

STRUCTURAL CALCULATIONS

PROJECT: BURGER KING - PORTLAND
CO2 TANK ATTACHMENT

DESIGN ASSUMPTIONS

Concrete Strength at 28 days: $f'_c := 2500 \text{ psi}$
Masonry (Grade N Concrete Block): $f'_m := 2000 \text{ psi}$
Mortar (Type S): $f'_m := 2000 \text{ psi}$
Grout: $f'_g := 2000 \text{ psi}$

Reinforcing Steel: A-615 Grade 40 (#4 and less) $F_{y40} := 40000 \text{ psi}$
Grade 60 (#5 and larger) $F_{y60} := 60000 \text{ psi}$
Structural Steel: A-36 Yield Strength: $F_y := 50000 \text{ psi}$
Lumber: Douglas Fir-Larch $E_s := 29 \cdot 10^6 \text{ psi}$
4x Joists, Beams and Posts: #2 (UON) $F_{v4x} := 95 \text{ psi}$
 $F_{b4x} := 900 \text{ psi}$
 $E4x := 1.6 \cdot 10^6 \text{ psi}$
6x Joists, Beams and Posts: #1 (UON) $F_{v6x} := 85 \text{ psi}$
 $F_{b6x} := 1350 \text{ psi}$
 $E6x := 1.6 \cdot 10^6 \text{ psi}$

Studs: Stud Grade (UON)

Manufactured: PSL: $F_{vPSL} := 290 \text{ psi}$ LSL: $F_{vLSL} := 310 \text{ psi}$
 $F_{bPSL} := 2900 \text{ psi}$ $F_{bLSL} := 2325 \text{ psi}$
 $EPSL := 2.0 \cdot 10^6 \text{ psi}$ $ELSL := 1.55 \cdot 10^6 \text{ psi}$

DESIGN LOADS:

ROOF DEAD LOAD

	SLOPING	FLAT
Roofing:		4 psf
Plywood:		2 psf
Joists:		5 psf
Insul. & Clg:		1 psf
Sprink:		2 psf
Misc:		2 psf

$DL_{roofslope} := 16 \text{ psf}$

$DL_{roofflat} := 16 \text{ psf}$

FLOOR DEAD LOAD

	INTERIOR	EXTERIOR
Flooring:	2 psf	4 psf
Plywood:	2 psf	2 psf
Joists:	5 psf	5 psf
Insul. & Clg:	2 psf	2 psf
Sprink:	2 psf	2 psf
Misc:	1 psf	1 psf

$DL_{floorint} := 14 \text{ psf}$

$DL_{floorext} := 16 \text{ psf}$

WALL DEAD LOAD

$DL_{wallext} := 16 \text{ psf}$
 $DL_{wallint} := 10 \text{ psf}$

ROOF LIVE LOAD

$LL_{roofslope} := 20 \text{ psf}$
 $LL_{roofflat} := 20 \text{ psf}$

FLOOR LIVE LOAD

$LL_{floorext} := 60 \text{ psf}$
 $LL_{floorint} := 40 \text{ psf}$
 $LL_{floorexit} := 100 \text{ psf}$

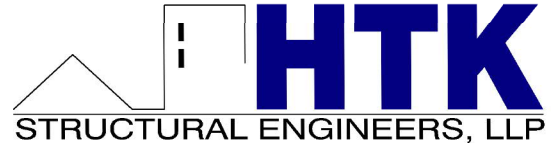
SOIL DESIGN VALUES

Report by: N/A (CBC MINIMUMS)
Report Date: -

Reporty No.: -
Bearing Pressure: $q_b := 1500 \text{ psf}$

These calculations are limited only to the items included herein, selected by the client and do not imply approval of any other portion of the structure by this office. These calculations are not valid if altered in any way, or not accompanied by a wet stamp and signature of the Engineer of Record.

Job No.	Designed By	Date
20-372.00	RT	9/29/2022



STRUCTURAL CALCULATIONS

BURGER KING - POTLAND

Anchorage Analysis for CO2 tank

$$Sds := 0.737$$

$$w_{tank} := 670 \text{ lbf}$$

$$h_{tank} := 169.55 \text{ in} \quad l_{tank} := 8 \text{ in}$$

$$a_p := 1$$

$$R_p := 2.5$$

$$I_p := 1.0$$

$$E_v := 0.6 - 0.7 \cdot 0.2 \cdot Sds = 0.5$$

NDS 2018 (12M), Wpullout=2850(G^2)D (per inch of penetration)

$$screw_{pull} := 117 \frac{\text{lbf}}{\text{in}} \cdot 2.5 \text{ in} = 292.5 \text{ lbf} \quad \begin{array}{l} \text{(#8 Wood Screws for tank strap)} \\ \text{8-#8 x 2" screws} \end{array}$$

$$screw_{shear} := 103 \text{ lbf} \quad \text{(Table 12M w/ steel plate)}$$

CO2 TANK (670 lb max)

$$F_{ptank} := \frac{0.4 \cdot a_p \cdot Sds \cdot w_{tank}}{\frac{R_p}{I_p}} \cdot (1 + 2 \cdot (1))$$

$$F_{ptank} = 237.02 \text{ lbf}$$

$$Mot_{tank} := F_{ptank} \cdot \left(\frac{h_{tank}}{2} \right) = 1674.442 \text{ lbf} \cdot \text{ft}$$

$$Mres_{tank} := E_v \frac{2 \cdot (w_{tank}) \cdot (l_{tank})}{2} = 221.913 \text{ lbf} \cdot \text{ft}$$

$$OT_{tank} := \frac{Mot_{tank} - Mres_{tank}}{l_{tank}} = 2178.79 \text{ lbf}$$

$$screw_t := \frac{OT_{tank}}{screw_{pull}} = 7.45$$

-Use strap per manuf or CS16
(H) min) w/ 4-#8 x 2.5" screw
ea. side (8 total).

STRUCTURAL CALCULATIONS

Worst Case Stud Check

$$l_{stud} := 12 \text{ ft}$$

$$F_{ptank} = 237.02 \text{ lbf}$$

$$Mot_{stud} := \frac{F_{ptank} \cdot (12 \text{ ft})}{(4 \cdot 4)} = 177.76 \text{ lbf} \cdot \text{ft} \quad \text{ea stud}$$

$$Sx_{req} := \frac{Mot_{stud} \cdot 12 \frac{\text{in}}{\text{ft}}}{900 \text{ psi} \cdot 1.6} \quad Sx_{req} = 1.48 \text{ in}^3 \quad \text{2x6 stud is adequate}$$