



City of Portland, Oregon - Bureau of Development Services



1900 SW Fourth Avenue • Portland, Oregon 97201 • 503-823-7300 • www.portlandoregon.gov/bds

Permit Revision Application and Submittal Requirements

A Permit Revision is required when there are proposed changes to the project after the permit has been issued. This may arise due to discrepancies between the city-approved permit drawings and actual field conditions, or the customer has changed their mind about an aspect of the project. In all cases, a revision to the existing permit must be submitted, reviewed and approved.

Applicants will provide: ☐ A copy of this application ☐ Three (3) sets of plans that clearly reflect the proposed change(s). Drawings and calculations must be stamped and signed by the Architect and/or the Engineer of Record, if applicable.	 One (1) copy of the original city approved permit drawings. (NOTE: If your project has an assigned process manager please contact them regarding submittal of the revision). Two (2) sets of calculations, if applicable Inspector's correction notice, if revision is due to an inspection correction Revision fee (paid at time of submittal)
Contact Information:	
Address	
	Zip Code
Phone Email _	
Value of proposed revision \Z, 000	Issued permit # 12-146349
Description of revision Change metall	Issued permit # 12-146349
Fees:	
The De 11 De 11 de 12 de	

The Permit Revisions are subject to fees associated with plan review, processing and any increase in project value. Additional fees may apply if adding plumbing fixtures.

The Bureau of Development Services fee schedule is available under the fees tab on the BDS web site at: www.portlandoregon.gov/bds. Fees are updated annually on July 1st.

Helpful Info	ormation:
--------------	-----------

Bureau of Development Services
City of Portland, Oregon
1900 SW 4th Avenue, Portland, OR 97201
www.portlandoregon.gov/bds

Submit your plans in person to:

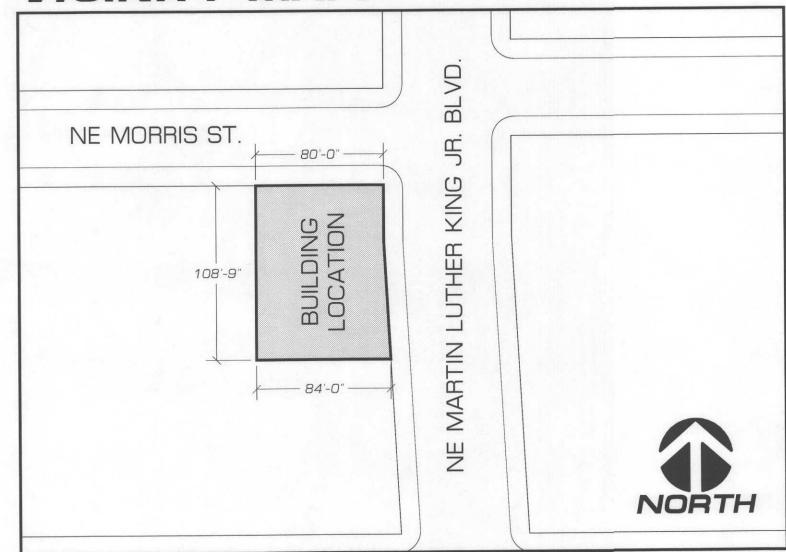
Development Services Center (DSC), First Floor, Tuesday - Friday: 7:30 am - 12:00 pm Closed Mondays

Important Telephone Numbers:

BDS main number	503-823-7300
DSC automated information line	503-823-7310
Building code information	503-823-1456
BDS 24 hour inspection request line	503-823-7000
Residential information for	
one and two family dwelling	503-823-7388
General Permit Processing and	
Fee Estimate info	503-823-7357
City of Portland TTY	503-823-6868

NEW 14-UNIT APARTMENT BUILDING @ 2955 NE Martin Luther King Jr. Blvd. -Martin Apartments

VICINITY MAP:



SITE AERIAL:



PROJECT CONTACT INFORMATION:

FOSLER PORTLAND ARCHITECTURE LLC STEVE FOSLER, ARCHITECT 720 SW ANKENY ST. PORTLAND, OR 97205 p. 503.241.9339 / f. 503.220.0754

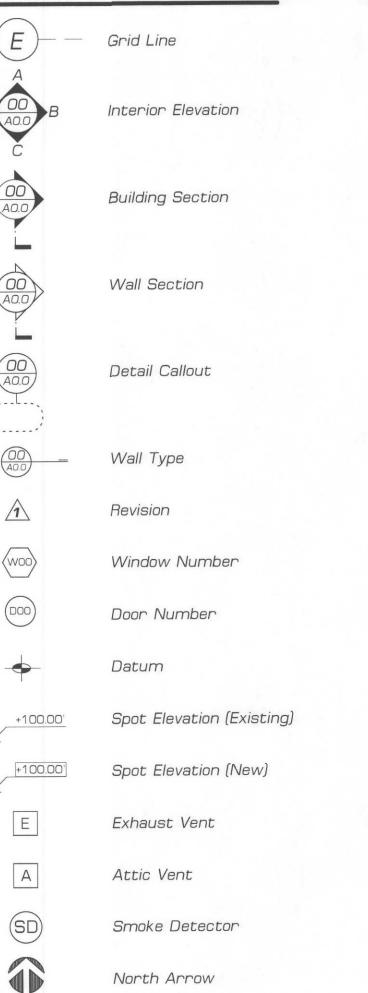
steve@foslerarchitecture.com

STRUCTURAL ENGINEER: VLMK CONSULTING ENGINEERS 3933 SW KELLY AVE. PORTLAND, OR 97239 p. 503.222.4453 / f. 503.248.9263

INDEX OF DRAWINGS:

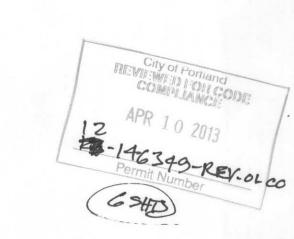
STRUCTURAL: **ARCHITECTURE:** AO.O | TITLE, PROJECT INFORMATION, & SO.O | STRUCTURAL NOTES AND SPECIAL INSPECTIONS DRAWING INDEX A3.1 ELEVATIONS S4.1 FLOOR AND MISC. FRAMING DETAILS ELEVATIONS A5.7 | BALCONY DETAILS

SYMBOLS:



ABBREVIATIONS:

CENTERLINE PROPERTY LINE **TYPICAL CONTROL JOINT** SIMILAR REV REVERSE **GWB** GYPSUM WALL BOARD CONCRETE PAINT **PAINTED GLAZING** TEMPERED GLASS **FLOOR DRAIN METAL** WD WOOD **CEILING** CLG DETAIL MINUTE **MANUFACTURER** REFLECTED CEILING PLAN **FINISHED FLOOR** FINISHED GRADE HOLLOW METAL **FLOOR** SHOWER DOWNSPOUT AND SCUPPER RD/OF ROOF DRAIN AND OVERFLOW STRUCTURAL STRUCT **UNLESS OTHERWISE NOTED** UON **EXISTING** NEW T.O. TOP OF B.O. **BOTTOM OF** SILL HEIGHT SH SSESD SUBSLAB SOIL EXHAUST SYSTEM DUCT





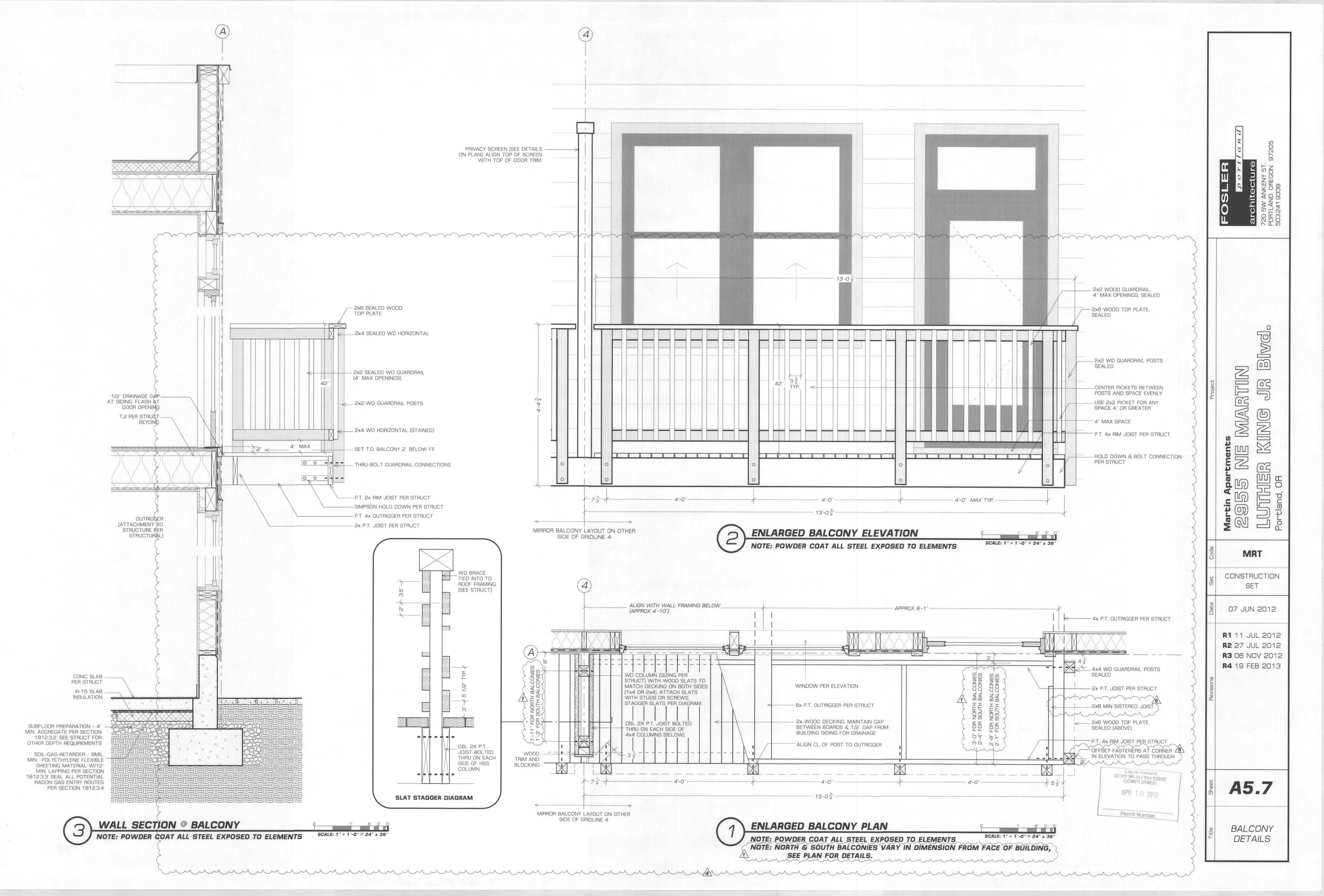
R4 19 FEB 2013

A0.0

TITLE,. PROJECT INFORMATION & DRAWING INDEX







DIVISION 1 - GENERAL

A. GENERAL

A.1. THESE NOTES SET MINIMUM STANDARDS FOR CONSTRUCTION. THE DRAWINGS GOVERN OVER THE GENERAL NOTES TO THE EXTENT SHOWN.

A.2. CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS ON DRAWINGS AND IN FIELD. COORDINATE LOCATIONS OF OPENINGS THROUGH FLOORS, ROOFS AND WALLS WITH ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS. NOTIFY VLMK CONSULTING ENGINEERS (VLMK) OF ANY DISCREPANCIES OR IF ACTUAL CONDITIONS DIFFER FROM THOSE SHOWN OR

A.3. CONTRACTOR SHALL PROVIDE ALL NECESSARY TEMPORARY SUPPORT PRIOR TO COMPLETION OF VERTICAL AND LATERAL LOAD SYSTEMS. VLMK HAS NOT BEEN RETAINED TO PROVIDE ANY SERVICES RELATED TO JOB SITE SAFETY PRECAUTIONS, OR TO REVIEW THE MEANS, METHODS, TECHNIQUES, SEQUENCES, OR PROCEDURES FOR THE CONTRACTOR TO PERFORM WORK. UNLESS WE ARE SPECIFICALLY RETAINED AND COMPENSATED TO DO OTHERWISE, OUR WORK IS LIMITED TO THE DESIGN OF WORK DESCRIBED ON OUR DRAWINGS FOR THIS PROJECT.

A.4. WHERE REFERENCE IS MADE TO ASTM, AISC, ACI OR OTHER STANDARDS, THE LATEST ISSUE SHALL APPLY.

A.5. INSPECTION AND/OR JOB SUPERVISION IS NOT PROVIDED BY VLMK.

A.6. ALL WORK SHALL BE IN STRICT COMPLIANCE WITH THE 2009 INTERNATIONAL BUILDING CODE (IBC) AS AMENDED BY THE STATE OF OREGON (2010 OREGON STRUCTURAL SPECIALTY CODE) AND ALL OTHER STATE AND LOCAL BUILDING REQUIREMENTS THAT APPLY.

A.7. DESIGN CRITERIA:

A.7.a. FLOOR LIVE LOADS (PER IBC 1603.1.1)

	2ND FLOOR AND LOFTS		40 PSF
	BALCONIES (NOT EXCEEDING 60 SQFT)		60 PSF
	CORRIDORS AND STAIRS		100 PSF
A.7.b.	ROOF SNOW LOAD (PER IBC 1603.1.3)		
	FLAT-ROOF SNOW LOAD	(Pf)	25 PSF
	SNOW EXPOSURE FACTOR	(Ce)	1.0
	SNOW LOAD IMPORTANCE FACTOR	(Is)	1.0
	THERMAL FACTOR	(Ct)	1.0
A.7.c.	WIND LOAD (PER IBC 1603.1.4)		
	BASIC WIND SPEED (3-SECOND GUST)		95 MPH
	WIND IMPORTANCE FACTOR	(Iw)	1.0
	OCCUPANCY CATEGORY		11
	WIND EXPOSIBE		В

WIND EXPOSURE (GCpi) ±0.18 INTERNAL PRESSURE COEFFICIENT A.7.e EARTHQUAKE DESIGN DATA (PER IBC 1603.1.5) SEISMIC IMPORTANCE FACTOR

OCCUPANCY CATEGORY MAPPED SPECTRAL RESPONSE ACCELERATIONS $Ss = 97.5\%, S_1 = 34.0\%$ SITE CLASS SPECTRAL RESPONSE COEFFICIENTS Sds = 72.2%, $Sd_1 = 39.0\%$ SEISMIC DESIGN CATEGORY LIGHT FRAMED WOOD BASIC SEISMIC-FORCE-RESISTING SYSTEM: SHEATHED SHEARWALLS SEISMIC RESPONSE COEFFICIENTS 6.5 RESPONSE MODIFICATION FACTOR

ANALYSIS PROCEDURE USED A.8. MECHANICAL EQUIPMENT, MECHANICAL AND SPRINKLER PIPING LARGER THAN 2 INCHES IN DIAMETER OR ANY OTHER ITEMS PRODUCING A HANGER OR SUPPORT LOAD OF OVER 50 POUNDS SHALL BE HUNG BY A SYSTEM APPROVED BY VLMK. ANY HANGER OR SUPPORT PRODUCING A LOAD OVER 200 POUNDS SHALL HAVE ADDITIONAL FRAMING INSTALLED TO TRANSFER THESE LOADS TO THE MAIN STRUCTURAL BEAMS OR WALL UNLESS OTHERWISE APPROVED.

A.9. WHERE REQUIRED, SEISMIC BRACING FOR MECHANICAL AND ELECTRICAL EQUIPMENT, SPRINKLER PIPING OR OTHER NON-STRUCTURAL COMPONENTS SHALL BE PROVIDED BY THE CONTRACTOR IN ACCORDANCE WITH CHAPTER 13 OF ASCE 7-05. STRUCTURAL CALCULATIONS AND DETAILS STAMPED BY A LICENSED ENGINEER SHALL BE SUBMITTED TO VLMK FOR REVIEW.

A.10. THESE DRAWINGS HAVE BEEN PREPARED SOLELY FOR USE IN THE CONSTRUCTION OF THE MARTIN DIVISION 4 - MASONRY (NOT USED) APARTMENTS PROJECT LOCATED IN PORTLAND, OREGON. POSSESSION OF THESE DRAWINGS DOES NOT GRANT A LICENSE TO CONSTRUCT OR FABRICATE THE WHOLE, OR PARTS OF THIS PROJECT IN OTHER LOCATIONS.

A.11. PROVIDE SHOP DRAWINGS FOR ALL STRUCTURAL PRODUCTS DELIVERED TO THE PROJECT. FOR BIDDER DESIGNED ITEMS (DEFERRED SUBMITTALS), THE DRAWINGS AND CALCULATIONS SHALL BEAR THE STAMP OF AN ENGINEER (PRODUCT ENGINEER) REGISTERED IN THE STATE WHERE THE A. GENERAL PROJECT IS LOCATED. SUCH DRAWINGS SHALL CERTIFY THAT THE DRAWINGS HAVE NOT DEVIATED FROM THE VLMK DESIGN IN ANY WAY, OR SHALL CALL TO THE ATTENTION OF VLMK THAT THE PRODUCT ENGINEER HAS DEVIATED FROM THE VLMK DESIGN DRAWINGS. SUCH DEVIATION MAY BE CAUSE FOR REJECTING THE SHOP DRAWINGS. SEE DRAWINGS FOR ITEMS CLASSIFIED AS DEFERRED SUBMITTALS. VLMK WILL REVIEW AND MARK-UP A MAXIMUM OF THREE COPIES OF SUBMITTALS. THE CONTRACTOR WILL BE RESPONSIBLE FOR DUPLICATING MARK-UPS IF ADDITIONAL COPIES ARE REQUIRED. SUBMITTALS WILL BE STAMPED AND RETURNED TO THE GENERAL CONTRACTOR WITHIN TWO WEEKS. FAX SUBMITTALS WILL NOT BE ACCEPTED WITHOUT PRIOR APPROVAL. ALL SUBMITTALS SHALL BE REVIEWED AND STAMPED BY THE CONTRACTOR PRIOR TO SUBMITTAL TO THE ENGINEER. UNLESS OTHERWISE INDICATED, SHOP DRAWINGS SHALL BE PROVIDED ON THE FOLLOWING:

A.11.a. PREFABRICATED STRUCTURAL MEMBERS AND COMPONENTS (E.G. WOOD CHORD PLYWOOD WEB TRUSSES, ENGINEERED WOOD HEADERS AND BEAMS, GLULAM BEAMS, ETC.). A.11.b. ALL REINFORCING STEEL.

A.11.c. ALL STRUCTURAL STEEL (GUARDRAILS AND BALCONY SCREENS).

DIVISION 2 - SITE WORK

A. GENERAL

A.1. REMOVE ALL ORGANIC MATERIAL AND TOPSOIL FROM AREAS UNDER THE BUILDING OR UNDER PAVED AREAS. A.2. FILL MATERIAL TO CONSIST OF A GRANULAR MATERIAL OR CONDITIONED SITE MATERIAL. PLACE

FILL IN LIFTS NOT TO EXCEED 8 INCHES AND COMPACT TO 95% STANDARD ASTM D-1557 UNDER FOOTINGS, UNDER PAVED AREAS AND UNDER FLOOR SLABS. A.3. BASE MATERIAL IMMEDIATELY UNDER SLAB TO BE 6 INCH LAYER OF COMPACTED CRUSHED ROCK WITH LESS THAN 5% PASSING THE #200 SIEVE.

A.4. SEE SPECIFICATIONS FOR ADDITIONAL INFORMATION.

A.5. DESIGN SOIL PRESSURE 1,500 PSF LL PLUS DL PER BUILDING CODE (AT OWNER'S DIRECTION). A.6. ALL FOOTINGS TO BEAR A MINIMUM 18 INCHES BELOW FINAL GRADE ON FIRM, UNDISTURBED SOIL OR APPROVED COMPACTED FILL. NOTIFY VLMK BEFORE PROCEEDING IF ANY UNUSUAL CONDITIONS ARE ENCOUNTERED IN THE FOOTING EXCAVATIONS.

A.7. DO NOT EXCAVATE CLOSER THAN 2:1 SLOPE BELOW FOOTING EXCAVATIONS.

A.8. CLEAN ALL FOOTING EXCAVATIONS OF LOOSE MATERIAL.

A.9. EXCAVATIONS MAY BE MADE UNDER CONTINUOUS FOOTINGS FOR PIPES. BACKFILL TO BE APPROVED BY GEOTECHNICAL ENGINEER.

DIVISION 3 - CONCRETE AND REINFORCING

A. CONCRETE MIX/MATERIAL

A.1. STRENGTH: AVERAGE 28 DAY CONCRETE STRENGTH DETERMINED BY JOB CAST LAB CURED CYLINDER TO BE AS INDICATED BELOW PLUS INCREASE DEPENDING ON THE PLANT'S STANDARD DEVIATION AS SPECIFIED IN ACI 318. PROVIDE MIX DESIGNS TO THE ENGINEER FOR ALL CONCRETE TO BE USED. CLEARLY LABEL ALL MIX DESIGNS FOR PROPOSED AREA OF USE.

MINIMUM MIX REQUIREMENTS:					
LOCATION	MIN. COMP. STRENGTH	SLUMP	MIN. CEM.	ADMIXTURES	
	SIRENGIH	(a)			
FOOTINGS AND STEM WALLS	3,000 PSI	0"-5"	470 LB	NONE	
INTERIOR SLAB ON GRADE	3,500 PSI	2"-4"	517 LB	(b), (d)	
(MAX WATER 250 PCY)					
EXTERIOR SLAB ON GRADE	3,000 PSI	2"-4"	517 LB	(b), (c), (c	
MISCELLANEOUS	3,000 PSI	0"-5"	470 LB	(b)	

(a) - SLUMP EXCEEDING SPECIFIED LIMITS SHALL NOT BE INCORPORATED IN THE PROJECT EXCEPT BY WRITTEN APPROVAL FROM ENGINEER.

(b) - WRA = WATER REDUCING AGENT.

(c) - AE = AIR ENTRAINMENT

(d) - UP TO 1 1/2 PERCENT CALCIUM CHLORIDE ALLOWED DURING COLD WEATHER.

(e) - POZZUTEC 20 REQUIRED FOR CONCRETE PLACED BELOW 40 DEGREES FAHRENHEIT. A.3. USE TYPE I CEMENT, PER ASTM C-150 UNLESS OTHERWISE APPROVED.

A.4. AGGREGATE ASTM C33. SIZE TO BE 3/4 INCH MAXIMUM SIZE AGGREGATE. A.5. MAXIMUM WATER ALLOWED IN SLAB ON GRADE MIX DESIGNS IS 250 POUNDS PER CUBIC YARD

UNLESS OTHERWISE APPROVED BY ENGINEER. A.6. WATER REDUCING AGENT (WRA) SHALL BE POLYHEED R-1 OR DURACEM 55 (MINIMUM 6 OZ. PER

100 POUNDS CEMENT). COMPLY WITH ASTM C-494. A.7. AIR ENTRAINMENT (AE) SHALL COMPLY WITH ASTM C-260 AND SHALL BE USED ONLY WHERE

SPECIFIED AND/OR APPROVED BY ENGINEER. A.8. ACCELERATORS: POZZUTEC 20 NON-CHLORIDE FOR COMPONENTS EXPOSED TO WEATHER IN THE COMPLETED STRUCTURE (WALLS, EXTERIOR PAVING, ETC.), CALCIUM CHLORIDE IN THE INTERIOR SLABS ON GRADE. QUANTITY (DOSAGE) TO BE DETERMINED BY CONTRACTOR AND APPROVAL OBTAINED FROM ENGINEER BEFORE USE OF ACCELERATORS.

A.9. PLACE AND CURE ALL CONCRETE PER ACI CODES AND STANDARDS.

A.10. PROVIDE CONTROL JOINTS IN ALL INTERIOR SLABS ON GRADE AT 25'-0" ON CENTER EACH WAY MAXIMUM UNLESS OTHERWISE SHOWN ON PLANS. PROVIDE HEAVY (3/4 INCH) TOOLED JOINTS AT 5'-0" ON CENTER IN ALL EXTERIOR WALKS UNLESS OTHERWISE NOTED.

A.11. SLEEVES, PIPES OR CONDUITS OF ALUMINUM SHALL NOT BE EMBEDDED IN STRUCTURAL CONCRETE UNLESS EFFECTIVELY COATED.

B.1. ALL REINFORCING TO BE ASTM A615 GRADE 60 UNLESS OTHERWISE NOTED.

B.2. USE ONLY A706 WELDABLE REBAR IF REBAR IS TO BE WELDED. USE ONLY LOW HYDROGEN ELECTRODES. ALL WELDING TO BE IN COMPLIANCE WITH AWS D1.4.

B.3. FABRICATE AND INSTALL REINFORCING STEEL ACCORDING TO THE MANUAL OF STANDARD

PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES - ACI STANDARD 315. B.4. PROVIDE 2'-0" x 2'-0" CORNER BARS TO MATCH HORIZONTAL REINFORCING IN

POURED-IN-PLACE WALLS AND FOOTINGS AT ALL CORNERS AND INTERSECTIONS. B.5. SPLICES IN WALL REINFORCING SHALL BE LAPPED 40 BAR DIAMETERS (2'-0" MINIMUM) AND

SHALL BE STAGGERED AT LEAST 4 FEET AT ALTERNATE BARS. B.6. ALL OPENINGS SMALLER THAN 30 INCHES x 30 INCHES THAT DISRUPT REINFORCING SHALL HAVE AN AMOUNT OF REINFORCING EQUAL TO THE AMOUNT DISRUPTED PLACED BOTH SIDES OF

B.7. PROVIDE (2) #4 CONTINUOUS BARS AT TOP AND BOTTOM AND AT DISCONTINUOUS ENDS OF ALL

B.8. PROVIDE DOWELS FROM FOOTINGS TO MATCH ALL VERTICAL WALL, PILASTER AND COLUMN REINFORCING. (POURED-IN-PLACE COLUMNS AND WALLS). B.9. LAP ALL BARS IN CONTINUOUS AND INTERSECTING FOOTINGS 2'-0" OR 40 BAR DIAMETERS,

WHICHEVER IS GREATER. B.10. ALL VERTICAL WALL REINFORCING TO BE PLACED IN CENTER OF WALL UNLESS SHOWN OTHERWISE

ON DRAWINGS. B.11. EXTEND REINFORCING TO WITHIN 1 INCH OF WALL AND SLAB EDGES.

DIVISION 5 - METAL

EQUIVALENT LATERAL FORCE

A.1. ALL STRUCTURAL MEMBERS TO BE DETAILED, FABRICATED AND ERECTED IN ACCORDANCE WITH THE LATEST EDITION OF AISC STANDARDS. SUBMIT SHOP DRAWINGS OF COLUMNS AND MISCELLANEOUS STEEL.

A.2. ALL BIDDER DESIGN STRUCTURAL SYSTEMS TO INCLUDE PLANS AND CALCULATIONS STAMPED BY AN ENGINEER REGISTERED IN THE GOVERNING JURISDICTION. A.3. UNLESS OTHERWISE NOTED, CONTRACTOR WILL BE REQUIRED TO PROVIDE A SEPARATE PERMIT

FOR BIDDER DESIGN STRUCTURAL SYSTEMS. B. MATERIALS

B.3. ANCHOR BOLTS AND WOOD CONNECTIONS TO BE A307. PROVIDE STANDARD PLATE WASHERS

UNDER ALL BOLT HEADS AND NUTS IN CONTACT WITH WOOD.

B.4. ANCHOR RODS TO BE ASTM 1554, GRADE 36 UNLESS OTHERWISE NOTED. B.5. LAG BOLTS IN WOOD TO BE SAE J429 GRADE 1 WITH MINIMUM DIMENSIONS MEETING ANSI/ASME

B.18.2.1. C. DRILLED ANCHOR BOLTS:

ADHESIVE ANCHORS I.C.C.# PRODUCT ESR-2508 SIMPSON SET-XP ADHESIVE ANCHOR CONCRETE ESR-2322 HILTI HIT-RE 500-SD ADHESIVE ANCHOR CONCRETE ESR-3013 CONCRETE HILTI HIT-HY 150 MAX-SD ADHESIVE ANCHOR ESR-2583 POWERS PE1000+ EPOXY ADHESIVE ANCHOR CONCRETE **EXPANSION ANCHORS:** BASE MATERIAL PRODUCT ESR-1771 SIMPSON STRONG-BOLT WEDGE ANCHOR CONCRETE ESR-1917 HILTI KWIK BOLT TZ ANCHOR CONCRETE

POWERS POWER-STUD+ SD1 ANCHOR

ESR-2502 POWERS POWER-STUD+ SD2 ANCHOR CONCRETE C.1. OTHER ANCHORS ARE ACCEPTABLE ONLY WITH PRIOR WRITTEN APPROVAL OF ENGINEER. INSTALL ALL ANCHORS PER MANUFACTURER'S INSTRUCTIONS. PROVIDE MINIMUM EMBEDMENT AND SPACING UNLESS NOTED OTHERWISE ON DRAWINGS. SPECIAL INSPECTION OF ANCHOR INSTALLATION IS REQUIRED UNLESS SPECIFICALLY NOTED OTHERWISE ON DRAWINGS.

D.1. ALL WELDS TO BE MADE BY CERTIFIED WELDERS TO AWS STANDARDS WITH E70XX ELECTRODES. CONTRACTOR SHALL PAY SPECIAL ATTENTION TO THE MEANS AND METHODS OF CONSTRUCTION THEY ANTICIPATE EMPLOYING ON THE PROJECT. SOME WELDS SHOWN AS SHOP WELDS MAY NEED TO BE FIELD WELDS TO AID ERECTION.

DIVISION 6 - WOOD

CONCRETE

A. ROUGH CARPENTRY

A.1. LUMBER SPECIES AND GRADES TO BE AS FOLLOWS UNLESS OTHERWISE SHOWN ON DRAWINGS: DOUGLAS FIR #2 JOISTS, BEAMS & STRINGERS:

DOUGLAS FIR #1 6 INCH NOMINAL AND GREATER: BUCKS, BLOCKING, BRIDGING AND MISCELLANEOUS: DOUGLAS FIR OR HEM FIR #3 DOUGLAS FIR #3 STRIPPING: DOUGLAS FIR STUDS 2x4 STUDS: DOUGLAS FIR #2 2x6 STUDS: PRESSURE TREATED HEM FIR #2 SILLS, LEDGERS, PLATES, STRIPPING, ETC. IN CONTACT WITH CONCRETE, AND WOOD CURBS: DOUGLAS FIR #1 OR PSL 1.8E POSTS, COLUMNS (SAWN):

ROOF, FLOOR AND WALL SHEATHING:

APA RATED SHEATHING, EXPOSURE 1, CONFORMING TO APA PERFORMANCE STANDARD PS 1-95 AND EXCERPT NER-108. SEE DRAWINGS FOR REQUIRED THICKNESS OF SHEATHING AND/OR SPAN RATING. INSTALL ROOF AND FLOOR SHEATHING WITH LONG DIMENSION PERPENDICULAR TO SUPPORTS UNLESS OTHERWISE NOTED. USE SPACERS TO PROVIDE 1/8 INCH END AND EDGE JOINTS (1/4 INCH FOR JUMBO PANELS) WHENEVER SHEATHING IS EXPOSED TO WET WEATHER. FILL ANY JOINT SPACING OVER 3/8 INCH UNLESS OTHERWISE APPROVED BY ENGINEER. INSTALL 2x4 OR THICKER BLOCKING AT UNSUPPORTED JOINTS IN WALL SHEATHING. BLOCK ROOF OR FLOOR SHEATHING WHERE NOTED ON DRAWINGS.

-Fb = 2400 PSI, E = 1,800,000 PSI PER ANSI A190.1 SPECIFICATION. -SIMPLE SPANS COMBINATION: 24F-V4

-CANTILEVER SPANS COMBINATION: 24F-V8 -SUBMIT CERTIFICATE OF CONFORMANCE INDICATING MEMBERS MEET THE REQUIREMENTS OF ANSI A190.1.

PARALLAM (PSL): -ALL PSL SHALL BE MANUFACTURED BY ILEVEL.

-Fb = 2900 PSI, E = 2,000,000 PSI, Fv = 290 PSI

-Fb = 2400 PSI, E = 1,800,000 PSI, FcII = 2500 PSI

TIMBERSTRAND (LSL): -ALL LSL SHALL BE MANUFACTURED BY ILEVEL.

-Fb = 2325 PSI, E = 1,550,000 PSI, FcII = 2325 PSI, Fv = 400 PSI

-Fb = 2325 PSI, E = 1,550,000 PSI, Fv = 310 PSIMICROLLAM (LVL) BEAMS:

-ALL LVL SHALL BE MANUFACTURED BY ILEVEL. -Fb = 2600 PSI, E = 1,900,000 PSI, Fv = 285 PSI

A.2. PROVIDE FIRE BLOCKING AS REQUIRED AND BLOCKING FOR ALL SPECIFIED WALL-MOUNTED ITEMS INCLUDING CABINETS, DOORSTOPS, ETC.

A.3. CONNECTORS: A.3.a. TIE STRAPS, FRAMING ANCHORS, HANGERS, STIRRUPS, COLUMN CAPS, COLUMN BASES,

ETC., TO BE SIMPSON AS DETAILED, OR AS APPROVED. A.3.b. FULLY FASTEN ACCORDING TO MANUFACTURER'S SCHEDULE USING LARGEST SIZE SHOWN. A.3.c. ALL BEAMS AND JOISTS NOT BEARING ON ANOTHER MEMBER TO CONNECT WITH 'U'

TYPE HANGERS UNLESS OTHERWISE NOTED ON DRAWINGS. A.3.d. NAILING: ALL NAILS SPECIFIED IN THE DRAWINGS TO BE "COMMON" NAILS UNLESS OTHERWISE NOTED. ALL NAILING SHALL COMPLY WITH IBC TABLE 2304.9.1, NAILING

SCHEDULE. OBTAIN ENGINEERS APPROVAL OF ALL PROPRIETARY NAILING SYSTEMS. A.3.e. NAILS AT ROOF AND WALL DIAPHRAGMS TO BE DRIVEN SO THAT THEIR HEAD OR CROWN IS FLUSH WITH THE SURFACE OF THE SHEATHING. CHECK AND RESINK IF NEEDED,

IMMEDIATELY BEFORE ROOFING. A.3.f. CORROSION PROTECTION:

A.3.f.a. CONFIRM REQUIRED CORROSION PROTECTION FOR HARDWARE AND FASTENERS WITH SPECIFIC RECOMMENDATIONS FROM PRESSURE TREATING MANUFACTURER OR HANGER MANUFACTURER (USE MOST CONSERVATIVE) FOR SPECIFIC WOOD TREATMENTS USED.

MINIMUM CORROSION PROTECTION ON METAL CONNECTORS EXPOSED TO THE ENVIRONMENT OR PRESSURE TREATED LUMBER TO BE PER ASTM A653 CLASS 185 (SIMPSON ZMAX) OR ASTM A123.

FASTENERS FOR PRESSURE TREATED LUMBER MUST BE MINIMUM HOT-DIP GALVANIZED, STAINLESS STEEL, SILICON BRONZE OR COPPER. A.3.f.d. HOT-DIP GALVANIZED HARDWARE AND FASTENERS MUST COMPLY WITH ASTM

A153, STAINLESS STEEL FASTENERS TO BE TYPE 304 OR TYPE 316. A.3.f.e. HARDWARE AND FASTENERS USED TOGETHER MUST BE THE SAME TYPE (E.G. HOT-DIP GALVANIZED NAILS WITH HOT-DIP GALVANIZED HANGERS).

4 DELIVERY AND STORAGE: ALL LUMBER AND SHEATHING DELIVERED TO THE SITE, SHALL BE STACKED OR STORED OFF THE GROUND AND PROPERLY PROTECTED AGAINST WEATHER.

NOTIFY ENGINEER FOR OBSERVATION OF ROOF SHEATHING NAILING. OBTAIN ENGINEER'S APPROVAL PRIOR TO STARTING ROOF INSTALLATION. SEE STRUCTURAL OBSERVATION NOTES FOR A.6. CUTTING AND NOTCHING OF JOISTS NOT ALLOWED. A 1 INCH DIAMETER HOLE MAY BE DRILLED

IN THE CENTER 1/3 OF THE MEMBER DEPTH. ALL OTHER HOLES TO BE APPROVED BY A.7. STUDS MAY BE NOTCHED IN THE LOWER 1/5 OF THE HEIGHT OF THE STUD FOR ELECTRIC AND PLUMBING PIPES, BUT NO PART OF THE NOTCH IS TO BE DEEPER THAN 25 PERCENT OF THE WIDTH OF THE STUD. HOLES OF DIAMETERS UP TO 1/3 OF WIDTH OF STUD MAY BE DRILLED IN

B. WOOD CHORD PLYWOOD WEB JOISTS (TJI'S)

B.1. ALL TJI'S SHALL BE MANUFACTURED BY REDBUILT OR A PRIOR APPROVED JOIST MANUFACTURER WITH ICC APPROVAL TO THE FOLLOWING LOAD REQUIREMENTS:

B.2. FLOOR JOISTS (MINIMUM REQUIREMENTS, UNLESS NOTED OTHERWISE):

B.2.a. 40 PSF LIVE LOAD (RESIDENTIAL)

B.2.b. 60 PSF LIVE LOAD (BALCONIES) B.2.c. 100 PSF LIVE LOAD (CORRIDORS AND STAIRS)

B.2.d. 15 PSF DEAD LOAD

ESR-2818

B.3. ALL BRIDGING, BEARING HARDWARE, BLOCKING, HANGARS, ETC., THAT CONNECT TO THE JOISTS

SHALL BE PER REDBUILT STANDARD DETAILS. B.4. JOIST ERECTOR TO INSTALL JOISTS IN ACCORDANCE WITH THE TRUSS SUPPLIER DRAWINGS AND INSTALLATION GUIDELINES.

B.5. RESIDENTIAL (STOCK) JOISTS: PROVIDE SHOP DRAWINGS.

B.6. JOIST MANUFACTURER TO INSPECT ALL JOISTS AFTER THEY HAVE BEEN ERECTED AND FLOOR AND/OR ROOF SHEATHING, BRIDGING, BLOCKING, ETC., HAS BEEN INSTALLED. MANUFACTURER TO SUBMIT CERTIFICATE TO VLMK THAT THE INSPECTION WAS MADE AND THAT THE JOISTS ARE IN ACCEPTABLE CONDITION AND MEET THE MANUFACTURER'S DESIGN AND INSTALLATION REQUIREMENTS.

FOUNDATIONS AND SOILS:	COMMENTS	DURATION	INSPECTION AGENCY
FILL MATERIAL, PLACEMENT AND COMPACTION	REF. GEOTECHNICAL REPORT	С	GEOTECHNICAL
GRADING AND SUBGRADE COMPACTION	REF. PROJECT SPECIFICATIONS	Р	GEOTECHNICAL
FOOTING EXCAVATION AND COMPACTION		Р	GEOTECHNICAL
FOOTING FORMWORK AND REINFORCEMENT		Р	TESTING LAB
CONCRETE:			
VERIFY USE OF ENGINEER APPROVED CONCRETE MIX DESIGNS		Р	TESTING LAB
PLACING OF REINFORCING FOR SLAB-ON-GRADE		Р	TESTING LAB
PLACING OF CONCRETE FOR REINFORCED SLAB-ON-GRADE	SEE NOTE 6, ITEM A	С	TESTING LAB
PRIOR TO AND DURING CONCRETE PLACEMENT FOR CAST ANCHOR BOLTS AND EMBEDMENT PLATES		С	TESTING LAB
DRILLED—IN ADHESIVE ANCHORS, RODS AND DOWELS SHALL BE CONTINUOUSLY INSPECTED DURING INSTALLATION	PER ICC EVALUATION REPORT	Р	TESTING LAB
DRILLED-IN EXPANSION/WEDGE ANCHORS IN CONCRETE (UNLESS OTHERWISE NOTED ON DRAWINGS)	PER ICC EVALUATION REPORT	Р	TESTING LAB
WOOD:			
ROOF DIAPHRAGM SHEATHING AND NAIL SPACING		Р	TESTING LAB
SHEARWALL SHEATHING, NAIL SPACING AND HOLDOWNS		Р	TESTING LAB
~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~		~~~	~~~~
			in. I

#### SPECIAL INSPECTION PROGRAM NOTES:

1. DURATION REFERS TO TIME AND FREQUENCY OF INSPECTION FOR THE PORTIONS OF WORK INDICATED. C = CONTINUOUS INSPECTION IN WHICH THE SPECIAL INSPECTOR IS ON SITE AT ALL TIMES, OBSERVING THE WORK REQUIRING

P = PERIODIC INSPECTION IN WHICH THE SPECIAL INSPECTOR IS ON SITE AT TIME INTERVALS NECESSARY TO CONFIRM THAT THE WORK REQUIRING SPECIAL INSPECTION IS IN CONFORMANCE WITH APPROVED PERMIT DRAWINGS AND SPECIFICATIONS.

2. THE INSPECTION AGENCIES ARE AS FOLLOWS:

VLMK ENGINEERS ENGINEER: GEOTECHNICAL: TO BE DETERMINED

TESTING LAB: TO BE DETERMINED TESTING LAB SHALL PERFORM INSPECTIONS OF ALL PORTIONS OF WORK DESIGNATED IN THE PROGRAM. THE SELECTED INSPECTION AGENCY SHALL BE AN ACCREDITED, APPROVED SPECIAL INSPECTION AGENCY EMPLOYED BY THE OWNER OR OR OWNER'S AGENT, NOT THE CONTRACTOR OR SUB CONTRACTOR, PER I.B.C. SECTION 1704.1. THE SPECIAL INSPECTORS DUTIES REGARDING THE PORTIONS OF WORK ARE DESCRIBED IN CHAPTER 17 OF THE I.B.C. THE SPECIAL INSPECTOR SHALL BE A QUALIFIED PERSON WHO SHALL DEMONSTRATE COMPETENCE. TO THE SATISFACTION OF THE BUILDING OFFICIAL, FOR INSPECTION OF THE PARTICULAR TYPE OF CONSTRUCTION OR OPERATION REQUIRING SPECIAL INSPECTION.

3 PRIOR TO THE BEGINNING OF CONSTRUCTION, A PHONE CONFERENCE SHALL BE HELD BETWEEN THE CONTRACTOR, ENGINEER AND SPECIAL INSPECTOR TO REVIEW THE SPECIAL INSPECTION REQUIREMENTS. STRUCTURAL OBSERVATIONS AND DEFERRED SUBMITTAL ITEMS.

4. DUTIES OF THE SPECIAL INSPECTOR TO INCLUDE, BUT ARE NOT LIMITED TO: A. ACKNOWLEDGE THE SPECIAL INSPECTION PROGRAM AND THE SPECIAL INSPECTION AND TESTING AGREEMENT, PROVIDED BY THE LOCAL JURISDICTION.

B. THE SPECIAL INSPECTOR SHALL OBSERVE THE WORK FOR CONFORMANCE WITH THE APPROVED PERMIT DRAWINGS AND SPECIFICATIONS. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR FOR CORRECTION, THEN, IF UNCORRECTED, TO THE ENGINEER AND TO THE BUILDING OFFICIAL.

THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS FOR EACH INSPECTION TO THE BUILDING OFFICIAL, ARCHITECT, ENGINEER, CONTRACTOR AND (OTHER DESIGNATED PARTIES). IN A TIMELY MANNER. D. THE SPECIAL INSPECTOR SHALL SUBMIT A FINAL REPORT STATING WHETHER THE WORK REQUIRING SPECIAL INSPECTION WAS INSPECTED, AND WHETHER THE WORK IS IN GENERAL CONFORMANCE WITH THE APPROVED PERMIT DRAWINGS AND SPECIFICATIONS AND THE APPLICABLE WORKMANSHIP PROVISIONS OF THE INTERNATIONAL BUILDING CODE.

5. DUTIES OF THE CONTRACTOR INCLUDE BUT ARE NOT LIMITED TO: A. NOTIFY THE SPECIAL INSPECTOR THAT THE WORK IS READY FOR INSPECTION AT LEAST 24 HOURS BEFORE SUCH INPECTION.

B. ALL WORK REQUIRING SPECIAL INSPECTION SHALL REMAIN ACCESSIBLE AND EXPOSED UNTIL IT HAS BEEN OBSERVED AND INDICATED TO BE IN CONFORMANCE BY THE SPECIAL INSPECTOR AND APPROVED BY THE BUILDING OFFICIAL C. PROVIDE THE SPECIAL INSPECTOR WITH ACCESS TO APPROVED PERMIT DRAWINGS AND SPECIFICATIONS AT THE JOB SITE. D. MAINTAIN AT THE JOB SITE, COPIES OF ALL REPORTS SUBMITTED BY THE SPECIAL INSPECTOR.

6. THE FOLLOWING INSPECTIONS AND TESTING REQUIREMENTS SHALL BE PERFORMED IN ADDITION TO THOSE

REQUIRED BY THE I.B.C.: A. CONCRETE INSPECTION AND TESTING REQUIREMENTS ARE AS FOLLOWS: 1) CONTINUOUS OBSERVATION OF SLAB ON GRADE CONCRETE PLACEMENT IS REQUIRED. OBTAIN (4) TEST CYLINDERS FOR EACH 100 CUBIC YARDS PLACED (OR PORTION THEREOF IF LESS COMPLIANCE THAN 100 CUBIC YARDS ARE PLACED). BREAK (1) CYLINDER AT 7 DAYS AND (2) CYLINDER\$ AT 28 DAYS. (IF THE FIRST 28 DAY CYLINDER BREAK IS LESS THAN REQUIRED STRENGTH. HOLD (2) CYLINDERS FOR 56 DAY BREAK) HOLD 4TH CYLINDER FOR 56 DAY BREAK. BREAK 4TH CYLINDER ONLY IF REQUIRED STRENGTH IS NOT MET WITH (2) CYLINDERS AT 28 DAYS.

3) VERIFY THAT THE APPROVED MIX DESIGNS ARE BEING DELIVERED TO THE SITE. BATCH TICKETS SHOULD BE CHECKED FOR ALL TRUCKS ENTERING THE SITE. Permit Number

	STRUCTURAL OBSERVATION:			
	WOOD FRAMING PRIOR TO COVER	Р	ENGINEER	
	AT COMPLETION OF FLOOR DIAPHRAGM NAILING	Р	ENGINEER	
	AT COMPLETION OF ROOF DIAPHRAGM NAILING	Р	ENGINEER	
_	AT COMPLETION OF GREEN ROOF AND PLANTER FRAMING PRIOR TO COVER	P	ENGINEER	
-				

### STRUCTURAL OBSERVATION NOTES:

1. THE CONTRACTOR SHALL BE RESPONSIBLE FOR NOTIFYING THE ENGINEER OF RECORD A MINIMUM OF 24 HOURS IN ADVANCE OF REQUIRED OBSERVATION(S). APPROVAL BY THE MUNICIPAL INSPECTOR DOES NOT PRECLUDE OBSERVATION BY THE ENGINEER OF RECORD AND APPROVAL BY THE ENGINEER OF RECORD DOES NOT PRECLUDE THE INSPECTION PROCESS BY THE MUNICIPAL INSPECTOR AND ANY OTHER CODE REQUIREMENTS FOR INSPECTION.

2. UPON COMPLETION OF WORK THE STRUCTURAL OBSERVER SHALL SUBMIT A REPORT TO OWNER AND BUILDING OFFICIAL WITH WET STAMP AND SIGNATURE ATTESTING TO THE VISUAL OBSERVATIONS MADE. THE REPORT SHALL IDENTIFY ANY REPORTED DEFICIENCIES WHICH HAVE NOT BEEN RESOLVED.

EXPIRES: 6-30-2013

CONSULTING ENGINEERS 3933 SW Kelly Avenue Portland • Oregon 97239-4393 03.222.4453 vlmk@vlmk.com

503.248.9263 www.vlmk.com

MRT

PERMIT SET

JUNE 08 2012

R1 JULY 11 2012 R2 JULY 27 2012 R3 NOV 06 2012 R4 FEB 19 2013

STRUCTURA NOTES AND SPECIAL

