# SECREST-SARVER RESIDENCE

4131 SE GRANT ST| PORTLAND, OR

20-153029 RS REV 01

#### **DESIGN REVISION CALCS**

25 MAY 2021

PREPARED FOR: INK BUILT ARCHITECTURE PORTLAND, OR



# SECREST-SARVER RESIDENCE

4131 SE GRANT ST| PORTLAND, OR

### PROJECT DESCRIPTION:

NEW SINGLE FAMILY RESIDENCE OF LIGHT FRAMED CONSTRUCTION ON CONCRETE FOUNDATIONS

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#### **DESIGN CRITERIA**

1)	BUILDING CODE:	
	A) 2019 OREGON STRUCTURAL SPECIALTY CODE (OSSC)	
2)	LOADS AND ALLOWABLES	
	A) LIVE LOAD:	
	I) RESIDENTIAL FLOORS	40 PSF
	II) DECKS	60 PSF
	III) ROOF	25 PSF (SNOW)
	B) WIND:	
	I) BASIC WIND SPEED (LRFD, 3-SEC. GUST)	100 MPH
	II) EXPOSURE CATEGORY	B
3)	FOUNDATION DESIGN CRITERIA (DESIGN PRESSURES PER 2012 IBC, U.O.N.)	
	A) ALLOWABLE FOUNDATION BEARING:	1500 PSF
	B) ACTIVE EARTH PRESSURE:	35 PCF
	C) PASSIVE FARTH RESISTANCE	150 PSE/ET

#### MATERIAL PROPERTIES

1)	WOOD	
	A) GLUE LAMINATED BEAMS	COMB 24F-V4 (24F-V8 WHERE NOTED)
		NO.2 D.FIR/LARCH
	C) ROOF SHEATHING	
	D) WALL SHEATHING	7/16" C-D, (1/2", "C-D", STRUCT 1 WHERE NOTED) INDEX 32/16
2)	CONCRETE:	,
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# SECREST-SARVER RESIDENCE

4131 SE GRANT ST| PORTLAND, OR

**GRAVITY FRAMING CALCULATIONS** 

SECREST SARVER REV02 | 05/25/21 | PG. 4 / 19

FORTE WEB JOB SUMMARY REPORT

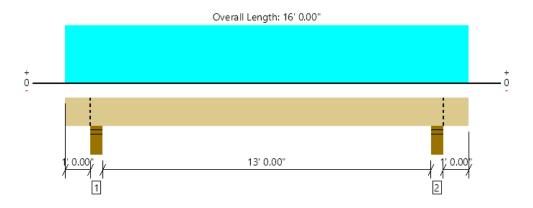
20020 - INKBUILT - SECREST SARVER REV02

ROOF					
Member Name	Results	Current Solution	Comments		
RJ210	Passed	1 piece(s) 2 x 10 DF No.2 @ 24" OC			
DBL 2X10	Passed	2 piece(s) 2 x 10 DF No.2			
2X10 STRUCT FASCIA GRID 1	Passed	1 piece(s) 2 x 10 DF No.2			
H48 grid 1	Passed	1 piece(s) 4 x 8 DF No.2			
H24-2	Passed	2 piece(s) 2 x 4 DF No.2			
GLB48 DROPPED BM. BACK PORCH	Passed	1 piece(s) 3 1/2" x 7 1/2" 24F-V4 DF Glulam			
RJ28 PORCH RAFTER	Passed	1 piece(s) 2 x 8 DF No.2 @ 24" OC			
H68 HEADER GRID 2	Passed	1 piece(s) 6 x 8 DF No.2			
UPPER FLOOR			<u> </u>		
Member Name	Results	Current Solution	Comments		
B410LSL grid 2/3	Passed	1 piece(s) 3 1/2" x 9 1/2" 1.55E TimberStrand® LSL			
B410LSL BRIDGE BEAM	Passed	1 piece(s) 1 3/4" x 9 1/2" 1.55E TimberStrand® LSL			
B410LSL grid 1	Passed	1 piece(s) 3 1/2" x 9 1/2" 1.55E TimberStrand® LSL			

ForteWEB Software Operator	Job Notes
Zachary Blodget Structural Dept LLC (503) 334-1870 zac@structuraldept.com	



#### ROOF, RJ210 1 piece(s) 2 x 10 DF No.2 @ 24" OC



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	721 @ 1' 3.00"	3825 (6.00")	Passed (19%)		1.0 D + 1.0 S (Adj Spans)
Shear (lbs)	517 @ 2' 3.25"	1915	Passed (27%)	1.15	1.0 D + 1.0 S (Adj Spans)
Moment (Ft-lbs)	2000 @ 8' 0.00"	2334	Passed (86%)	1.15	1.0 D + 1.0 S (Alt Spans)
Live Load Defl. (in)	0.231 @ 8' 0.00"	0.675	Passed (L/701)		1.0 D + 1.0 S (Alt Spans)
Total Load Defl. (in)	0.412 @ 8'	0.900	Passed (L/393)		1.0 D + 1.0 S (Alt Spans)

System: Roof
Member Type: Joist
Building Use: Residential
Building Code: IBC 2018
Design Methodology: ASD
Member Pitch: 0/12

- Deflection criteria: LL (L/240) and TL (L/180).
- Overhang deflection criteria: LL (2L/240) and TL (2L/180).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- A 15% increase in the moment capacity has been added to account for repetitive member usage.
- Applicable calculations are based on NDS.

	E	Bearing Length			o Supports		
Supports	Total	Available	Required	Dead	Snow	Total	Accessories
1 - Stud wall - SPF	6.00"	6.00"	1.50"	320	401	721	Blocking
2 - Stud wall - SPF	6.00"	6.00"	1.50"	320	401	721	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	4' 8.00" o/c	
Bottom Edge (Lu)	16' 0.00" o/c	

<sup>•</sup>Maximum allowable bracing intervals based on applied load.

Vertical Load	Location (Side)	Spacing	Dead (0.90)	Snow (1.15)	Comments
1 - Uniform (PSF)	0 to 16' 0.00"	24"	20.0	25.0	25PSF SNOW

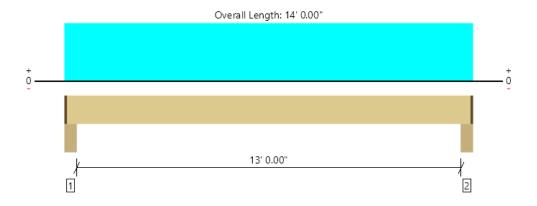
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#### ROOF, DBL 2X10 2 piece(s) 2 x 10 DF No.2



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	1152 @ 4.50"	8906 (4.75")	Passed (13%)		1.0 D + 1.0 S (All Spans)
Shear (lbs)	957 @ 1' 3.25"	3830	Passed (25%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	3666 @ 7' 0.00"	4059	Passed (90%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.219 @ 7'	0.663	Passed (L/726)		1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.366 @ 7' 0.00"	0.883	Passed (L/435)		1.0 D + 1.0 S (All Spans)

System : Roof Member Type : Flush Beam Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD Member Pitch : 0/12

- Deflection criteria: LL (L/240) and TL (L/180).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- Applicable calculations are based on NDS.

	Bearing Length			Loads t	o Supports		
Supports	Total	Available	Required	Dead	Snow	Total	Accessories
1 - Beam - DF	6.00"	4.75"	1.50"	468	700	1168	1 1/4" Rim Board
2 - Beam - DF	6.00"	4.75"	1.50"	468	700	1168	1 1/4" Rim Board

<sup>•</sup> Rim Board is assumed to carry all loads applied directly above it, bypassing the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	7' 8.00" o/c	
Bottom Edge (Lu)	13' 10.00" o/c	

<sup>•</sup>Maximum allowable bracing intervals based on applied load.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Snow (1.15)	Comments
0 - Self Weight (PLF)	1.25" to 13' 10.75"	N/A	7.0		
1 - Uniform (PSF)	0 to 14' 0.00" (Front)	4' 0.00"	15.0	25.0	Default Load

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#### ROOF, 2X10 STRUCT FASCIA GRID 1 1 piece(s) 2 x 10 DF No.2



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	851 @ 9' 0.00"	1406 (1.50")	Passed (60%)		1.0 D + 1.0 S (Alt Spans)
Shear (lbs)	678 @ 8' 2.75"	1915	Passed (35%)	1.15	1.0 D + 1.0 S (Alt Spans)
Moment (Ft-lbs)	1618 @ 5' 2.34"	2029	Passed (80%)	1.15	1.0 D + 1.0 S (Alt Spans)
Live Load Defl. (in)	0.068 @ 5' 1.68"	0.387	Passed (L/999+)		1.0 D + 1.0 S (Alt Spans)
Total Load Defl. (in)	0.110 @ 5' 1.76"	0.517	Passed (L/848)		1.0 D + 1.0 S (Alt Spans)

System : Roof Member Type : Flush Beam Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD Member Pitch : 0/12

- Deflection criteria: LL (L/240) and TL (L/180).
- Overhang deflection criteria: LL (2L/240) and TL (2L/180).
- · Allowed moment does not reflect the adjustment for the beam stability factor.
- Applicable calculations are based on NDS.

	Bearing Length			Loads t	o Supports		
Supports	Total	Available	Required	Dead	Snow	Total	Accessories
1 - Beam - DF	6.00"	6.00"	1.50"	449	719	1168	Blocking
2 - Hanger on 9 1/4" DF beam	6.00"	Hanger <sup>1</sup>	1.50"	366	595	961	See note 1

- Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.
- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- $\bullet\,\,^{\rm 1}$  See Connector grid below for additional information and/or requirements.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	6' 7.00" o/c	
Bottom Edge (Lu)	9' 0.00" o/c	

<sup>•</sup>Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie							
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories	
2 - Face Mount Hanger	LUS28	1.75"	N/A	6-10dx1.5	3-10d		

<sup>•</sup> Refer to manufacturer notes and instructions for proper installation and use of all connectors.

		T 11	Dead	Snow	
Vertical Loads	Location (Side)	Tributary Width	(0.90)	(1.15)	Comments
0 - Self Weight (PLF)	0 to 9' 0.00"	N/A	3.5		
1 - Uniform (PSF)	0 to 9' 6.00" (Top)	5' 6.00"	15.0	25.0	Default Load

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#### ROOF, H48 grid 1 1 piece(s) 4 x 8 DF No.2



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	536 @ 0	3281 (1.50")	Passed (16%)		1.0 D + 1.0 L (All Spans)
Shear (lbs)	400 @ 8.75"	3045	Passed (13%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	770 @ 2' 10.50"	2989	Passed (26%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.014 @ 2' 10.50"	0.192	Passed (L/999+)		1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.026 @ 2' 10.50"	0.287	Passed (L/999+)		1.0 D + 1.0 L (All Spans)

System: Wall Member Type: Header Building Use: Residential Building Code: IBC 2018 Design Methodology: ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- Applicable calculations are based on NDS.

	Bearing Length			Loads t	o Supports (		
Supports	Total	Available	Required	Dead	Floor Live	Total	Accessories
1 - Trimmer - DF	1.50"	1.50"	1.50"	248	288	536	None
2 - Trimmer - DF	1.50"	1.50"	1.50"	248	288	536	None

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	5' 9.00" o/c	
Bottom Edge (Lu)	5' 9.00" o/c	

<sup>•</sup>Maximum allowable bracing intervals based on applied load.

			Dead	Floor Live	
Vertical Loads	Location	Tributary Width	(0.90)	(1.00)	Comments
0 - Self Weight (PLF)	0 to 5' 9.00"	N/A	6.4		
1 - Uniform (PSF)	0 to 5' 9.00"	4' 0.00"	20.0	25.0	Default Load

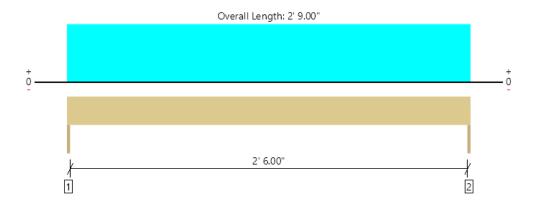
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#### ROOF, H24-2 2 piece(s) 2 x 4 DF No.2



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	313 @ 0	2813 (1.50")	Passed (11%)		1.0 D + 1.0 L (All Spans)
Shear (lbs)	218 @ 5.00"	1260	Passed (17%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	215 @ 1' 4.50"	689	Passed (31%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.009 @ 1' 4.50"	0.092	Passed (L/999+)		1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.017 @ 1' 4.50"	0.138	Passed (L/999+)		1.0 D + 1.0 L (All Spans)

System: Wall Member Type: Header Building Use: Residential Building Code: IBC 2018 Design Methodology: ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- Applicable calculations are based on NDS.

	Bearing Length			Loads to Supports (lbs)			
Supports	Total	Available	Required	Dead	Floor Live	Total	Accessories
1 - Trimmer - DF	1.50"	1.50"	1.50"	141	172	313	None
2 - Trimmer - DF	1.50"	1.50"	1.50"	141	172	313	None

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	2' 9.00" o/c	
Bottom Edge (Lu)	2' 9.00" o/c	

<sup>•</sup>Maximum allowable bracing intervals based on applied load.

			Dead	Floor Live	
Vertical Loads	Location	Tributary Width	(0.90)	(1.00)	Comments
0 - Self Weight (PLF)	0 to 2' 9.00"	N/A	2.7		
1 - Uniform (PSF)	0 to 2' 9.00"	5' 0.00"	20.0	25.0	Default Load

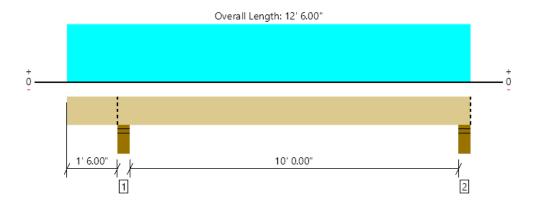
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#### ROOF, GLB48 DROPPED BM. BACK PORCH 1 piece(s) 3 1/2" x 7 1/2" 24F-V4 DF Glulam



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	2808 @ 1' 9.00"	11865 (6.00")	Passed (24%)		1.0 D + 1.0 S (All Spans)
Shear (lbs)	1768 @ 2' 7.50"	5333	Passed (33%)	1.15	1.0 D + 1.0 S (All Spans)
Pos Moment (Ft-lbs)	5146 @ 7' 0.35"	7547	Passed (68%)	1.15	1.0 D + 1.0 S (Alt Spans)
Neg Moment (Ft-lbs)	-607 @ 1' 9.00"	5817	Passed (10%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.341 @ 6' 11.56"	0.519	Passed (L/365)		1.0 D + 1.0 S (Alt Spans)
Total Load Defl. (in)	0.447 @ 6' 11.64"	0.692	Passed (L/279)		1.0 D + 1.0 S (Alt Spans)

System : Roof Member Type : Drop Beam Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD Member Pitch : 0/12

- Deflection criteria: LL (L/240) and TL (L/180).
- Overhang deflection criteria: LL (2L/240) and TL (2L/180).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- Critical positive moment adjusted by a volume factor of 1.00 that was calculated using length L = 10' 2.30".
- Critical negative moment adjusted by a volume factor of 1.00 that was calculated using length L = 2' 0.54".
- The effects of positive or negative camber have not been accounted for when calculating deflection.
- The specified glulam is assumed to have its strong laminations at the bottom of the beam. Install with proper side up as indicated by the manufacturer.
- Applicable calculations are based on NDS.

	Bearing Length			Loads to Supports (lbs)			
Supports	Total	Available	Required	Dead	Snow	Total	Accessories
1 - Stud wall - SYP	6.00"	6.00"	1.50"	683	2126	2809	Blocking
2 - Stud wall - DF	6.00"	6.00"	1.50"	522	1647	2169	Blocking

• Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	12' 6.00" o/c	
Bottom Edge (Lu)	12' 6.00" o/c	

Maximum allowable bracing intervals based on applied load.

			Dead	Snow	
Vertical Loads	Location (Side)	Tributary Width	(0.90)	(1.15)	Comments
0 - Self Weight (PLF)	0 to 12' 6.00"	N/A	6.4		
1 - Uniform (PSF)	0 to 12' 6.00" (Front)	6' 0.00"	15.0	50.0	Default Load

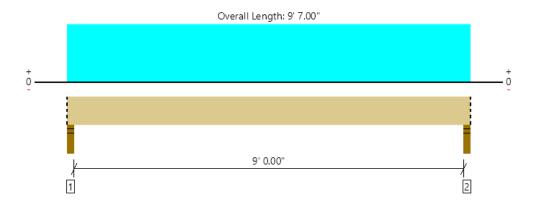
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#### ROOF, RJ28 PORCH RAFTER 1 piece(s) 2 x 8 DF No.2 @ 24" OC



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	527 @ 2.50"	2231 (3.50")	Passed (24%)		1.0 D + 1.0 S (All Spans)
Shear (lbs)	429 @ 10.75"	1501	Passed (29%)	1.15	1.0 D + 1.0 S (All Spans)
Moment (Ft-lbs)	1155 @ 4' 9.50"	1564	Passed (74%)	1.15	1.0 D + 1.0 S (All Spans)
Live Load Defl. (in)	0.167 @ 4' 9.50"	0.458	Passed (L/660)		1.0 D + 1.0 S (All Spans)
Total Load Defl. (in)	0.229 @ 4' 9.50"	0.611	Passed (L/480)		1.0 D + 1.0 S (All Spans)

System: Roof
Member Type: Joist
Building Use: Residential
Building Code: IBC 2018
Design Methodology: ASD
Member Pitch: 0/12

- Deflection criteria: LL (L/240) and TL (L/180).
- · Allowed moment does not reflect the adjustment for the beam stability factor.
- A 15% increase in the moment capacity has been added to account for repetitive member usage
- · Applicable calculations are based on NDS.

	Bearing Length			Loads to Supports (lbs)			
Supports	Total	Available	Required	Dead	Snow	Total	Accessories
1 - Stud wall - SPF	3.50"	3.50"	1.50"	144	383	527	Blocking
2 - Stud wall - SPF	3.50"	3.50"	1.50"	144	383	527	Blocking

<sup>•</sup> Blocking Panels are assumed to carry no loads applied directly above them and the full load is applied to the member being designed.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	8' 1.00" o/c	
Bottom Edge (Lu)	9' 7.00" o/c	

<sup>•</sup>Maximum allowable bracing intervals based on applied load.

Vertical Load	Location (Side)	Spacing	Dead (0.90)	Snow (1.15)	Comments
1 - Uniform (PSF)	0 to 9' 7.00"	24"	15.0	40.0	25PSF SNOW

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#### ROOF, H68 HEADER GRID 2 1 piece(s) 6 x 8 DF No.2



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	1528 @ 0	5156 (1.50")	Passed (30%)		1.0 D + 1.0 L (All Spans)
Shear (lbs)	1250 @ 9.00"	4675	Passed (27%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	3152 @ 4' 1.50"	3222	Passed (98%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.083 @ 4' 1.50"	0.275	Passed (L/999+)		1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.154 @ 4' 1.50"	0.412	Passed (L/644)		1.0 D + 1.0 L (All Spans)

System: Wall
Member Type: Header
Building Use: Residential
Building Code: IBC 2018
Design Methodology: ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- Applicable calculations are based on NDS.

	Bearing Length			Loads to Supports (lbs)			
Supports	Total	Available	Required	Dead	Floor Live	Total	Accessories
1 - Trimmer - DF	1.50"	1.50"	1.50"	703	825	1528	None
2 - Trimmer - DF	1.50"	1.50"	1.50"	703	825	1528	None

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	8' 3.00" o/c	
Bottom Edge (Lu)	8' 3.00" o/c	

<sup>•</sup>Maximum allowable bracing intervals based on applied load.

			Dead	Floor Live	
Vertical Loads	Location	Tributary Width	(0.90)	(1.00)	Comments
0 - Self Weight (PLF)	0 to 8' 3.00"	N/A	10.4		
1 - Uniform (PSF)	0 to 8' 3.00"	8' 0.00"	20.0	25.0	Default Load

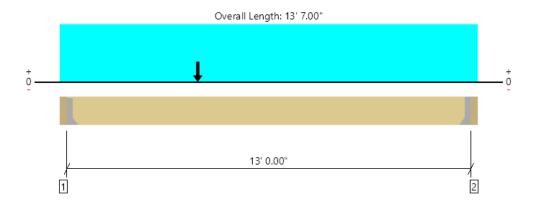
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### UPPER FLOOR, B410LSL grid 2/3 1 piece(s) 3 1/2" x 9 1/2" 1.55E TimberStrand® LSL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	2163 @ 3.50"	4725 (1.50")	Passed (46%)		1.0 D + 1.0 L (All Spans)
Shear (lbs)	1969 @ 1' 1.00"	6872	Passed (29%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	7104 @ 5' 8.01"	10422	Passed (68%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.429 @ 6' 7.70"	0.433	Passed (L/364)		1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.582 @ 6' 7.71"	0.650	Passed (L/268)		1.0 D + 1.0 L (All Spans)

System : Floor Member Type : Flush Beam Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.

	Bearing Length			Loads to Supports (lbs)			
Supports	Total	Available	Required	Dead	Floor Live	Total	Accessories
1 - Hanger on 9 1/2" SPF beam	3.50"	Hanger <sup>1</sup>	1.50"	583	1648	2231	See note 1
2 - Hanger on 9 1/2" SPF beam	3.50"	Hanger <sup>1</sup>	1.50"	506	1426	1932	See note 1

- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- ¹ See Connector grid below for additional information and/or requirements.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	13' 0.00" o/c	
Bottom Edge (Lu)	13' 0.00" o/c	

<sup>•</sup>Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie									
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories			
1 - Face Mount Hanger	HHUS48	3.00"	N/A	22-10d	8-10d				
2 - Face Mount Hanger	HHUS48	3.00"	N/A	22-10d	8-10d				

Refer to manufacturer notes and instructions for proper installation and use of all connectors.

Vertical Loads	Location (Side)	Tributary Width	Dead (0.90)	Floor Live (1.00)	Comments
0 - Self Weight (PLF)	3.50" to 13' 3.50"	N/A	10.4		
1 - Uniform (PSF)	0 to 13' 7.00" (Front)	4' 6.00"	12.0	40.0	Default Load
2 - Point (lb)	4' 6.00" (Front)	N/A	220	629	Linked from: B410LSL BRIDGE BEAM, Support 1

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The product application, input design loads, dimensions and support information have been provided by ZJB

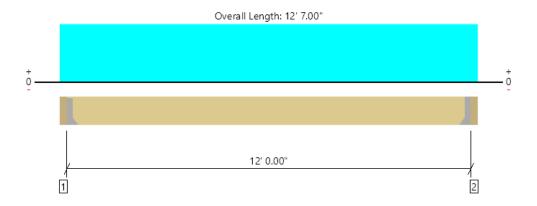
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File Name: 20020 - INKBUILT - SECREST SARVER REV02

### UPPER FLOOR, B410LSL BRIDGE BEAM 1 piece(s) 1 3/4" x 9 1/2" 1.55E TimberStrand® LSL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	811 @ 3.50"	2363 (1.50")	Passed (34%)		1.0 D + 1.0 L (All Spans)
Shear (lbs)	704 @ 1' 1.00"	3436	Passed (20%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	2433 @ 6' 3.50"	5211	Passed (47%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.257 @ 6' 3.50"	0.300	Passed (L/561)		1.0 D + 1.0 L (All Spans)
Total Load Defl. (in)	0.347 @ 6' 3.50"	0.600	Passed (L/415)		1.0 D + 1.0 L (All Spans)

System : Floor Member Type : Flush Beam Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD

- Deflection criteria: LL (L/480) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.

	Bearing Length			Loads to Supports (lbs)			
Supports	Total	Available	Required	Dead	Floor Live	Total	Accessories
1 - Hanger on 9 1/2" SPF beam	3.50"	Hanger <sup>1</sup>	1.50"	220	629	849	See note 1
2 - Hanger on 9 1/2" SPF beam	3.50"	Hanger <sup>1</sup>	1.50"	220	629	849	See note 1

- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- ¹ See Connector grid below for additional information and/or requirements.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	11' 11.00" o/c	
Bottom Edge (Lu)	12' 0.00" o/c	

<sup>•</sup>Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie									
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories			
1 - Face Mount Hanger	IUS1.81/9.5	2.00"	N/A	8-10d	2-10dx1.5				
2 - Face Mount Hanger	IUS1.81/9.5	2.00"	N/A	8-10d	2-10dx1.5				

Refer to manufacturer notes and instructions for proper installation and use of all connectors.

			Dead	Floor Live	
Vertical Loads	Location (Side)	Tributary Width	(0.90)	(1.00)	Comments
0 - Self Weight (PLF)	3.50" to 12' 3.50"	N/A	5.2		
1 - Uniform (PSF)	0 to 12' 7.00" (Front)	2' 6.00"	12.0	40.0	Default Load

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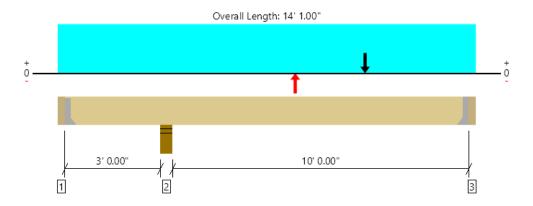
The product application, input design loads, dimensions and support information have been provided by ZJB

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zac@structuraldept.com		W



File Name: 20020 - INKBUILT - SECREST SARVER REV02

# UPPER FLOOR, B410LSL grid 1 1 piece(s) 3 1/2" x 9 1/2" 1.55E TimberStrand® LSL



All locations are measured from the outside face of left support (or left cantilever end). All dimensions are horizontal.

Design Results	Actual @ Location	Allowed	Result	LDF	Load: Combination (Pattern)
Member Reaction (lbs)	3237 @ 3' 6.50"	8925 (6.00")	Passed (36%)		1.0 D + 1.0 L (All Spans)
Shear (lbs)	1508 @ 4' 7.00"	6872	Passed (22%)	1.00	1.0 D + 1.0 L (All Spans)
Moment (Ft-lbs)	-3050 @ 3' 6.50"	10422	Passed (29%)	1.00	1.0 D + 1.0 L (All Spans)
Live Load Defl. (in)	0.091 @ 9' 2.12"	0.342	Passed (L/999+)		1.0 D + 1.0 L (Alt Spans)
Total Load Defl. (in)	0.121 @ 9' 2.20"	0.512	Passed (L/999+)		1.0 D + 1.0 L (Alt Spans)

System : Floor Member Type : Flush Beam Building Use : Residential Building Code : IBC 2018 Design Methodology : ASD

- Deflection criteria: LL (L/360) and TL (L/240).
- Allowed moment does not reflect the adjustment for the beam stability factor.
- -709 lbs uplift at support located at 3.50". Strapping or other restraint may be required.

	В	Bearing Length			oads to Sup			
Supports	Total	Available	Required	Dead	Floor Live	Wind	Total	Accessories
1 - Hanger on 9 1/2" SPF beam	3.50"	Hanger <sup>1</sup>	1.50"	-98	400/-611	205	605/- 709	See note 1
2 - Stud wall - SPF	6.00"	6.00"	2.18"	834	2402	-726	3236/- 726	None
3 - Hanger on 9 1/2" SPF beam	3.50"	Hanger <sup>1</sup>	1.50"	334	978	520	1832	See note 1

- At hanger supports, the Total Bearing dimension is equal to the width of the material that is supporting the hanger
- ¹ See Connector grid below for additional information and/or requirements.

Lateral Bracing	Bracing Intervals	Comments
Top Edge (Lu)	13' 6.00" o/c	
Bottom Edge (Lu)	13' 6.00" o/c	

<sup>•</sup>Maximum allowable bracing intervals based on applied load.

Connector: Simpson Strong-Tie										
Support	Model	Seat Length	Top Fasteners	Face Fasteners	Member Fasteners	Accessories				
1 - Face Mount Hanger	LUS48	2.00"	N/A	6-16d	4-16d					
3 - Face Mount Hanger	LUS410	2.00"	N/A	8-10d	6-10d					

<sup>•</sup> Refer to manufacturer notes and instructions for proper installation and use of all connectors.

			Dead	Floor Live	Wind	
Vertical Loads	Location (Side)	Tributary Width	(0.90)	(1.00)	(1.60)	Comments
0 - Self Weight (PLF)	3.50" to 13' 9.50"	N/A	10.4			
1 - Uniform (PSF)	0 to 14' 1.00" (Front)	5' 6.00"	12.0	40.0	-	Default Load
2 - Point (lb)	8' 0.00" (Front)	N/A	-	-	-2000	HOLDOWN STRAP
3 - Point (lb)	10' 4.01" (Front)	N/A	-	-	2000	HOLDOWN STRAP

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#### SECREST SARVER REV02 | 05/25/21 | PG. 16 / 19\_

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COMPANY **PROJECT** 

May 25, 2021 14:49:24

4 2X8 COL 20FT.wwc

#### **Design Check Calculation Sheet**

Sizer 2004

#### LOADS: (lbs, psf, or plf)

Load	Туре	Distribution	Magnitude	Location [ft]	Pattern
			Start End	Start End	Load?
M	Wind	Full Area	20.00 (5.00)*		No

<sup>\*</sup>Tributary Width (ft)

#### MAXIMUM REACTIONS (lbs):

0' 20' Dead 1000 1000 Live 1000 1000 Total

#### Lumber n-ply, D.Fir-L, No.2, 2x8", 4-Plys

Self Weight of 10.33 plf automatically included in loads;

Pinned base; Loadface = width(b); Built-up fastener: nails; Ke x Lb: 1.00 x 0.00= 0.00 [ft]; Ke x Ld: 1.00 x 20.00= 20.00 [ft]; Lateral support: top = Lb, bottom = Lb; Load combinations: ICC-IBC;

WARNING: Member length exceeds typical stock length of 18.0 [ft]

#### SECTION vs. DESIGN CODE NDS-2001: ( stress=psi, and in )

Criterion	Analysis Value	Design Value	Analysis/Design
Shear	fv = 34	Fv' = 288	fv/Fv' = 0.12
Bending(+)	fb = 1141	Fb' = 1728	fb/Fb' = 0.66
Axial	fc = 5	Fc' = 402	fc/Fc' = 0.01
Axial Bearing	fc = 5	Fc* = 1276	fc/Fc* = 0.00
Combined (axia)	compression + s	de load bending)	Eq.3.9-3 = 0.67
Live Defl'n	1.18 = L/203	1.33 = L/180	0.89
Total Defl'n	1.18 = L/203	1.33 = L/180	0.89

#### ADDITIONAL DATA:

FACTORS	: F	CD	CM	Ct	CL/CP	CF	Cfu	Cr	Cfrt	Ci	LC#
Fb'+	900	1.60	1.00	1.00	1.000	1.200	1.00	1.00	1.00	1.00	2
Fv'	180	1.60	1.00	1.00	-	-	-	-	1.00	1.00	2
Fcp'	625	-	1.00	1.00	_	-	-	-	1.00	1.00	_
Fc'	1350	0.90	1.00	1.00	0.315	1.050	-	-	1.00	1.00	1
Fc'comb	1350	1.60	-	-	0.185	-	-	-	-	-	2
E'	1.6	million	1.00	1.00	-	-	-	-	1.00	1.00	2
Fc*	1350	0.90	1.00	1.00	-	1.050	-	-	1.00	1.00	1

Bending(+): LC# 2 = .6D+W, M = 5000 lbs-ft

Shear : LC# 2 = .6D+W, V = 1000, V design = Deflection: LC# 2 = .6D+W EI= 76.21e06 lb-in2/ply 1000 lbs

Total Deflection = 1.50(Dead Load Deflection) + Live Load Deflection.

Axial : LC# 1 = D only, P = 207 lbs Kf = 1.00

Combined : LC# 2 = .6D+W; (1 - fc/FcE) = 0.99

(D=dead L=live S=snow W=wind I=impact C=construction CLd=concentrated)

(All LC's are listed in the Analysis output)

#### **DESIGN NOTES:**

- 1. Please verify that the default deflection limits are appropriate for your application.
- 2. BUILT-UP COLUMNS: nailed or bolted built-up columns shall conform to the provisions of NDS Clause 15.3.
- 3. SCL Columns (Structural Composite Lumber): the attached SCL selection is for preliminary design only. For final member design contact your local SCL manufacturer.

# SECREST-SARVER RESIDENCE

4131 SE GRANT ST| PORTLAND, OR

LATERAL CALCULATIONS

		1	Loidown	-1.9 NOT REQ'D	Zi	-7.5 NOT REQ'D	-7.7 NOT REQ'D	C28	В	0.0 NOT REQ'D	2	0.0 NOT REQ'D	Zi	C40	2	C28	
		-	O L	NOT	0.8 DTT2Z	NOT	NOT	0.8 MSTC28	6.9 SWSB	NOT	2.4 HDU2	NOT	0.9 DTT2Z	1.9 MSTC40	2.6 HDU2	0.3 MSTC28	l
		SUM T/C	( <del>X</del>	-1.9	8.0	-7.5	-7.7	8.0	6.9	0.0	2.4	0.0	0.9	1.9	2.6	0.3	
		Sum Mo	(ft-k)	11.3	31.1	11.3	31.2	4.7	14.4	0.0	6.9	0.0	7.6	7.6	14.9	2.4	I
	n grid lin	ī	sum DL	2.4	3.1	7.8	9.8	0.2	0.3	0.0	0.2	0.0	0.5	0.3	0.5	0.4	
	est) pier o	.6dl rf./flr	12 psf(k)	1.5	0.2	2.0	0.2	0.1	0.0	0.0	0.1	0.0	0.1	0.1	0.1	0.1	
	ortest(light	rf/fir trib 60	(ft)	8.5	2.0	8.5	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	2.0	
	L's based on shortest(lightest) pier on grid line	6dl Wall Pier rf,	(k) 12psf	6.0	0.5	2.8	9.0	0.1	0.1	0.0	0.2	0.0	0.4	0.2	0.2	0.3	
. DESINGS	s,Tc		) (K)	0.5	2.5	0.3	0.5	1.0	4.9	0.0	2.6	0.0	1.4	2.2	3.1	9.0	
SECREST SARVER SHEAR WALL DESINGS		Mo (ft.	⊋	11.3	19.8	11.3	19.9	4.7	6.7	0.0	6.9	0.0	7.6	9.7	14.9	2.4	
ARVER SHI		MS	Design	9MS	310 SW4	9MS (	9MS	146 SW6	608 SWSB	9WS 0	324 SW4	9MS 0	154 SW6	242 SW4	343 SW4	9MS 89	
EST SA		Vpiers	(plf)	47	310	29	67	146	309	0	357	)	157	242	343	39	
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	5 R(		1	12.0	8.0	39.0	28.0	2.3	2.0	0.0	2.7	0.0	5.5	3.5	2.5	4.0	
	Pwind (psf)	11111111111	vtotal (lbs)	1125	2480	1130	2485	675	1215	0	864	0	846	846	1656	270	
		PLATE	눞	10	8	10	8	7	8	6	8	6	6	6	6	6	İ
N		Atrib	roof	0	1	1	1	0	0	0	0	0	0	0	0	0	ĺ
3NS - WI	9 WALLS	Atrib	walls	125	150	125	150	75	09	0	96	0	94	94	06	30	ĺ
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S	WALL DESIGNS - SEISMIC	MIC	_																				
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Trib		Bldg.	PLATE	\(\rac{1}{2}\)		LE	Length of Wall		Piers (ft.				Vpiers	rs SW	/ Mo (ft	ft/ ft.	.6dl Wall Pier	rf/flr trib	.6dl rf./flr	2	Sum Mo	SUM T/C	מיייטקוסו
width		Width	보	V LOLAII (IIDS)	1	2	3	4	2	9	3 /	8 SUM	M (plf)	) Design	gn k)	- - -	(k) 12psf	(£)	12 psf(k)	Sull DL	(ft-k)	( <u>k</u>	IMODIOL
6		18	10	1155	12.0	12.0		-	-	-	1	24.0		48 SW6	11.6		0.5 0.9	8.5	1.5	2.4	11.6		-1.9 NOT REQ'D
6		18	8	1855	8.0	,	-		-	:	1	∞	8.0 23	232 SW6	14.8		1.9 0.5	2.0	0.2	3.1	26.4		0.2 DTT2Z
6		18	10	1155	39.0	,	-				:	39.0		30 SW6	11.6		0.3 2.8	8.5	5.0	7.8	11.6	-7.5	NOT REQ'D
6		18	8	1855	28.0	- 0.6	-			-	:	37.0		50 SW6	14.8		0.4 0.6	2.0	0.2	8.6	26.4		-7.9 NOT REQ'D
15	-	44	7	788	2.3	2.3					:	4	4.6 17	170 SW6	5.	5.5	1.2 0.1	2.0	0.1	0.2	5.5		1.0 MSTC28
9		44	8	826	2.0		-		-	-	:	2	2.0 48	489 SWSB		7.8 3.	3.9 0.1	2.0	0.0	0.3	13.3		6.3 SWSB
0	0	44	6	0	0.0		-			-	:	0	0.0	9WS 0	0	0.0	0.0 0.0	2.0	0.0	0.0	0.0		0.0 NOT REQ'D
1	10	44	8	318	2.7				-		:	2	2.7 11	119 SW6		2.5	1.0 0.2	2.0	0.1	0.2	2.5		0.7 DTT2Z
_	0	44	6	0	0.0		-				:	0	0.0	9WS 0	0.	0.0	0.0 0.0	2.0	0.0	0.0	0.0		0.0 NOT REQ'D
1	14	44	6	445	- 5.5		-			-	:	5	5.5	81 SW6	4.	4.0	0.7 0.4	2.0	0.1	0.5	4.0		0.2 DTT2Z
_	19	44	6	866	3.5						:	3	3.5 28	285 SW4	9.	9.0	2.6 0.2	2.0	0.1	0.3	9.0		2.3 MSTC40
	6	44	6	1284	2.5	2.3					:	4	4.8 26	266 SW4	11.6		2.4 0.2	2.0	0.1	0.5	11.6		1.9 DTT2Z
		44	6	368	4.0						:	4	4.0	92 SW6	3.3		0.8 0.3	2.0	0.1	0.4	3.3		0.5 MSTC28
	7	44	6	290	1.0						1	1	1.0 59	590 SWSB	3 5.3		5.3 0.1	0.0	0.0	0.4	8 6		8.2 SWSB