safety

- END -

UNTREATED - CANADA

50 psf	8'-0"	11'-6"	11'-6"
75 psf		10'-0"	10'-0"
500 lb.		8'-6"	9'-6"
Workers & Tools (25 psf + 250 plf)		10'-6"	10'-6"
Workers & Materials (75 psf + 265 plf)		8'-0"	8'-0"
50 psf		10'-6"	10'-6"
75 psf		9'-0"	9'-0"
500 lb.		7'-0"	8'-6"
Workers & Tools (25 psf + 250 plf)		9'-6"	9'-6"
Workers & Materials (75 psf + 265 plf)		7'-6"	7'-6"

1. Live load deflection is limited to $L/80$.
2. No increases in allowable unit stresses are included for load-sharing systems.
3. PLF loads are applied across the plank direction at centerline.

MICROLLAM® LVL SCAFFOLD PLANK DESIGN PROPERTIES

1½" and less	Greater than 1½"	1½" and less	Greater than 1½"
2500 psi	2400 psi	2500 psi	2400 psi
2.15×10^6	2.15×10^6	2.15×10^6	2.15×10^6
745 psi	745 psi	745 psi	745 psi
12%	12%	12%	12%

1. Design properties are determined in accordance with ANSI A10.8-1988, Appendix C.

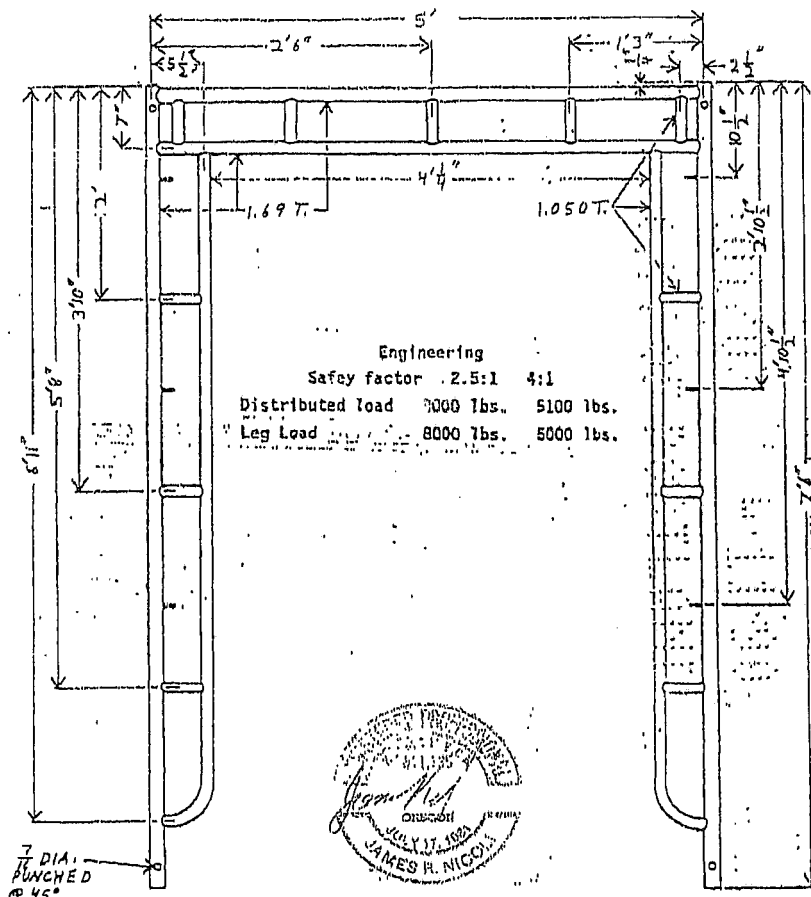
2. These properties apply only to Microllam® LVL scaffold planks used in conditions where the moisture content of the plank is not expected to exceed 19%. These values apply only to planks used in the flat orientation.

3. F_b , F_w , and E shall be adjusted by a factor of 0.80 when conditions of use are such that the moisture content of the plank is expected to exceed 19%.

4. Fastener values (nails, bolts, screws) shall be as provided for sawn Douglas fir per CAN-086-M84, *Engineering Design and Wood (working stress design)*.

From:

07/30/2001 10:58 #087 P.003



5' x 7' CANOPY FRAME

SUNBELT RENTALS, INC.
 P.O. # 325
 7628 N.E. KILLINGSWORTH
 PORTLAND, OR 97218



July 3, 2001
Revised July 16, 2001

Mr. Dan Cornwell
CC&L Roofing Company
3319 SE 92nd Avenue
Portland, OR 97266

RE: Scaffolding Loads on Existing Sidewalk Beams
Bank of California Building
SW Sixth and Stark Portland, OR

Dear Dan:

We have reviewed the capacity of the existing sidewalk beams for additional loads from scaffolding for the upcoming re-roofing of the building as requested. We based our analysis on existing building drawings dated March 1924, and on our conversation during last weeks meeting with yourself and the engineer for the scaffolding.

We understand that the scaffolding engineer will lay out the scaffolding in a manner such that no load will be imposed on the existing slab. The existing slab has negligible reserved capacity for scaffolding loads, as we understand the sidewalk will remain open to pedestrians during the re-roofing project.

It is our understanding that two point loads will be imposed on each existing steel beam. A plan of the existing floor framing is attached. Per our discussion, we have assumed that the loading condition be as follows for the two street sides:

- Sixth Avenue: The first load will be approximately 3 feet from the face of the existing building and the second load 5 feet away from the first (8 feet from face of building). The existing 15" deep steel beams have the capacity to support point loads of 2,300 pounds each with this loading configuration. This is in addition to 150 psf pedestrian traffic live load.
- Stark Street: The first load will be approximately 1 foot from the face of existing building and the second load 5 feet away from the first (6 feet from face of building). The existing 12" deep steel beams have the capacity to support point loads of 1,700 pounds each in addition to pedestrian traffic.

If you have any questions or need additional information, please do not hesitate to call.

Sincerely,


Anne Birgitte Monnier, P.E.
Associate

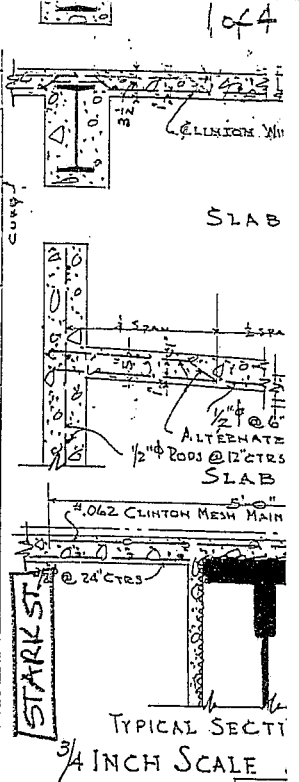
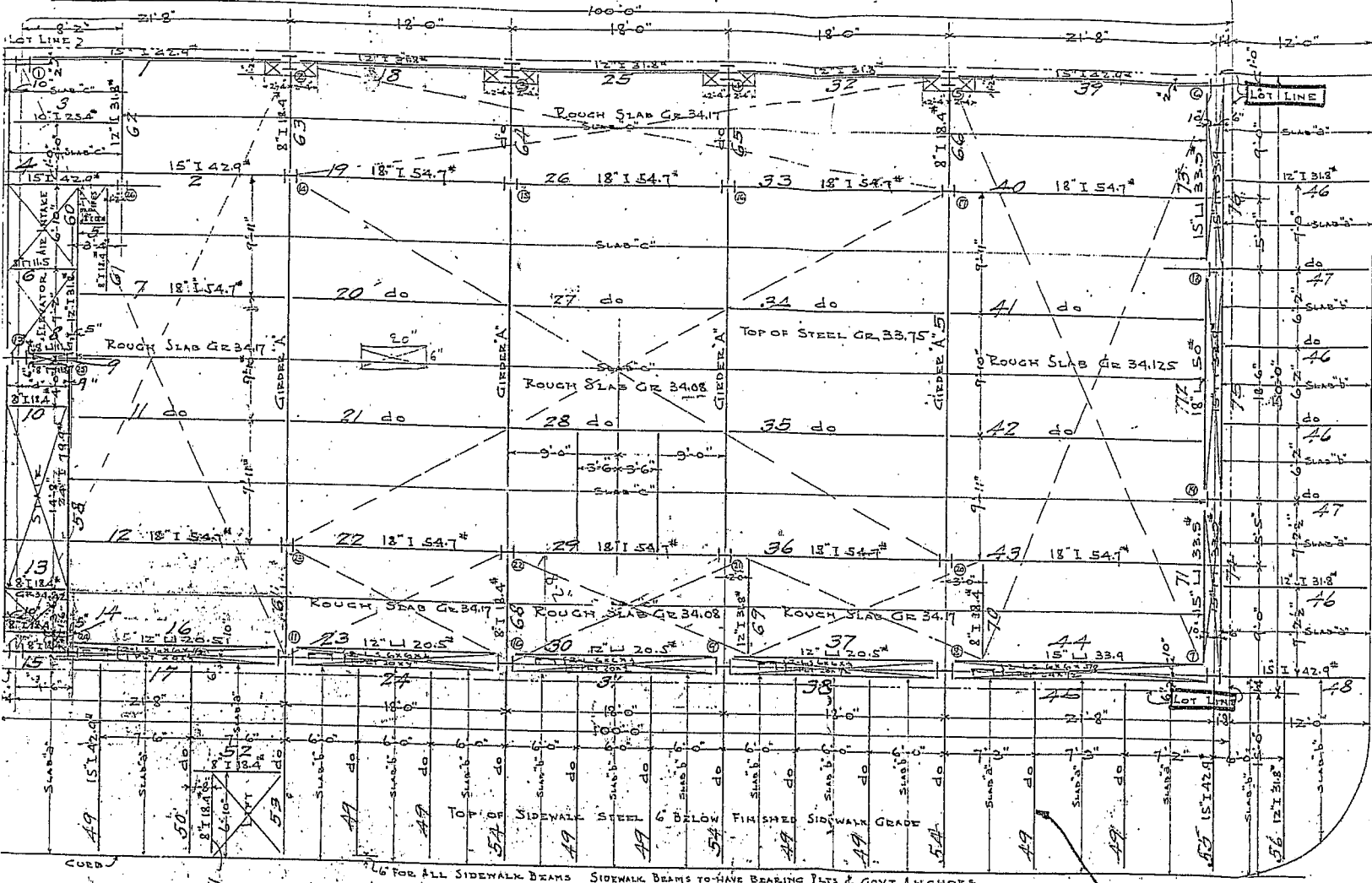
Attachments



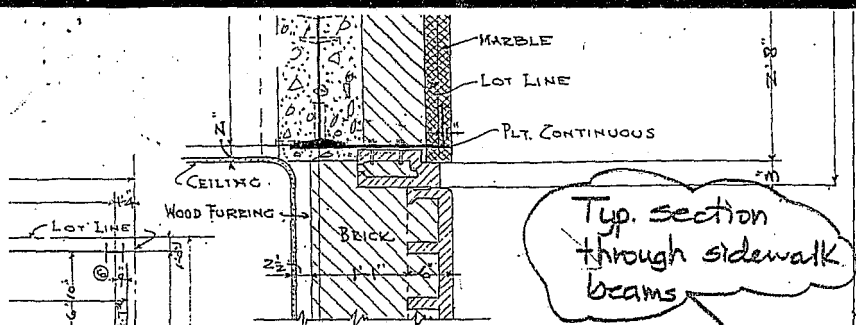
(AM-201329/Scaffolding Loads Letter.doc)

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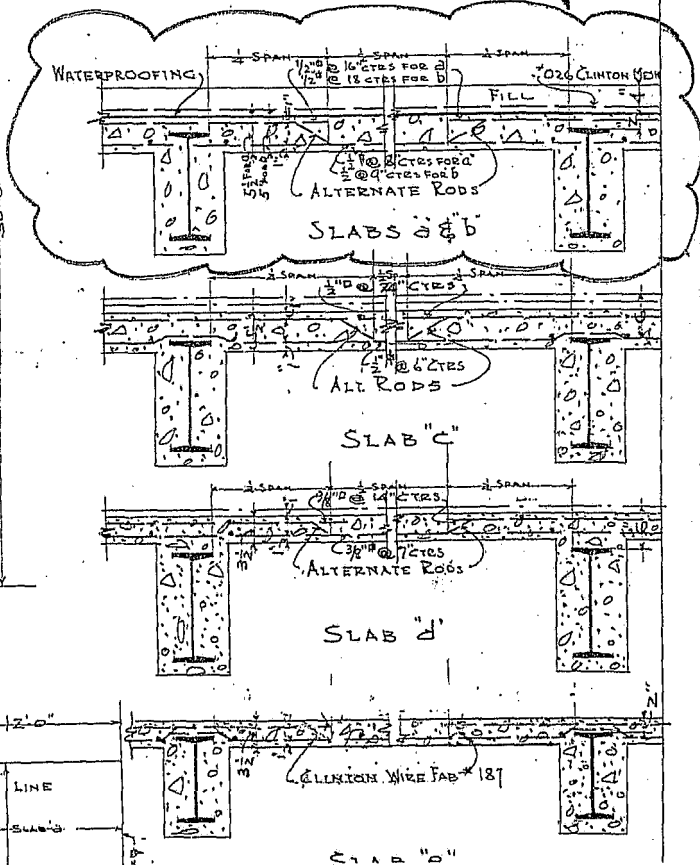
02-150532-2



STRUCTURAL
BUILDING - I
BANK OF CALIFORNIA
NATIONAL ASSOCIATION
SIXTH & STARK
A. E. DOYLE
JOB 408 MAR 1924



$\frac{3}{4}$ INCH SCALE SECTION OF CORNICE



SLAB "C"

SLAB "A"

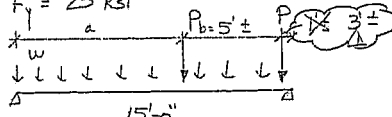
У. А. Д. "Д"

Project	B. OF CA (Bidwell Building)	By	ADM	Sheet No.	3 of 4
Location		Date	7-2-01		
Client		Revised	7-16-01	Job No.	

CAPACITY OF (E) STEEL BEAMS AT SIDEWALK :

AT 6th Ave :

- (E) 15" I 42.9 # (American Standard)
- 15' span
- $F_y = 25$ Ksi



$$W_{DL} = \left(\frac{5.5 + 4}{12} \right) 150 \text{ pcf} \times 7.5 \text{ ft} + 42.9 \text{ #/ft}$$

$$+ \left(\frac{12 \times 24}{144} \right) \times 150 \text{ pcf}$$

$$= 890 + 42.9 + 300 \text{ plf}$$

$$= 1.23 \text{ Klf}$$

$$W_{LL} = 150 \text{ pcf} \times 7.5 \text{ ft} \quad (1997 \text{ OESC Table 16-8})$$

$$= 1.13 \text{ Klf}$$

$$M_{cap} = S \times F_b = 58.9 \times (0.66 \times 25)$$

$$= 97.2 \text{ K-in} = 81.0 \text{ K-ft}$$

$$M_w = \frac{1}{8} \times (1.23 + 1.13) (15)^2 = 66.4 \text{ K-ft}$$

Reserved bending capacity for P :

$$M_{cap, P} = 81.0 - 66.4 = 14.6 \text{ K-ft}$$

$$M_p = P \times \frac{a-b}{4} \Rightarrow \max P = \frac{14.6 \text{ K-ft} \times 15 \text{ ft}}{\left(\frac{9 \times 6}{4} \right) + \left(\frac{1 \times 1}{4} \right)}$$

$$\Rightarrow P_{\max} = \frac{3,200 \text{ #}}{2,300 \text{ #}}$$

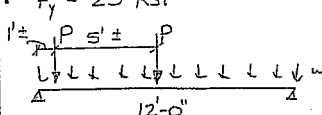
Check deflection :

$$\Delta_{LL} = \frac{5}{384} \times \frac{1.13 \left(\frac{1}{2} \right) (15 \times 12)^4}{29,000 \times 441.8} + \frac{3.2 \times 9 \times 6 \left(9 \times 2 \times 6 \right) (12)^3 \sqrt{3 \times 12 \left(9 \times 12 + 2 \times 6 \times 12 \right)}}{27 \times 29,000 \times 441.8 \times 15 \times 12}$$

$$= 0.10 + 0.03 = 0.13" \sim 1/393 < 1/360 \text{ OK}$$

AT Stark St :

- (E) 12" I 31.8 #
- 12' span
- $F_y = 25$ Ksi



$$W_{DL} = 1.23 \text{ Klf} - \text{cons.}$$

$$W_{LL} = 1.13 \text{ Klf}$$

$$M_{cap} = S \times F_b = 36.0 \times (0.66 \times 25)$$

$$= 59.4 \text{ K-in} = 49.5 \text{ K-ft}$$

$$M_w = \frac{1}{8} \times (1.23 + 1.13) (12)^2 = 42.5 \text{ K-ft}$$

Reserved bending capacity for P :

$$M_{cap, P} = 49.5 - 42.5 = 7.0 \text{ K-ft}$$

$$\max P = \frac{7.0 \times 12}{(1 \times 11) + (6 \times 6)}$$

$$\Rightarrow P_{\max} = 1,700 \text{ #}$$

Check deflection :

$$\Delta_{LL} = \frac{5}{384} \times \frac{1.13 \left(\frac{1}{2} \right) (12 \times 12)^4}{29,000 \times 215.8} + \frac{1.7 \left(\frac{1}{2} \right) (12 \times 12)^3}{41 \times 29,000 \times 215.8}$$

$$= 0.08 + 0.02 = 0.10" \sim 1/1223 \text{ OK}$$

kprll

Consulting Engineers

Richard Dwyer

Project: E OF CA (Bidwell L. dng)

By ABM

Sheet No.

4 of 4

Location

Date 7-2-01

Client

Revised

Job No.

Date

Check shear :

$$R_{max} = 5(1.23 + 1.13) \times 15' + \frac{9}{15} \times 3.2^k + \frac{14}{15} \times 3.2^k$$

$$= 22.6^k$$

$$f_v = \frac{22.6^k}{12.3 in^2} = 1.8 \text{ ksi} \ll \text{OK}$$

$\therefore (E) 15" I 42.9 \#$ Beams adequate
for 3200 # point loads from scaffolding

Check shear :

$$R_{max} = \frac{1}{2} \times (1.23 + 1.13) \times 12' + \frac{1}{2} \times 1.7^k + \frac{11}{12} \times 1.7^k$$

$$= 16.6^k$$

$$f_v = \frac{16.6^k}{9.3 in^2} = 1.8 \text{ ksi} \ll \text{OK}$$

$\therefore (E) 12" I 31.8 \#$ Beams adequate
for 1,700 # point loads from scaffolding

LARGE CONCRETE T.C. TILE
1/2" T.G. FLOORING
1/2" WOOD JOISTS 16" O.C.
CONG. FILL

ROUGH CAST CEMENT PLASTER

CONG. WALL
1/2" EDD & 1/2" CHS BOTH WALLS

COPPER FLASHING
20 YRS. PAROS ROOF

COPPER GUTTER

ROUGH CAST CEMENT PLASTER

SEC'D BEAMS NO. 5'S 2-1/2" EDD CONTINUOUS

CONG. CORNG.

CONG. WALL
1/2" EDD & 1/2" CHS BOTH WALLS

COPPER FLASH

TILE ROOF 2

1/2" EDD CONTINUOUS

COPPER FLASHING

TILE ROOF 2

1/2" EDD CONTINUOUS

COPPER FLASHING

TILE ROOF 2

1/2" EDD CONTINUOUS

COPPER FLASHING

TILE ROOF 2

1/2" EDD CONTINUOUS

COPPER FLASHING

TILE ROOF 2

1/2" EDD CONTINUOUS

COPPER FLASHING

TILE ROOF 2

1/2" EDD CONTINUOUS

COPPER FLASHING

TILE ROOF 2

1/2" EDD CONTINUOUS

COPPER FLASHING

TILE ROOF 2

1/2" EDD CONTINUOUS

COPPER FLASHING

TILE ROOF 2

1/2" EDD CONTINUOUS

COPPER FLASHING

TILE ROOF 2

1/2" EDD CONTINUOUS

COPPER FLASHING

TILE ROOF 2

1/2" EDD CONTINUOUS

COPPER FLASHING

TILE ROOF 2

1/2" EDD CONTINUOUS

Replace existing copper gutter
w/ new to match ext.

LARGE CONCRETE T.C. TILE
NO. 2X7/4 1/2" EDD

1/2" WOOD JOISTS 16" O.C.
1/2" T.G. FLOORING
CONG. FILL

6 1/2"

1/2" WOOD JOISTS 16" O.C.
1/2" T.G. FLOORING
CONG. FILL

6 1/2"

1/2" WOOD JOISTS 16" O.C.
1/2" T.G. FLOORING
CONG. FILL

6 1/2"

1/2" WOOD JOISTS 16" O.C.
1/2" T.G. FLOORING
CONG. FILL

6 1/2"

1/2" WOOD JOISTS 16" O.C.
1/2" T.G. FLOORING
CONG. FILL

6 1/2"

1/2" WOOD JOISTS 16" O.C.
1/2" T.G. FLOORING
CONG. FILL

6 1/2"

1/2" WOOD JOISTS 16" O.C.
1/2" T.G. FLOORING
CONG. FILL

6 1/2"

1/2" WOOD JOISTS 16" O.C.
1/2" T.G. FLOORING
CONG. FILL

6 1/2"

1/2" WOOD JOISTS 16" O.C.
1/2" T.G. FLOORING
CONG. FILL

6 1/2"

1/2" WOOD JOISTS 16" O.C.
1/2" T.G. FLOORING
CONG. FILL

6 1/2"

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1/2" T.G. FLOORING
CONG. FILL

6 1/2"

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1/2" T.G. FLOORING
CONG. FILL

6 1/2"

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1/2" T.G. FLOORING
CONG. FILL

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1/2" T.G. FLOORING
CONG. FILL

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CONG. FILL

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CONG. FILL

6 1/2"

1/2" WOOD JOISTS 16" O.C.
1/2" T.G. FLOORING
CONG. FILL

6 1/2"

1/2" WOOD JOISTS 16" O.C.
1/2" T.G. FLOORING
CONG. FILL

6 1/2"

1/2" WOOD JOISTS 16" O.C.
1/2" T.G. FLOORING
CONG. FILL

6 1/2"

3/4" INCH SCALE SECTION THRU COURT.

VESTIBULE CHAIRING

3/4" INCH SCALE SECTION OF COURT

3/4"

WATERPROOFING

1/2" WOOD JOISTS 16" O.C.

1/2" T.G. FLOORING

CONG. FILL

SLAB 5'6"

1/2" WOOD JOISTS 16" O.C.

1/2" T.G. FLOORING

CONG. FILL

SLAB 6'0"

1/2" WOOD JOISTS 16" O.C.

1/2" T.G. FLOORING

CONG. FILL

SLAB 6'6"

1/2" WOOD JOISTS 16" O.C.

1/2" T.G. FLOORING

CONG. FILL

SLAB 7'0"

1/2" WOOD JOISTS 16" O.C.

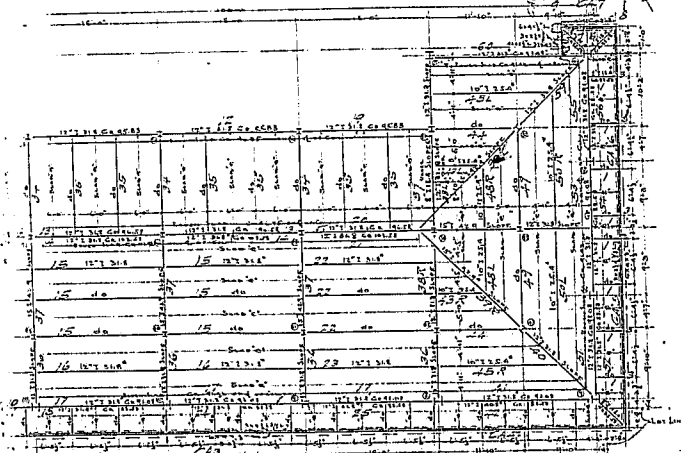
1/2" T.G. FLOORING

CONG. FILL

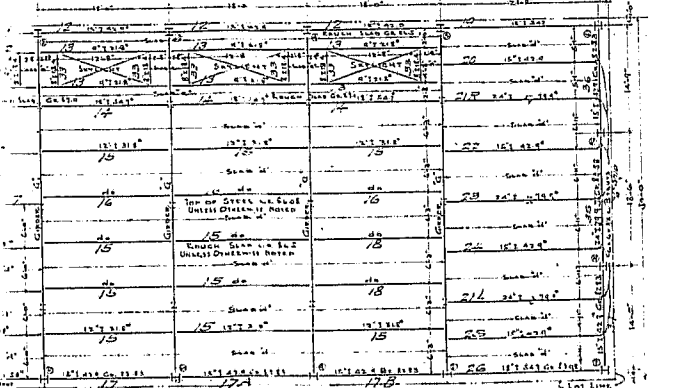
City of Portland

CLINTON WINE BAR

SLAB 8'0"



ROOF PLAN

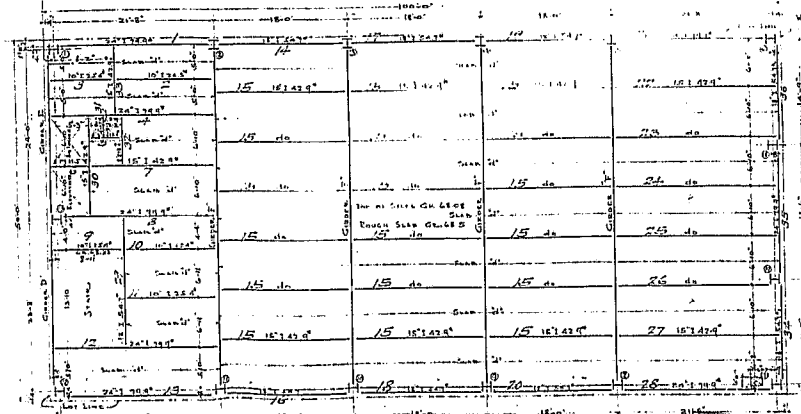


THIRD FLOOR PLAN

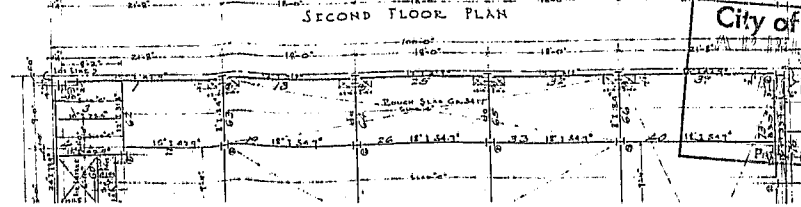
SCHEDULE

NO.	DESCRIPTION	QUANTITY	UNIT
1	1/2" WOOD JOISTS 16" O.C.	100	LF
2	1/2" T.G. FLOORING	100	SQ. YD.
3	CONG. FILL	100	CY
4	1/2" WOOD JOISTS 16" O.C.	100	LF
5	1/2" T.G. FLOORING	100	SQ. YD.
6	CONG. FILL	100	CY
7	1/2" WOOD JOISTS 16" O.C.	100	LF
8	1/2" T.G. FLOORING	100	SQ. YD.
9	CONG. FILL	100	CY
10	1/2" WOOD JOISTS 16" O.C.	100	LF
11	1/2" T.G. FLOORING	100	SQ. YD.
12	CONG. FILL	100	CY

SCALE 1/4" = 1'-0"



SECOND FLOOR PLAN



City of Portland

CLINTON WINE BAR

SLAB 8'0"

City of Portland
APPROVED
AUG 03 2001
Permit Number

