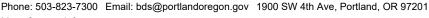
Development Services

From Concept to Construction



More Contact Info (http://www.portlandoregon.gov//bds/article/519984)





APPEAL SUMMARY

Appeal ID: 20580	Project Address: 4415 SW Fairview Blvd
Hearing Date: 7/3/19	Appellant Name: Mark Engberg
Case No. : B-004	Appellant Phone: 503 416 0139
Appeal Type: Building	Plans Examiner/Inspector: Mike Walkiewicz, Amit Kumar, Jason Butler-Brown
Project Type: residential	Stories: 2 Occupancy: R-1 Construction Type: V
Building/Business Name: None	Fire Sprinklers: No
Appeal Involves: Alteration of an existing structure	LUR or Permit Application No.: 19-183014-RS
Plan Submitted Option: pdf [File 1] [File 2] [File 3] [File 4] [File 5] [File 6]	Proposed use: Single Family

APPEAL INFORMATION SHEET

Appeal item 1

Code	Section	

OSSC/16/#2

Requires

The PDOT has supplemental requirements to OSSC/16/#2 for ramps or decks that are adjacent to a public roadway. Under these requirements the ramps or decks need to be designed for live load requirements of AASHTO HS-20. Design should accommodate for Trucks such as Garbage trucks and Moving trucks. See enclosed BDS Code Guide.

Proposed Design

The Project scope is to replace an aging elevated driveway that was built in 1976. The existing structure is comprised of a concrete slab on plywood on 2x8 joists supported by a wood glulam beam which is supported by 4x4 wood columns supported on concrete piers and a concrete retaining wall. The existing elevated driveway is nearing the end of its useful life. The slab and wood section will be removed and replaced. The new replacement design is the same size and shape in plan. It is comprised os a 4 inch Concrete slab on PT Plywood on PT sleeper for slope on PT plywood supported on 4x8 PT joists supported by a steel beam W12x19 which is supported on 4 lnch round steel columns supported by the existing concrete foundation. The new driveway is designed to support 40 psf and a point load of 3,000 lbs. The driveway will also be flashed properly to help maintain its integrity.

The existing conditions on the site provide a circular on-grade driveway with two points of entry or exit. The on-grade driveway is long enough and wide enough to accommodate the largest truck, listed in the code, to park and turn around without crossing the elevated driveway section. The elevated driveway is 35.5 feet east of the Fairview Blvd roadway and 15.8 feet from the property line. See enclosed document BCA1.0 and BCS 1.1.

Reason for alternative

The reason for the alternate is that IF the new elevated friveway is designed for truck loads, we will need to remove and replace the existing concrete footings and retaining wall for both the driveway and existing house. That will potentially put the house and foundation in jeopardy during removal of the existing foundation. This would also require the removal of 2 mature trees adjacent to the driveway. The new design will make the elevated driveway safer in terms of longevity plus was are adding a safety curb to each side of the drive. The access to the house will remain the same. Energy conservation will not change Large truck and fire apparatus access is currently already in place on the existing circular on-grade driveway.

APPEAL DECISION

Replacement of existing elevated driveway with reduction in minimum required structural design load: Granted provided signage is posted "No Heavy Trucks".

Appellant may contact John Butler (503 823-7339) with questions.

The Administrative Appeal Board finds with the conditions noted, that the information submitted by the appellant demonstrates that the approved modifications or alternate methods are consistent with the intent of the code; do not lessen health, safety, accessibility, life, fire safety or structural requirements; and that special conditions unique to this project make strict application of those code sections impractical.

Pursuant to City Code Chapter 24.10, you may appeal this decision to the Building Code Board of Appeal within 180 calendar days of the date this decision is published. For information on the appeals process and costs, including forms, appeal fee, payment methods and fee waivers, go to www.portlandoregon.gov/bds/appealsinfo, call (503) 823-7300 or come in to the Development Services Center.

ABBREVIATIONS

(A) AB	ABOVE	oc	ON CENTER
AB'S	ANCHOR BOLT ANCHOR BOLTS	OPP OSSC	OPPOSITE HAND OREGON STRUCTURAL
ADD'L	ADDITIONAL	0330	SPECIALTY CODE
(B)	BELOW	OWJ	OPEN WEB JOIST
BF B.O. DECK	BRACED FRAME BOTTOM OF DECK	PREP PLF	PREPARE, PREPARATION POUNDS PER LINEAR
B.O. FOOTING	BOTTOM OF FOOTING	PLF	FOOT
CBC	CALIFORNIA BUILDING CODE	PLY	PLYWOOD
CONC	CONCRETE	PSL	PARALLAM PSL BY
CONN DEMO	CONNECTION DEMOLISH		
DIA	DIAMETER	PT P/T	PRESSURE TREATED POST—TENSIONED
(E)	EXISTING	REINF	REINFORCEMENT
EA	EACH	SHEATH.	SHEATHING
EL ELEV	ELEVATION ELEVATOR	SIM	SIMILAR
EO	EDGE OF	S.O. GRADE STD	SLAB ON GRADE STANDARD
EXT	EXTERIOR	STRUCT	STRUCTURAL
FF	FINISHED FLOOR	T&B	TOP AND BOTTOM
FG	FINISHED GRADE	T.O. FOOTING	TOP OF FOOTING
FLR FTG	FLOOR FOOTING	T.O. SLAB	TOP OF SLAB
FNDN	FOUNDATION	T.O. STEEL T.O. STRUCT	TOP OF STEEL TOP OF STRUCTURE
GL	GLULAM	T.O. WALL	TOP OF WALL
GYP.	GYPSUM BOARD	TYP	TYPICAL
HORIZ	HORIZONTAL		
IBC	INTERNATIONAL BUILDING CODE	UNO	UNLESS NOTED
INFO	INFORMATION	(V)	OTHERWISE VERIFY, TO BE VERIFIED
INT	INTERIOR	(v)	BY CONTRACTOR
LSL	TIMBERSTRAND LSL BY	V.I.F.	VERIFY IN FIELD
LVL	TRUS-JOIST MACMILLAN MICROLLAM LVL BY	VERT	VERTICAL
LVL	TRUS-JOIST MACMILLAN	w w/	WIDE, WIDTH WITH
MF	MOMENT FRAME	w/ WWF	WILH WELDED WIRE FABRIC
MFR	MANUFACTURER	@	AT
/ki/	NICTA/		

GENERAL NOTES

STRUCTURAL SUMMARY:

GENERAL		
BUILDING CODE		
RISK CATEGORY	II	
<u>GEOTECHNICAL</u>		
ALLOWABLE BEARING PRESSURE, PA	2000	PSF
BY	ORIGINAL PER	MIT DRAWINGS
DATE	04/20	/1976
	LIVE LOAD	
FLOOR LIVE LOAD	DISTRIBUTED LOAD	POINT LOAD
DRIVEWAY	40 PSF	3000 LBS
SEISMIC (REFERENCE ONLY)		
MAPPED SPECTRAL RESPONSE	S _S =	1.000G
ACCELERATION PARAMETERS	S ₁ =	0.432G
SITE CLASS	D	
DESIGN SPECTRAL RESPONSE	S _{DS} =	0.733G
ACCELERATION PARAMETERS	S _{D1} =	0.451G
IMPORTANCE FACTOR, I _E	1.0	
SEISMIC DESIGN CATEGORY	D	
WIND (REFERENCE ONLY)		
ULTIMATE DESIGN WIND SPEED, V _{ULT}	120	МРН
EXPOSURE	В	

TEMPORARY CONDITIONS: THE STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. THE DRAWINGS DO NOT INDICATE THE METHOD OF CONSTRUCTION. THE STRUCTURE IS DESIGNED TO BE STABLE AS COMPLETED. THE CONTRACTOR IS RESPONSIBLE FOR THE DESIGN, ERECTION, AND INSPECTION OF TEMPORARY SHORES, BRACES, ETC. THAT SUPPORT THE STRUCTURE AGAINST ALL ANTICIPATED LOADS INCLUDING GRAVITY, WIND, AND LATERAL EARTH PRESSURES UNTIL THE COMPLETION OF THE STRUCTURE.

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SUBMITTALS: SHALL BE SUBMITTED TO THE ENGINEER VIA THE ARCHITECT PRIOR TO FABRICATION FOR THE FOLLOWING: STRUCTURAL STEEL, BIDDER DESIGNED ITEMS.

ALL STRUCTURAL MATERIALS SHALL HAVE CURRENT ICC ENGINEERING REPORTS ALTERATIONS TO THE STRUCTURAL DRAWINGS SHALL BEAR THE STAMP OF AN ENGINEER REGISTERED IN THE STATE OF OREGON.

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SPECIAL INSPECTION: THE OWNER SHALL EMPLOY AN ICC CERTIFIED SPECIAL INSPECTOR TO PROVIDE INSPECTION OF THE FOLLOWING ITEMS PER IBC SECTION

ITEM	INSPECTION FREQUENCY	NOTES
CONCRETE: EXPANSION ANCHORS EPOXY ANCHORS	CONTINUOUS PERIODIC	

GENERAL NOTES, CONT.

CONCRETE:

CONCRETE WORK SHALL COMPLY WITH CHAPTER 19 OF THE IBC. CONTRACTOR SHALL SUBMIT TEST DATA. MINIMUM COMPRESSIVE STRENGTH SHALL BE:

f'c = 4000 PSIWATER CEMENT RATIO BY WEIGHT NOT TO EXCEED .44 FOR NON AIR ENTRAINED AND .48 FOR AIR ENTRAINED.

AIR ENTRAIN 5% FOR CONCRETE EXPOSED TO WEATHER. AIR ENTRAINMENT TO CONFORM TO ASTM C 260 MINIMUM CEMENT PER CUBIC YARD = 400 LBS.

- EXISTING CONCRETE SURFACES THAT ARE BONDED TO NEW CONCRETE SHALL
- BE CLEANED AND ROUGHENED TO 1/4" AMPLITUDE. SUBMIT TEST DATA FOR CONCRETE MIX DESIGNS 2 WEEKS PRIOR TO
- PLACEMENT. USE A WATER-REDUCING ADMIXTURE IN CONFORMANCE WITH ASTM C494. MAX AGGREGATE SIZE = 3/4". MAX SLUMP = 4".
- AGGREGATE TO CONFORM TO ASTM C33. FOR EXPOSED CONCRETE, UTILIZE MICRO-FIBERS (POLYPROPYLENE FIBER OR EQUIVALENT PER ASTM C 1116) AND SUPERPLASTICIZERS (PER ASTM C 1017) TO HELP MINIMIZE PLASTIC SHRINKAGE AND CRACKING. CONTRACTOR TO ENSURE ADEQUATE FINISHING AND CURING PROCESSES ARE PERFORMED TO MINIMIZE CRACKING OF THE CONCRETE SURFACE. WET CURING IS REQUIRED. ENVIRONMENTAL FACTORS (SUCH AS HUMIDITY, TEMPERATURE, AND WIND) SHALL BE FACTORED INTO THE CURING PROCESS.

REINFORCING STEEL: REINFORCING STEEL SHALL BE GRADE Fy = 60 KSI IN CONFORMANCE WITH ASTM A615, INCLUDING S1. LAP ALL REÍNFORCING BARS A MINIMUM OF 24" OR AS NOTED. REINFORCING STEEL SHALL HAVE THE FOLLOWING CLEARANCES:

1" CLR SLAB ON GRADE 1 1/2" CLR RETAINING WALLS

LAP SPLICE SCHEDULE: REFER TO IBC CHAPTER 19 FOR REQUIREMENTS OF CLASS "A" AND CLASS "B" LAP SPLICES. FOR OTHER CONDITIONS REFER TO 2014 OSSC.

CONCRETE ACCESSORIES: EXPANSION ANCHORS SHALL BE SIMPSON STRONG-BOLT (ICC ESD-1771) OR AN EQUIVALENT EXPANSION ANCHOR WITH A CURRENT ICC EVALUATION REPORT INDICATING CONFORMANCE WITH ICC ACCEPTANCE CRITERIA AC 193. EPOXY ANCHORS SHALL BE F1554 GR 36 ALL-THREAD RODS IN PRE-DRILLED HOLES ANCHORED TO CAST CONCRETE WITH SIMPSON SET XP ADHESIVE (ICC ESR-2508), HILTI HY-200 (ICC ESR-3187), OR AN EQUIVALENT EPOXY ADHESIVE WITH A CURRENT ICC EVALUATION REPORT INDICATING CONFORMANCE WITH ICC ACCEPTANCE CRITERIA AC 308. DO NOT CUT REINFORCEMENT FOR EXPANSION ANCHOR OR EPOXY ANCHOR PLACEMENT. ANCHORS EXPOSED TO WEATHER SHALL BE GALVANIZED.

STRUCTURAL STEEL

STEEL SHAPE: **GRADE AND YIELD STRENGTH:**

ASTM A992, GRADE 50 CHANNELS, PLATES, AND ANGLES ASTM A500 GRADE B Fy=46 KSI, ASTM A1085

SPECIFICATION: DESIGN, FABRICATION, AND ERECTION SHALL FOLLOW THE "AISC SPECIFICATION FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" WITH THE "COMMENTARY" AND "CODE OF STANDARD

Fy=50 KSI

- BOLTS: BOLTS SHALL BE TWIST-OFF ASTM F3125 BOLTS UNLESS NOTED OTHERWISE. FAYING SURFACES AT SLIP-CRITICAL BOLTS SHALL BE FREE OF PAINT. ALL SLIP-CRITICAL BOLTS SHALL BE INSTALLED PER AISC REQUIREMENTS AND BOLT MANUFACTURER.
- WELDING SPECIFICATION: WELDING SHALL CONFORM TO THE AMERICAN WELDING SOCIETY (AWS) CODES FOR ARC AND GAS WELDING. ALL WELDING SHALL BE PRE-QUALIFIED AND PERFORMED WITH A WELDING PROCEDURE SPECIFICATION (WPS) PER AWS D1.1.
- 4. ALL WELDERS SHALL BE QUALIFIED BY THE STATE OF OREGON. WELD MATERIAL: ALL WELDS SHALL BE MADE WITH E70XX ELECTRODES AND SHALL BE 3/6" MINIMUM UNLESS NOTED OTHERWISE.

SAWN LUMBER: SAWN LUMBER SHALL CONFORM TO THE WESTERN WOOD PRODUCTS ASSOCIATION GRADING RULES. LUMBER SHALL BE KD-DRY (KILN DRY) WITH MAXIMUM MOISTURE CONTENT OF 19% TO BE SUBMITTED BY CONTRACTOR PROTECT STORED WOOD ON SITE FROM MOISTURE. THE SPECIES AND GRADE

DIMENSIONAL LUMBER 2" TO 4" THICK HEADERS/BEAMS

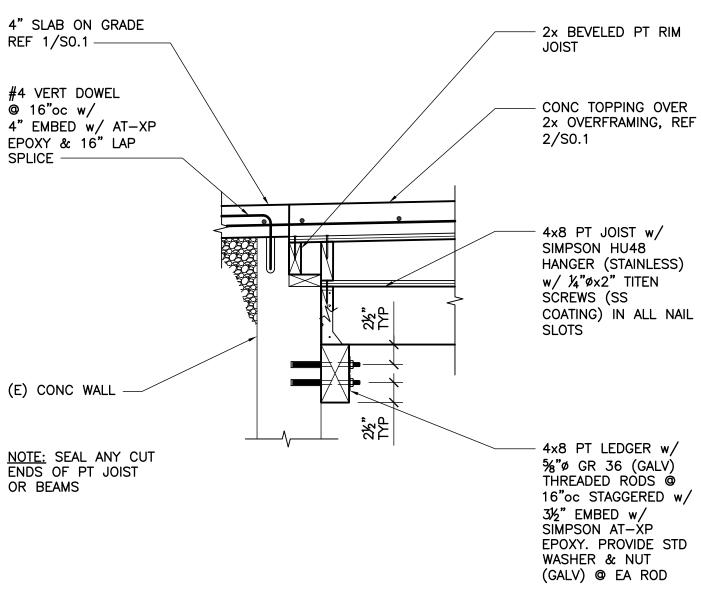
SHALL BE AS NOTED BELOW:

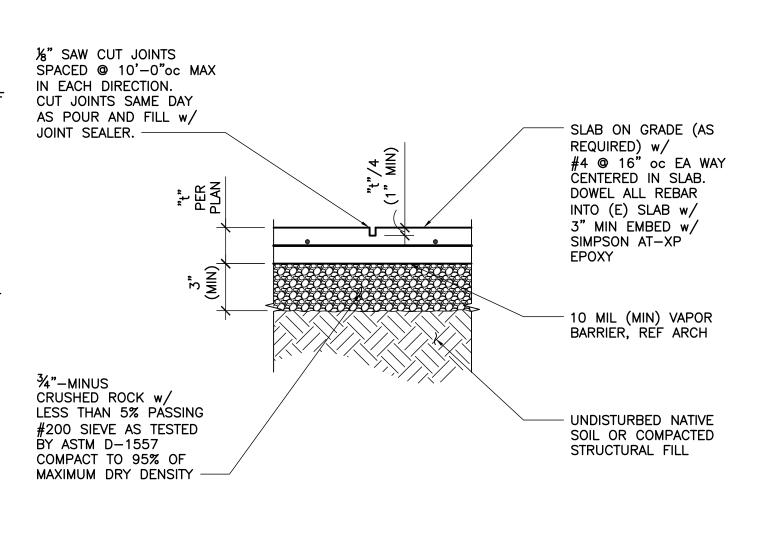
DOUGLAS FIR-LARCH #2 DOUGLAS FIR-LARCH # DOUGLAS FIR-LARCH #

NAILING NOT SHOWN SHALL BE AS INDICATED ON FASTENING SCHEDULE. ALL BOLTS AND LAG SCREWS (HAND TIGHTEN) SHALL BE INSTALLED WITH STANDARD CUT WASHERS. CUTTING AND NOTCHING OF STUDS SHALL CONFORM TO IBC CHAPTER 23. ALL LUMBER IN CONTACT WITH CONCRETE SHALL BE PRESSURE TREATED. FRAMING ACCESSORIES AND STRUCTURAL FASTENERS SHALL BE MANUFACTURED BY SIMPSON STRONG TIE. ALL FASTENERS AND HANGERS EXPOSED TO WEATHER SHALL BE STAINLESS BY SIMPSON OR APPROVED EQUAL

PLYWOOD OR OSB SHEATHING: PLYWOOD OR OSB PANELS SHALL CONFORM TO THE REQUIREMENTS OF U.S. PRODUCT STANDARD PS 1 (PLYWOOD) OR PS-2 (OSB) FOR CONSTRUCTION. PANELS SHALL BE APA RATED SHEATHING, EXPOSURE 1. PLYWOOD INSTALLATION SHALL BE IN CONFORMANCE WITH APA RECOMMENDATIONS. ALLOW 1/8 INCH SPACING AT ROOF PANEL ENDS AND EDGES, UNLESS OTHERWISE RECOMMENDED BY THE PANEL MANUFACTURER. SHEATHING TO BE AS FOLLOWS:

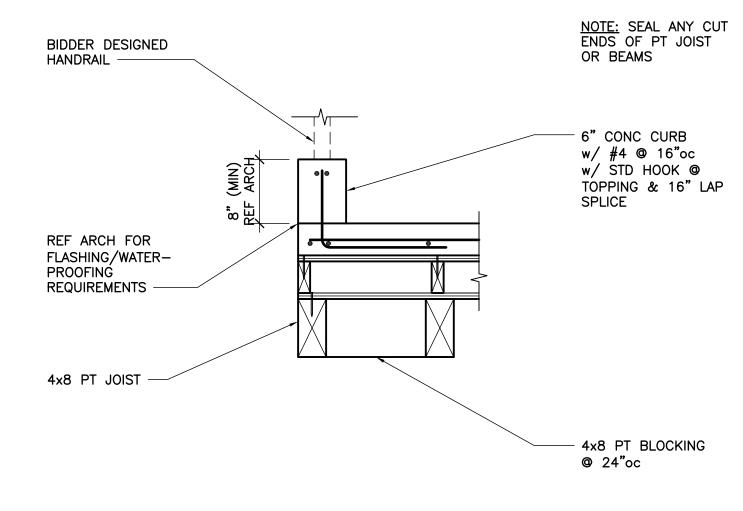
FLOOR SHEATHING: 34" STRUCTURAL 1 PLYWOOD OR ORIENTED STRAND BOARD (w/ EXTERIOR GLUE), RATED 32/16. GLUE & NAIL w/ 0.148"ø x 3" NAILS @ 6" oc ALONG PANEL EDGES/12" oc IN FIELD

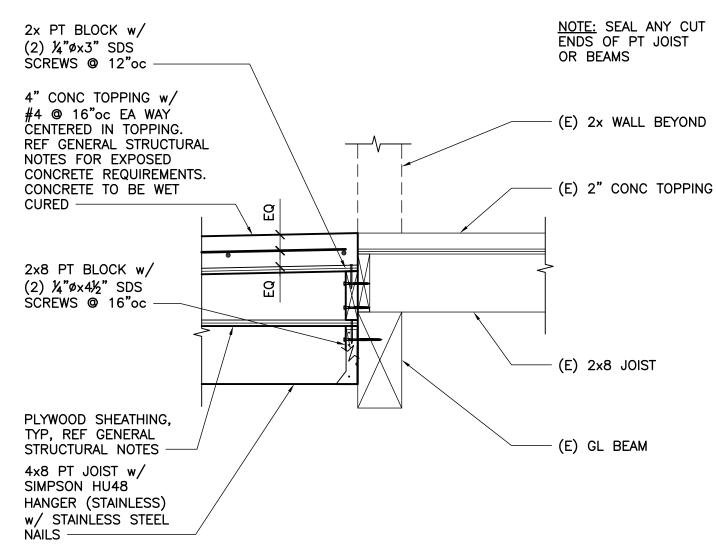




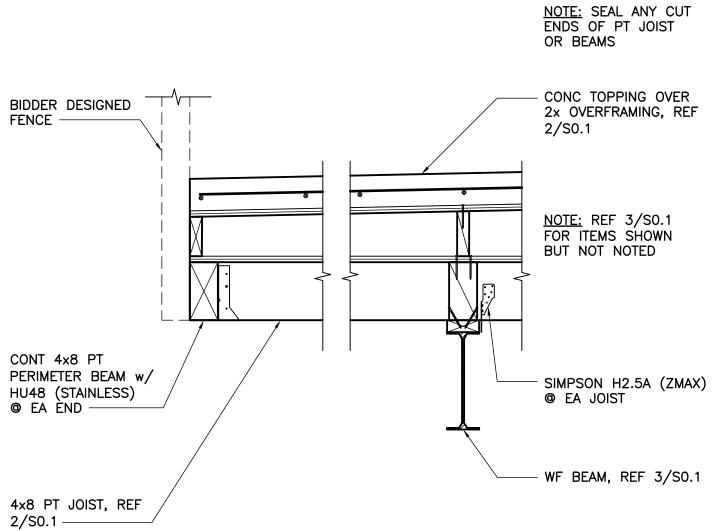
JOIST TO (E) CONC WALL

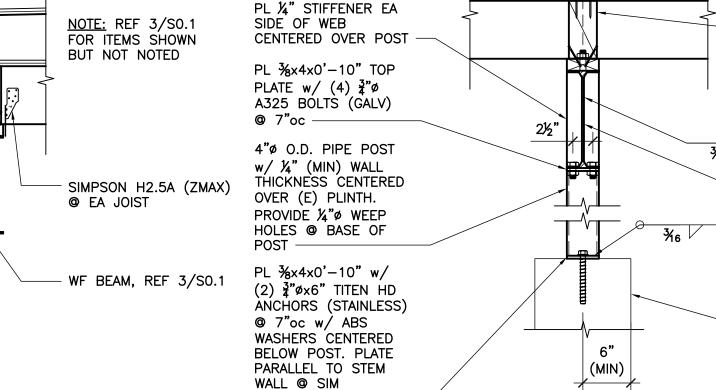












LOCATION

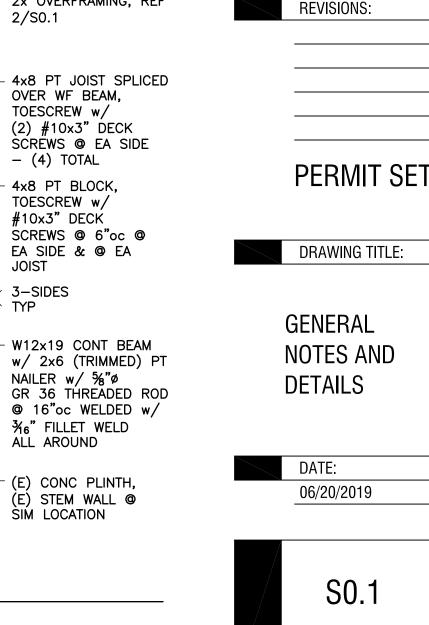
1.) ALL STEEL TO BE

OR POWDER COATED

ENDS OF PT JOIST OR

2.) SEAL ANY CUT

HOT-DIPPED GALVANIZED



CONC TOPPING OVER

2x OVERFRAMING, REF

JOIST

CANTILEVERD DECK JOIST

erson a Morldon II

EXPIRES 12-31-20

(5 (5)

S

RHODE

I

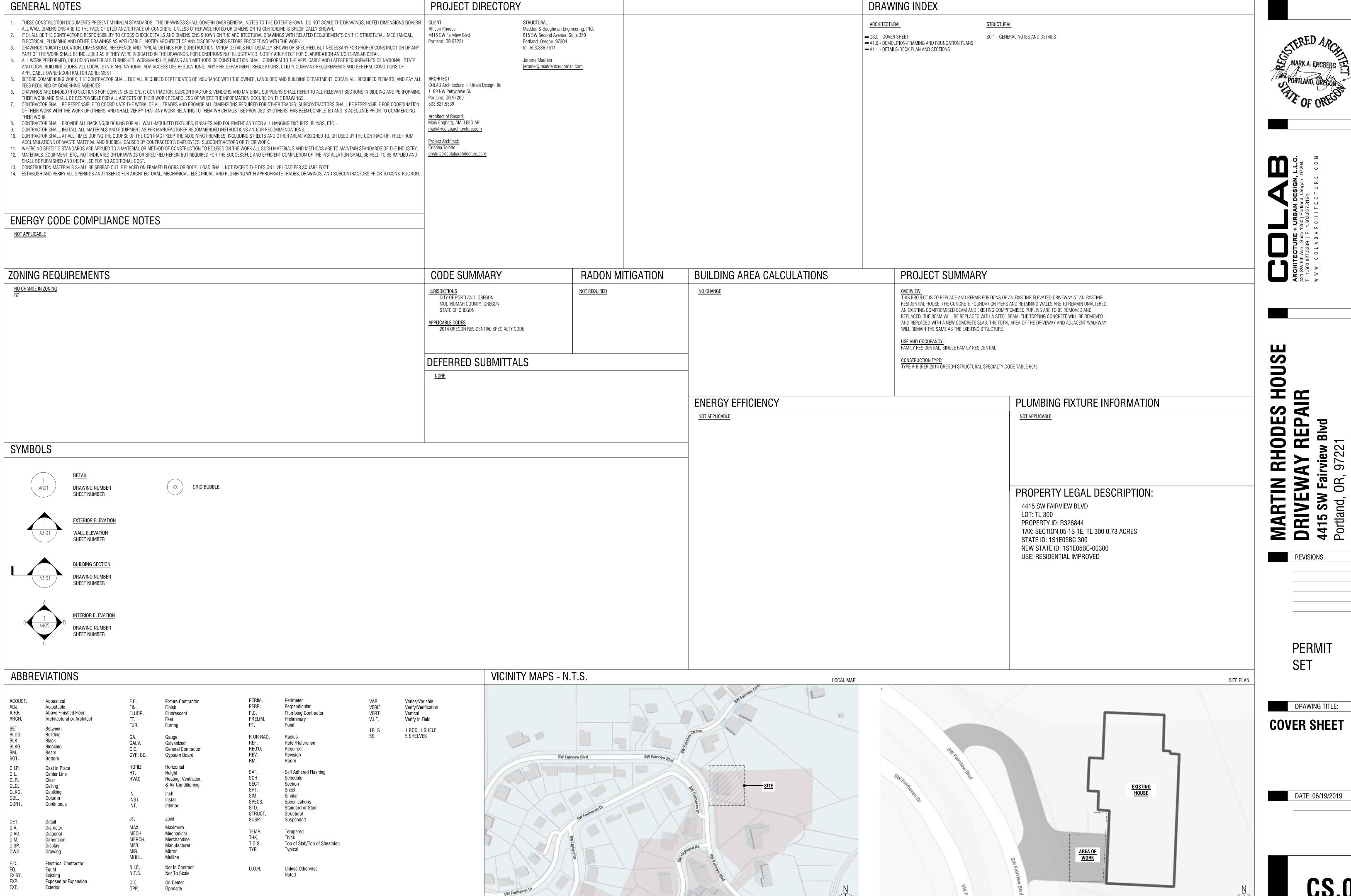
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R

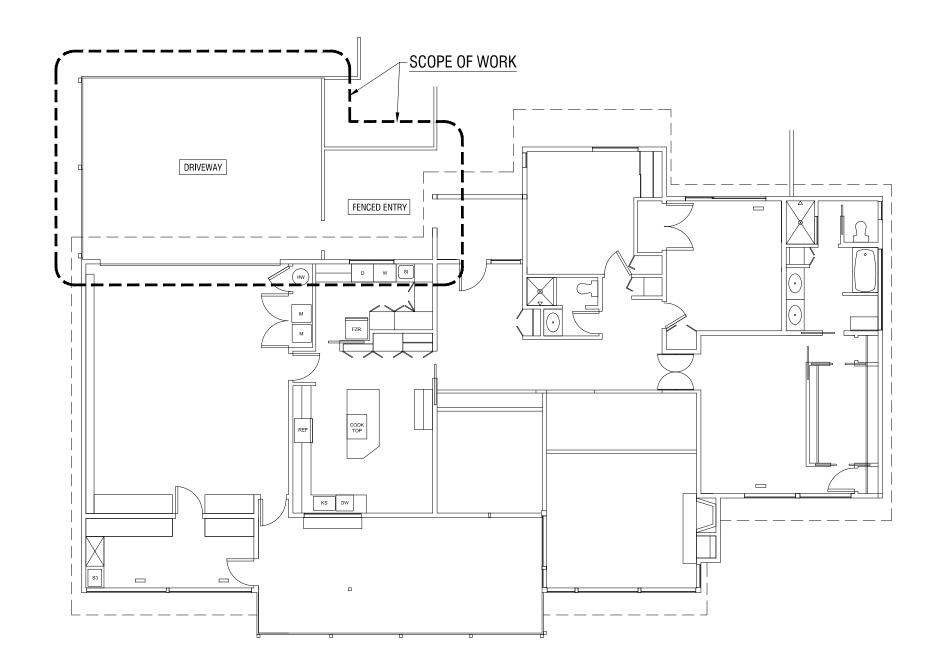
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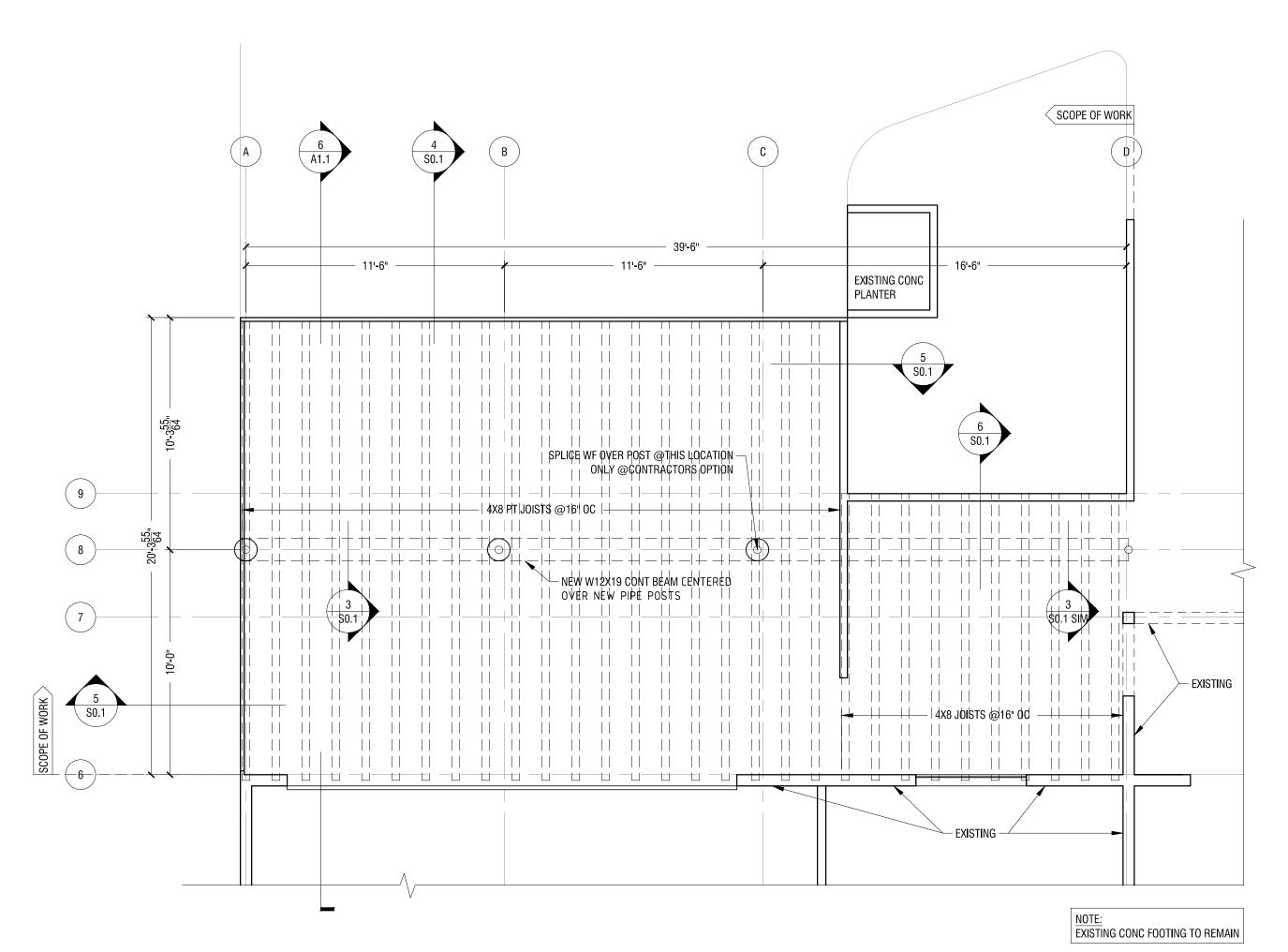
节音

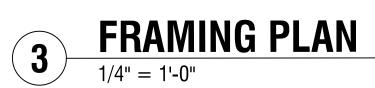


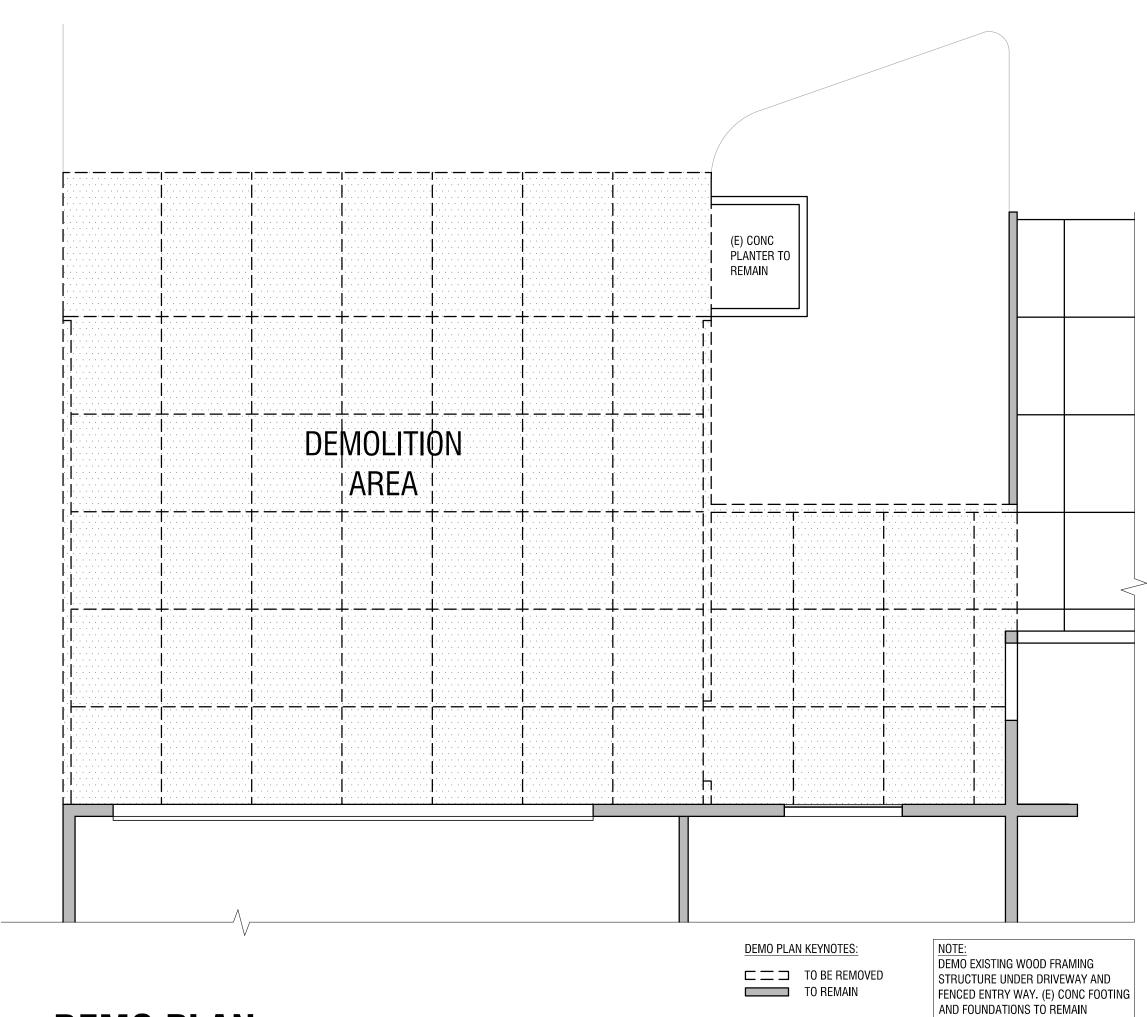




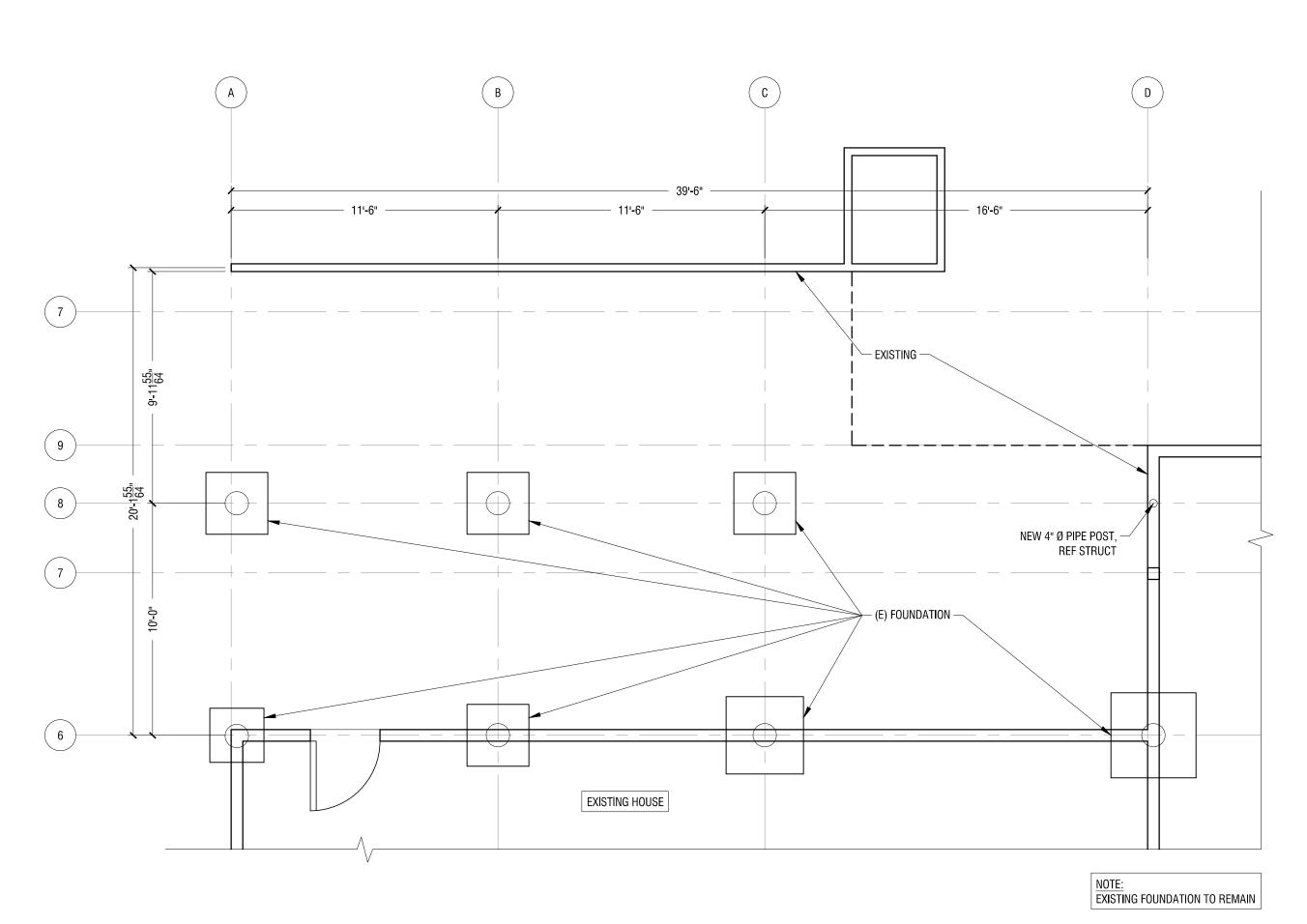




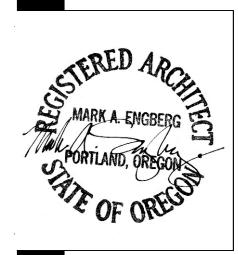




DEMO PLAN1/4" = 1'-0"



FOUNDATION PLAN1/4" = 1'-0"



**RCHITECTURE + URBAN DESIGN, L.L.C. 21 SW 6th Ave., Suite 1250 | Portland, Oregon 97204 : 1.503.827.5339 | F: 1.503.827.8164 | W V. COLABAR CHITECTURE. COM

RTIN RHODES HOUSE
VEWAY REPAIR

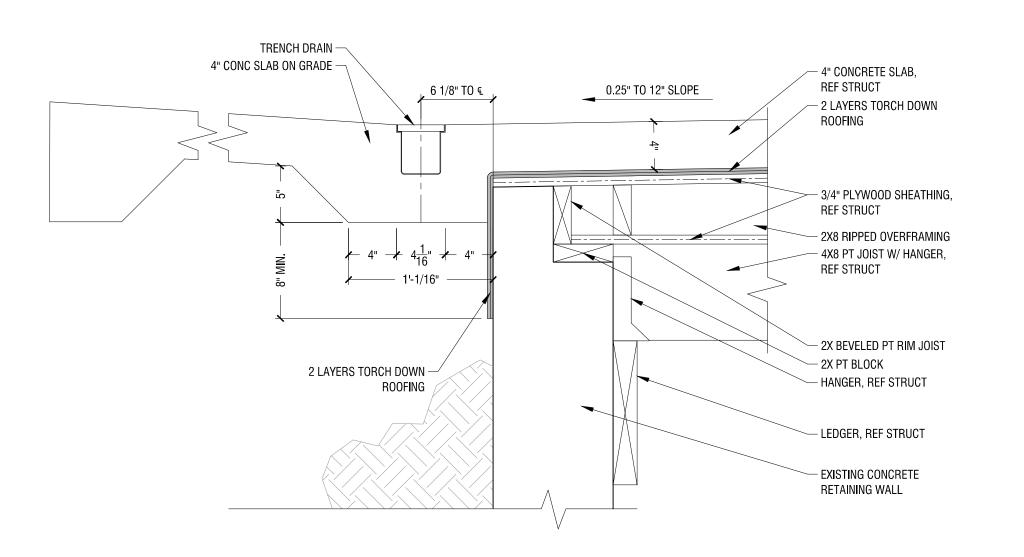
REVISIONS:

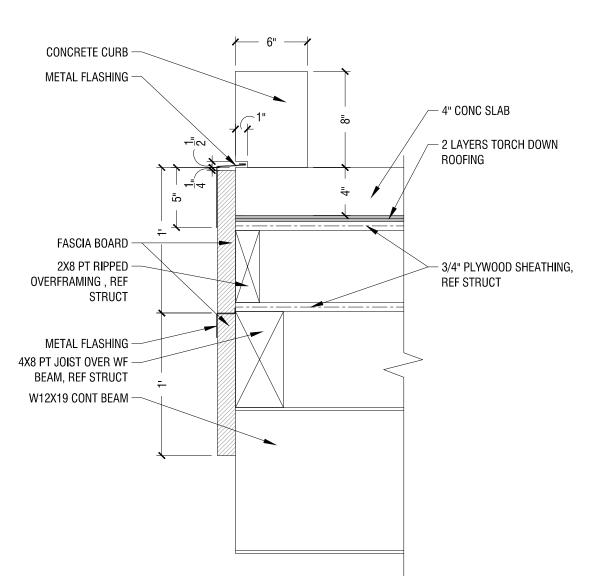
PERMIT SET

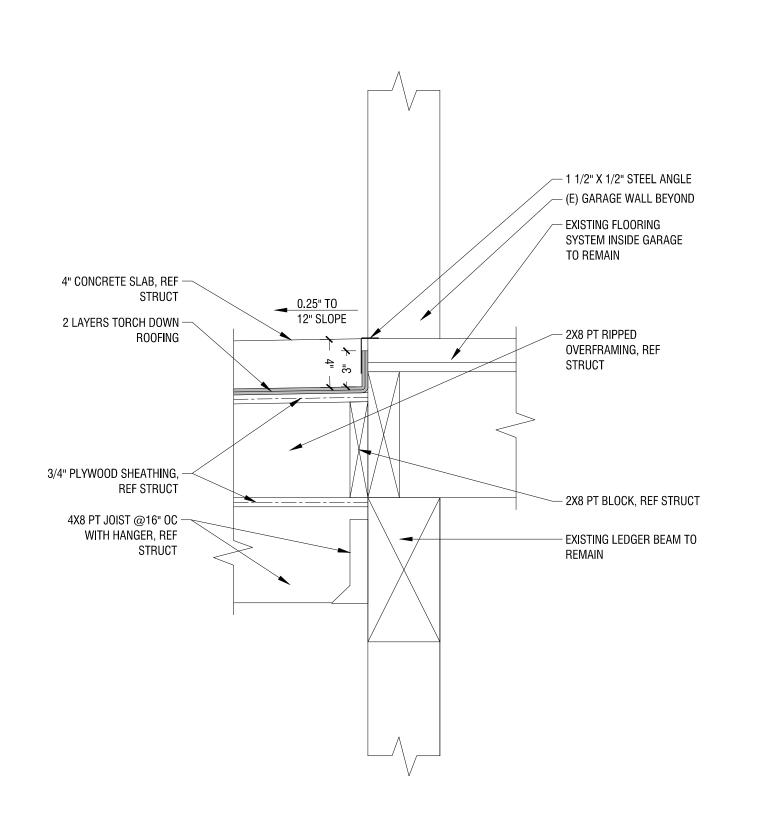
DEMO
FRAMING
FOUNDATION
PLAN

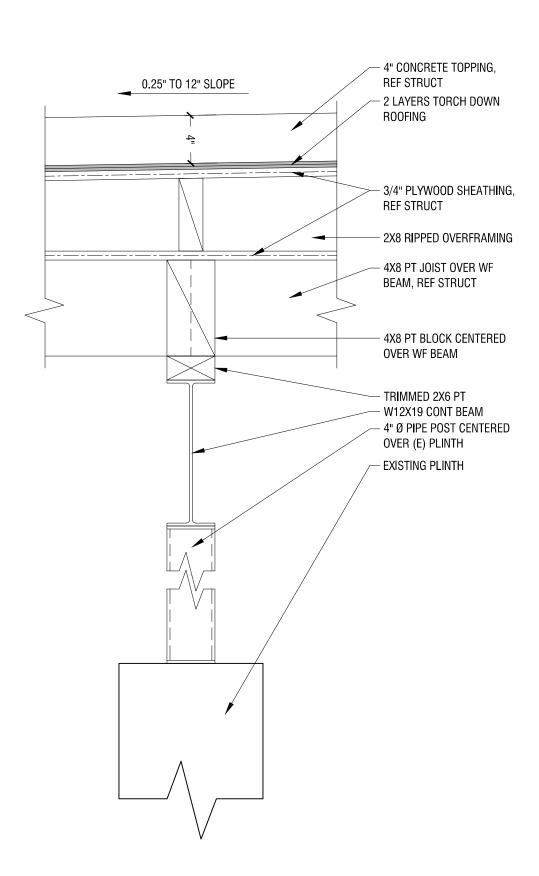
DATE: 06/19/2019

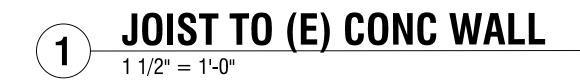
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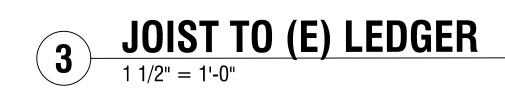




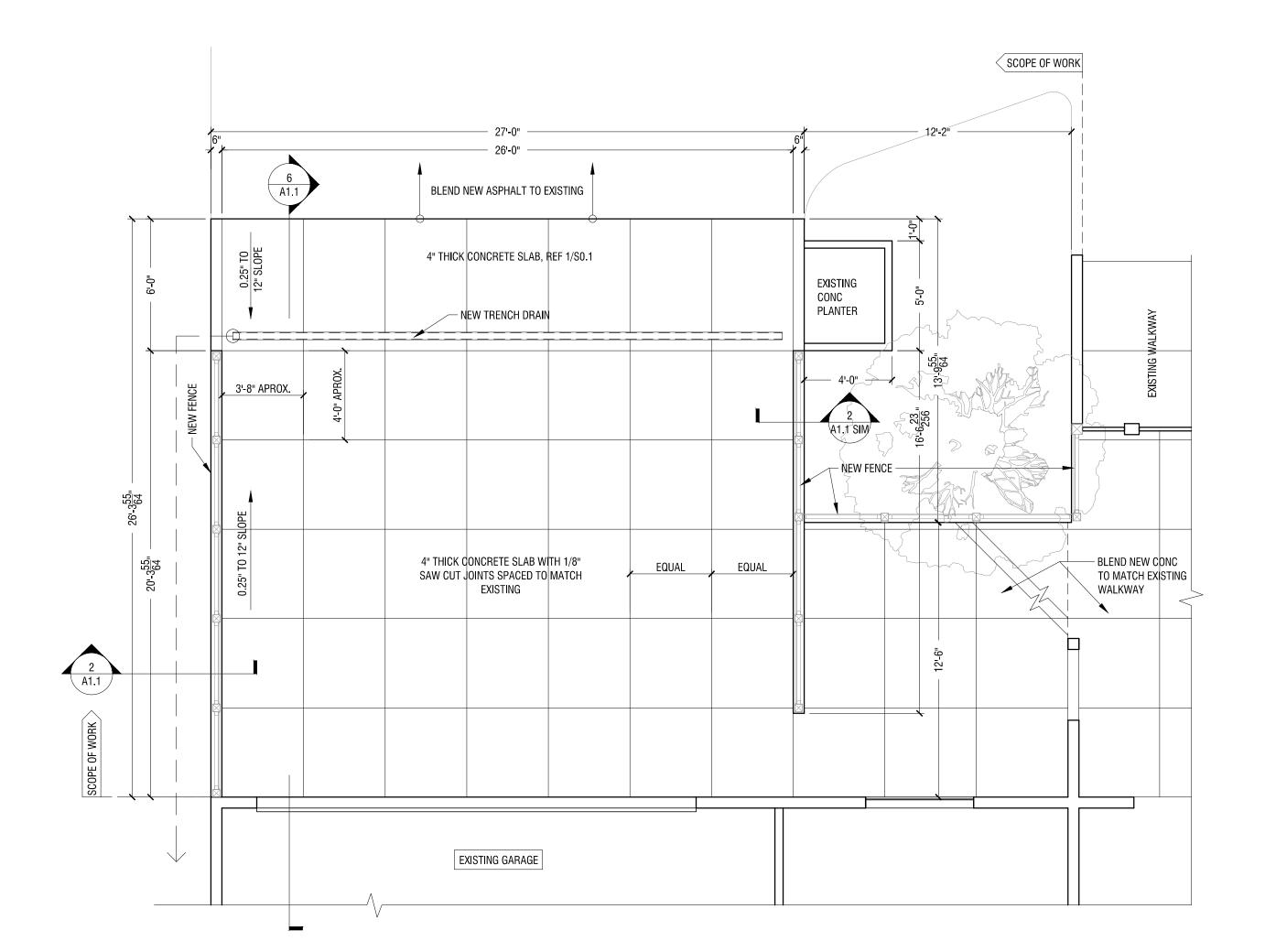


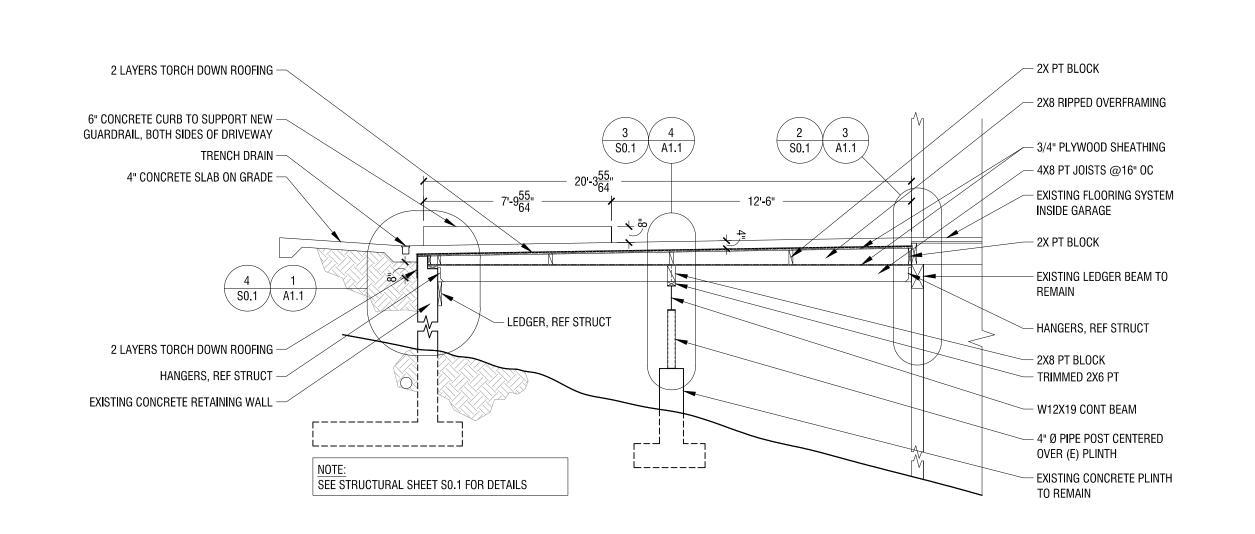


















ARCHITECTURE + URBAN DESIGN, L.L.C. 421 SW 6th Ave., Suite 1250 | Portland, Oregon 97204 T: 1.503.827.5339 | F: 1.503.827.8164 W W W COLABARCHITECTURE.COM

MARTIN RHODES HOUSE
IRIVEWAY REPAIR
A15 SW Fairview Blvd
ortland, OR, 97221

PERMIT

REVISIONS:

DRAWING TITLE:

DETAILS DECK PLAN SECTION

DATE: 06/19/2019

A1.1

ABBREVIATIONS

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- BE CLEANED AND ROUGHENED TO 1/4" AMPLITUDE. SUBMIT TEST DATA FOR CONCRETE MIX DESIGNS 2 WEEKS PRIOR TO
- PLACEMENT. USE A WATER-REDUCING ADMIXTURE IN CONFORMANCE WITH ASTM C494. MAX AGGREGATE SIZE = 3/4". MAX SLUMP = 4".
- AGGREGATE TO CONFORM TO ASTM C33. FOR EXPOSED CONCRETE, UTILIZE MICRO-FIBERS (POLYPROPYLENE FIBER OR EQUIVALENT PER ASTM C 1116) AND SUPERPLASTICIZERS (PER ASTM C 1017) TO HELP MINIMIZE PLASTIC SHRINKAGE AND CRACKING. CONTRACTOR TO ENSURE ADEQUATE FINISHING AND CURING PROCESSES ARE PERFORMED TO MINIMIZE CRACKING OF THE CONCRETE SURFACE. WET CURING IS REQUIRED. ENVIRONMENTAL FACTORS (SUCH AS HUMIDITY, TEMPERATURE, AND WIND) SHALL BE FACTORED INTO THE CURING PROCESS.

REINFORCING STEEL: REINFORCING STEEL SHALL BE GRADE Fy = 60 KSI IN CONFORMANCE WITH ASTM A615, INCLUDING S1. LAP ALL REÍNFORCING BARS A MINIMUM OF 24" OR AS NOTED. REINFORCING STEEL SHALL HAVE THE FOLLOWING CLEARANCES:

1" CLR SLAB ON GRADE 1 1/2" CLR RETAINING WALLS

LAP SPLICE SCHEDULE: REFER TO IBC CHAPTER 19 FOR REQUIREMENTS OF CLASS "A" AND CLASS "B" LAP SPLICES. FOR OTHER CONDITIONS REFER TO 2014 OSSC.

CONCRETE ACCESSORIES: EXPANSION ANCHORS SHALL BE SIMPSON STRONG-BOLT (ICC ESD-1771) OR AN EQUIVALENT EXPANSION ANCHOR WITH A CURRENT ICC EVALUATION REPORT INDICATING CONFORMANCE WITH ICC ACCEPTANCE CRITERIA AC 193. EPOXY ANCHORS SHALL BE F1554 GR 36 ALL-THREAD RODS IN PRE-DRILLED HOLES ANCHORED TO CAST CONCRETE WITH SIMPSON SET XP ADHESIVE (ICC ESR-2508), HILTI HY-200 (ICC ESR-3187), OR AN EQUIVALENT EPOXY ADHESIVE WITH A CURRENT ICC EVALUATION REPORT INDICATING CONFORMANCE WITH ICC ACCEPTANCE CRITERIA AC 308. DO NOT CUT REINFORCEMENT FOR EXPANSION ANCHOR OR EPOXY ANCHOR PLACEMENT. ANCHORS EXPOSED TO WEATHER SHALL BE GALVANIZED.

STRUCTURAL STEEL

STEEL SHAPE: **GRADE AND YIELD STRENGTH:**

ASTM A992, GRADE 50 CHANNELS, PLATES, AND ANGLES ASTM A500 GRADE B Fy=46 KSI, ASTM A1085

SPECIFICATION: DESIGN, FABRICATION, AND ERECTION SHALL FOLLOW THE "AISC SPECIFICATION FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" WITH THE "COMMENTARY" AND "CODE OF STANDARD

Fy=50 KSI

- BOLTS: BOLTS SHALL BE TWIST-OFF ASTM F3125 BOLTS UNLESS NOTED OTHERWISE. FAYING SURFACES AT SLIP-CRITICAL BOLTS SHALL BE FREE OF PAINT. ALL SLIP-CRITICAL BOLTS SHALL BE INSTALLED PER AISC REQUIREMENTS AND BOLT MANUFACTURER.
- WELDING SPECIFICATION: WELDING SHALL CONFORM TO THE AMERICAN WELDING SOCIETY (AWS) CODES FOR ARC AND GAS WELDING. ALL WELDING SHALL BE PRE-QUALIFIED AND PERFORMED WITH A WELDING PROCEDURE SPECIFICATION (WPS) PER AWS D1.1.
- 4. ALL WELDERS SHALL BE QUALIFIED BY THE STATE OF OREGON. WELD MATERIAL: ALL WELDS SHALL BE MADE WITH E70XX ELECTRODES AND SHALL BE 3/6" MINIMUM UNLESS NOTED OTHERWISE.

SAWN LUMBER: SAWN LUMBER SHALL CONFORM TO THE WESTERN WOOD PRODUCTS ASSOCIATION GRADING RULES. LUMBER SHALL BE KD-DRY (KILN DRY) WITH MAXIMUM MOISTURE CONTENT OF 19% TO BE SUBMITTED BY CONTRACTOR PROTECT STORED WOOD ON SITE FROM MOISTURE. THE SPECIES AND GRADE

DIMENSIONAL LUMBER 2" TO 4" THICK HEADERS/BEAMS

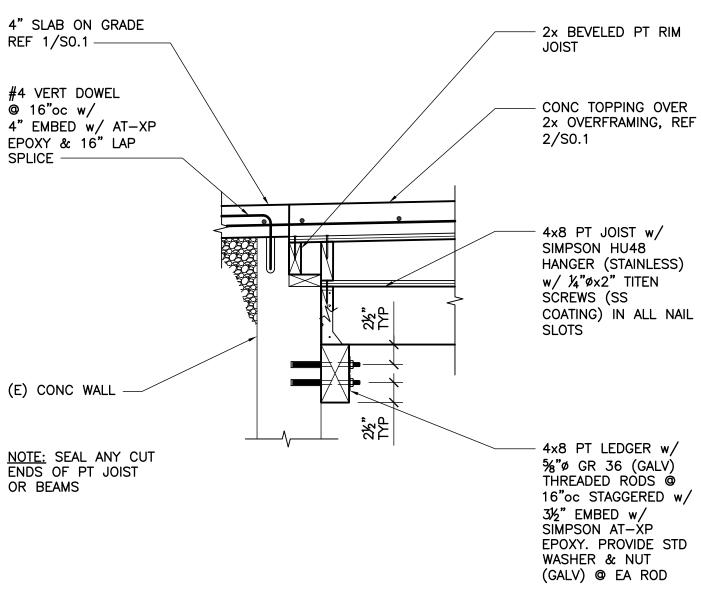
SHALL BE AS NOTED BELOW:

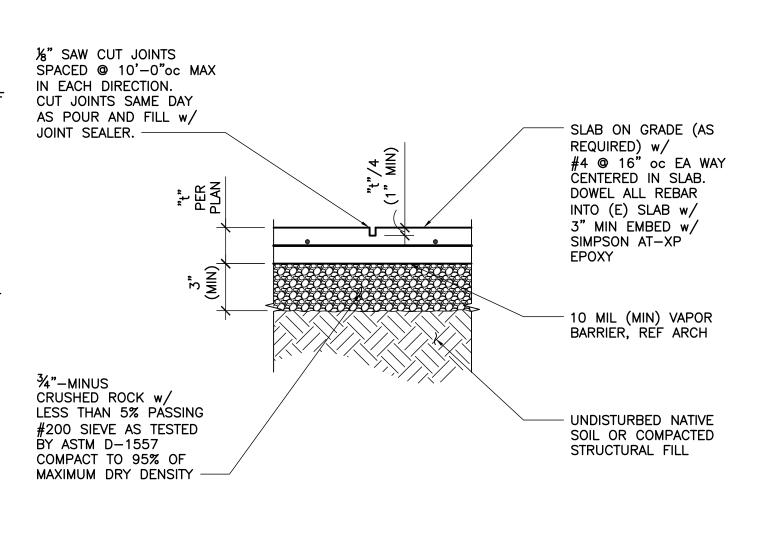
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NAILING NOT SHOWN SHALL BE AS INDICATED ON FASTENING SCHEDULE. ALL BOLTS AND LAG SCREWS (HAND TIGHTEN) SHALL BE INSTALLED WITH STANDARD CUT WASHERS. CUTTING AND NOTCHING OF STUDS SHALL CONFORM TO IBC CHAPTER 23. ALL LUMBER IN CONTACT WITH CONCRETE SHALL BE PRESSURE TREATED. FRAMING ACCESSORIES AND STRUCTURAL FASTENERS SHALL BE MANUFACTURED BY SIMPSON STRONG TIE. ALL FASTENERS AND HANGERS EXPOSED TO WEATHER SHALL BE STAINLESS BY SIMPSON OR APPROVED EQUAL

PLYWOOD OR OSB SHEATHING: PLYWOOD OR OSB PANELS SHALL CONFORM TO THE REQUIREMENTS OF U.S. PRODUCT STANDARD PS 1 (PLYWOOD) OR PS-2 (OSB) FOR CONSTRUCTION. PANELS SHALL BE APA RATED SHEATHING, EXPOSURE 1. PLYWOOD INSTALLATION SHALL BE IN CONFORMANCE WITH APA RECOMMENDATIONS. ALLOW 1/8 INCH SPACING AT ROOF PANEL ENDS AND EDGES, UNLESS OTHERWISE RECOMMENDED BY THE PANEL MANUFACTURER. SHEATHING TO BE AS FOLLOWS:

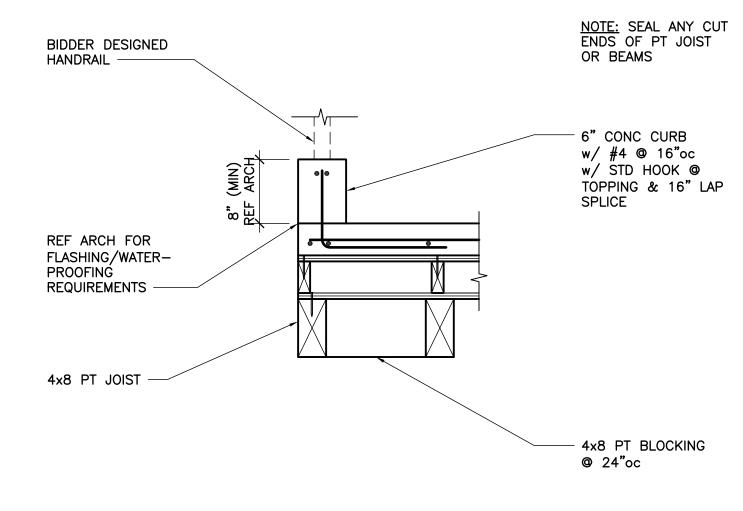
FLOOR SHEATHING: 34" STRUCTURAL 1 PLYWOOD OR ORIENTED STRAND BOARD (w/ EXTERIOR GLUE), RATED 32/16. GLUE & NAIL w/ 0.148"ø x 3" NAILS @ 6" oc ALONG PANEL EDGES/12" oc IN FIELD

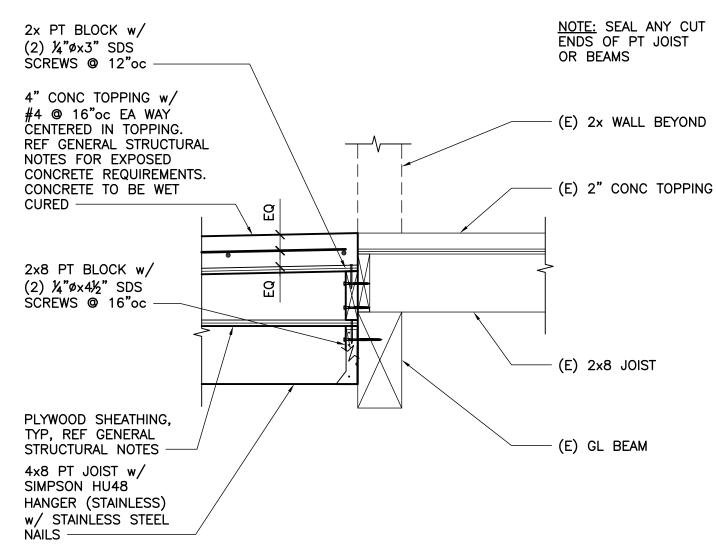




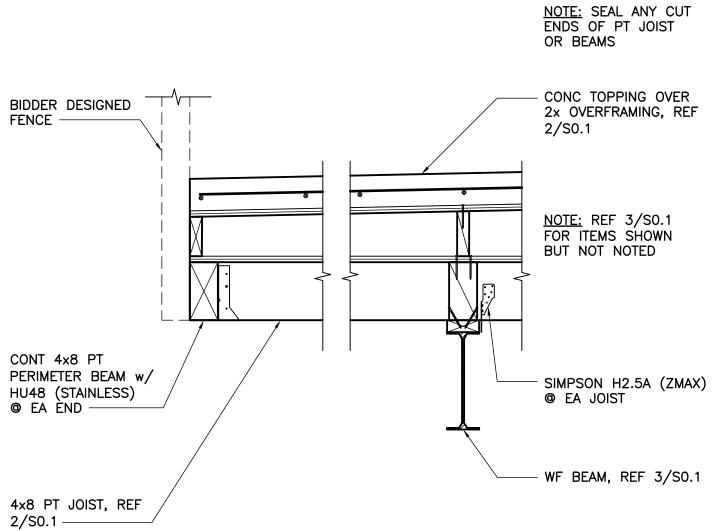
JOIST TO (E) CONC WALL

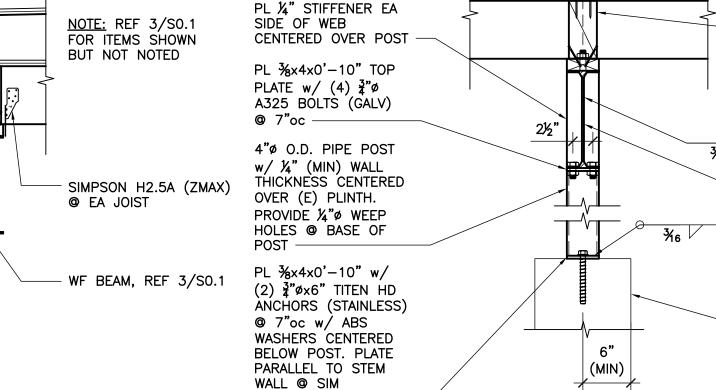












LOCATION

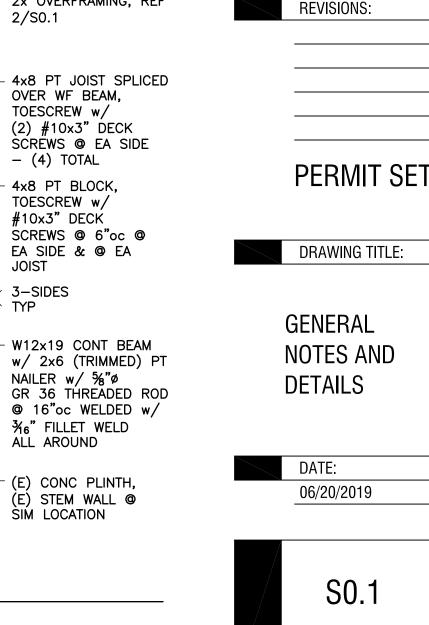
1.) ALL STEEL TO BE

OR POWDER COATED

ENDS OF PT JOIST OR

2.) SEAL ANY CUT

HOT-DIPPED GALVANIZED



CONC TOPPING OVER

2x OVERFRAMING, REF

JOIST

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EXPIRES 12-31-20

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TOPIC: Structural Design – OSSC/16/#2

CODE: Oregon Structural Specialty Code, 2014 Edition

Oregon Fire Code, 2014 Edition

REVISED: September 16, 2016 [Paul L. Scarlett] Director

REFERENCE: Chapter 16, Oregon Structural Specialty Code

Section 503, Oregon Fire Code

SUBJECT: Elevated Private Driveways and Parking Decks -

Structural Design Loads

QUESTION: What are the structural loading requirements for private elevated

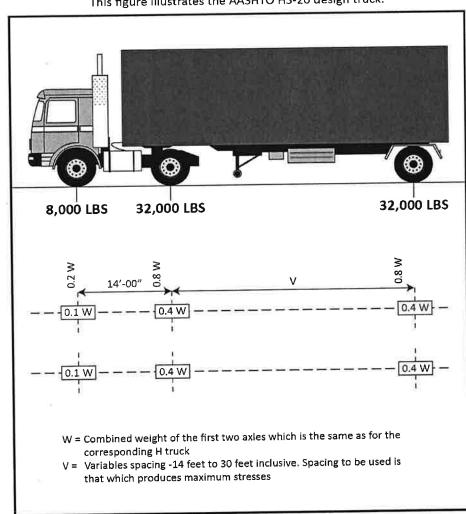
driveways and parking decks?

RESPONSE: Private elevated driveway structures and parking decks will be subjected to the loading caused by heavy vehicles such as garbage trucks, delivery trucks, and moving vans. The Portland Bureau of Transportation (PBOT) requires ramps or decks that are constructed in the street right-of-way to meet the live load design requirements of the AASHTO HS- 20 Design Truck as a minimum. To meet PBOT standards, elevated private driveway structures and parking decks shall be designed to support the same vehicle load as the adjacent street or shoulder (HS-20 Design Truck minimum, see illustration on page 2). However, if fire apparatus access is provided by an elevated private driveway or parking deck that access must be designed and constructed to meet AASHTO HS-25 loading per requirements of the Oregon Fire Code.

Overhead barriers that restrict the height of vehicles entering a driveway or parking area will not be accepted as an alternative to providing the minimum structural load design.

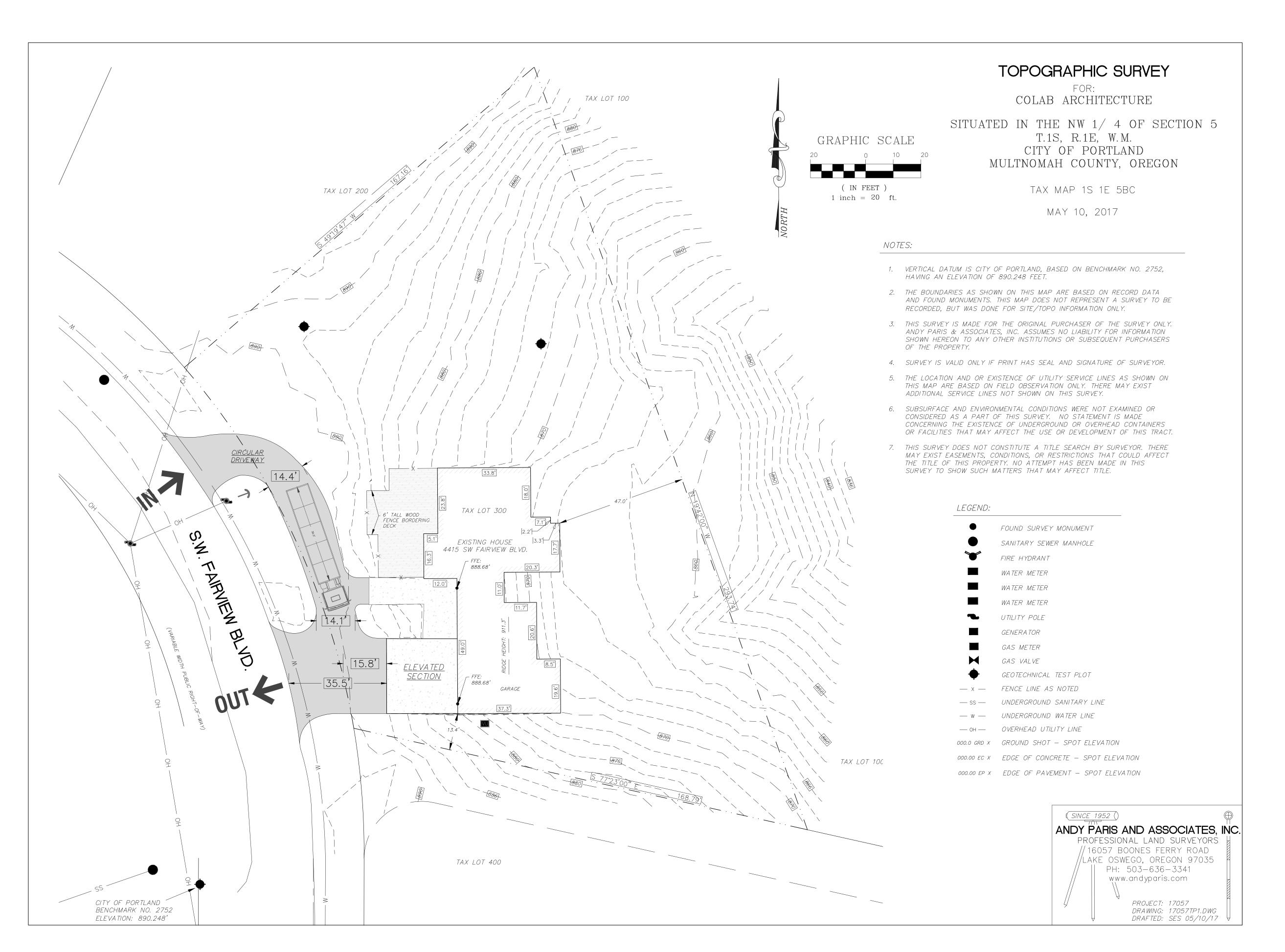
OSSC/16/#2 Elevated Private Driveways and Parking Decks-Structural Design Loads Page 2 of 2 September 16, 2016

Questions regarding the street design loads should be directed to the Portland Bureau of Transportation at 503-823-7002.



This figure illustrates the AASHTO HS-20 design truck.

Updates March 1, 1999 edition Updates July 1, 1996 edition Replaces Code Guide UBC/23/#2 which replaced Policy & Procedures # D-39





RCHITECTURE + URBAN DESIGN, L.L.C.

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BUILDING CODE APPEAL DOC

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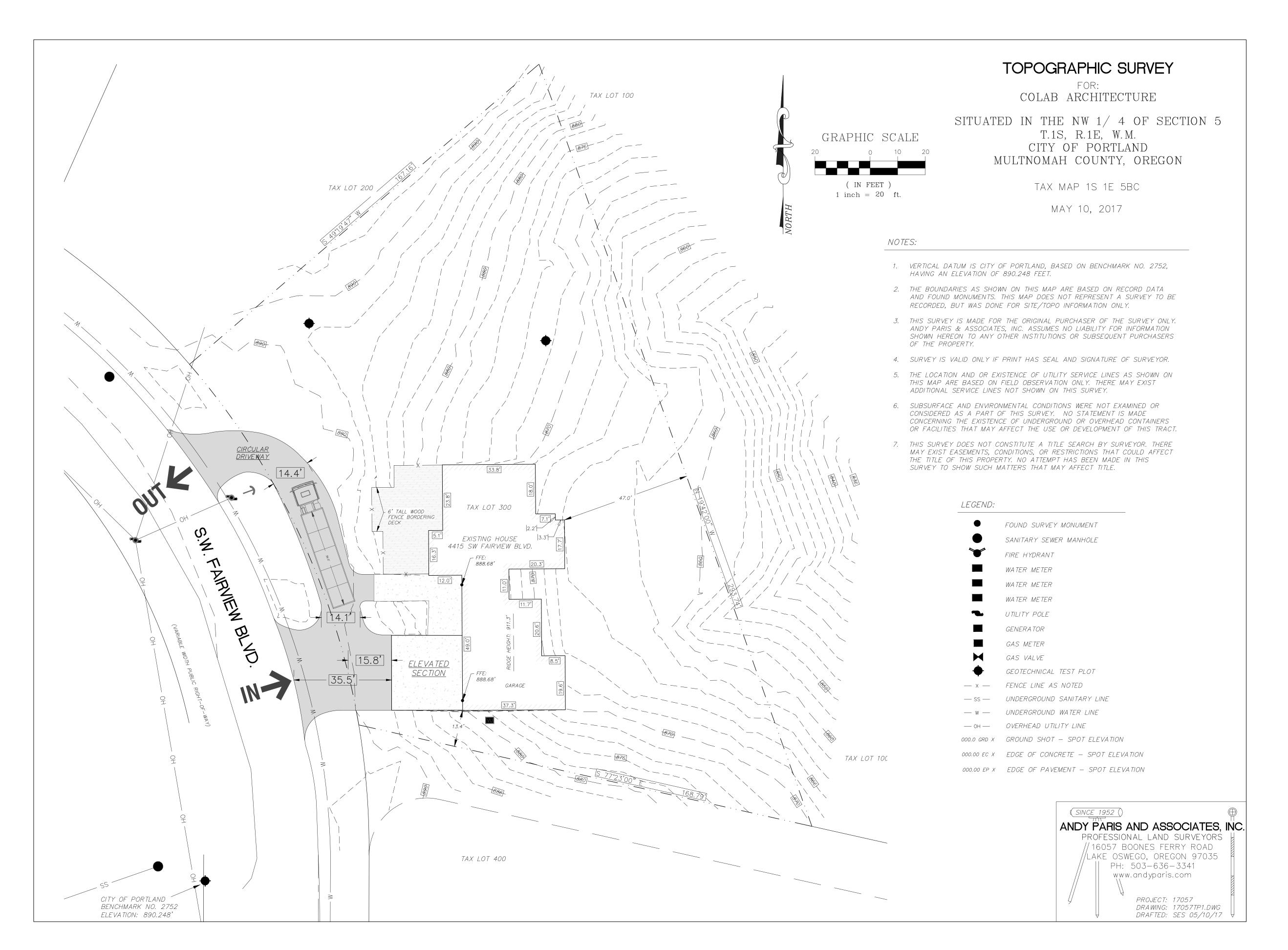
REVISIONS:

DRAWING TITLE:

VEHICLE ACCESS AND CIRCULATION

DATE: 06/25/2019

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ARCHITECTURE + URBAN DESIGN, L.L.C.

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REVISIONS:

DRAWING TITLE:

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DATE: 06/25/2019

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