7300 SW OLESON RD. DFS-01 C003-102648

Co. 03. 102648 DFS. 01.

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MICROFILMED

9

MCB

Tecna Industries

Roof and Floor Truss Manufacturing

Tecna Industries, Inc.
2003 Colvin Court
McMinnville, OR 97128
Email: Tecna@onlinemac.com
Telephone: 503 472-8880
Fax: 503 472-2790

Fax

To:	JHC Commercial LLC	From:	Dwane Carver
Fax:	503-684-~~[illegible]~~ 5295	Pages:	12
Phone:	503-624-7100	Date:	7/22/2005
Re:	Engineering & Layout for Olesen Apts.	CC:	

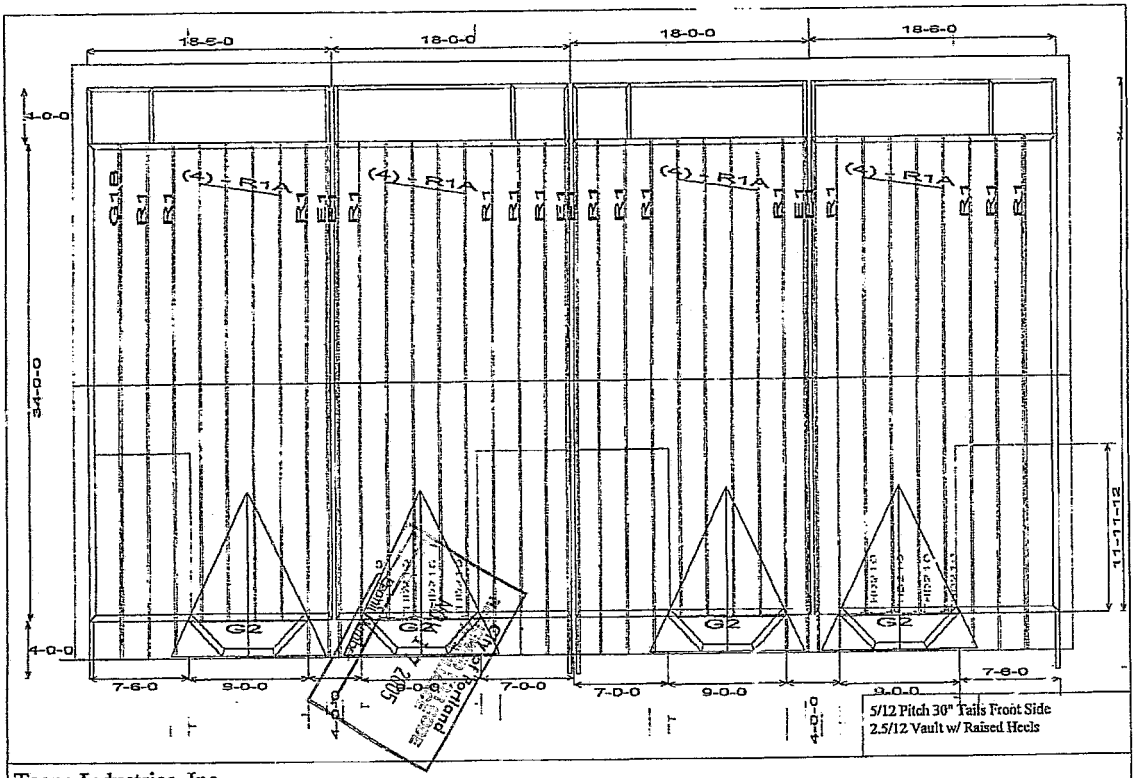
☐ Urgent ☐ For Review ☐ Please Comment ☐ Please Reply ☐ Please Recycle

• Comments:

City of Portland
Revised Site Plan
NOV 17 2005
Item Number

9

03-102648-DFS-01-EW



Tecna Industries, Inc.

2083 Colvin Court
McMinnville
OR 97128
tcc.o@mlinemac.com
Telephone: 503-472-8880
Fax: 503-472-2790

Name: JHC Commercial, LLC
Address: Garden Hom-Summit Apt.
7410 SW Olcson Rd
Portland
OR 97223
Telephone:

Cameron P Dutz Architect-8-uni

Job: 6880

Scale: 1 : 96 | Date: 1/5/2005 | Drawn By: dw

MiTek®

MiTek Industries, Inc.
7777 Greenback Lane
Suite 109
Citrus Heights, CA, 95610
Telephone 916/676-1900
Fax 916/676-1909

Re: 6880
Cameron P Dutz Arch.- 8 unit Apt.

The truss drawing(s) referenced below have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by Tecna Industries.

Pages or sheets covered by this seal: R14792457 thru R14792462

My license renewal date for the state of Oregon is December 31, 2005.

REGISTERED PROFESSIONAL ENGINEER
66808PE
OREGON
NOVEMBER 13, 2001
ROBERT C. ANDERSON

EXPIRATION DATE: 12-31-05

City of Portland
NOV 17 2005
Permit Number

January 6,2005

Anderson, Bob

The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI-1995 Sec. 2.

Job	Truss	Truss Type	Qty	Ply	Cameron P Dutz Arch - 3 Unit Apt.	
6669	E1	COMMON	5	1	Job Reference (optional)	R1479245T

TECHA INDUSTRIES, McMINNVILLE, OR 97128, Dean Wagner — 6.000 s Jun 17 2004 MiTek Industries, Inc. Wed Jun 05 16:37:22 2005 Page 1

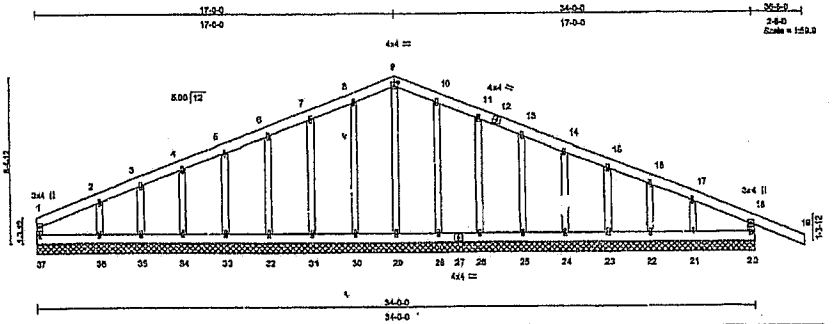


Plate Offsets (X,Y): [8:0-2-0,0-2-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/def	L/d	PLATES	GRIP
TCLL	25.0	Plates Increase	1.15	TC	0.45	Vert(LL)	-0.06	12	n/r	120	MT20	220/195
TCDL	7.0	Lumber Increase	1.15	BC	0.03	Vert(TL)	-0.07	19	n/r	120		
BCLL	0.0	Rep Stress Incr	YES	WB	0.17	Horz(TL)	0.00	20	n/a	n/a		
BCDL	10.0	Code IBC2003/TPI2002		(Matrix)							Weight: 251 lb	

LUMBER
TOP CHORD 2 X 6 DF SS G
BOT CHORD 2 X 6 DF SS G
WEBS 2 X 4 DF Std G
OTHERS 2 X 4 DF Std G

BRACING
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) 37=94/34-0-0, 20=363/34-0-0, 29=170/34-0-0, 30=170/34-0-0, 31=168/34-0-0, 32=168/34-0-0, 33=167/34-0-0, 34=172/34-0-0, 35=145/34-0-0, 36=247/34-0-0, 28=164/34-0-0, 26=162/34-0-0, 25=168/34-0-0, 24=168/34-0-0, 23=166/34-0-0, 22=165/34-0-0, 21=112/34-0-0
Max Horz 37=91(load case 7)
Max Uplift 37=-21(load case 6), 20=-107(load case 8), 30=-28(load case 7), 31=-34(load case 7), 32=-40(load case 7), 33=-48(load case 7), 34=-52(load case 7), 35=-27(load case 7), 36=-115(load case 7), 28=-30(load case 8), 26=-55(load case 8), 25=-49(load case 8), 24=-48(load case 8), 23=-50(load case 8), 22=-47(load case 8), 21=-109(load case 8)
Max Grav 37=165(load case 2), 20=623(load case 3), 29=170(load case 1), 30=234(load case 2), 31=239(load case 2), 32=239(load case 2), 33=234(load case 2), 34=239(load case 2), 35=205(load case 2), 36=328(load case 2), 28=235(load case 3), 26=231(load case 3), 25=236(load case 3), 24=234(load case 3), 23=232(load case 3), 22=231(load case 3), 21=161(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-75/78, 2-3=-67/68, 3-4=-59/60, 4-5=-50/54, 5-6=-50/59, 6-7=-60/115, 7-8=-82/132, 8-9=-61/141, 9-10=-20/141, 10-11=-27/132, 11-12=-22/115, 12-13=-28/112, 13-14=-28/99, 14-15=-28/84, 15-16=-28/68, 16-17=-34/55, 17-18=-62/37, 18-19=0/0, 1-37=-132/75, 19-20=-409/129
BOT CHORD 36-37=0/0, 35-36=0/0, 34-35=0/0, 33-34=0/0, 32-33=0/0, 31-32=0/0, 30-31=0/0, 29-30=0/0, 28-29=0/0, 27-28=0/0, 26-27=-58/124, 25-26=-58/124, 24-25=-58/124, 23-24=-58/124, 22-23=-58/124, 21-22=-58/124, 20-21=-58/124
WEBS 9-29=-129/0, 8-30=-194/37, 7-31=-198/82, 6-32=-194/57, 5-33=-195/50, 4-34=-197/58, 3-35=-178/45, 2-36=-269/100, 10-28=-201/37, 11-26=-197/82, 13-25=-194/57, 14-24=-195/50, 16-23=-192/50, 16-22=-221/63, 17-21=-109/147

NOTES
1) Wind: ASCE 7-98; 90mph; h=20ft; TCDL=4.0psf; BCDL=2.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left exposed; porch left and right exposed; Lumber DOL=1.15 plate grip DOL=1.15.
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see MiTek "Standard Gable End Detail"
3) TCLL: ASCE 7-98; Pf=25.0 psf (roof snow); Exp B; Fully Exp.
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.

Continued on page 2

City of Portland
RECEIVED
NOV 17 2005
Permit Number

REGISTERED PROFESSIONAL ENGINEER
56808PE
OREGON
NOVEMBER 13, 2001
ROBERT C. ANDERSON

EXPIRATION DATE: 12-31-05
January 6,2005

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MiTEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

3777 Greenback Lane
Suite 180
Citrus Heights, CA, 95610
MiTek®

Job	Truss	Truss Type	Qty	Ply	Cameron P Dietz Arch.- 8 unit Apt. R14782457
8050	E1	COMMON	6	1	Job Reference (optional)

TECNA INDUSTRIES, McMINNVILLE, OR 97128, Dean Wagner 6.000 s Jun 17 2004 MiTek Industries, Inc. Wed Jan 05 16:37:23 2005 Page 2

NOTES

6) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
7) All plates are 1.5x4 MT20 unless otherwise indicated.
8) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
9) Gable requires continuous bottom chord bearing.
10) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
11) Gable studs spaced at 2-0-0 oc.
12) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
13) A plate rating reduction of 20% has been applied for the green lumber members.
14) This truss is designed in accordance with the 2003 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

City of Portland
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NOV 17 2005
Permit Number

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MiTek REFERENCE PAGE MT-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Application of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

7777 Greenback Lane
Suite 109
Citrus Heights, CA, 95610
MiTek®

Job	Truss	Truss Type	Qty	Ply	Cameron P Dutz Arch.- 8 unit Apt.
5660	G1B	ROOF TRUSS	2	2	R147924EB

Job Reference (optional)

TECNA INDUS IES, McMINNVILLE, OR 97128, Dean Wagner

8.500 s Jun 17 2004 MiTek Industries, Inc. Wed Jan 05 16:37:24 2005 Page 1

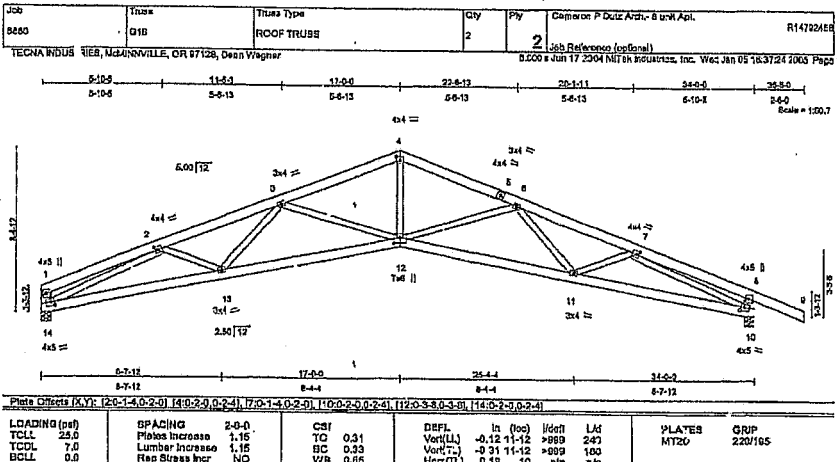

Plate Offsets (X,Y): [2:0-1-4,0-2-0] [4:0-2-0,0-2-4] [7:0-1-4,0-2-0] [10:0-2-0,0-2-4] [12:0-3-8,0-3-8] [14:0-2-0,0-2-4]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL.	in (loc)	l/def	L/d	PLATES	GRIP
TCLL	25.0	Plates Increase	1.15	TC	0.31	Vert(LL)	-0.12 11-12	>999	240	MT20	220/195
TCDL	7.0	Lumber Increase	1.15	BC	0.33	Vert(TL)	-0.31 11-12	>999	180		
BCLL	0.0	Rep Stress Incr	NO	WB	0.66	Horz(TL)	0.19 10	n/a	n/a		
BCDL	10.0	Code IBC2003/TPI2002		(Matrix)						Weight: 447 lb	

LUMBER
TOP CHORD 2 X 6 DF SS G
BOT CHORD 2 X 6 DF SS G
WEBS 2 X 4 DF Std G *Except*
1-14 2 X 6 DF SS-G, 8-10 2 X 6 DF SS-G

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 14=1751/0-5-8, 10=1088/0-5-8
Max Horz14=-158(load case 8)
Max Uplift14=-561(load case 7), 10=-787(load case 8)
Max Grav14=2020(load case 2), 10=2395(load case 8)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1817/550, 2-3=-4826/1666, 3-4=-3732/1295, 4-5=-3547/1314, 5-6=-3732/1295, 6-7=-4469/1518, 7-8=-1161/425, 8-9=0/150, 1-14=-671/324, 9-10=-1209/395
BOT CHORD 13-14=-1476/4233, 12-13=-1408/4235, 11-12=-1180/4114, 10-11=-1167/4038
WEBS 2-13=-109/431, 3-13=-165/217, 3-12=-1136/395, 4-12=-891/2369, 5-12=-1026/397, 6-11=-160/206, 7-11=-136/449

NOTES
1) 2-ply truss to be connected together with 10d Common(.148"x3") Nails as follows:
Top chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
Bottom chords connected as follows: 2 X 6 - 2 rows at 0-9-0 oc.
Webs connected as follows: 2 X 4 - 1 row at 0-9-0 oc.
2) All loads are considered equally applied to all plies, except if noted as front (F) or back (B) face in the LOAD CASE(S) section. Ply to ply connections have been provided to distribute only loads noted as (F) or (B), unless otherwise indicated.
3) Wind: ASCE 7-98; 90mph; h=20ft; TCDL=4.0psf; BCDL=2.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left exposed; porch left and right exposed; Lumber DOL=1.15 plate grip DOL=1.15.
4) TCLL: ASCE 7-98; Pf=25.0 psf (roof snow); Exp B; Fully Exp.
5) Unbalanced snow loads have been considered for this design.
6) This truss has been designed for 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
7) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
8) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
9) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
10) A plate rating reduction of 20% has been applied for the green lumber members
11) Bearing at joint(s) 14, 10 considers parallel to grain value using ANSI/TPI 1-1995 angle to grain formula. Building designer should verify capacity of bearing surface.

City of Portland
NOV 17 2005
Permit Number

REGISTERED PROFESSIONAL ENGINEER
66808PE
OREGON
NOVEMBER 13, 2001
ROBERT C. ANDERSON

EXPIRATION DATE: 12-31-05
January 6,2005

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MiTEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

MiTek Industries, Inc.
7777 Greenback Lane
Suite 109
Citrus Heights, CA, 95610
MiTek®

Job	Truss	Truss Type	Qty	Ply	Cameron P Butz Arch - 8 unit Apt. R14702456
8580	G1B	ROOF TRUSS	2	2	Job Reference (optional)

TECRA INDUSTRIES, McMINNVILLE, OR 97128, Dean Wagner 8.000 s Jun 17 2004 MiTek Industries, Inc. Wed Jan 05 16:37:21 2005 Page 2

NOTES

12) This truss is designed in accordance with the 2003 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

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7777 Greenback Lane
Suite 109
Citrus Heights, CA, 95610

MiTek

Job	Truss	Truss Type	Qty	Ply	Cameron P Dutz Arch.- 8 unit Apt. R1-792459
6650	G2	COMMON	4	1	Job Reference (optional)

TECNA INDUSTRIES, NEWBERGVILLE, OR 97128, Dean Wagner 8.003 s Jun 17 2004 MiTek Industries, Inc. Wed Jan 05 10:37:28 2005 Page 1

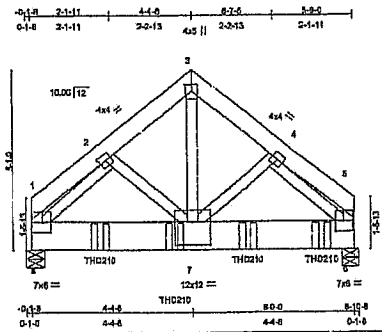

Plate Offsets (X,Y): [3:0-2-4,0-2-0], [5:0-2-12,0-3-8], [7:0-4-0,0-8-0], [8:0-2-12,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plates Increase	1.15	TC	0.13	Vert(LL)	-0.02	6-7	>999	240	MT20	220/195
TCDL	7.0	Lumber Increase	1.15	BC	0.41	Vert(TL)	-0.05	6-7	>999	180		
BCLL	0.0	Rep Stress Incr	NO	WB	0.39	Horz(TL)	0.01	6	n/a	n/a		
BCDL	10.0	Code	IBC2003/TPI2002	(Matrix)							Weight: 70 lb	

LUMBER
TOP CHORD 2 X 6 DF SS G
BOT CHORD 2 X 10 DF SS G
WEBS 2 X 4 DF Std G *Except*
3-7 2 X 4 DF No.1&Btr-G

BRACING
TOP CHORD Structural wood sheathing directly applied or 5-7-12 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS (lb/size) 8=3157/0-5-8, 6=3157/0-5-8
Max Horz 8=82(load case 6)
Max Uplift 8=-852(load case 8), 6=-852(load case 7)
Max Grav 8=3210(load case 2), 6=3210(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-89/44, 2-3=-2232/709, 3-4=-2232/709, 4-5=-89/44, 1-8=-156/57, 5-6=-156/56
BOT CHORD 7-8=-438/1389, 6-7=-400/1369
WEBS 2-7=-207/588, 3-7=-898/2878, 4-7=-208/588, 2-8=-2281/666, 4-6=-2281/666

NOTES
1) Wind: ASCE 7-98; 90mph h=20ft; TCDL=4.0psf; BCDL=2.0psf; Category II; Exp B; enclosed; MWFRS gable and zone; cantilever left and right exposed ; end vertical left exposed; porch left and right exposed; Lumber DOL=1.15 plate grip DOL=1.15.
2) TCLL: ASCE 7-98; Pf=25.0 psf (roof snow); Exp B; Fully Exp.
3) Unbalanced snow loads have been considered for this design.
4) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
5) This truss requires plate inspection per the Truss Count Method when this truss is chosen for quality assurance inspection.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) A plate rating reduction of 20% has been applied for the green lumber members.
8) This truss is designed in accordance with the 2003 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
9) Girder carries tie-in span(s): 24-0-0 from 0-0-0 to 8-0-0
10) Use USP THD210 (With 16d nails into Girder & NAILS nails into Truss) or equivalent spaced at 2-0-0 oc max. starting at 2-0-0 from the left end to 8-0-0 to connect truss(es) to (1 ply 2 X 6 DF) to front face of bottom chord.
11) Fill all nail holes where hanger is in contact with lumber.
12) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

Continued on page 2

City of Portland
Building Codes Division
NOV 17 2005
Permit Number

REGISTERED PROFESSIONAL ENGINEER
56808PE
OREGON
NOVEMBER 13, 2001
ROBERT C. ANDERSON

EXPIRATION DATE: 12-31-05
January 6,2005

WARNING - Verify design parameters and READ NOTES ON THIS AND INDIVIDUAL TRUSS DESIGNS PRIOR TO PLATE INSTALLATION. Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Application of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

7777 Greenback Lane
Suite 109
Citrus Heights, CA, 95610
MiTek®

Job	Truss	Truss Type	Qty	Ply	Cameron P Dutz Arch. 8 unit Apt.
6880	G2	COMMON	4	1	R1482462
					Job Reference (optional)

TECNA INDUSTRIES, MCMINNVILLE, OR 97128, Dean Wagner 8.000 s Jun 17 2004 MiTek Industries, Inc. Wed Jan 05 16:37:26 2005 Page 2

LOAD CASE(S) Standard
1) Snow: Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)
Vert: 8-8=-882(F=-662), 1-3=-54, 3-5=-54

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Suite 109
Citrus Heights, CA 95610

MiTek®

Job	Truss	Truss Type	Qty	Ply	Cameron P Dutz Akin, 8 Unit Apt.	
6060	R1	ROOF TRUSS	4	1	Job Reference (optional)	R1479240C

TECNA INDUSTRIES, McMINNVILLE, OR 97128, Dean Wagner

8.000 s Jun 17 2004 MiTek Industries, Inc. Wed Jun 03 16:37:28 2005 Page 1

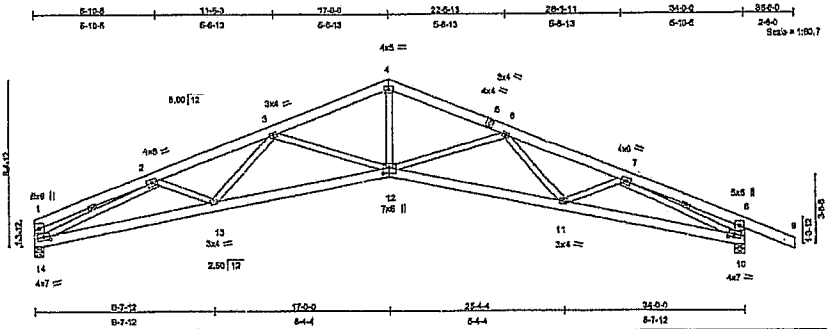


Plate Offsets (X,Y): [10:0-3-4,0-2-8], [12:0-3-8,0-3-8], [14:0-3-4,0-2-8]

LOADING (psf)		SPACING 2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plates Increase 1.15	TC	0.43	Vert(LL)	-0.20	11-12	>999	240	MT20	220/195
TCDL	7.0	Lumber Increase 1.15	BC	0.47	Vert(TL)	-0.49	11-12	>822	180		
BCLL	0.0	Rep Stress Incr YES	WB	0.85	Horz(TL)	0.31	10	n/a	n/a		
BCDL	10.0	Code IBC2003/TPI2002	(Matrix)							Weight: 223 lb	

LUMBER
TOP CHORD 2 X 6 DF SS G
BOT CHORD 2 X 6 DF SS G
WEBS 2 X 4 DF Std G *Except*
1-14 2 X 6 DF SS-G, 8-10 2 X 6 DF SS-G

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-9 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-0-14 oc bracing.
WEBS 1 Row at midpt 2-14, 7-10

REACTIONS (lb/size) 14=1401/0-5-8, 10=1500/0-5-8
Max Horz 14=-127(load case 8)
Max Uplift 14=-328(load case 7), 10=-830(load case 8)
Max Grav 14=1616(load case 2), 10=1895(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1214/440, 2-3=-3700/1335, 3-4=-2980/1038, 4-5=-2918/1051, 5-6=-2986/1030, 6-7=-3531/1214, 7-8=-929/340, 8-9=0/20, 1-14=-786/259, 8-10=-560/3
BOT CHORD 13-14=-1181/3365, 12-13=-1127/3390, 11-12=-950/3323, 10-11=-933/3229
WEBS 2-13=-87/345, 3-13=-132/173, 3-12=-912/318, 4-12=-704/1885, 6-12=-670/317, 6-11=-140/165, 7-11=-109/360, 2-14=-2706/881, 7-10=-2843/846

NOTES
1) Wind: ASCE 7-98; 90mph; h=20ft; TCDL=4.0psf; BCDL=2.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left exposed; porch left and right exposed; Lumber DOL=1.15 plate grip DOL=1.15.
2) TCLL: ASCE 7-98; Pf=28.0 psf (roof snow); Exp B; Fully Exp.
3) Unbalanced snow loads have been considered for this design.
4) This truss has been designed for 2.00 times flat roof load of 25.0 psf on overhangs non-concurrent with other live loads.
5) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
6) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) A plate rating reduction of 20% has been applied for the green lumber members.
9) Bearing at joint(s) 14, 10 considers parallel to grain value using ANSI/TPI 1-1995 angle to grain formula. Building designer should verify capacity of bearing surface.
10) This truss is designed in accordance with the 2003 International Building Code section 2300.1 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

City of Portland
Hearings Officer
NOV 17 2005
Permit Number

REGISTERED PROFESSIONAL ENGINEER
65808PE
OREGON
NOVEMBER 13, 2001
ROBERT C. ANDERSON

EXPIRATION DATE: 12-31-05
January 1, 2005

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MiTEK REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

7777 Greenback Lane
Suite 109
Citrus Heights, CA, 95610

MiTek®

Job	Truss	Truss Type	Qty	Ply	Cameron P Dutz Arch.-8 unit Apt.	R14792461
0000	R1A	ROOF TRUSS	18	1	Job Reference (optional)	

TECNA INDUSTRIES, MCMINNVILLE, OR 97128, Dean Wagner 8.000 s Jun 17 2004 MiTek Industries, Inc. Wed Jan 05 16:37:29 2005 Page 1

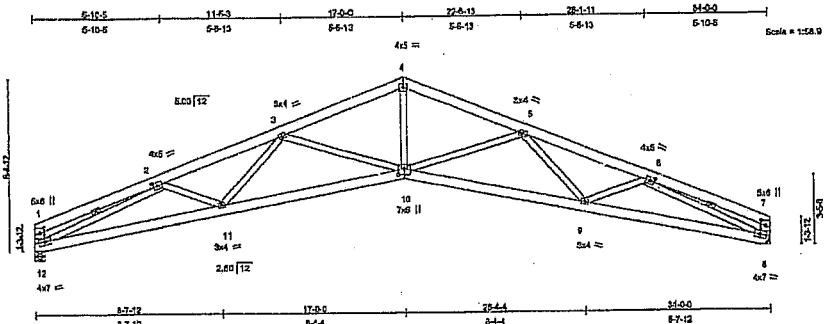


Plate Offsets (X,Y): [1:0-3-0,0-2-4], [2:0-2-4,0-2-0], [6:0-2-4,0-2-0], [7:0-3-0,0-2-4], [10:0-3-8,0-3-8]

LOADING (psf)		SPACING	2-0-0	CSI		DEFL	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plates Increase	1.15	TC	0.30	Vert(LL)	-0.20	9-10	>999	240	MT20	220/195
TCDL	7.0	Lumber Increase	1.15	BC	0.47	Vert(TL)	-0.40	9-10	>818	180		
BCLL	0.0	Rep Stress Incr	YES	WB	0.64	Horz(TL)	0.31	8	n/a	n/a		
BCDL	10.0	Code	IBC2003/TPI2002	(Matrix)							Weight: 217 lb	

LUMBER
TOP CHORD 2 X 6 DF SS G
BOT CHORD 2 X 6 DF SS G
WEBS 2 X 4 DF Std G *Except*
1-12 2 X 6 DF SS-G, 7-8 2 X 6 DF SS-G

BRACING
TOP CHORD Structural wood sheathing directly applied or 4-2-7 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 7-0-0 oc bracing.
WEBS 1 Row at midpt 2-12, 6-8

REACTIONS (lb/size) 12=1408/0-5-8, 8=1408/Mechanical
Max Horz12=100(load case 7)
Max Uplift12=-620(load case 7), 8=-530(load case 8)
Max Grav12=1810(load case 2), 8=1810(load case 3)

FORCES (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=-1215/442, 2-3=-3705/1346, 3-4=-3015/1052, 4-5=-3015/1056, 5-6=-3705/1282, 6-7=-1215/423, 1-12=-785/260, 7-8=-785/251
BOT CHORD 11-12=-1205/3390, 10-11=-1154/3396, 9-10=-992/3398, 6-9=-1059/3390
WEBS 2-11=-65/351, 3-11=-138/172, 3-10=-911/314, 4-10=-715/1906, 5-10=-911/322, 5-9=-138/172, 5-9=-108/351, 2-12=-2710/669, 6-8=-2710/632

NOTES
1) Wind: ASCE 7-98; 90mph; h=20ft; TCDL=4.0psf; BCDL=2.0psf; Category II; Exp B; enclosed; MWFRS gable end zone; cantilever left and right exposed ; end vertical left exposed; porch left and right exposed; Lumber DOL=1.15 plate grip DOL=1.15.
2) TCLL: ASCE 7-98; Pf=25.0 psf (roof snow); Exp B; Fully Exp.
3) Unbalanced snow loads have been considered for this design.
4) As requested, plates have not been designed to provide for placement tolerances or rough handling and erection conditions. It is the responsibility of the fabricator to increase plate sizes to account for these factors.
5) This truss requires plate inspection per the Tooth Count Method when this truss is chosen for quality assurance inspection.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) A plate rating reduction of 20% has been applied for the green lumber members.
8) Refer to girder(s) for truss to truss connections.
9) Bearing at joint(s) 12 considers parallel to grain value using ANSI/TPI 1-1995 angle to grain formula. Building designer should verify capacity of bearing surface.
10) This truss is designed in accordance with the 2003 International Building Code section 2308.1 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard

City of Portland
Bureau of Development Services
NOV 17 2005
Permit Number

REGISTERED PROFESSIONAL ENGINEER
65808PE
OREGON
NOVEMBER 18, 2001
ROBERT C. ANDERSON

EXPIRATION DATE: 12-31-05
January 6,2005

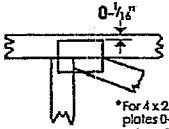
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MiTek REFERENCE PAGE MII-7473 BEFORE USE.
Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult ANSI/TPI1 Quality Criteria, DSB-89 and BCSI1 Building Component Safety Information available from Truss Plate Institute, 583 D'Onofrio Drive, Madison, WI 53719.

7777 Greenback Lane
Suite 109
Citrus Heights, CA, 95610
MiTek®

Symbols

PLATE LOCATION AND ORIENTATION

*Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and securely seat.



*For 4 x 2 orientation, locate plates 0-1/16" from outside edge of truss.

*This symbol indicates the required direction of slots in connector plates.

*Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 x 4 — The first dimension is the width perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING

Indicated by symbol shown and/or by text in the bracing section of the output. Use T, I or Eliminator bracing if indicated.

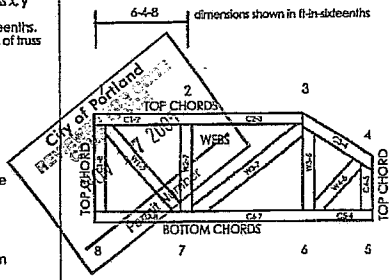
BEARING

Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI1: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

CONNECTOR PLATE CODE APPROVALS

BC	96-31, 95-43, 96-20-1, 96-67, 84-32
ICBO	4922, 5243, 5363, 3907
SBCCI	9667, 9730, 9604B, 9511, 9432A

MiTek®

TEE-LOK

MiTek Engineering Reference Sheet: MII-7473

City of Portland
JUL 22 2005
Permit Expiration

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI1.
2. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
3. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
4. Cut members to bear tightly against each other.
5. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI1.
6. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI1.
7. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
8. Unless expressly noted, this design is not applicable for use with fire retardant or preservative treated lumber.
9. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
10. Plate type, size, orientation and location dimensions shown indicate minimum plating requirements.
11. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
12. Top chords must be sheathed or purlins provided at spacing shown on design.
13. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
14. Connections not shown are the responsibility of others.
15. Do not cut or alter truss member or plate without prior approval of a professional engineer.
16. Install and load vertically unless indicated otherwise.

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