APPROVED
Urban Forestry

Multi-Family Street Tree Requirements:
TWO 2-inch caliper Street Trees Required.
Plant trees per Urban Forestry guidelines.
Call 503-823-8733 for more information.

7614 SE WASHINGTON
MAP NO. 1S2E05AB 12OO
PROP ID # R198871
ZONING: R1
PERMIT 17–180493 /496 /497 /498
/500–RS /501–SD

5250 SF LOT AREA
67.5 SF ALLEY DEDICATION
150 SF STREET DEDICATION

4975 SF LOT
ALLOWED: 5 UNITS
ALLOWED COVERAGE: 2985 SF (60%)
TOTAL COVERAGE: 2087 SF

LANDSCAPING REQMT:
4975 X 20% = 995SF
696SF IN SETBACK
301SF IN REC AREAS
997SF TOTAL

TREE REQUIREMENTS
SIDE SETBACK: 99.5'.
FRONT SETBACK: 50'
10 SMALL TREES @ 15' EA = 150'

TREE CANOPY = 20%
4975 X 20% = 995SF
SMALL TREE = 300SF
10 SMALL TREES PROVIDED = 3000SF

IMPERVIOUS AREA:
2993 SF ROOF
331 SF UNCOVERED CONC
3324 SF TOTAL

PERVIOUS DRIVEWAY NOT COUNTED

48"X5' CONC DRYWELL = 300SF
5 DECID TREES = 500SF
ALLOWED = 3324 / 10 = 332

TOTAL MITIGATION = 3332SF

INCENSE CEDAR,
CALOCEDRUS DECURRENS

ITALIAN CYPRESS, CUPRESSUS
SEMPERVIREMS CVS.

SWEET VIBURNUM, VIBURNUM
ODORATISSIMUM V. AWABUKI

(N) 2 GAL EVERGREEN VIOLET.
PLANT AT 18" O.C. TRIANGLES
(SHOWN SPORADICALLY ON
PLAN FOR CLARITY)

6'X6' SQUARE
3'X4' CC CONN TO DRIVEWAY, 12SF
8'R
AC DRIVEWAY

TYP OUTDOOR AREA.
ADD'L LANDSCAPING IS
EQUAL TO TOTAL
OUTDOOR AREA LESS
12SF. END UNITS HAVE
LARGER AREAS AS
SHOWN ON SITE PLAN.

"WRONG WAY" REFLECTIVE
SIGN ON POST (4 TOTAL)

ENERGY EFFICIENCY MEASURES: TABLE N1101.1(2)

MEASURE 2:
EXT WALLS: R–21 INTERMEDIATE FRAMING
VAULTED CEILINGS: R–30
FLAT CEILINGS: R–49
FLOORS: R–38
WINDOWS: U–0.30

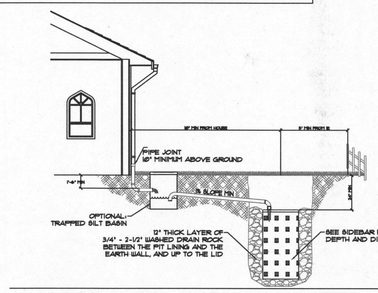
ALL DOORS U–0.20 OR ADDITIONAL 15% OF PERMANENTLY INSTALLED LIGHTING AS HIGH EFFICACY FIXTURES (e.g. CFL)

MEASURE C:
8.5+ HSPF MINI SPLIT

PEDESTRIAN STANDARDS:
CONNECTION FROM UNIT 1 TO THE STREET IS MADE

NO INTERNAL CONNECTIONS REQ'D, SITE UNDER 10,000SF

BICYCLE PARKING:
SHARED SHORT TERM BICYCLE PARKING FOR 10 UNITS: 2



1. Provide protection from all vehicle traffic, equipment staging, and foot traffic in proposed infiltration areas prior to, during and after construction.
2. Siting Criteria: Gravelly sand, gravelly loamy sand or other equally porous material must occur in a continuous 5' deep stratum within 12' of the ground surface. Drywell shall not be placed where base of facility has less than 5' of separation to water table.
3. Sizing: Exhibit 2-36 is used to size the drywell(s) based on impervious area.
4. Top of drywell must be below lowest finished floor.
5. Setbacks: Drywell must be 10' from foundations, 5' from property lines, and 20' from cesspools.
6. Piping must be cast iron, ABS or PVC. 3" pipe required for facilities draining up to 1500 sf, otherwise 4" minimum pipe. Uniform Plumbing Code also applies.
7. Trapped Silt Basin: Optional for roof runoff or pedestrian only paved areas.

DRYWELL DETAIL – 3/8" = 1'

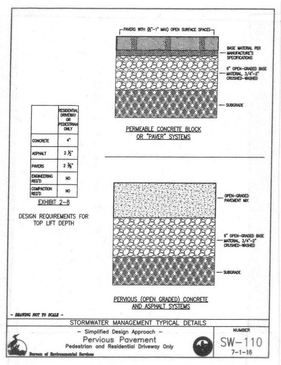
BES Pollution Prevention
Approved By: ANDRE M Date: 5/16/18
D. Hulse 5/16/18

BES Pollution Prevention:
If any de-watering is proposed to be discharged into a City sanitary or storm sewer system, pre-authorization MUST be obtained from the BES Pretreatment of Stormwater Programs. Call 503-823-7122 for more information.

BES STORMWATER FACILITY INSPECTION REQUIRED AT TIME OF CONSTRUCTION. SEE GREEN BES INSPECTION CARD. To schedule, contact the automated inspection request (IVR) system at 503-823-7000 and request inspection #487 BES Onsite Stormwater Facility Eval - OR you may contact our office directly at 503-823-2059

SEPARATE BDS COMMERCIAL PLUMBING PERMIT REQUIRED FOR INSPECTION OF DOWNSPOUTS AND PRIVATE STORMWATER PIPING OUTSIDE OF STORM FACILITY

SEPARATE SEWER CONNECTION PERMIT REQUIRED



SEE SEPARATE PERMIT 90672 FOR INFORMATION ON PUBLIC RIGHT-OF-WAY IMPROVEMENTS

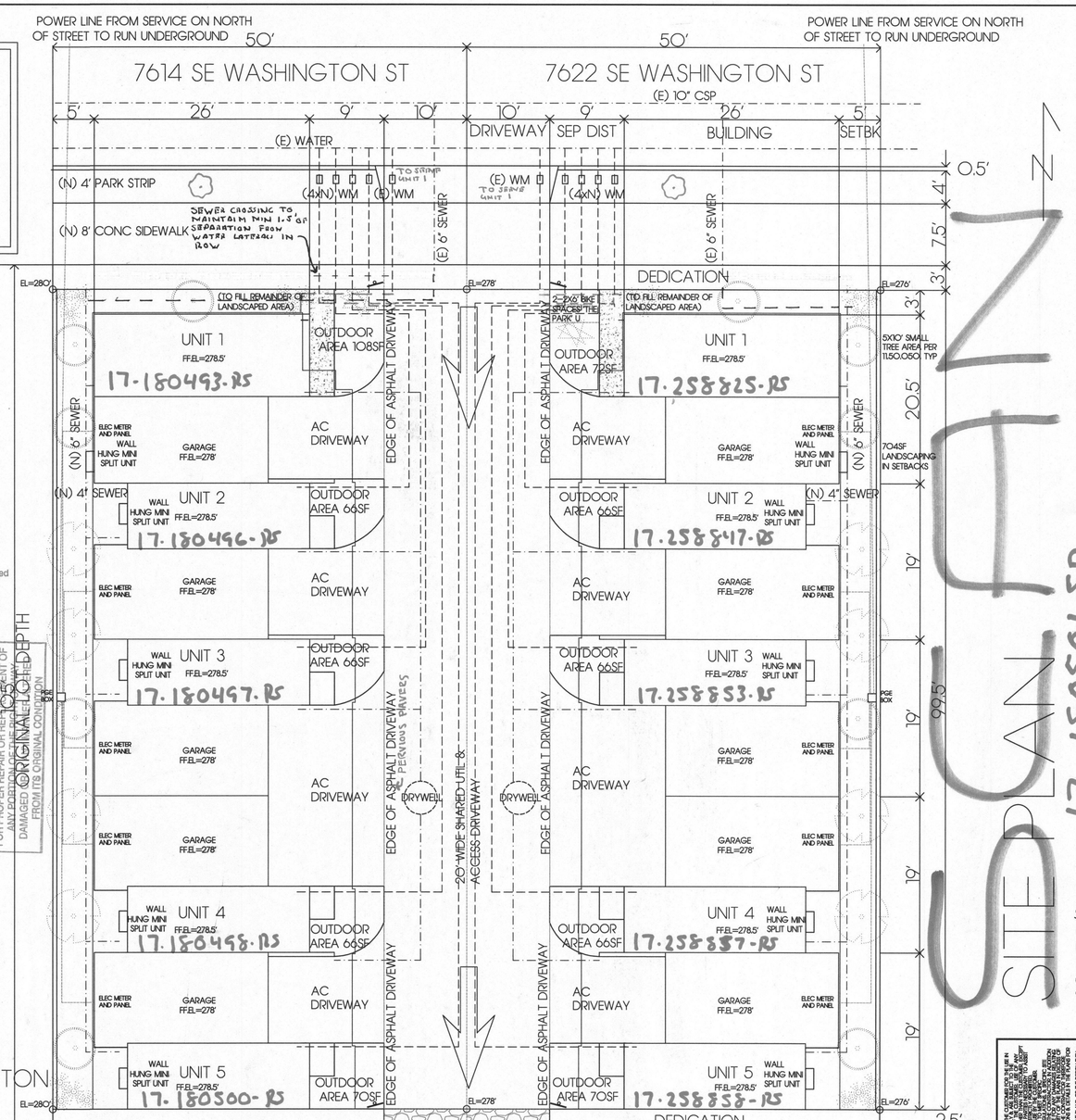
APPLICANT/PERMITTEE IS RESPONSIBLE FOR PROPER REPAIR OR REPLACEMENT OF ANY PEDESTRIAN FACILITY DAMAGED DURING CONSTRUCTION

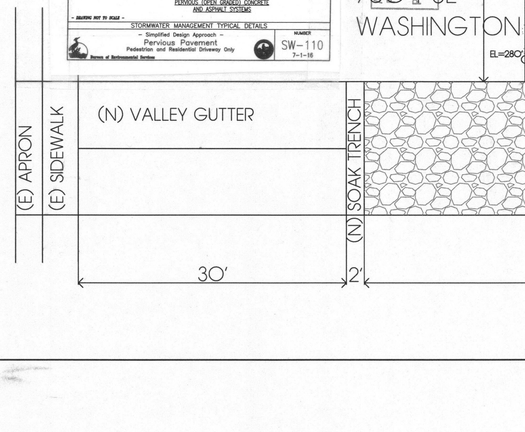
ORIGINAL LOT DEPTH

CONDITION OF FINAL OCCUPANCY APPROVAL: DEDICATED RIGHT-OF-WAY IMPROVEMENTS UNDER THE ASSOCIATED MINOR IMPROVEMENT PERMIT MUST BE APPROVED BY PBOT INSPECTION CONTACT (503-823-7002 OPTION 1)

POWER LINE FROM SERVICE ON NORTH OF STREET TO RUN UNDERGROUND

POWER LINE FROM SERVICE ON NORTH OF STREET TO RUN UNDERGROUND





7614 SE WASHINGTON

SITE PLAN

SCALE: 1/8" = 1'

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PLAN# 7614 SE
NAME WASHINGTON
SHEET SITE PLAN
DATE 1/30/18
SHEET# 0

City of Portland
REVIEWED FOR CODE COMPLIANCE
MAY 22 2018
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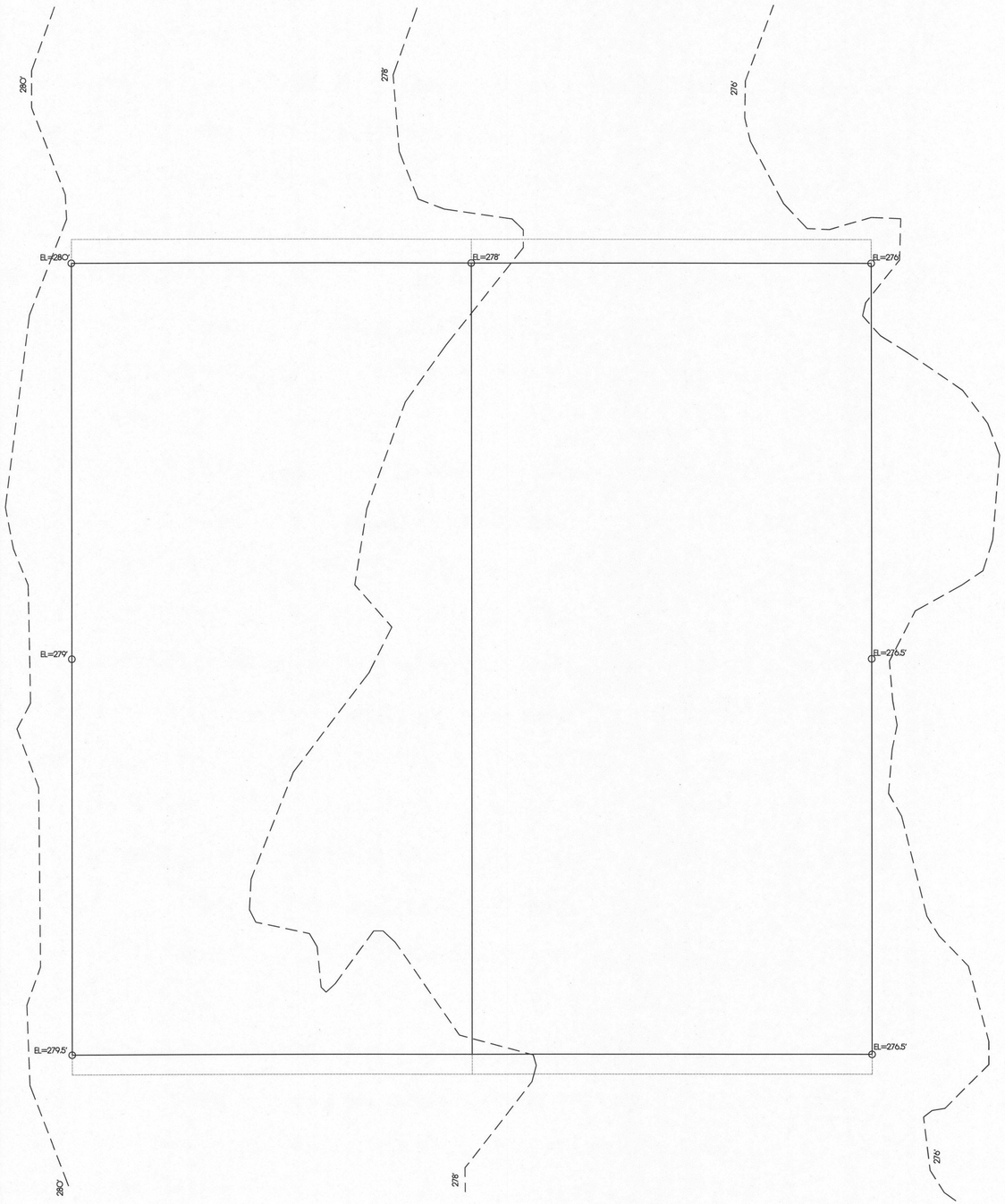
Bureau of Development Services
By: Date: 5/16/18
Approved by Planning and Zoning Review

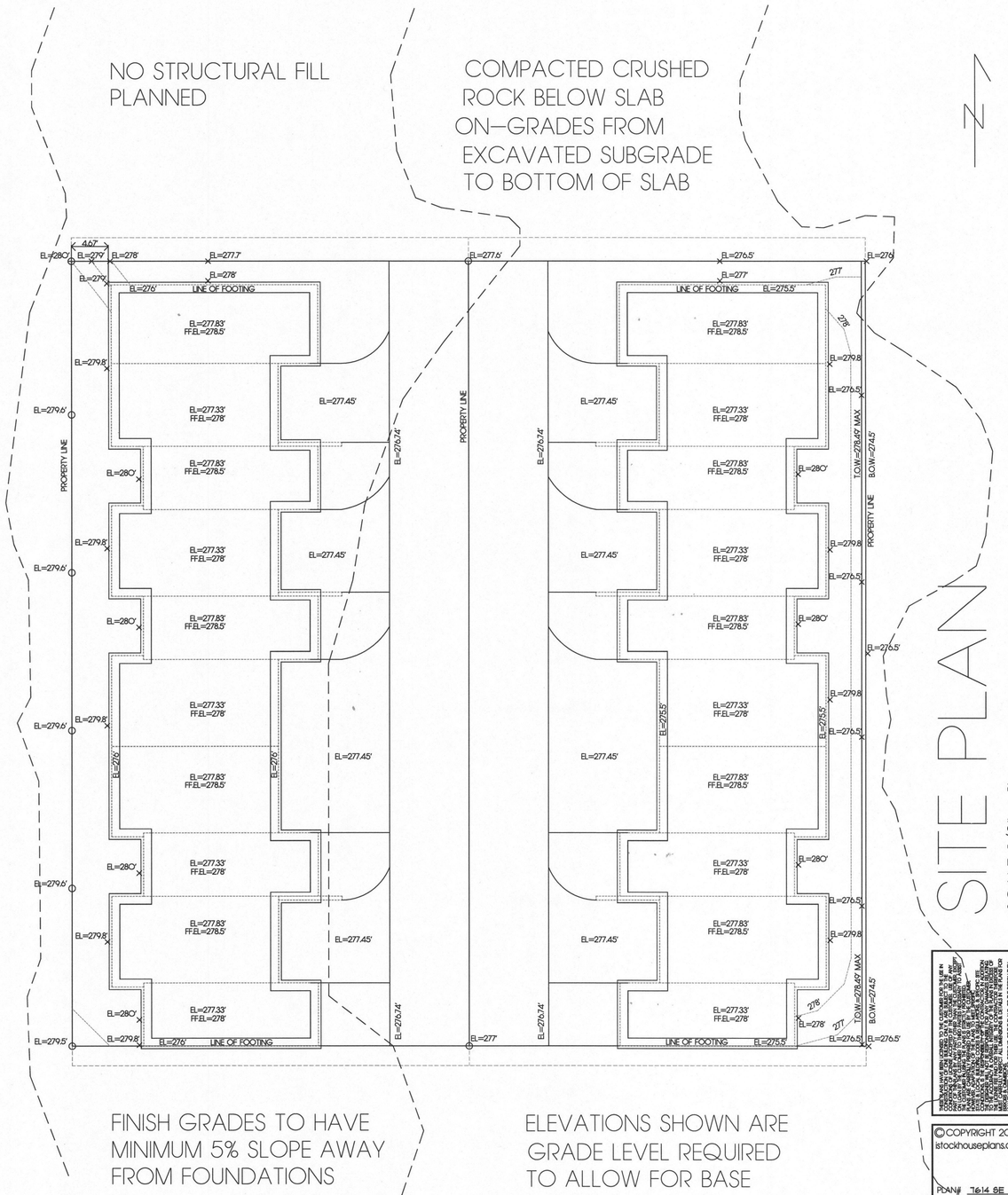
17.258825/47/53/57/58.RS
17.180501-SD
17.180493/496/497/498/500-RS



EXISTING CONDITIONS

NO STRUCTURAL FILL PLANNED

COMPACTED CRUSHED ROCK BELOW SLAB ON-GRADES FROM EXCAVATED SUBGRADE TO BOTTOM OF SLAB



FINISH GRADES TO HAVE MINIMUM 5% SLOPE AWAY FROM FOUNDATIONS

ELEVATIONS SHOWN ARE GRADE LEVEL REQUIRED TO ALLOW FOR BASE LAYER OF GRAVEL

PROPOSED CONDITIONS

SITE PLAN

SCALE: 1/8" = 1'

ALL WORK SHALL CONFORM TO THE 2014 OREGON RESIDENTIAL SPECIALTY CODE AND ALL OTHER APPLICABLE LOCAL CODES, ORDINANCES AND REGULATIONS. THE CONTRACTOR SHALL VISIT THE SITE AND BE RESPONSIBLE FOR ALL CONDITIONS AND DIMENSIONS. DO NOT SCALE DRAWINGS. NOTIFY DESIGNER OF ANY DISCREPANCIES. DESIGNER IS NOT RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES, OR FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK. THESE DRAWINGS ARE NOT FOR CONSTRUCTION UNTIL PERMITTED AND SIGNED BY THE BUILDING OFFICIAL.

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PLAN# 1614 BE
NAME WASHINGTON
SHEET GRADING
DATE 1/30/18
SHEET# 0.2

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ALL SLOPES TO BE
5% FROM BUILDING

EL=279.8'

EL=276'

EL=277.5'

TRANSITION SLOPE
AT CORNER

7622 FACADE FOR CONTEXT

7634 SE WASHINGTON
7622 SE WASHINGTON

GLAZING ASSESSMENT

813SF X 15% =
121.95SF GLAZING

6/Ox5/6 = 33
3/Ox5/6 = 16.5
3/Ox6/6 = 19.5
2/6x5/6 = 13.75
2/6x5/6 = 13.75
2/6x5/O = 12.5
3/Ox5/O = 15

TOTAL = 124SF
MEETS

30 YEAR COMP ASPHALT
SHINGLES OVER
UNDERLAYMENT OVER
3/8" RATED SHEATHING

HARDI-PANEL SIDING
OVER BUILDING WRAP
OVER 1/2" RATED
SHEATHING

5/4X8" CORNER TRIM
(OPTIONAL)

5/4X4" WINDOW TRIM
(OPTIONAL)

ALL SLOPES TO BE
5% FROM BUILDING

TYP REAR SLOPE

EXISTING GRADE

EL=279.8'

EL=279.6'

TRANSITION SLOPE
AT CORNER

7614 SE WASHINGTON
7604 SE WASHINGTON

EXISTING GRADE

EL=277.5'

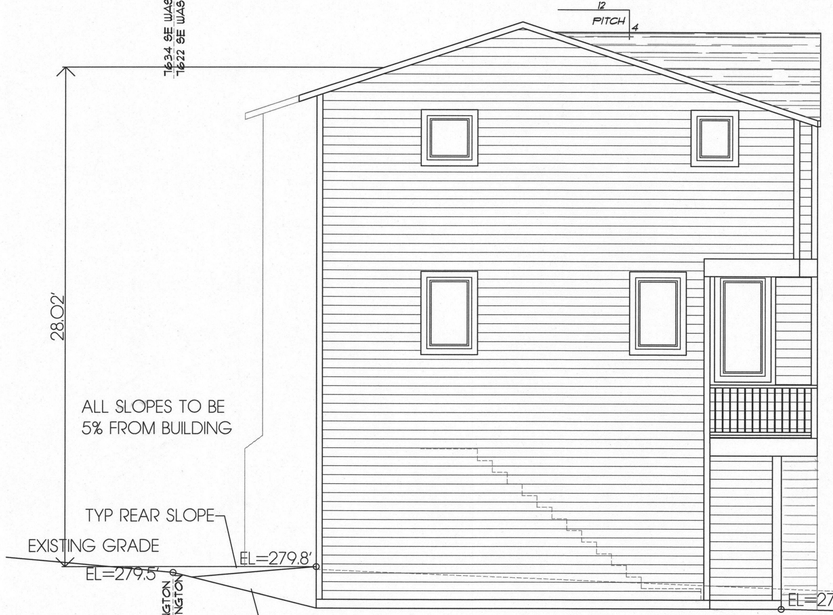
NEW DRIVEWAY

7614 WASHINGTON STREET FACADE

7622 SE WASHINGTON
7614 SE WASHINGTON

ELEVATIONS

SCALE: 1/4" = 1'



ALL SLOPES TO BE
5% FROM BUILDING

TYP REAR SLOPE

EXISTING GRADE

EL=279.8'

EL=279.5'

TRANSITION SLOPE
AT CORNER

7614 SE WASHINGTON
7604 SE WASHINGTON

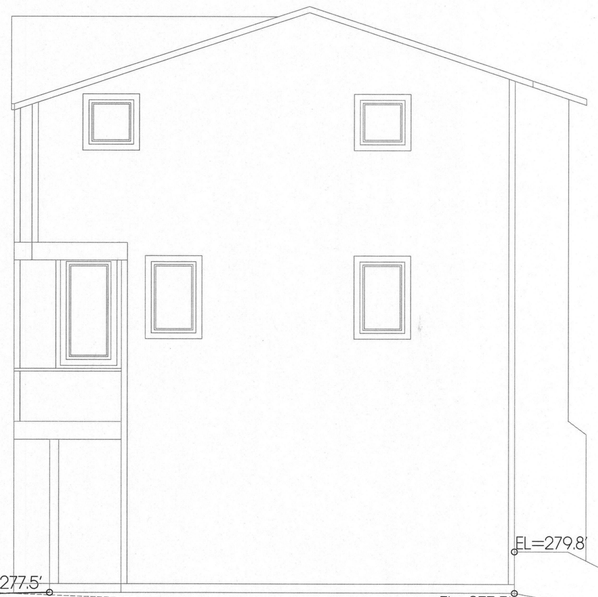
7614 ALLEYWAY FACADE

7622 SE WASHINGTON
7614 SE WASHINGTON

EXISTING GRADE

EL=277.5'

NEW DRIVEWAY



ALL SLOPES TO BE
5% FROM BUILDING

EL=279.8'

EL=278.49'

EL=277.5'

EL=276'

TRANSITION SLOPE
AT CORNER

7634 SE WASHINGTON
7622 SE WASHINGTON

7622 FACADE FOR CONTEXT

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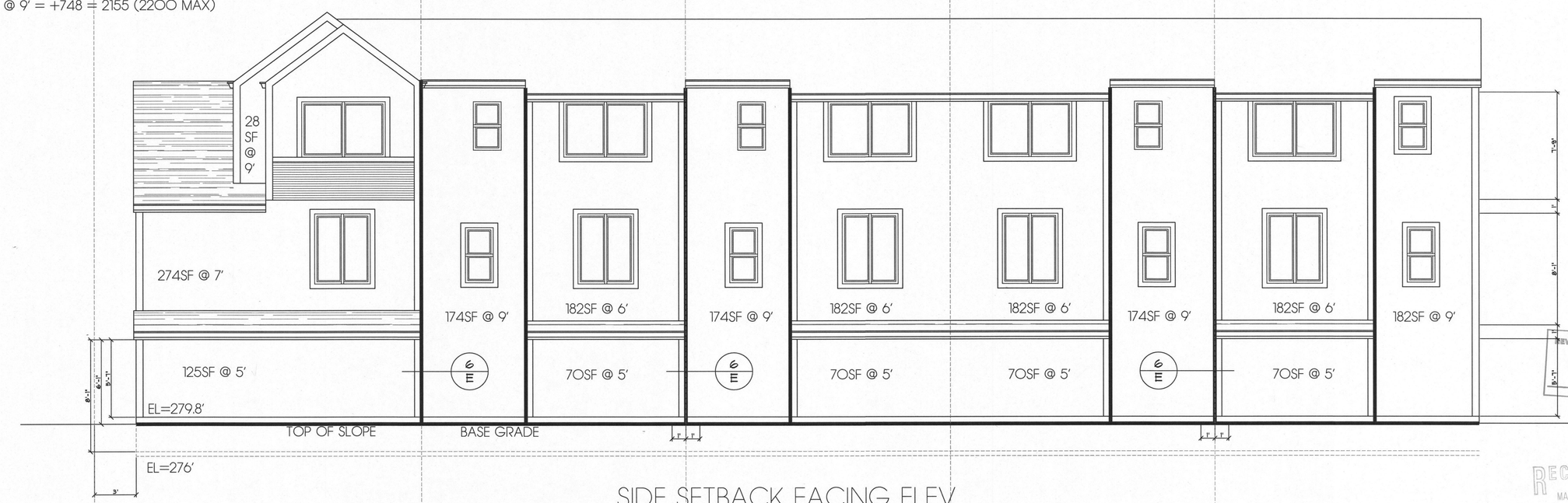
City of Portland
Bureau of
Development Services
By
Date 5/16/18
Approved by
Planning and Zoning Review

PLAN# 7614 SE
NAME WASHINGTON
SHEET ELEVATIONS
DATE 1/30/18
SHEET# 1.1/A

DRIVEWAY FACING ELEV

@ 5' = 405SF (1000 MAX)
@ 6' = +728 = 1133 (1300 MAX)
@ 7' = +274 = 1407 (1600 MAX)
@ 8' = +0 = 1407 (1900 MAX)
@ 9' = +748 = 2155 (2200 MAX)



SIDE SETBACK FACING ELEV

ELEVATIONS

SCALE: 1/4" = 1'

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PLAN# 1614 8E
NAME WASHINGTON
SHEET ELEVATIONS
DATE 1/30/18
SHEET# 1.2

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City of Portland
Bureau of Development Services
By Planning Review Date 3/16/18
Approved by
Planning and Zoning Review

PCC 24.10.080 Fills:

B. Ground preparation. The ground surface shall be prepared to receive fill by removing vegetation, noncomplying fill, top-soil, and other unsuitable materials scarifying to provide a bond with the new fill, and where slopes are steeper than 5 to 1, and the height greater than 5 feet, by benching into competent material or sound bedrock as determined by the soils engineer. The bench under the toe of a fill on a slope steeper than 5 to 1 shall be at least 10 feet wide. The area beyond the toe of fill shall be sloped for sheet overflow or a paved drain shall be provided. Where fill is to be placed over a cut the bench under the toe of a fill shall be at least 10 feet wide but the cut must be made before placing fill and approved by the soils engineer and engineering geologist as a suitable foundation for fill. Unsuitable soil is soil which in the opinion of the Director or the civil engineer or the soils engineer or the engineering geologist, is not competent to support either soil or fill, to support structures or to satisfactorily perform the other functions for which the soil is intended.

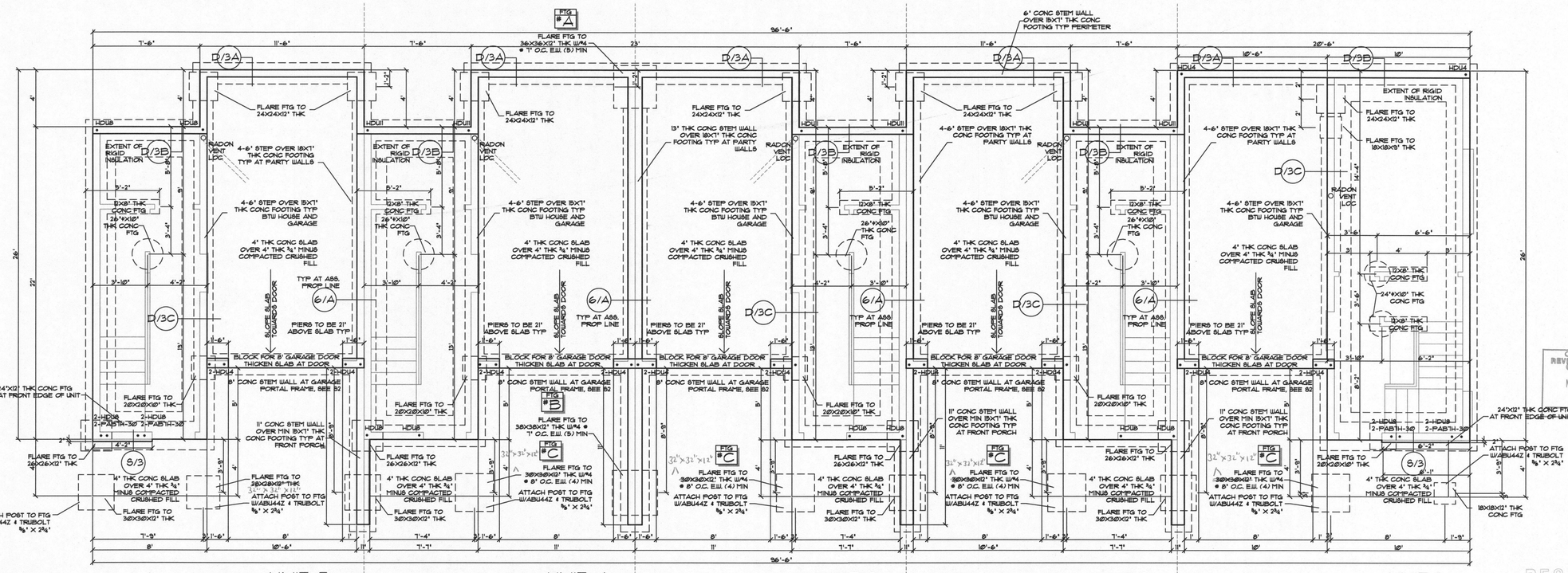
C. Fill material. Only permitted material free from tree stumps, detrimental amounts of organic matter, trash, garbage, sod, peat, and similar materials shall be used. Rocks larger than 6 inches in greatest dimension shall not be used unless the method of placement is properly devised, continuously inspected, and approved by the Director.

The following shall also apply:

1. Rock sizes greater than 6 inches in maximum dimension shall be 10 feet or more below grade, measured vertically.

2. Rocks shall be placed so as to assure filling all voids with fines. Topsoil may be used in the top 12-inch surface layer to aid in planting and landscaping.

D. Compaction of fill. All fills shall be compacted to a minimum relative dry density of 90 percent as determined in accordance with ASTM Standard D-1557-78. Field density verification shall be determined in accordance with ASTM Standard D-1556-82 or equivalent and must be submitted for any fill 12 inches or more in depth where such fill may support the foundation for a structure. A higher relative dry density, or additional compaction tests, or both, may be required at any time by the Director.



FTC #C

REDLINED PER BRIAN D'AGOSTINI'S FOOTING CALCULATION 3-19-18

FOUNDATION

SCALE: 1/4" = 1'

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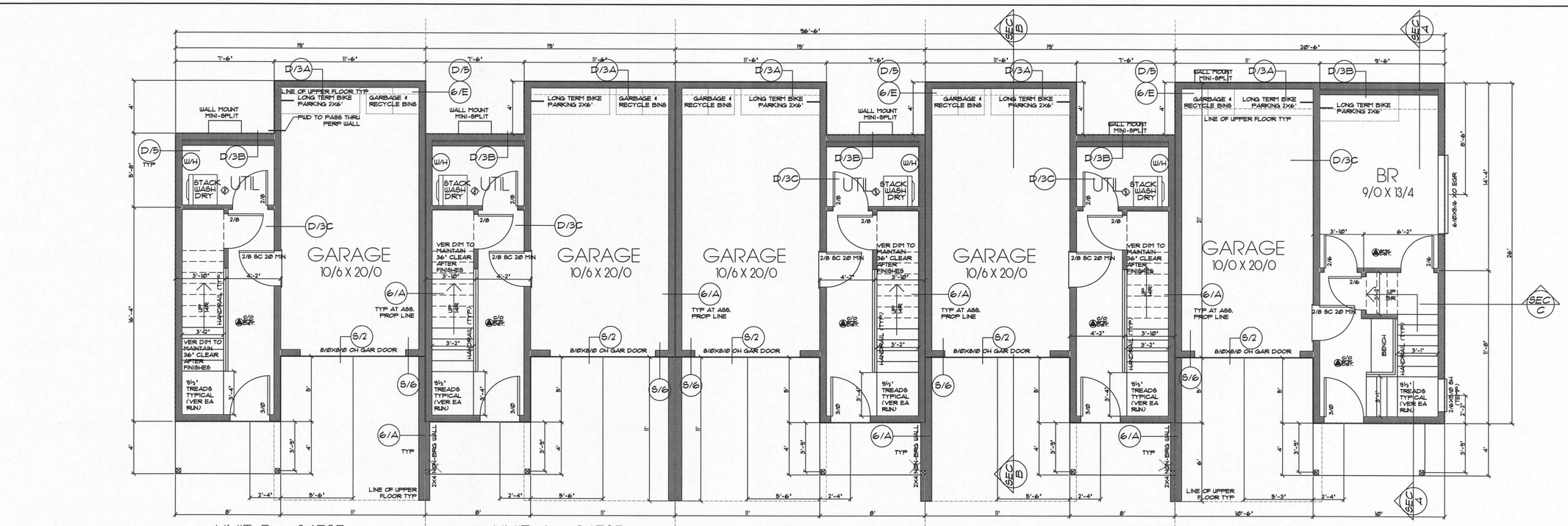
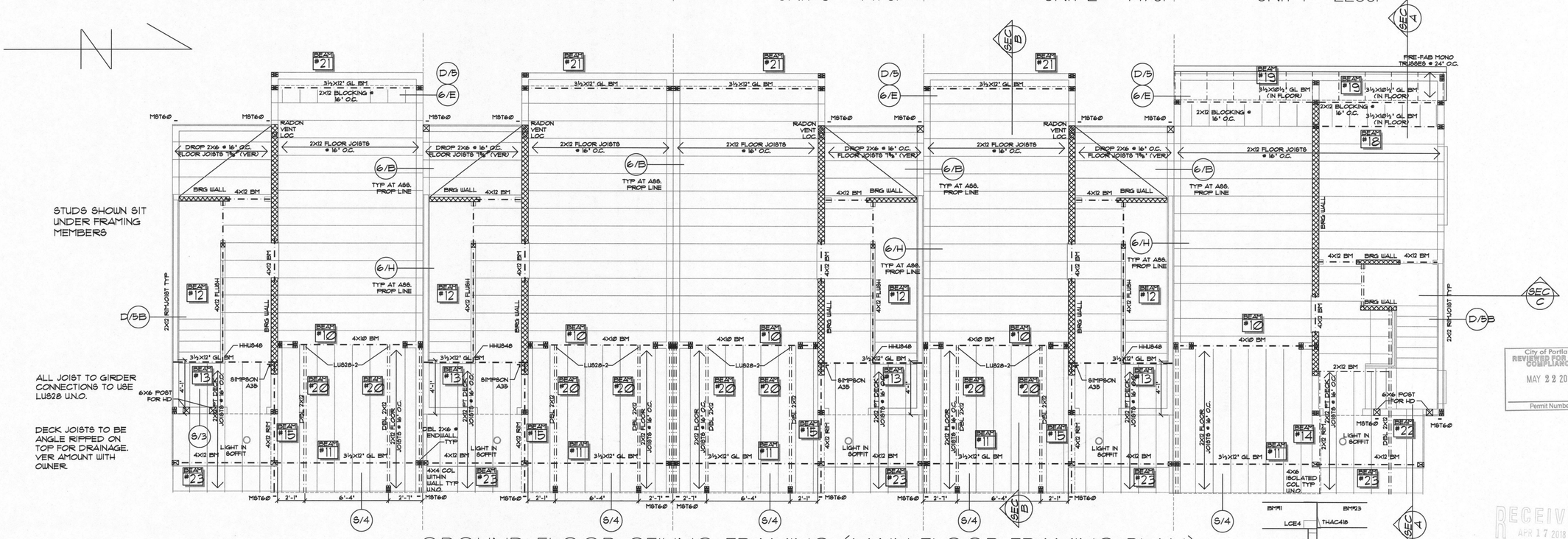
PLAN# 1614 SE
NAME WASHINGTON
SHEET FDN
DATE 4/9/18
SHEET# 2 A

GROUND FLOOR & CEILING FRAMING

SCALE: 1/4" = 1'





GROUND FLOOR CEILING FRAMING (MAIN FLOOR FRAMING PLAN)

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NAME WASHINGTON
SHEET GRD FLR
DATE 4/9/18
SHEET# 3 A

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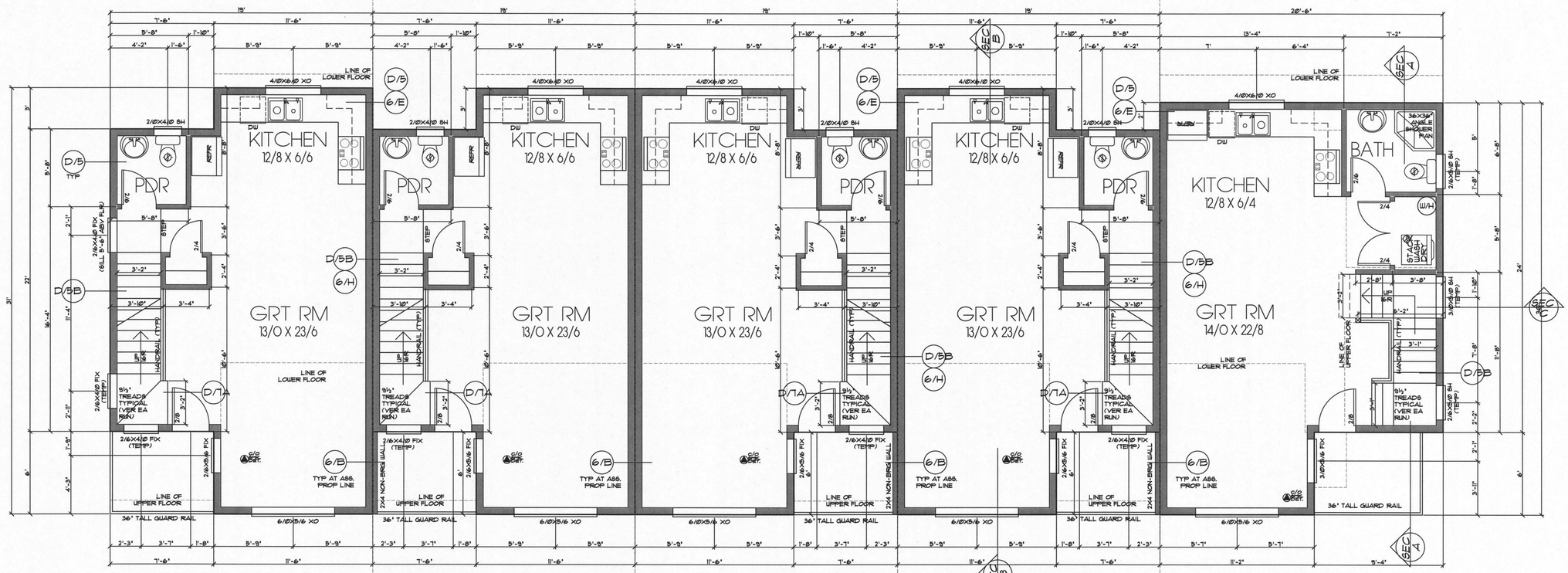

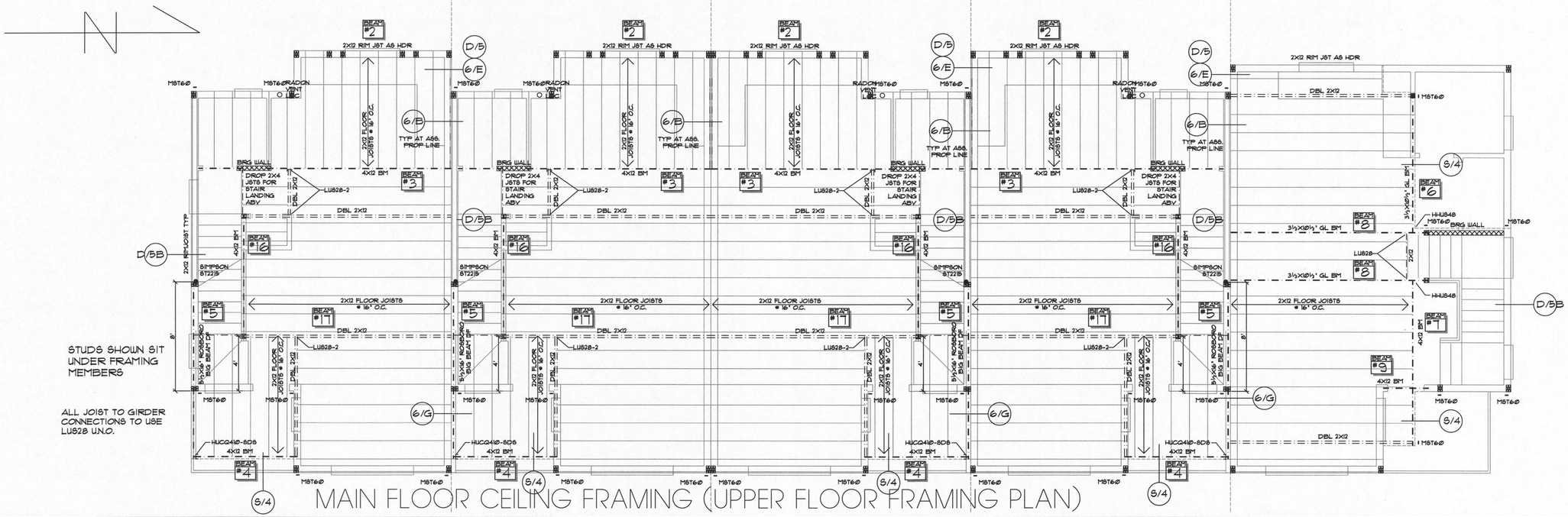

STUDS SHOWN SIT UNDER FRAMING MEMBERS

ALL JOIST TO GIRDER CONNECTIONS TO USE LUS28 U.N.O.

MAIN FLOOR & CEILING FRAMING

SCALE: 1/4" = 1'

MAIN FLOOR CEILING FRAMING (UPPER FLOOR FRAMING PLAN)

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NAME WASHINGTON
SHEET MAIN FLR
DATE 4/9/18
SHEET# 4 A

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UPPER FLOOR & ROOF FRAMING

SCALE: 1/4" = 1'

UNIT 5 – 473SF | UNIT 4 – 473SF | UNIT 3 – 473SF | UNIT 2 – 473SF | UNIT 1 – 353SF

FRONT ROOF OF UNIT 1:

1725F ROOF AREA
172 / 3Ø2Ø X 144 =
838GIN VENTILATION
5Ø% = 42ØGIN
TOP VENTS = 8ØIN NFA
42/5Ø = 1 VENTS MIN
MIN 42NFA BIRD BLOCK VENTING

MAIN ROOF OF UNIT 1:

3956F ROOF AREA
395 / 3Ø2Ø X 144 =
1926GIN VENTILATION
5Ø% = 963GIN
TOP VENTS = 8ØIN NFA
96/5Ø = 2 VENTS MIN
MIN 96NFA BIRD BLOCK VENTING

MAIN ROOF OF UNITS 2-5:

5825F ROOF AREA
395 / 3Ø2Ø X 144 =
2738GIN VENTILATION
5Ø% = 1369GIN
TOP VENTS = 8ØIN NFA
140/5Ø = 3 VENTS MIN
MIN 14ØNFA BIRD BLOCK VENTING

STUDS SHOWN SIT UNDER FRAMING MEMBERS

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PLAN# 7614 SE
NAME WASHINGTON
SHEET UPPER/ROOF
DATE 4/9/18
SHEET# 5A

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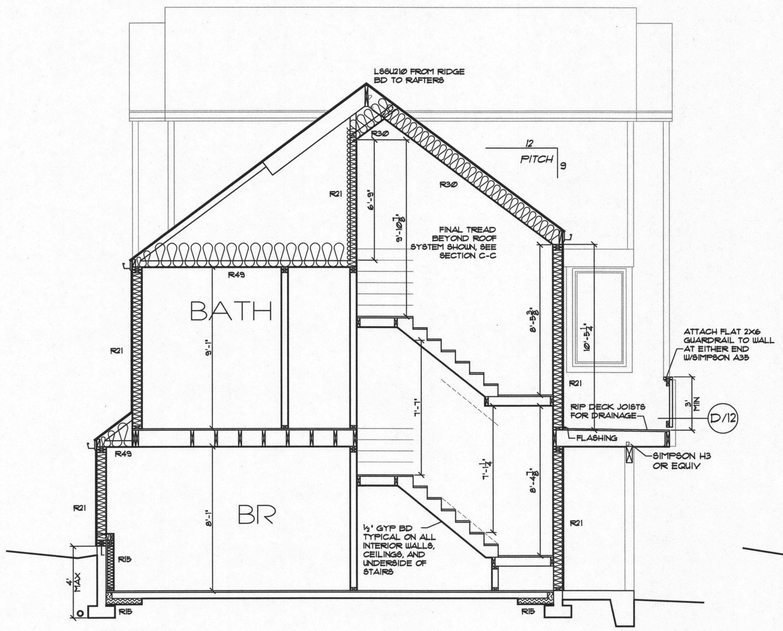
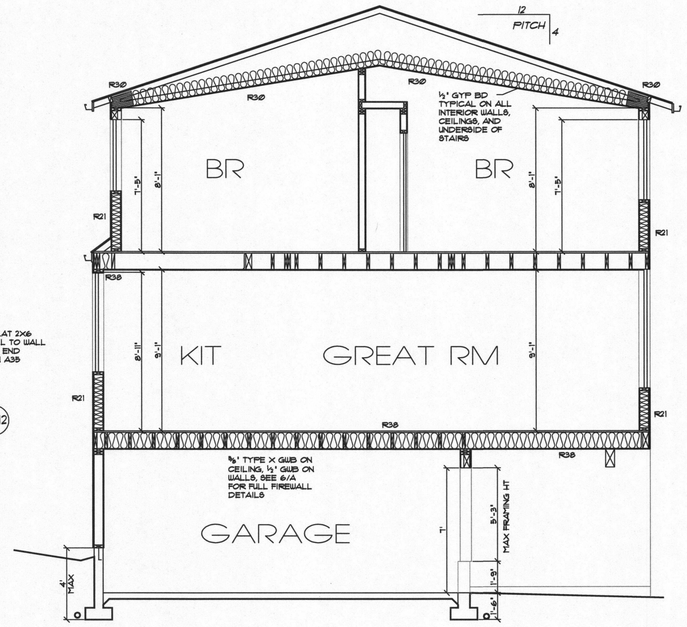
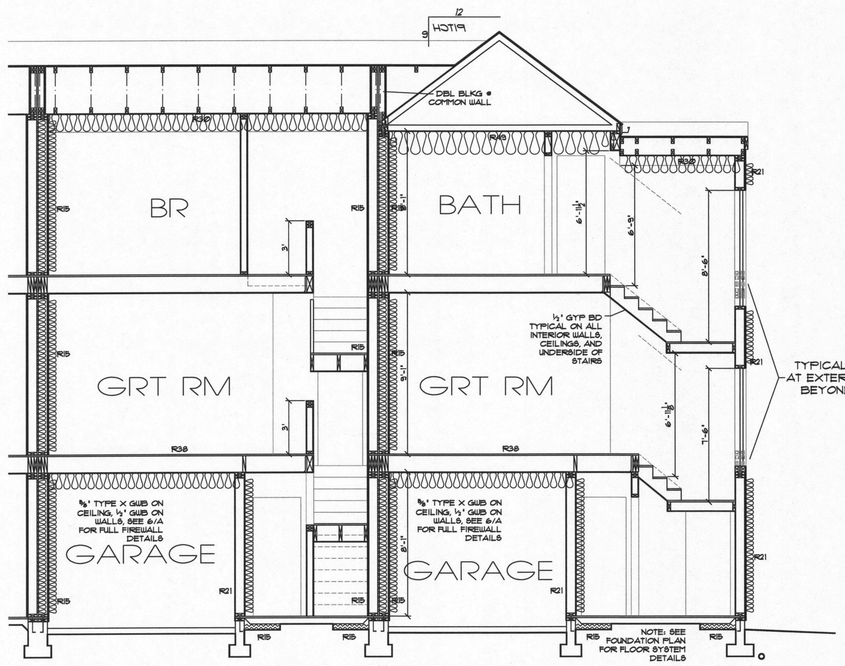


SECTION A—A



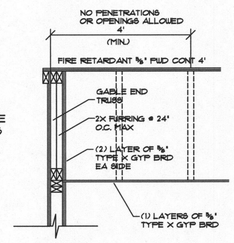
SECTION B—B



TYPICAL FRAMING AT EXTERIOR WALL BEYOND WINDOW

SECTION C—C

SHEATHINGS ON BOTH SIDES TO BE CONTINUOUS THROUGH ALL LEVELS AND ATTIC TO THE UNDERSIDE OF ROOF DECKING.

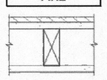


GA FILE NO. WP 8109	PROPRIETARY*	1 HOUR FIRE

GYPSUM PANEL PRODUCTS, FIBER-CEMENT SIDING, WOOD STUDS

EXTERIOR SIDE: **Base** layer ⅝" proprietary type X gypsum sheathing or glass mat gypsum substrate applied parallel to 2 x 4 wood studs 16" o.c. with 1¼" galvanized roofing nails 4" o.c. at vertical joints and 7" o.c. at intermediate studs and top and bottom plates. Joints of gypsum sheathing may be left untreated. **Face** layer ½" proprietary fiber-cement siding fastened through sheathing to studs. 3½" unfaced glass fiber friction fit in stud space.

INTERIOR SIDE: One layer ⅝" proprietary type X gypsum wallboard, glass mat gypsum substrate, water-resistant gypsum backing board, or gypsum veneer base applied parallel or at right angles to studs with 6d coated nails, 1⅞" long, 0.0915" shank, ¼" heads, 7" o.c. **(LOAD BEARING)**

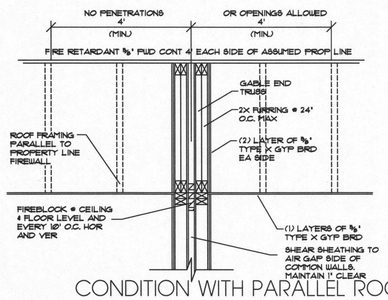


Thickness: 5⅛"
Approx. Weight: 9 psf
Fire Test: See WP 3510 (UL R3501-47, -48, 9-17-65, UL Design U309; UL R-1319-129, 7-22-70, UL Design U314)

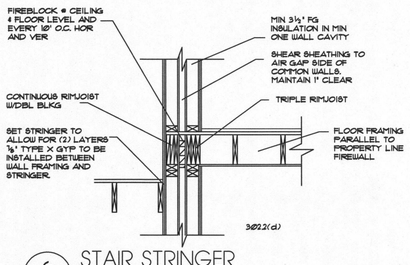
PROPRIETARY GYPSUM PANEL PRODUCTS

American Gypsum Company LLC	⅝" FireBloc® Type X ⅝" Exterior Sheathing Type X
CertainTeed Gypsum Inc.	⅝" CertainTeed® Type X Gypsum Board ⅝" GlasRoc® Sheathing Type X Gypsum Panels
Georgia Pacific Gypsum LLC	⅝" DensArmor Plus® Fireguard® Interior Panel ⅝" DensGlass® Fireguard® Sheathing
Lafarge North America Inc.	⅝" Firecheck® Type X
National Gypsum Company	⅝" Weather Defense® Platinum Sheathing ⅝" Gold Bond® Brand FIRE-SHIELD® Gypsum Board ⅝" Gold Bond® Brand FIRE-SHIELD® Gypsum Sheathing
PABCO Gypsum	⅝" FLAME CURB® Type X ⅝" Exterior Gypsum Sheathing Type X
Temple-Inland	⅝" Type X ⅝" GreenGlass Type X

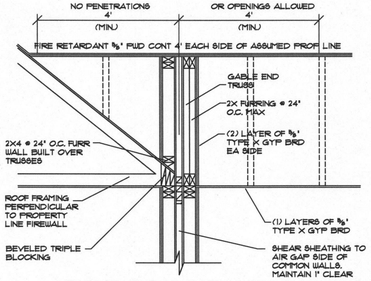
6/E EXTERIOR WALL PROTECTION
NOT TO SCALE



6/C CONDITION WITH PARALLEL ROOFS AT ASSUMED PROPERTY LINE.
NOT TO SCALE 3@22.4(d)

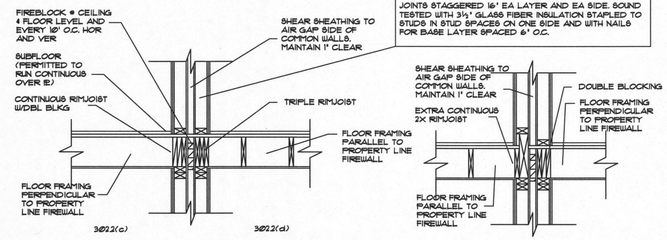


6/H STAIR STRINGER
NOT TO SCALE



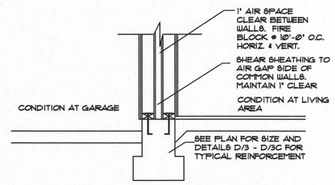
6/D CONDITION WITH PARALLEL AND PERPENDICULAR ROOF AT ASSUMED PROPERTY LINE.
NOT TO SCALE 3@22.4.3(a)

RATED 2 HR FIREWALL
GA FILE NO. WP3620
55 TO 59 STC SOUND



6/B FRAMED FLOOR
NOT TO SCALE

6/G BEAM #5
NOT TO SCALE



6/A FOUNDATION
NOT TO SCALE

NOTE:
2-HR RATED WALL ASSEMBLY WILL CONTINUE THROUGH THE FLOOR SYSTEM TO THE RATED ROOF. THE RATED WALL MUST BE CONTINUOUS BEHIND SHOWERS, TUBS, CABINETS, CASES, SOFFITS, ELECTRICAL PANELS, FIREPLACES, STAIR STRINGERS AND ADJOINING WALLS.

NO PENETRATIONS SHALL BE ALLOWED WITHIN THE WALL ASSEMBLY INCLUDING BUT NOT LIMITED TO ELECTRICAL, PLUMBING AND MECHANICAL DUCTS AND VENTS.

BASE LAYER:
⅝" TYPE 'X' GYPSUM WALLBOARD APPLIED AT RIGHT ANGLES TO EACH SIDE OF DBL ROW OF 2X4 WOOD STUDS @ 16" C/C WITH 6d COATED NAILS, 2½" LONG, MIN Ø.099 SHANK, ¼" HEAD, 6" C/C.

FACE LAYER:
⅝" TYPE 'X' GYPSUM WALLBOARD APPLIED AT RIGHT ANGLES TO EACH SIDE OF STUDS OVER BASE LAYER W/ 10d COATED NAILS, 3" LONG, MIN Ø.120 SHANK ¼" HEADS, 8" C/C. STAGGER JOINTS 16" C/C EACH LAYER AND SIDE. PROVIDE BLOCKING AT MID HEIGHT.

JOINTS STAGGERED 16" EA LAYER AND EA SIDE. SOUND TESTED WITH 3½" GLASS FIBER INSULATION STAPLED TO STUDS IN STUD SPACES ON ONE SIDE AND WITH NAILS FOR BASE LAYER SPACED 6" O.C.

CROSS SECTIONS

SCALE: 1/4" = 1'

DETAILS 6/A THROUGH 6/H ARE A PART OF ASSUMED PROPERTY LINE FIREWALL MODIFIED 2-HR APPROACH

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PLAN# T614 SE
NAME WASHINGTON
SHEET SECTIONS
DATE 4/9/18
SHEET# 6/A

SHEARWALL SHEATHING SCHEDULE

TYPE	THICKNESS	EDGE NAILING	SILL TO FLOOR	SILL TO FOUNDATION
A	15/32"	8D @ 6" O.C.	SDS25312 @ 16" O.C.	5/8"~ A.B. @ 36" O.C.
B	15/32"	8D @ 4" O.C.	(2) SDS25312 @ 16" O.C.	5/8"~ A.B. @ 24" O.C.
C	15/32"	8D @ 3" O.C.	(2) SDS25312 @ 12" O.C.	5/8"~ A.B. @ 18" O.C.

SHEARWALL HOLDDOWN SCHEDULE

#	TYPE
O	NO HOLDDOWN REQ'D
1	SIMPSON MST60

MST60 – FILL 23 HOLES TOP AND 23 HOLES BOTTOM W/16d

#	TYPE	AB DIA	COL
2	SIMPSON HDU2	5/8"	2–2X
3	SIMPSON HDU4	5/8"	2–2X
4	SIMPSON HDU8	7/8"	2–2X
5	SIMPSON HDU11	1"	6X

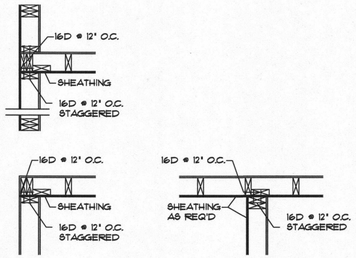
ALL HDU FILL ALL HOLES W/ 1¼" x 2½" SDS

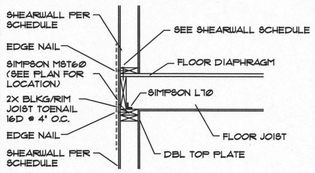
GENERAL SHEATHING NOTES

ROOF DIAPHRAGM:
MIN 1/2" CDX PWD (48/24) W/
8D @ 6" O.C. EDGES
8D @ 12" O.C. FIELD

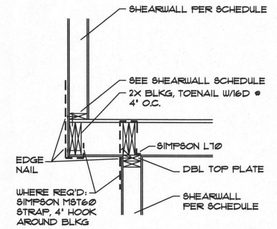
FLOOR DIAPHRAGM:
MIN 3/4" CDX PWD (32/16) W/
10D @ 6" O.C. EDGES
10D @ 12" O.C. FIELD

SHEATHING — SHEAR WALL TYPE
SHEAR WALL — HOLDDOWN TYPE



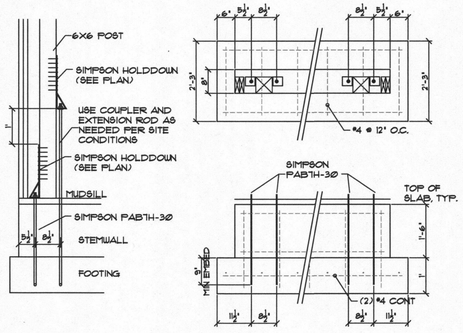


S/5 SHEARWALL TO SHEARWALL CONNECTION 1/2" = 1'-0"

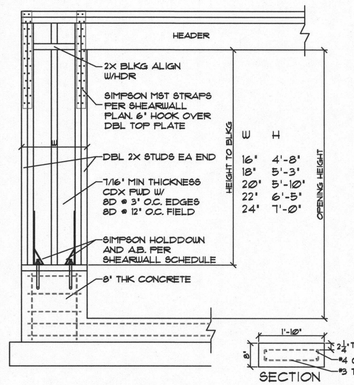


S/4 SHEAR STRAP @ CANTILEVER 1/2" = 1'-0"

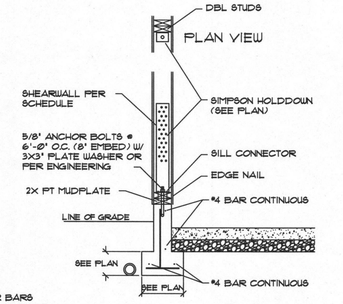
S/6 TYPICAL THROUGH SHEATHING DETAIL 1/2" = 1'-0"



S/3 DOUBLE HOLDDOWN DETAIL 1/2" = 1'-0"

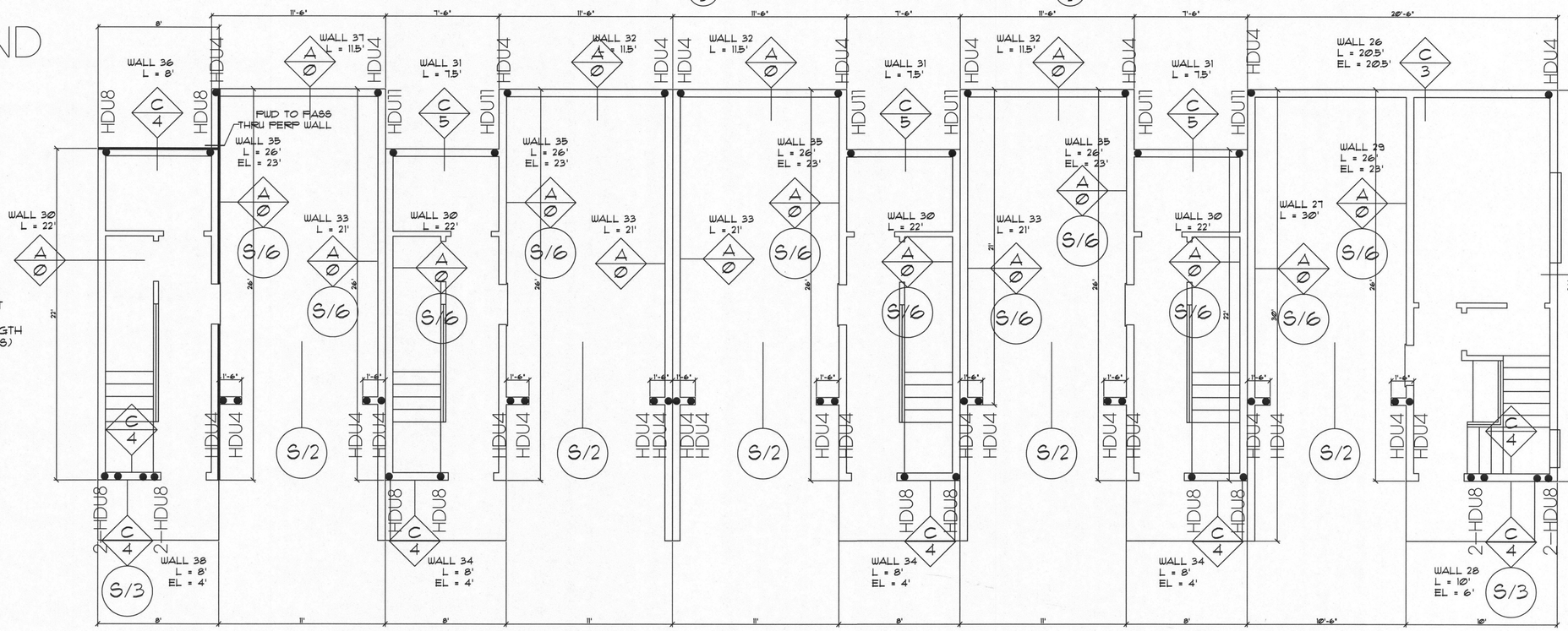


S/2 PORTAL FRAME 1/2" = 1'-0"



S/1 HOLDDOWN CONNECTION 1/2" = 1'-0"

GROUND FLOOR



WALL #, SEE CALC PACKET
L = FULL WALL LENGTH
EL = EFFECTIVE WALL LENGTH
(EXCLUDES PERFORATIONS)

STUDS TO BE 16" O.C
U.N.O.

DBL STUDS AT
EACH SHEARWALL
END AND HD U.N.O.

EXTEND
SHEARWALLS TO
ROOF DIAPHRAGM

EACH UNIT
STRUCTURALLY
INDEPENDENT

SHEAR ENGINEERING

SCALE: 3/16" = 1'

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PLAN# 7614 SE
NAME WASHINGTON
SHEET SHEAR
DATE 4/9/18
SHEET# S1A

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EACH UNIT
STRUCTURALLY
INDEPENDENT

MAIN FLOOR

WALL 20
L = 7.5'
EL = 5.5'

WALL 21
L = 11.5'
EL = 7.5'

WALL 19
L = 22'

WALL 24
L = 22'
EL = 15'

WALL 22
L = 31'

WALL 23
L = 11.5'
EL = 5.5'

WALL 15
L = 20.5'
EL = 16.5'

WALL 16
L = 30'

WALL 14
L = 24'
EL = 15.5'

WALL 18
L = 9.33'
EL = 6.5'

WALL 17
L = 11.17'
EL = 5'

WALL *, SEE CALC PACKET
L = FULL WALL LENGTH
EL = EFFECTIVE WALL LENGTH
(EXCLUDES PERFORATIONS)

STUDS TO BE 16" O.C
U.N.O.

DBL STUDS AT
EACH SHEARWALL
END AND HD U.N.O.

EXTEND
SHEARWALLS TO
ROOF DIAPHRAGM

UPPER FLOOR

WALL 10
L = 11.5'
EL = 5.5'

WALL 9
L = 7.5'
EL = 5.5'

WALL 11
L = 31'

WALL 8
L = 27'

WALL 12a
L = 19'
EL = 10.5'

WALL 12b
L = 19'
EL = 10.5'

WALL 13
L = 27'
EL = 22.5'

WALL 7
L = 11.17'
EL = 5'

WALL 4
L = 30'

WALL 5
L = 11.17'
EL = 5'

WALL 3a
L = 14'

WALL 2
L = 7'

WALL 6
L = 7'

WALL 1
L = 12'
EL = 8.5'

WALL 3b
L = 14'

SHEAR ENGINEERING

SCALE: 3/16" = 1'

THESE PLANS ARE SPECIFICALLY DESIGNED TO BE CONSTRUCTED FOR THE IN-TENDED USE AS DESCRIBED ON THESE PLANS AND ARE NOT TO BE USED FOR ANY OTHER PURPOSE. THE CONTRACTOR SHALL VERIFY ALL DIMENSIONS AND CONDITIONS AT THE JOB SITE AND SHALL NOTIFY THE ARCHITECT OF ANY DISCREPANCIES BEFORE PROCEEDING WITH THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES AND FOR COORDINATING ALL PORTIONS OF THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL JOB SITE SAFETY AND SECURITY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES AND FOR COORDINATING ALL PORTIONS OF THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL JOB SITE SAFETY AND SECURITY. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES AND FOR COORDINATING ALL PORTIONS OF THE WORK. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL JOB SITE SAFETY AND SECURITY.

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PLAN# 7614 SE
NAME WASHINGTON
SHEET SHEAR
DATE 4/9/18
SHEET# S2A

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GENERAL NOTES

1. DESIGN LOADS:

ROOF	25 PSF (LIVE LOAD)
FLOOR	40 PSF
STAIRS	100 PSF
GARAGE FLOOR	50 PSF
DECKS	60 PSF

2. INSULATION:

   A) CEILINGS (VAULTED) R-30A
   B) CEILINGS (FLAT) R-49
   C) WALLS R-21
   D) FLOOR (OVER UNHEATED AREA) R-38

   THE ABOVE VALUES ARE MINIMUM AND MAY BE INCREASED IF DESIRED, ALL EXPOSED INSULATION IS TO HAVE A FLAME SPREAD RATING OF LESS THAN 25 AND A SMOKE DESTINY RATING OF LESS THAN 450. PROVIDE INSULATION BAFFLES AT EAVE VENTS BETWEEN RAFTERS.

3. WINDOWS:

   A) EACH BEDROOM TO HAVE A MIN. WINDOW OPENING OF 5.7 SQ. FT. WITH A MIN. WIDTH OF 20", A MIN. HEIGHT OF 22" AND A SILL LESS THAN 44" ABOVE FIN. FLR.

   B) ALL EXTERIOR WINDOWS TO BE DOUBLE GLAZED. U=0.32

   C) ALL WINDOWS WITHIN 18" OF THE FLOOR AND WITHIN 12" OF ANY DOOR TO BE TEMPERED.

4. ALL ELECTRICAL OUTLETS IN GARAGES, BATHROOMS, AND KITCHENS TO BE G.F.I. PER ELECTRICAL CODE STANDARDS.

5. ALL BATHROOMS AND UTILITY ROOMS TO BE PROVIDED WITH A FAN VENTED TO THE OUTSIDE, WITH A MIN. OF 5 AIR CHANGES PER HOUR. THIS ALSO INCLUDES RANGE HOODS.

FRAMING NOTES

1. LUMBER SPECIES:

   A) POSTS, BEAMS, HEADERS, RAFTERS, AND JOISTS - NO. 2 D.F.L.

   B) STUDS - STUD GRADE D.F.L.

   C) SILLS, PLATES, BLOCKING, BRIDGING, ETC. - NO. 3 D.F.L.

   D) POST AND BEAM DECKING TO BE UTILITY GRADE D.F.L.

   E) SHEATHING - 1/2" CDX PWD 32/16 OR 7/16" O.S.B. NAILING: 8d 6" O.C. EDGES, 12" O.C. FIELD

2. ALL EXTERIOR HEADERS LESS THAN 5'-0" ARE TO BE 4 X 8. ALL HEADERS 5'-0" TO 8'-0" ARE TO BE 4 X 12. GARAGE HEADER TO BE 4 X 12. HEADERS MAY ALSO BE DESIGNED.

3. PROVIDE DOUBLE JOIST UNDER ALL PARTITION WALLS ABOVE RUNNING PARALLEL TO JOIST.

4. ATTIC ACCESS MUST BE 22" x 30" MIN. WITH 30" OF HEADROOM DIRECTLY ABOVE OPENING. ATTIC VENTS TO BE A MIN. OF 1/600 OF ATTIC AREA, 1/2 TO BE ROOF VENT OTHERS TO BE EAVE VENTS. EAVE VENTS TO BE COVERED WITH MIN. 1/4" CORROSION PROOF WIRE MESH.

5. ALL EXTERIOR POST AND BEAMS WILL BE PROVIDED WITH APPROVED PAINTED STEEL ANCHORS AND/OR CONNECTORS.

6. ALL STAIRS SHALL BE FURNISHED WITH RAILINGS OR BARRIERS WHICH ARE CONSTRUCTED SO THAT A 4"ø OBJECT CAN NOT PASS THROUGH AND ARE TO BE A MIN. OF 34" HIGH (36" AT LANDINGS). ALL DECKS 30" ABOVE GRADE MUST ALSO MEET THESE REQUIREMENTS.

FOUNDATION NOTES

1. EXCAVATE TO PROVIDE A MIN. OF 18" BEAM TO GRADE CLEARANCE.

2. FOOTINGS ARE TO BE ON ONLY UNDISTURBED SOIL.

3. ANY FILL UNDER SLABS TO BE A MIN. OF 4" GRANULAR FILL COMPACTED TO A MIN. OF 95%.

4. CONCRETE TO MEET FOLLOWING SPECS.:

   A) BASEMENT WALLS AND FOUNDATIONS EXPOSED TO EXTERIOR (2500 PSI)

   B) INTERIOR SLABS ON GRADE (2500 PSI)

   C) BASEMENT WALLS AND FOUNDATIONS EXPOSED TO INTERIOR (3000 PSI)

   D) PORCHES, STEPS, AND GRADE SLABS (3500 PSI)

5. CONCRETE SLABS TO HAVE CONTROL JOINTS NO MORE THAN 25' IN SPAN ANY DIRECTION.

6. COVER ENTIRE CRAWLSPACE WITH 6 MIL BLK. POLY. VAPOR BARRIER.

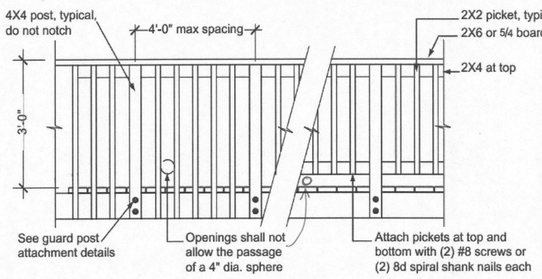
7. PROVIDE CRAWLSPACE DRAINS PER I.R.C. SECTION R405

8. REINFORCING STEEL IN FOUNDATION WALLS, FOOTINGS, RETAINING WALLS, AND PIERS WILL BE PROVIDED WHEN JOB SITE CONDITIONS AND OR CITY/COUNTY REQUIREMENTS WARRANT IT'S USE.

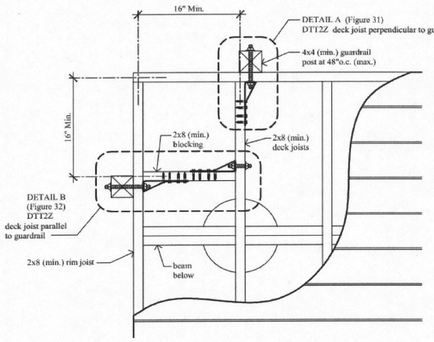
9. BEAM POCKETS TO HAVE 1/2" AIRSPACE AT ALL SIDES AND BEAM MUST HAVE A MIN. OF 3" BEARING.

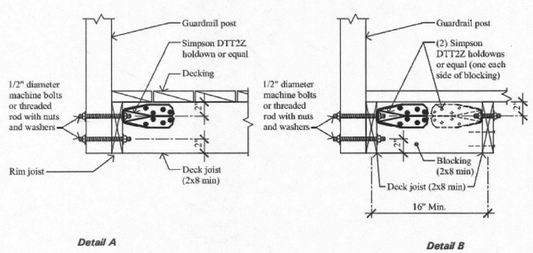
10. ALL WOOD IN CONTACT WITH CONCRETE TO BE PRESSURE TREATED.

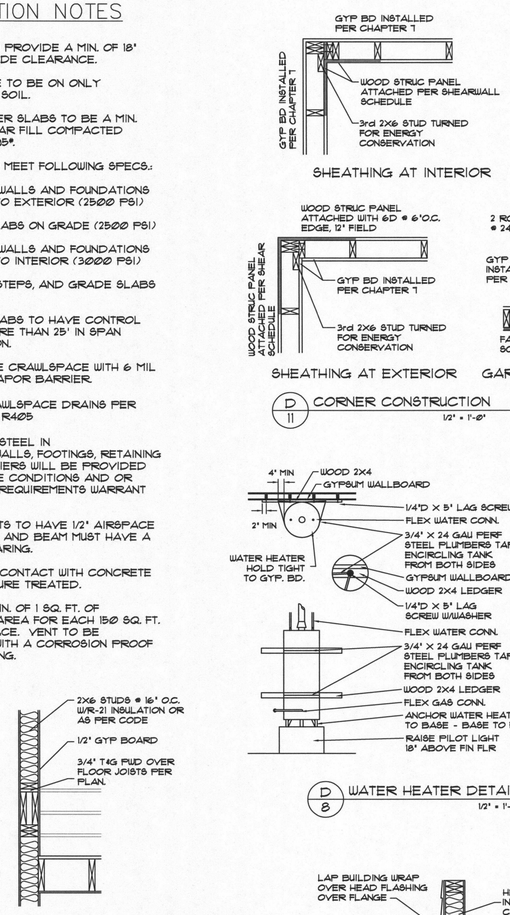
11. PROVIDE A MIN. OF 1 SQ. FT. OF VENTILATION AREA FOR EACH 150 SQ. FT. OF CRAWLSPACE. VENT TO BE CLOSEABLE WITH A CORROSION PROOF MESH COVERING.



D 12 DECK RAIL DETAILS 1/2" = 1'-0"

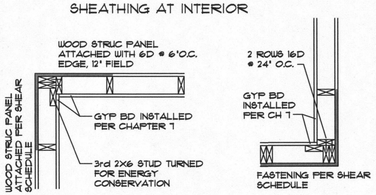






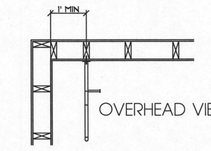
D 5B STAIR WALL WALL FRAMING AT EXTERIOR 1/2" = 1'-0"

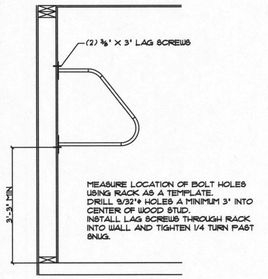
SHEATHING AT INTERIOR



SHEATHING AT EXTERIOR GARAGE CORNER

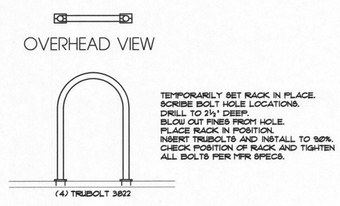
D 11 CORNER CONSTRUCTION 1/2" = 1'-0"





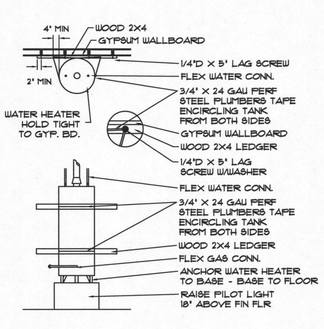
http://www.dero.com/product/ultra-space-saver-single/

D 14 INTERIOR BIKE RACK DETAIL 1/2" = 1'-0"

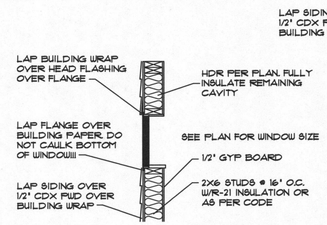


https://www.theparkcatalog.com/u-bike-rack

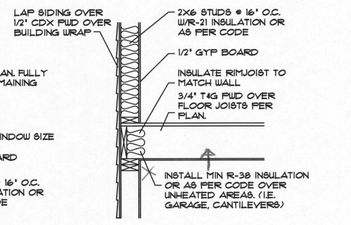
D 13 EXTERIOR BIKE RACK DETAIL 1/2" = 1'-0"



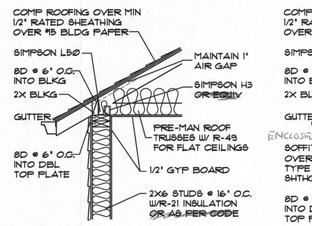
D 8 WATER HEATER DETAIL 1/2" = 1'-0"



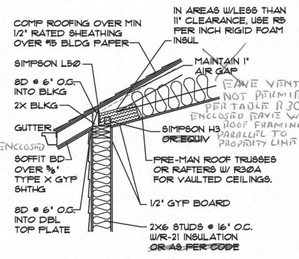
D 4 WINDOW AND DOOR DETAIL 1/2" = 1'-0"



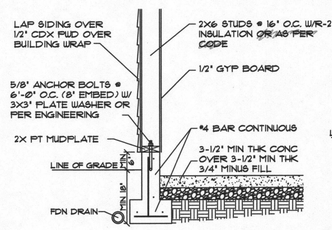
D 5 FRAMED FLOOR PLENUM DETAIL 1/2" = 1'-0"



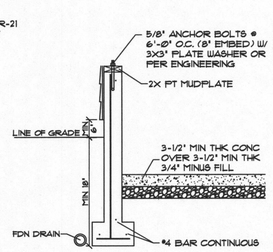
D 6 ROOF TRUSS DETAIL 1/2" = 1'-0"



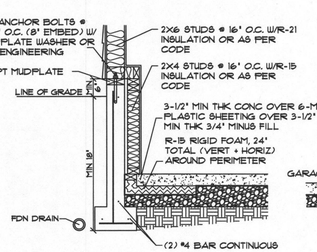
D 6A ROOF TRUSS DETAIL 1/2" = 1'-0"



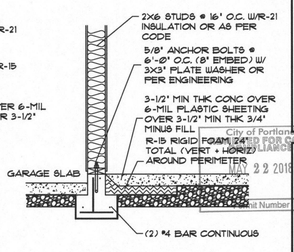
D 3 GARAGE SLAB DETAIL 1/2" = 1'-0"



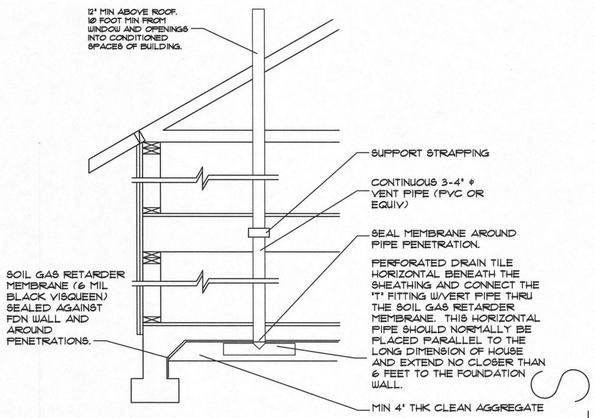
D 3A BURIED SLAB DETAIL 1/2" = 1'-0"



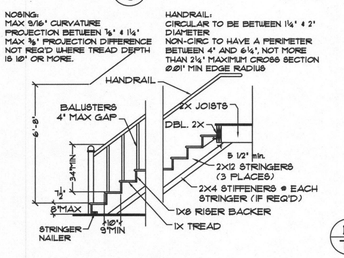
D 3B WARM SLAB DETAIL 1/2" = 1'-0"



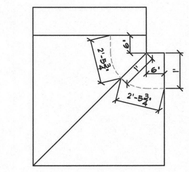
D 3C WALL BETWEEN HOUSE AND GAR 1/2" = 1'-0"



D 9 RADON MITIGATION DETAIL 1/2" = 1'-0"



D 7A STAIR DETAIL 1/2" = 1'-0"



D 7 WINDER DETAIL 1/2" = 1'-0"

DISCLAIMER

ALL WORK IS TO COMPLY WITH THE LATEST ADOPTED VERSION OF THE INTERNATIONAL RESIDENTIAL CODE OR APPLICABLE STATE, COUNTY, OR LOCAL REGULATIONS. THE CONTRACTOR IS RESPONSIBLE TO CHECK THE PLANS AND TO NOTIFY THE DESIGNER OF ANY ERRORS IN THE PLANS PRIOR TO THE START OF CONSTRUCTION. THE DESIGNER CAN NOT BE HELD RESPONSIBLE FOR MISINTERPRETATION OR MISUSE OF INFORMATION HEREIN, OR USE BY PEOPLE NOT KNOWLEDGEABLE OF BUILDING CONSTRUCTION AND IT'S DERIVATION AND IMPLEMENTATION.

REFERENCE THE COMPLETE CODE FROM YOUR SMART PHONE! VISIT HTTP://ORSC.ISTOCKHOUSEPLANS.COM

NOTES & DETAILS

SCALE: 1/4" = 1'-0"

THESE PLANS ARE NOT TO BE USED FOR CONSTRUCTION UNLESS THEY BEAR THE CITY OF PORTLAND STAMP "APPROVED FOR PERMIT". THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL CONSTRUCTION AND SHALL VERIFY ALL DIMENSIONS AND CONDITIONS ON THE JOB. ANY DISCREPANCIES SHALL BE REPORTED TO THE ARCHITECT BEFORE PROCEEDING WITH THE WORK. ALL WORK SHALL COMPLY WITH ALL APPLICABLE CODES AND REGULATIONS. THE ARCHITECT SHALL NOT BE RESPONSIBLE FOR CONSTRUCTION MEANS, METHODS, TECHNIQUES, SEQUENCES OR PROCEDURES, OR FOR SAFETY PRECAUTIONS AND PROGRAMS IN CONNECTION WITH THE WORK. THESE DRAWINGS ARE NOT TO BE USED FOR ANY WORK OTHER THAN SHOWN HEREIN.

City of Portland, Oregon - FOR PERMIT
BUREAU OF DEVELOPMENT SERVICES
MAY 22 2018
Permit Number:

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PLAN# _____
NAME NOTES
SHEET DETAILS
DATE 4/9/18

SHEET# NDA

City of Portland Bureau of Development Services
Approved by Planning and Zoning Review
By: Todd King Date: 5/16/18

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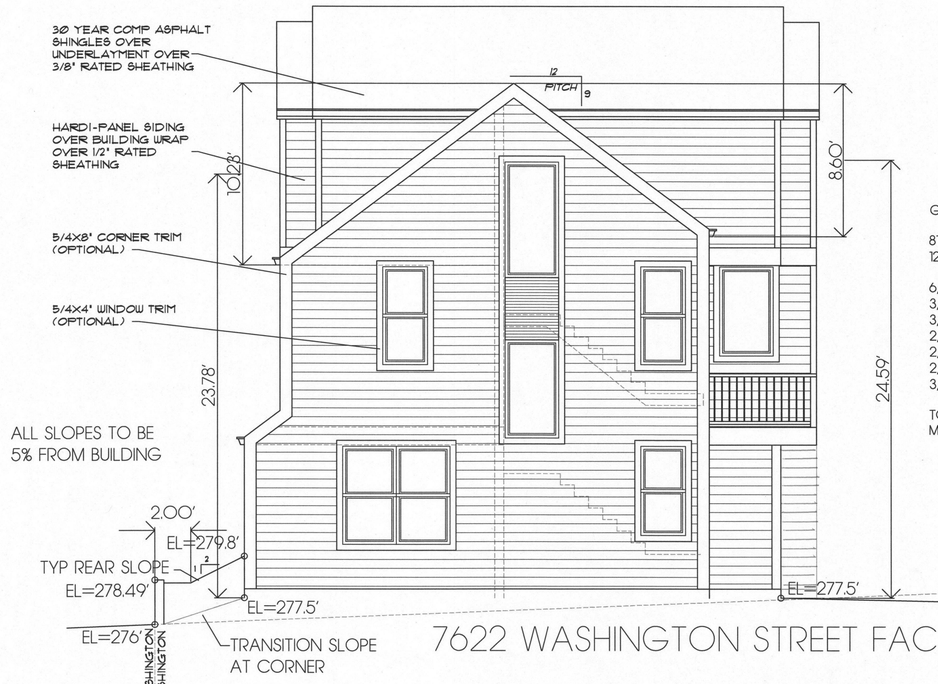


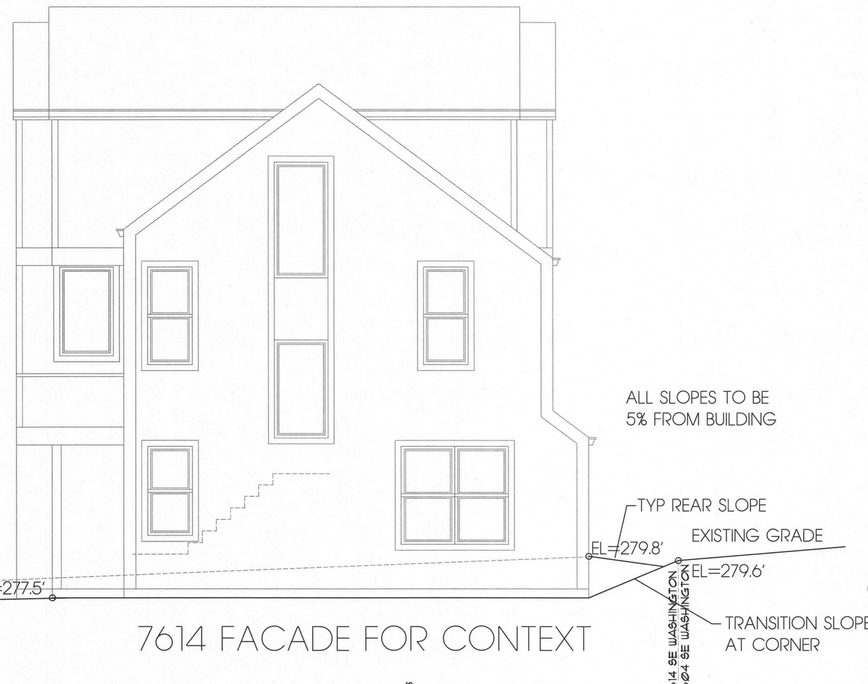
7622 WASHINGTON STREET FACADE

GLAZING ASSESSMENT

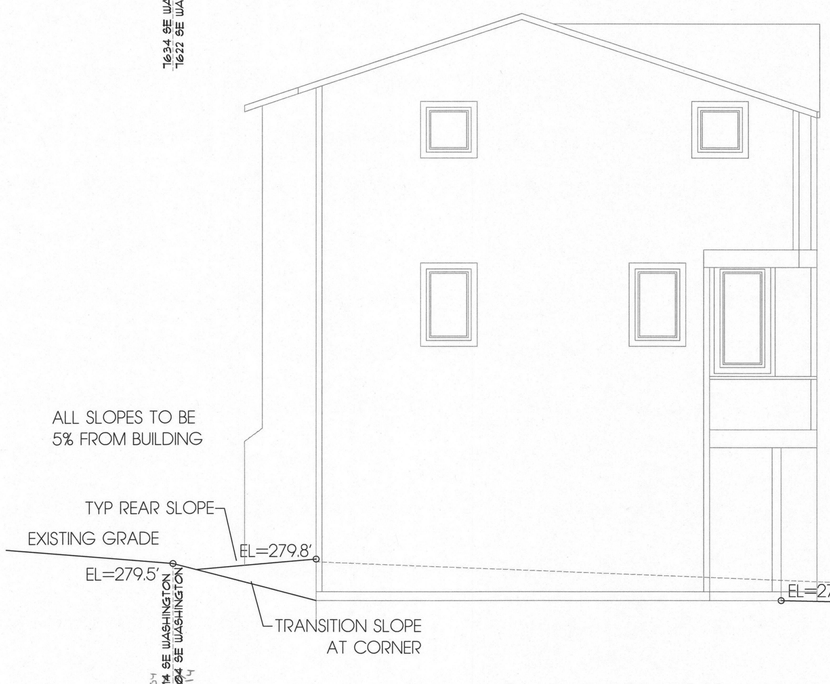
813SF X 15% =
121.95SF GLAZING

6/Ox5/6 = 33
3/Ox5/6 = 16.5
3/Ox6/6 = 19.5
2/6x5/6 = 13.75
2/6x5/6 = 13.75
2/6x5/O = 12.5
3/Ox5/O = 15

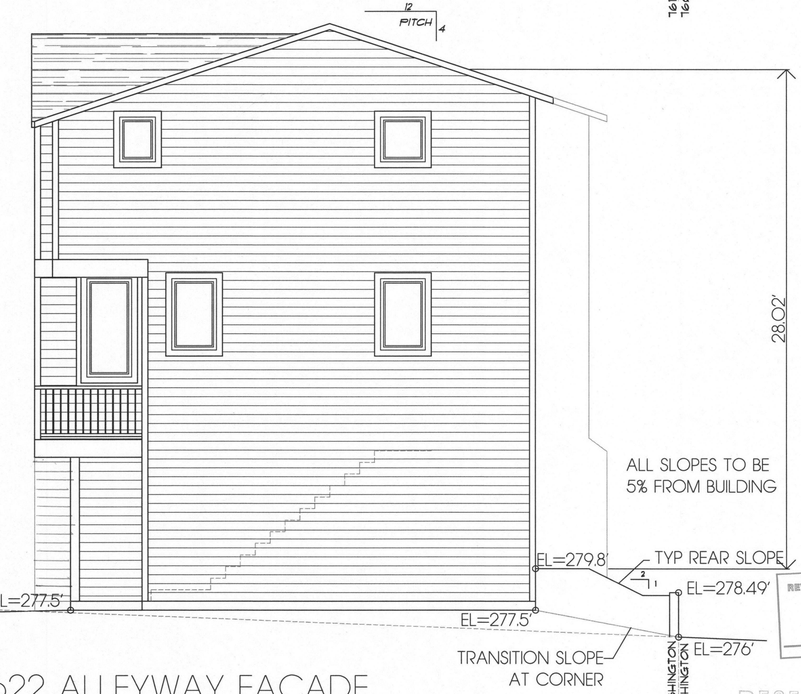
TOTAL = 124SF
MEETS



7614 FACADE FOR CONTEXT



7614 FACADE FOR CONTEXT



7622 ALLEYWAY FACADE

ELEVATIONS

SCALE: 1/4" = 1'

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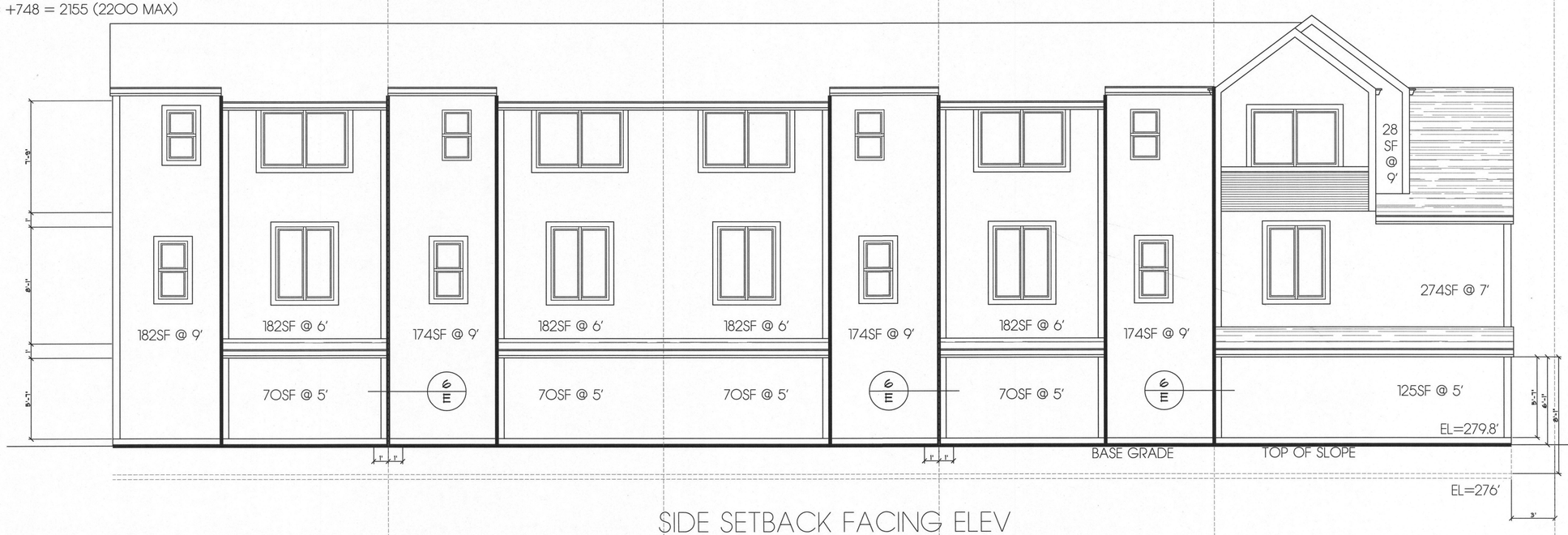
PLAN# 1622 8E
NAME WASHINGTON
SHEET ELEVATIONS
DATE 1/30/18
SHEET# 1.1

DRIVEWAY FACING ELEV

@ 5' = 405SF (1000 MAX)
@ 6' = +728 = 1133 (1300 MAX)
@ 7' = +274 = 1407 (1600 MAX)
@ 8' = +0 = 1407 (1900 MAX)
@ 9' = +748 = 2155 (2200 MAX)



SIDE SETBACK FACING ELEV

ELEVATIONS

SCALE: 1/4" = 1'

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PLAN# 1622 BE
NAME WASHINGTON
SHEET ELEVATIONS
DATE 1/30/18
SHEET# 1.2B

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PCC 24.10.080 Fills:

B. Ground preparation. The ground surface shall be prepared to receive fill by removing vegetation, noncomplying fill, top-soil, and other unsuitable materials scarifying to provide a bond with the new fill, and where slopes are steeper than 5 to 1, and the height greater than 5 feet, by benching into competent material or sound bedrock as determined by the soils engineer. The bench under the toe of a fill on a slope steeper than 5 to 1 shall be at least 10 feet wide. The area beyond the toe of fill shall be sloped for sheet overflow or a paved drain shall be provided. Where fill is to be placed over a cut the bench under the toe of a fill shall be at least 10 feet wide but the cut must be made before placing fill and approved by the soils engineer and engineering geologist as a suitable foundation for fill. Unsuitable soil is soil which in the opinion of the Director or the civil engineer or the soils engineer or the engineering geologist, is not competent to support either soil or fill, to support structures or to satisfactorily perform the other functions for which the soil is intended.

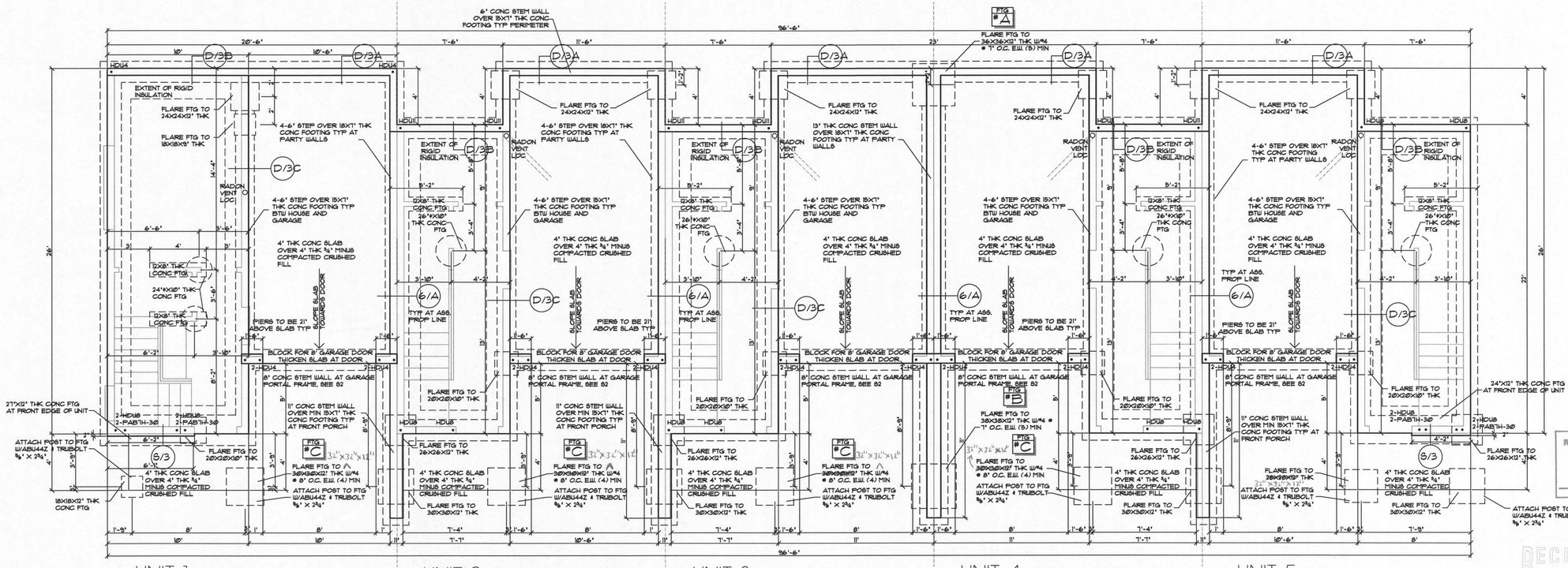
C. Fill material. Only permitted material free from tree stumps, detrimental amounts of organic matter, trash, garbage, sod, peat, and similar materials shall be used. Rocks larger than 6 inches in greatest dimension shall not be used unless the method of placement is properly devised, continuously inspected, and approved by the Director.

The following shall also apply:

1. Rock sizes greater than 6 inches in maximum dimension shall be 10 feet or more below grade, measured vertically.

2. Rocks shall be placed so as to assure filling all voids with fines. Topsoil may be used in the top 12-inch surface layer to aid in planting and landscaping.

D. Compaction of fill. All fills shall be compacted to a minimum relative dry density of 90 percent as determined in accordance with ASTM Standard D-1557-78. Field density verification shall be determined in accordance with ASTM Standard D-1556-82 or equivalent and must be submitted for any fill 12 inches or more in depth where such fill may support the foundation for a structure. A higher relative dry density, or additional compaction tests, or both, may be required at any time by the Director.



REDLINES PER BRIAN D'ACOSTINES' FOOTING CALCULATION 3-19-18

FOUNDATION

SCALE: 1/4" = 1'

ALL WORK SHALL BE CONSTRUCTED TO THE CODES IN EFFECT AT THE TIME OF PERMIT APPLICATION. ALL CONSTRUCTION, MATERIALS, AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE MINIMUM STANDARDS OF THE CODES AND ORDINANCES OF THE JURISDICTION HAVING AUTHORITY OVER THE PROJECT. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL CONSTRUCTION METHODS, TECHNIQUES, SEQUENCES AND PROCEDURES AND FOR COORDINATING ALL PORTIONS OF THE WORK UNDER THE CONTRACT. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ALL JOB SITE CONDITIONS DURING THE COURSE OF CONSTRUCTION, INCLUDING SAFETY OF ALL PERSONS AND PROPERTY. THIS REQUIREMENT SHALL APPLY CONTINUOUSLY AND NOT BE LIMITED TO NORMAL WORKING HOURS. THE CONTRACTOR SHALL DEFEND, INDEMNIFY AND HOLD THE OWNER AND ARCHITECT HARMLESS FROM ANY AND ALL LIABILITY, REAL OR ALLEGED, IN CONNECTION WITH THE PERFORMANCE OF WORK ON THIS PROJECT, EXCEPTING LIABILITY ARISING FROM THE SOLE NEGLIGENCE OF THE OWNER OR ARCHITECT.

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NAME WASHINGTON
SHEET FDN
DATE 4/9/18
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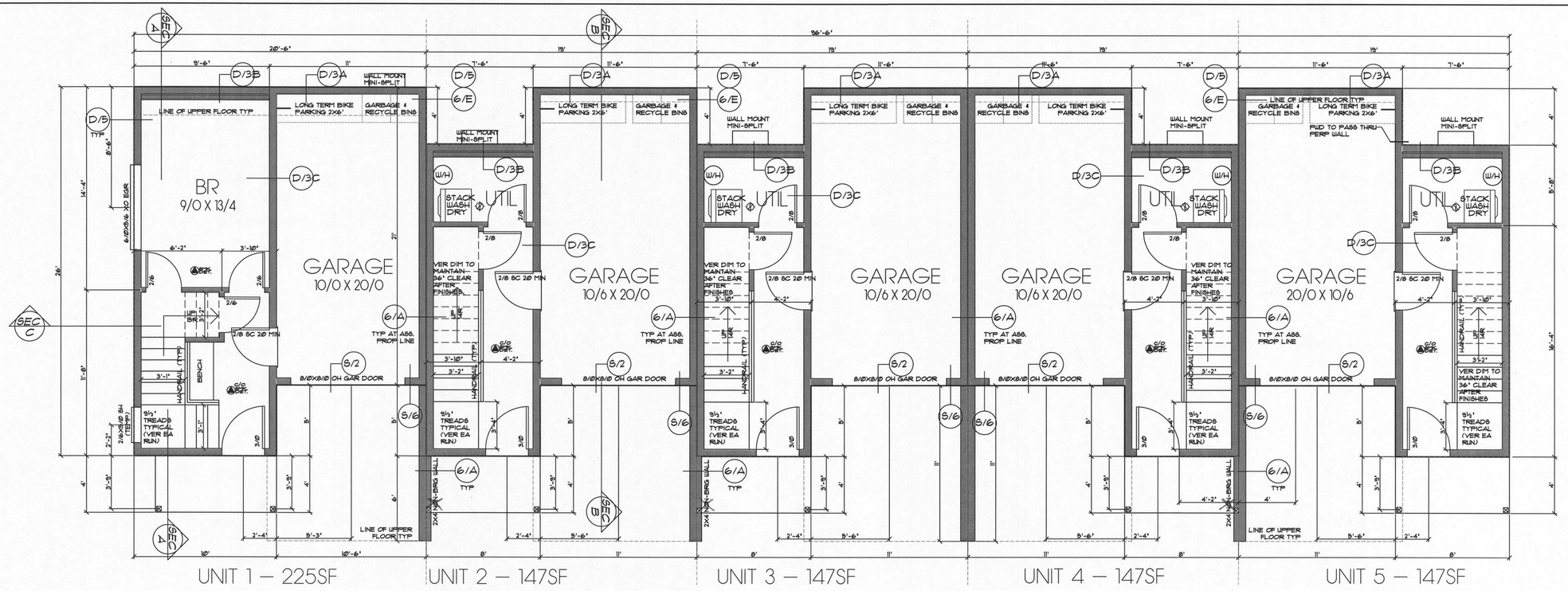
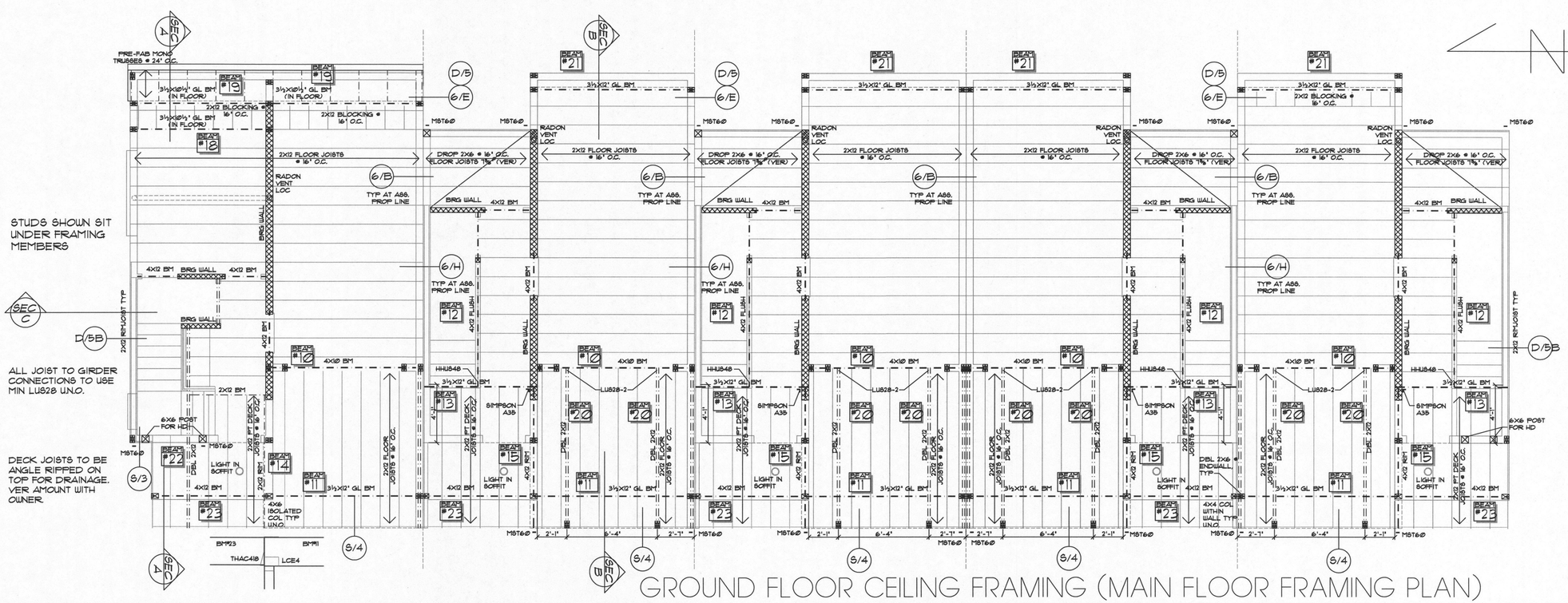
GROUND FLOOR & CEILING FRAMING

SCALE: 1/4" = 1'





GROUND FLOOR CEILING FRAMING (MAIN FLOOR FRAMING PLAN)

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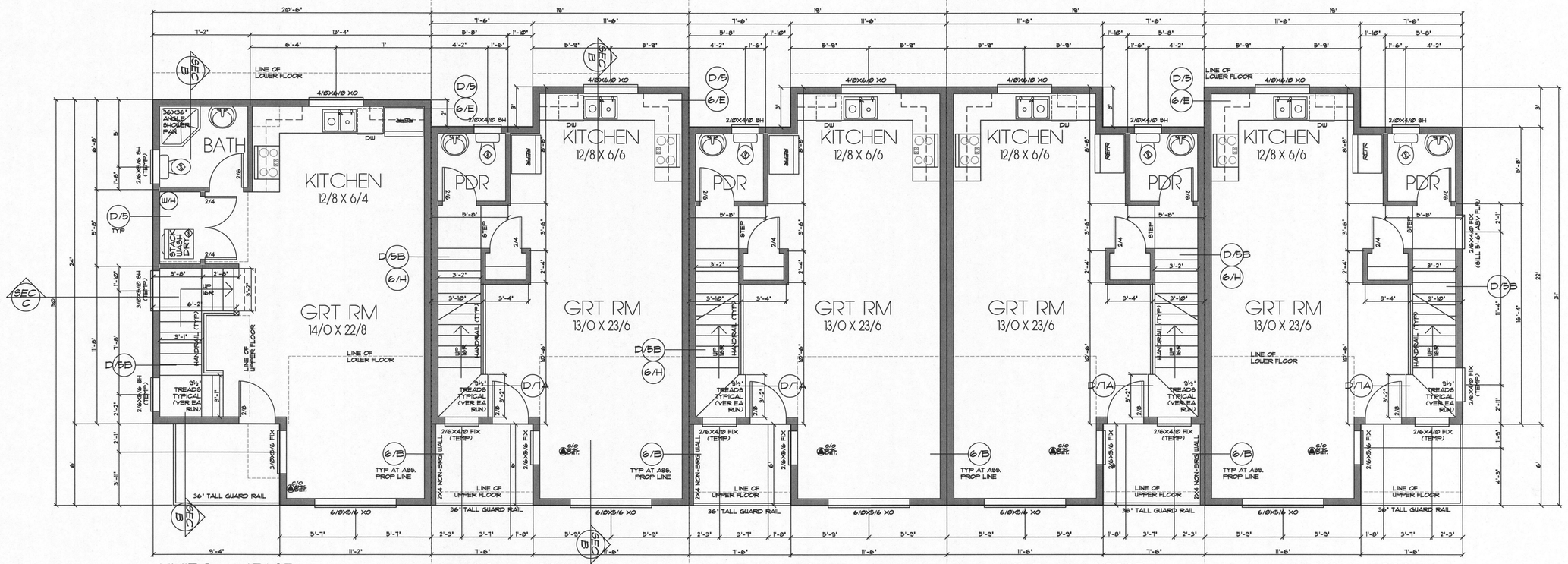
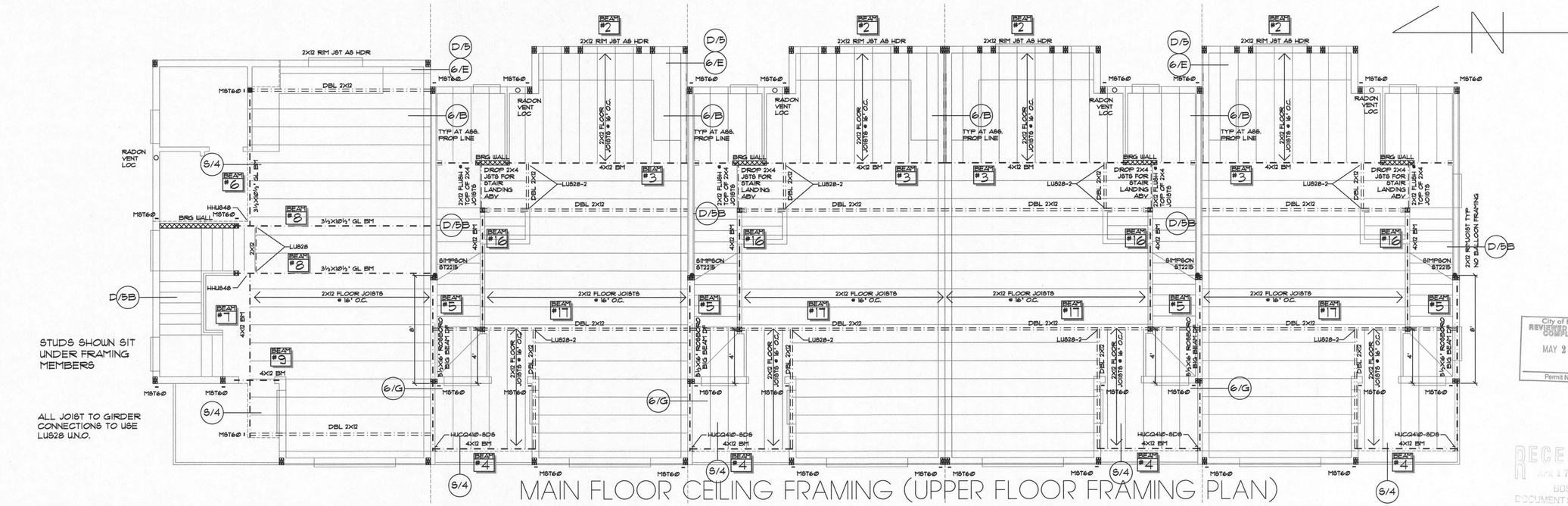
PLAN# T622 SE
NAME WASHINGTON
SHEET GRD FLR
DATE 4/9/18
SHEET# 3





MAIN FLOOR & CEILING FRAMING

SCALE: 1/4" = 1'

MAIN FLOOR CEILING FRAMING (UPPER FLOOR FRAMING PLAN)

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PLAN# T622 SE
NAME WASHINGTON
SHEET MAIN FLR
DATE 4/9/18
SHEET# 4B

UPPER FLOOR & ROOF FRAMING

SCALE: 1/4" = 1'

UNIT 1 – 353SF

UNIT 2 – 473SF

UNIT 3 – 473SF

UNIT 4 – 473SF

UNIT 5 – 473SF

FRONT ROOF OF UNIT 1:

172SF ROOF AREA
172 / 300 X 144 =
838CIN VENTILATION
50% = 419CIN
TOP VENTS = 50IN NFA
42/50 = 1 VENTS MIN
MIN 42NFA BIRD BLOCK VENTING

MAIN ROOF OF UNIT 1:

395SF ROOF AREA
395 / 300 X 144 =
1925CIN VENTILATION
50% = 955CIN
TOP VENTS = 50IN NFA
95/50 = 2 VENTS MIN
MIN 95NFA BIRD BLOCK VENTING

MAIN ROOF OF UNITS 2-5:

582SF ROOF AREA
395 / 300 X 144 =
2838CIN VENTILATION
50% = 1419CIN
TOP VENTS = 50IN NFA
142/50 = 3 VENTS MIN
MIN 140NFA BIRD BLOCK VENTING

STUDS SHOWN SIT UNDER FRAMING MEMBERS

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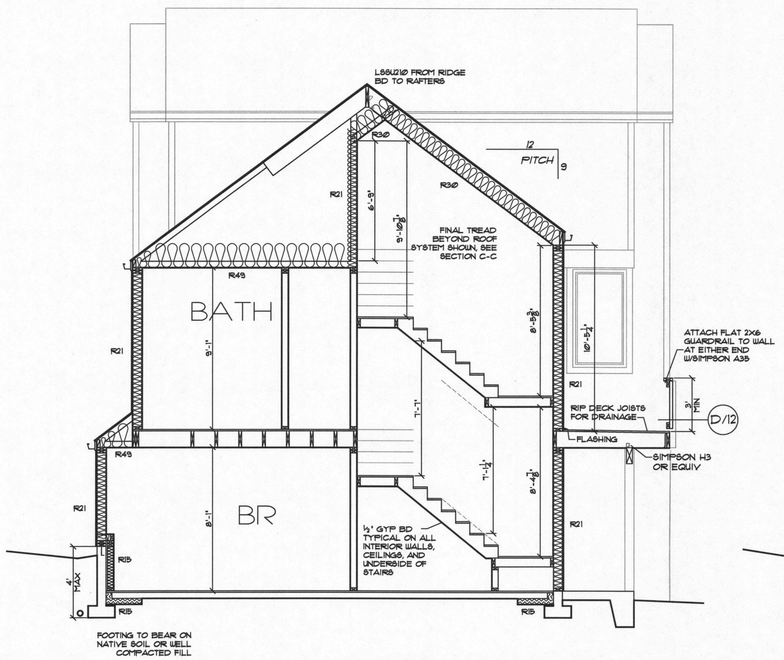
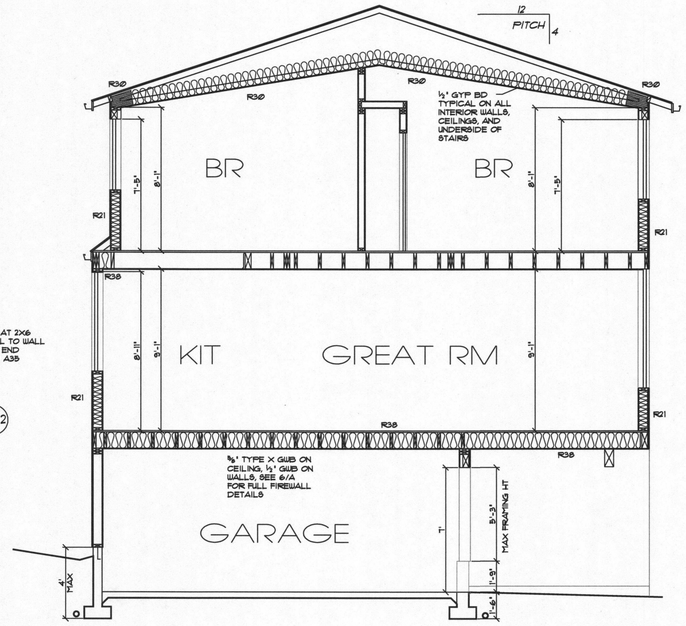
PLAN# 1622 BE
NAME WASHINGTON
SHEET UPPER/ROOF
DATE 4/9/18
SHEET# 5 B



SECTION A—A



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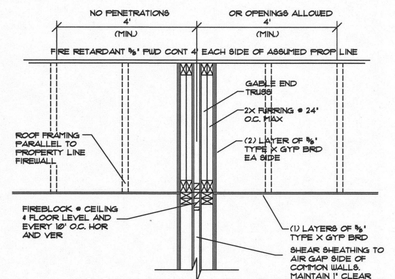
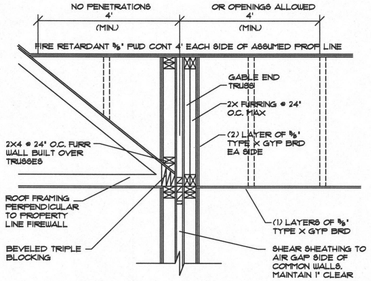


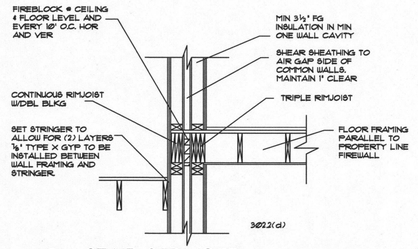
CONDITION WITH PARALLEL ROOFS AT ASSUMED PROPERTY LINE.

6/C NOT TO SCALE 3@22.(d)



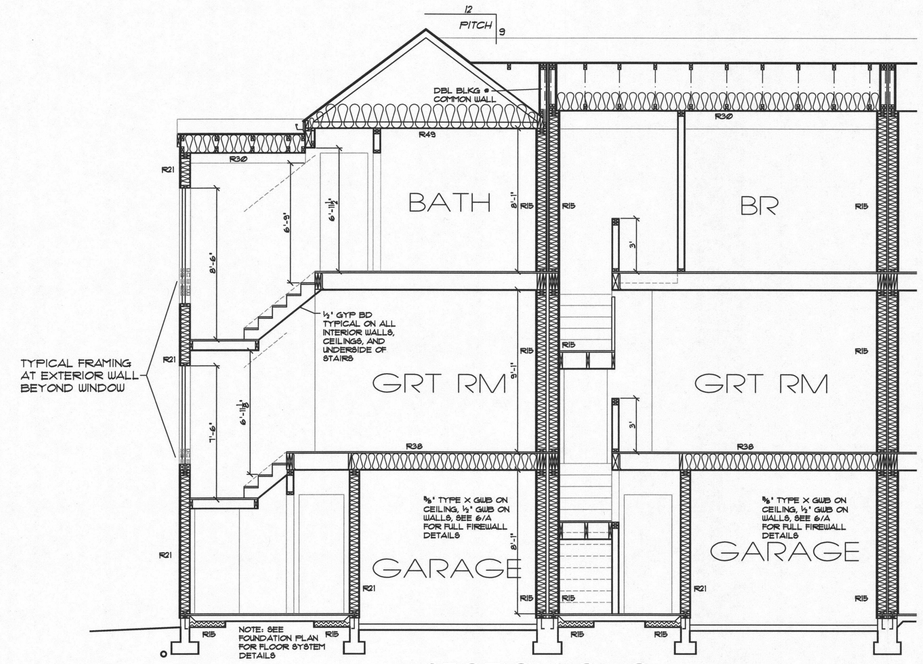
CONDITION WITH PARALLEL AND PERPENDICULAR ROOF AT ASSUMED PROPERTY LINE.

6/D NOT TO SCALE 3@22.4.3(c)



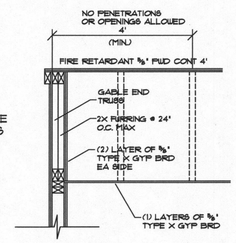
STAIR STRINGER

6/H NOT TO SCALE



SECTION C—C

SHEATHINGS ON BOTH SIDES TO BE CONTINUOUS THROUGH ALL LEVELS AND ATTIC TO THE UNDERSIDE OF ROOF DECKING.



GA FILE NO. WP 8109	PROPRIETARY*	1 HOUR FIRE

GYPSUM PANEL PRODUCTS, FIBER-CEMENT SIDING, WOOD STUDS

EXTERIOR SIDE: **Base** layer 5/8" proprietary type X gypsum sheathing or glass mat gypsum substrate applied parallel to 2 x 4 wood studs 16" o.c. with 1¼" galvanized roofing nails 4" o.c. at vertical joints and 7" o.c. at intermediate studs and top and bottom plates. Joints of gypsum sheathing may be left untreated. **Face** layer ¼" proprietary fiber-cement siding fastened through sheathing to studs. 3½" unfaced glass fiber friction fit in stud space.

INTERIOR SIDE: One layer 5/8" proprietary type X gypsum wallboard, glass mat gypsum substrate, water-resistant gypsum backing board, or gypsum veneer base applied parallel or at right angles to studs with 6d coated nails, 1⅞" long, 0.0915" shank, ¼" heads, 7" o.c. **(LOAD BEARING)**

PROPRIETARY GYPSUM PANEL PRODUCTS

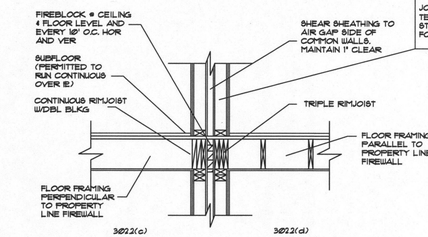
American Gypsum Company LLC	5/8" FireBloc® Type X 5/8" Exterior Sheathing Type X
CertainTeed Gypsum Inc.	5/8" CertainTeed® Type X Gypsum Board 5/8" GlasRoc® Sheathing Type X Gypsum Panels
Georgia Pacific Gypsum LLC	5/8" DensArmor Plus® Fiberguard® Interior Panel 5/8" DensGlass® Fireguard® Sheathing
Lafarge North America Inc.	5/8" Firecheck® Type X 5/8" Weather Defense® Platinum Sheathing
National Gypsum Company	5/8" Gold Bond® Brand FIRE-SHIELD® Gypsum Board 5/8" Gold Bond® Brand FIRE-SHIELD® Gypsum Sheathing
PABCO Gypsum	5/8" FLAME CURB® Type X 5/8" Exterior Gypsum Sheathing Type X
Temple-Inland	5/8" Type X 5/8" GreenGlass Type X

Thickness: 5⅛"
Approx. Weight: 9 psf
Fire Test: See WP 3510 (UL R3501-47, -48, 9-17-65, UL Design U309; UL R-1319-129, 7-22-70, UL Design U314)

EXTERIOR WALL PROTECTION

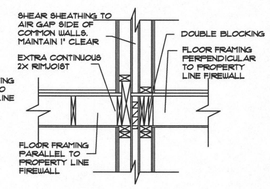
6/E NOT TO SCALE

RATED 2 HR FIREWALL GA FILE NO. WP3620 5/8 TO 5/8 STC 50@2



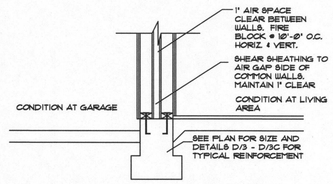
FRAMED FLOOR

6/B NOT TO SCALE



BEAM #5

6/G NOT TO SCALE



FOUNDATION

6/A NOT TO SCALE

BASE LAYER:
5/8" TYPE 'X' GYPSUM WALLBOARD APPLIED AT RIGHT ANGLES TO EACH SIDE OF DBL ROW OF 2X4 WOOD STUDS @ 16" C/C WITH 6d COATED NAILS, 2⅛" LONG, MIN Ø.2065 SHANK, ¼" HEAD, 6" C/C.

FACE LAYER:
5/8" TYPE 'X' GYPSUM WALLBOARD APPLIED AT RIGHT ANGLES TO EACH SIDE OF STUDS OVER BASE LAYER W/ 16d COATED NAILS, 3" LONG, MIN Ø.1620 SHANK, ¼" HEADS, 8" C/C. STAGGER JOINTS 16" C/C EACH LAYER AND SIDE. PROVIDE BLOCKING AT MID HEIGHT.

JOINTS STAGGERED 16" EA LAYER AND EA SIDE. SOUND TESTED WITH 3½" GLASS FIBER INSULATION STAPLED TO STUDS IN STUD SPACES ON ONE SIDE AND WITH NAILS FOR BASE LAYER SPACED 6" O.C.

NOTE:
2-HR RATED WALL ASSEMBLY WILL CONTINUE THROUGH THE FLOOR SYSTEM TO THE RATED ROOF. THE RATED WALL MUST BE CONTINUOUS BEHIND SHOWERS, TUBS, CABINETS, CASES, SOFFITS, ELECTRICAL PANELS, FIREPLACES, STAIR STRINGERS AND ADJOINING WALLS.

NO PENETRATIONS SHALL BE ALLOWED WITHIN THE WALL ASSEMBLY INCLUDING BUT NOT LIMITED TO ELECTRICAL, PLUMBING AND MECHANICAL DUCTS AND VENTS.

DETAILS 6/A THROUGH 6/H ARE A PART OF ASSUMED PROPERTY LINE FIREWALL MODIFIED 2-HR APPROACH

CROSS SECTIONS

SCALE: 1/4" = 1'

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PLAN# T622 SE
NAME WASHINGTON
SHEET SECTIONS
DATE 4/9/18
SHEET# 6 B

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SHEARWALL SHEATHING SCHEDULE

TYPE	THICKNESS	EDGE NAILING	SILL TO FLOOR	SILL TO FOUNDATION
A	15/32"	8D @ 6" O.C.	SDS25312 @ 16" O.C.	5/8"ø A.B. @ 36" O.C.
B	15/32"	8D @ 4" O.C.	(2) SDS25312 @ 16" O.C.	5/8"ø A.B. @ 24" O.C.
C	15/32"	8D @ 3" O.C.	(2) SDS25312 @ 12" O.C.	5/8"ø A.B. @ 18" O.C.

SHEARWALL HOLDDOWN SCHEDULE

#	TYPE
O	NO HOLDDOWN REQ'D
1	SIMPSON MST6O

MST6O – FILL 23 HOLES TOP AND 23 HOLES BOTTOM w/ 16d

#	TYPE	AB DIA	COL
2	SIMPSON HDU2	5/8"	2–2X
3	SIMPSON HDU4	5/8"	2–2X
4	SIMPSON HDU8	7/8"	2–2X
5	SIMPSON HDU11	1"	6X

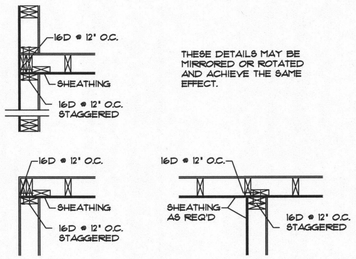
ALL HDU FILL ALL HOLES w/ 1/4" x 2 1/2" SDS

GENERAL SHEATHING NOTES

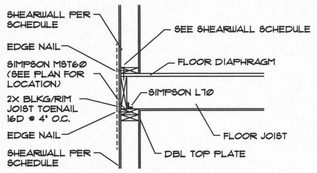
ROOF DIAPHRAGM:
MIN 1/2" CDX PWD (48/24) W/
8D @ 6" O.C. EDGES
8D @ 12" O.C. FIELD

FLOOR DIAPHRAGM:
MIN 3/4" CDX PWD (32/16) W/
10D @ 6" O.C. EDGES
10D @ 12" O.C. FIELD

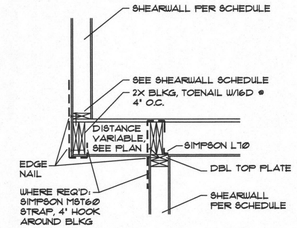
SHEATHING — SHEAR WALL TYPE
SHEAR WALL — HOLDDOWN TYPE



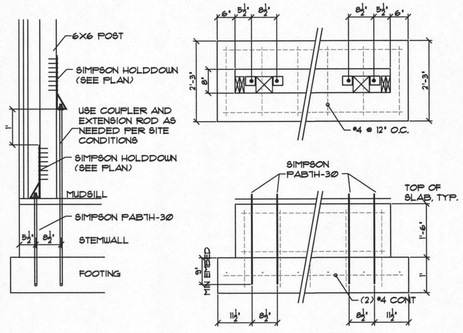
S/6 TYPICAL THROUGH SHEATHING DETAIL 1/2" = 1'-0"



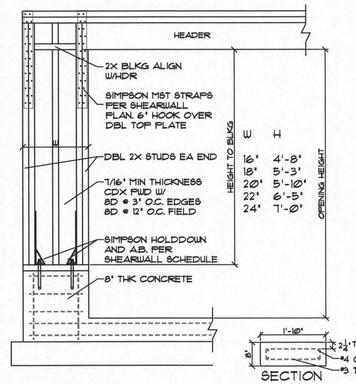
S/5 SHEARWALL TO SHEARWALL CONNECTION 1/2" = 1'-0"



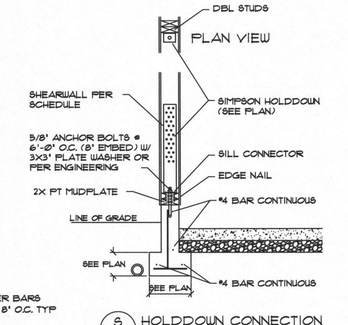
S/4 SHEAR STRAP @ CANTILEVER 1/2" = 1'-0"



S/3 DOUBLE HOLDDOWN DETAIL 1/2" = 1'-0"



S/2 PORTAL FRAME 1/2" = 1'-0"



S/1 HOLDDOWN CONNECTION 1/2" = 1'-0"

GROUND FLOOR

WALL *, SEE CALC PACKET
L = FULL WALL LENGTH
EL = EFFECTIVE WALL LENGTH (EXCLUDES PERFORATIONS)

STUDS TO BE 16" O.C U.N.O.

DBL STUDS AT EACH SHEARWALL END AND HD U.N.O.

EXTEND SHEARWALLS TO ROOF DIAPHRAGM

EACH UNIT STRUCTURALLY INDEPENDENT

SHEAR ENGINEERING

SCALE: 1/4" = 1'

THE REVIEW OF THESE PLANS PERTAINS ONLY TO THE CODE REQUIREMENTS SPECIFICALLY IDENTIFIED BY THE BUREAU OF DEVELOPMENT SERVICES AND DOES NOT INCLUDE REVIEW OF ALL APPLICABLE CODE REQUIREMENTS. THE APPROVAL OF THESE PLANS DOES NOT PERMIT THE VIOLATION OF ANY CODE, REGULATION OR ORDINANCE. THE BUILDING PERMIT SHALL BE VOID IF ANY CODE VIOLATIONS EXIST. IT IS THE RESPONSIBILITY OF THE PERMITTEE TO COMPLY WITH ALL APPLICABLE CODES AND REGULATIONS. THE BUREAU OF DEVELOPMENT SERVICES IS ENFORCING OREGON REVISED STATUTE 455.

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PLAN# 1622 SE
NAME WASHINGTON
SHEET SHEAR
DATE 4/9/18
SHEET# S1

EACH UNIT
STRUCTURALLY
INDEPENDENT

MAIN FLOOR

WALL *, SEE CALC PACKET
L = FULL WALL LENGTH
EL = EFFECTIVE WALL LENGTH
(EXCLUDES PERFORATIONS)

STUDS TO BE 16" O.C
U.N.O.

DBL STUDS AT
EACH SHEARWALL
END AND HD U.N.O.

EXTEND
SHEARWALLS TO
ROOF DIAPHRAGM

WALL 15
L = 20.5'
EL = 16.5'

WALL 14
L = 24'
EL = 15.5'

WALL 16
L = 30'

WALL 18
L = 9.33'
EL = 6.5'

WALL 17
L = 11.17'
EL = 5'

WALL 20
L = 7.5'
EL = 5.5'

WALL 21
L = 11.5'
EL = 7.5'

WALL 19
L = 22'

WALL 22
L = 31'

WALL 23
L = 11.5'
EL = 5.5'

WALL 24
L = 22'
EL = 15'

MST60

UPPER FLOOR

WALL 3a
L = 14'

WALL 2
L = 7'

WALL 7
L = 11.17'
EL = 5'

WALL 4
L = 30'

WALL 5
L = 11.17'
EL = 5'

WALL 6
L = 7'

WALL 9
L = 7.5'
EL = 5.5'

WALL 8
L = 27'

WALL 11
L = 31'

WALL 10
L = 11.5'
EL = 5.5'

WALL 12b
L = 19'
EL = 10.5'

WALL 12a
L = 19'
EL = 10.5'

WALL 1
L = 12'
EL = 8.5'

WALL 3b
L = 14'

WALL 13
L = 27'
EL = 22.5'

SHEAR ENGINEERING

SCALE: 3/16" = 1'

City of Portland
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PLAN# 1622 SE
NAME WASHINGTON
SHEET SHEAR
DATE 4/9/18
SHEET# S2

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GENERAL NOTES

1. DESIGN LOADS:

ROOF	25 PSF (LIVE LOAD)
FLOOR	40 PSF
STAIRS	100 PSF
GARAGE FLOOR	50 PSF
DECKS	60 PSF

2. INSULATION:

   A) CEILINGS (VAULTED) R-30A
   B) CEILINGS (FLAT) R-49
   C) WALLS R-21
   D) FLOOR (OVER UNHEATED AREA) R-38

   THE ABOVE VALUES ARE MINIMUM AND MAY BE INCREASED IF DESIRED. ALL EXPOSED INSULATION IS TO HAVE A FLAME SPREAD RATING OF LESS THAN 25 AND A SMOKE DESTINY RATING OF LESS THAN 450. PROVIDE INSULATION BAFFLES AT EAVE VENTS BETWEEN RAFTERS.

3. WINDOWS:

   A) EACH BEDROOM TO HAVE A MIN. WINDOW OPENING OF 5.7 SQ. FT. WITH A MIN. WIDTH OF 20", A MIN. HEIGHT OF 22" AND A SILL LESS THAN 44" ABOVE FIN. FLR.

   B) ALL EXTERIOR WINDOWS TO BE DOUBLE GLAZED. U=0.32

   C) ALL WINDOWS WITHIN 18" OF THE FLOOR AND WITHIN 12" OF ANY DOOR TO BE TEMPERED.

4. ALL ELECTRICAL OUTLETS IN GARAGES, BATHROOMS, AND KITCHENS TO BE G.F.I. PER ELECTRICAL CODE STANDARDS.

5. ALL BATHROOMS AND UTILITY ROOMS TO BE PROVIDED WITH A FAN VENTED TO THE OUTSIDE, WITH A MIN. OF 5 AIR CHANGES PER HOUR. THIS ALSO INCLUDES RANGE HOODS.

FRAMING NOTES

1. LUMBER SPECIES:

   A) POSTS, BEAMS, HEADERS, RAFTERS, AND JOISTS - NO. 2 D.F.L.

   B) STUDS - STUD GRADE D.F.L.

   C) SILLS, PLATES, BLOCKING, BRIDGING, ETC.. - NO. 3 D.F.L.

   D) POST AND BEAM DECKING TO BE UTILITY GRADE D.F.L.

   E) SHEATHING - 1/2" CDX PWD 32/16 OR 7/16" O.S.B.
   NAILING: 8d 6" O.C. EDGES, 12" O.C. FIELD

2. ALL EXTERIOR HEADERS LESS THAN 5'-0" ARE TO BE 4 X 8. ALL HEADERS 5'-0" TO 8'-0" ARE TO BE 4 X 12. GARAGE HEADER TO BE 4 X 12. HEADERS MAY ALSO BE DESIGNED.

3. PROVIDE DOUBLE JOIST UNDER ALL PARTITION WALLS ABOVE RUNNING PARALLEL TO JOIST.

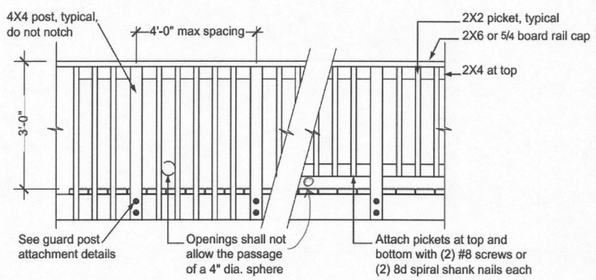
4. ATTIC ACCESS MUST BE 22" x 30" MIN. WITH 30" OF HEADROOM DIRECTLY ABOVE OPENING. ATTIC VENTS TO BE A MIN. OF 1/600 OF ATTIC AREA, 1/2 TO BE ROOF VENT OTHERS TO BE EAVE VENTS. EAVE VENTS TO BE COVERED WITH MIN. 1/4" CORROSION PROOF WIRE MESH.

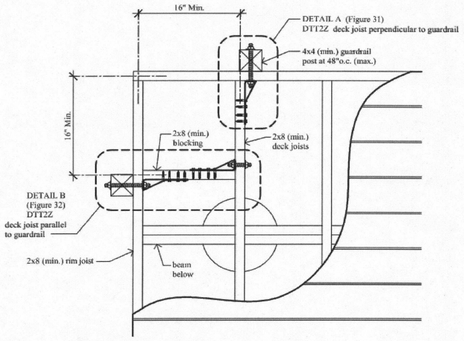
5. ALL EXTERIOR POST AND BEAMS WILL BE PROVIDED WITH APPROVED PAINTED STEEL ANCHORS AND/OR CONNECTORS.

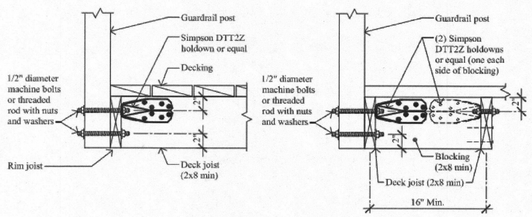
6. ALL STAIRS SHALL BE FURNISHED WITH RAILINGS OR BARRIERS WHICH ARE CONSTRUCTED SO THAT A 4"ø OBJECT CAN NOT PASS THROUGH AND ARE TO BE A MIN. OF 34" HIGH (36" AT LANDINGS). ALL DECKS 30" ABOVE GRADE MUST ALSO MEET THESE REQUIREMENTS.

FOUNDATION NOTES

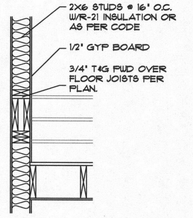
1. EXCAVATE TO PROVIDE A MIN. OF 18" BEAM TO GRADE CLEARANCE.
2. FOOTINGS ARE TO BE ON ONLY UNDISTURBED SOIL.
3. ANY FILL UNDER SLABS TO BE A MIN. OF 4' GRANULAR FILL COMPACTED TO A MIN. OF 95%.
4. CONCRETE TO MEET FOLLOWING SPECS.:
   A) BASEMENT WALLS AND FOUNDATIONS EXPOSED TO EXTERIOR (2500 PSI)
   B) INTERIOR SLABS ON GRADE (2500 PSI)
   C) BASEMENT WALLS AND FOUNDATIONS EXPOSED TO INTERIOR (3000 PSI)
   D) PORCHES, STEPS, AND GRADE SLABS (3500 PSI)
5. CONCRETE SLABS TO HAVE CONTROL JOINTS NO MORE THAN 25' IN SPAN ANY DIRECTION.
6. COVER ENTIRE CRAWLSPACE WITH 6 MIL BLK. POLY. VAPOR BARRIER.
7. PROVIDE CRAWLSPACE DRAINS PER I.R.C. SECTION R405
8. REINFORCING STEEL IN FOUNDATION WALLS, FOOTINGS, RETAINING WALLS, AND PIERS WILL BE PROVIDED WHEN JOB SITE CONDITIONS AND/OR CITY/COUNTY REQUIREMENTS WARRANT IT'S USE.
9. BEAM POCKETS TO HAVE 1/2" AIRSPACE AT ALL SIDES AND BEAM MUST HAVE A MIN. OF 3" BEARING.
10. ALL WOOD IN CONTACT WITH CONCRETE TO BE PRESSURE TREATED.
11. PROVIDE A MIN. OF 1 SQ. FT. OF VENTILATION AREA FOR EACH 150 SQ. FT. OF CRAWLSPACE. VENT TO BE CLOSEABLE WITH A CORROSION PROOF MESH COVERING.



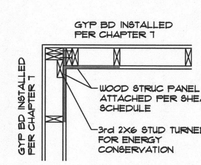




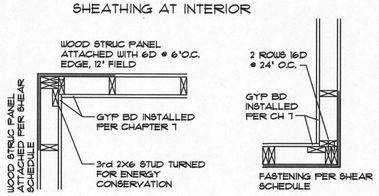
D 12 DECK RAIL DETAILS 1/2" = 1'-0"



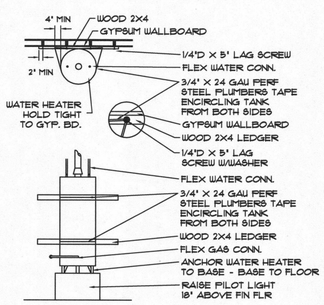
D 5B WALL FRAMING AT EXTERIOR STAIR WALL 1/2" = 1'-0"



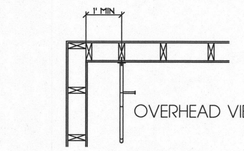
SHEATHING AT INTERIOR

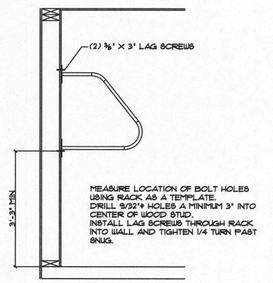


D 11 CORNER CONSTRUCTION 1/2" = 1'-0"



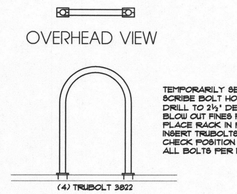
D 8 WATER HEATER DETAIL 1/2" = 1'-0"





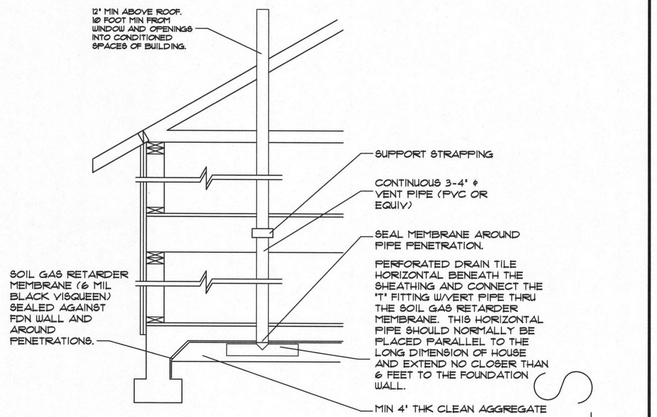
http://www.dero.com/product/ultra-space-saver-single/

D 14 INTERIOR BIKE RACK DETAIL 1/2" = 1'-0"

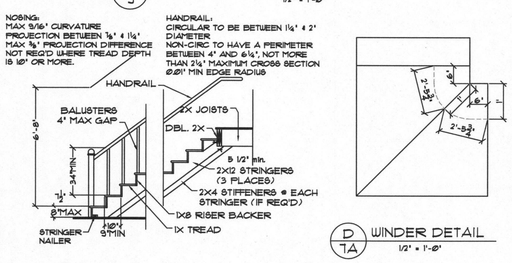


https://www.theparkcatalog.com/u-bike-rack

D 13 EXTERIOR BIKE RACK DETAIL 1/2" = 1'-0"

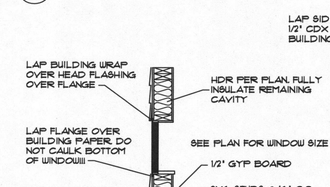


D 9 RADON MITIGATION DETAIL 1/2" = 1'-0"

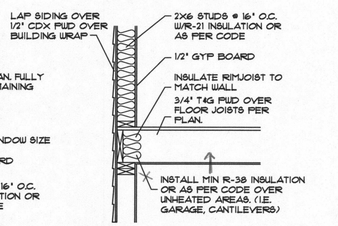


D 7 STAIR DETAIL 1/2" = 1'-0"

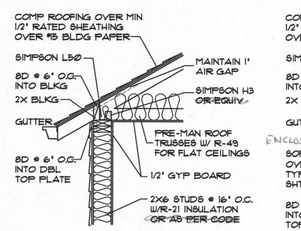
D 7A WINDER DETAIL 1/2" = 1'-0"



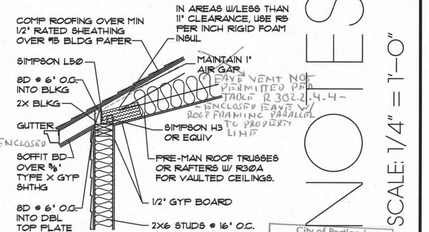
D 4 WINDOW AND DOOR DETAIL 1/2" = 1'-0"



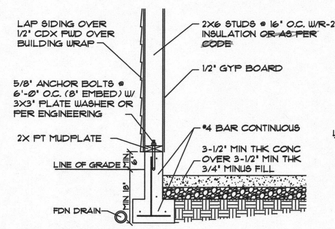
D 5 FRAMED FLOOR PLENUM DETAIL 1/2" = 1'-0"



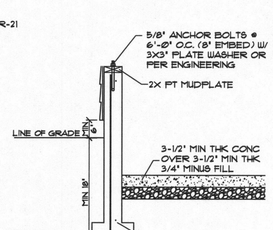
D 6 ROOF TRUSS DETAIL 1/2" = 1'-0"



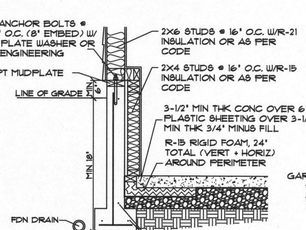
D 6A ROOF TRUSS DETAIL 1/2" = 1'-0"



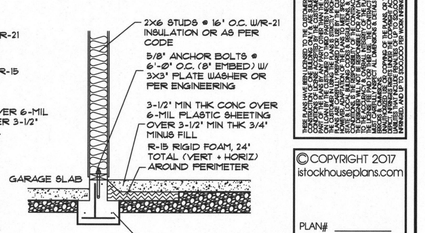
D 3 GARAGE SLAB DETAIL 1/2" = 1'-0"



D 3A BURIED SLAB DETAIL 1/2" = 1'-0"



D 3B WARM SLAB DETAIL 1/2" = 1'-0"



D 3C WALL BETWEEN HOUSE AND GAR 1/2" = 1'-0"

DISCLAIMER

ALL WORK IS TO COMPLY WITH THE LATEST ADOPTED VERSION OF THE INTERNATIONAL RESIDENTIAL CODE OR APPLICABLE STATE, COUNTY, OR LOCAL REGULATIONS. THE CONTRACTOR IS RESPONSIBLE TO CHECK THE PLANS AND TO NOTIFY THE DESIGNER OF ANY ERRORS IN THE PLANS PRIOR TO THE START OF CONSTRUCTION. THE DESIGNER CAN NOT BE HELD RESPONSIBLE FOR MISINTERPRETATION OR MISUSE OF INFORMATION HEREIN, OR USE BY PEOPLE NOT KNOWLEDGEABLE OF BUILDING CONSTRUCTION AND IT'S DERIVATION AND IMPLEMENTATION.

REFERENCE THE COMPLETE CODE FROM YOUR SMART PHONE! VISIT HTTP://ORSC.ISTOCKHOUSEPLANS.COM

DETAILS & NOTES

SCALE: 1/4" = 1'-0"

City of Portland REVIEWED FOR CODE COMPLIANCE MAY 22 2018

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PLAN# ____
NAME NOTES
SHEET DETAILS
DATE 4/9/18
SHEET# ND13

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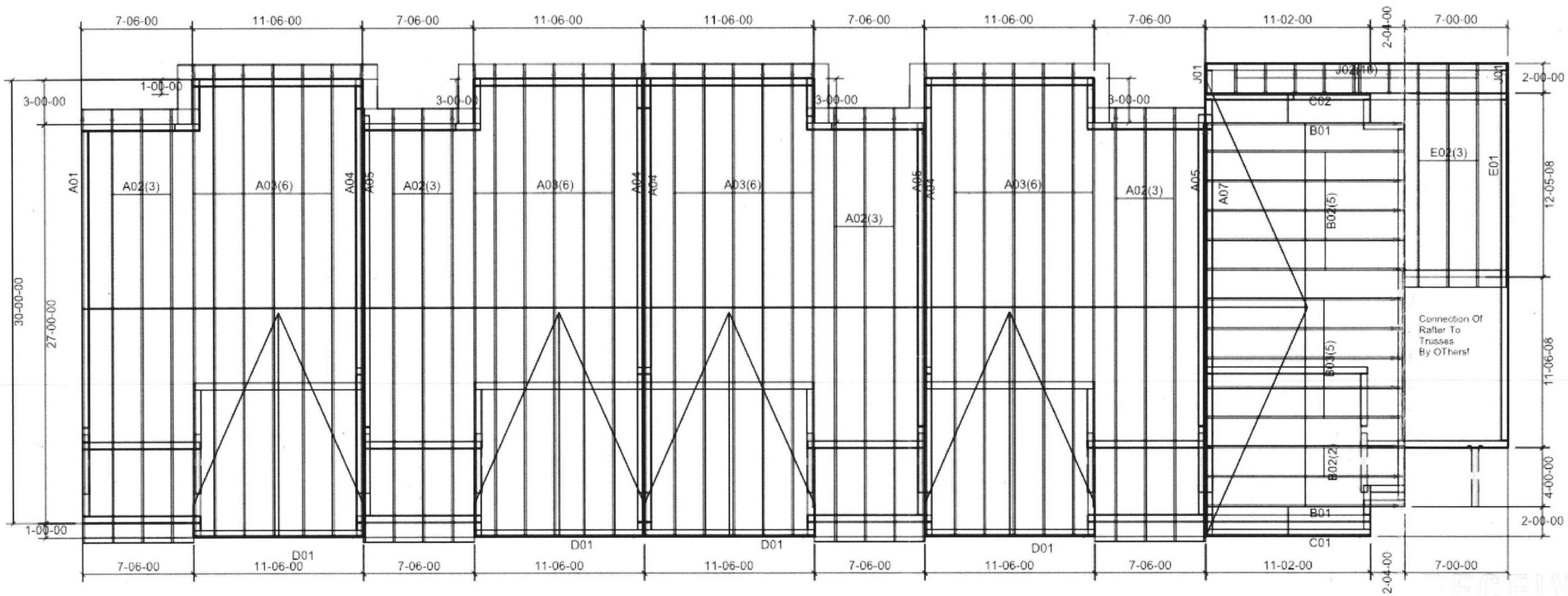

City of Portland
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MAY 17 2018
Permit Number

17-180493/496/497/498/500-RS

Client: Jessey Cereghino
Plan Name: 5 Plex
Sales Person: Oscar Trigueros
x131
Site :
Quote #: 17-OT9956
Order #: [???]
Lot : .
Pitch: 4 & 9/12
Overhang: Varies
Loading: 25-7-0-10
Date: 4-12-18

PRECISION TRUSS & TIMBER
11550 SE Jennifer St
Clackamas, OR 97015
(503) 656-2983
(503) 656-2647

7614 SE Washington St,17-180493 / 496 / 497 / 498 / 500 - RS

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SCANNED

MiTek USA, Inc.
250 Klug Circle
Corona, CA 92880
951-245-9525

Re: 17-OT9956
Jessey Cereghino

The truss drawing(s) referenced below have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Precision Roof Trusses, Inc.

Pages or sheets covered by this seal: K4488194 thru K4488209

My license renewal date for the state of Oregon is December 31, 2019.

REGISTERED PROFESSIONAL ENGINEER
89200PE
OREGON
MAY 14, 2014
DAVID MERRILL BAXTER
EXPIRES: 12-31-2019

April 12,2018

Baxter, David

City of Portland
REVIEWED FOR CODE COMPLIANCE
MAY 17 2018
Permit Number

IMPORTANT NOTE: Truss Engineer's responsibility is solely for design of individual trusses based upon design parameters shown on referenced truss drawings. Parameters have not been verified as appropriate for any use. Any location identification specified is for file reference only and has not been used in preparing design. Suitability of truss designs for any particular building is the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino
17-OT9956	A01	GABLE	1	1	K4488194
					Job Reference (optional)

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015 — 8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:40 2018 Page 1
ID:EKXFkE3nUaaiMJsziER1roynf_N-vq5wBzfTvh_qAZs0LtuqhgKMJ_o8e9FE7H7ufBzRU2n

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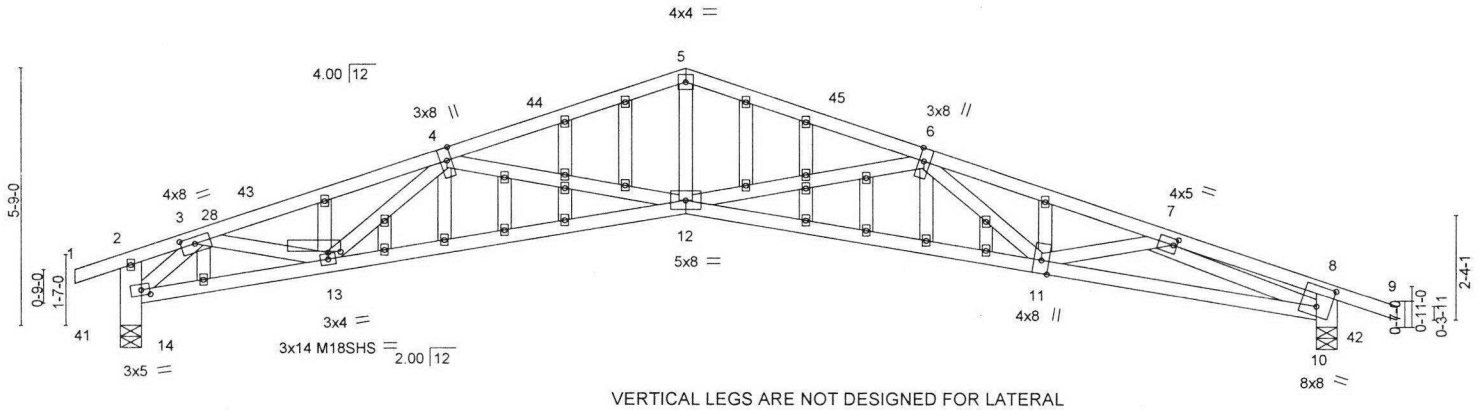

VERTICAL LEGS ARE NOT DESIGNED FOR LATERAL LOADS IMPOSED BY SUPPORTS (BEARINGS).

0-5-8	4-6-15	12-6-0	20-5-1	27-0-0
0-5-8	4-1-7	7-11-1	7-11-1	6-6-15

Plate Offsets (X,Y)-- [3:0-3-12,0-1-12], [4:0-0-4,0-0-1], [4:0-3-8,Edge], [6:0-3-8,Edge], [7:0-1-0,0-1-12], [8:0-2-14,0-0-0], [10:0-3-12,0-5-8], [11:0-0-14,0-0-2], [13:0-3-4,0-0-4], [14:0-2-4,0-1-8]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.38	Vert(LL)	-0.25	11-12	>999	240	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	0.56	Vert(TL)	-0.59	11-12	>542	180	M18SHS	185/148
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.88	Horz(TL)	0.31	42	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-AS							Weight: 141 lb	FT = 0%

LUMBER-
TOP CHORD 2x4 DF No.1&Btr
BOT CHORD 2x4 DF No.1&Btr
WEBS 2x4 HF Std *Except*
8-42,2-41: 2x6 DF 1800F 1.6E
OTHERS 2x4 HF Std

BRACING-
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied.

REACTIONS. (lb/size) 41=1192/0-5-8, 42=1216/0-5-8
Max Horz 41=-75(LC 17)
Max Uplift 41=-321(LC 8), 42=-352(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 3-4=-2601/976, 4-5=-2735/1083, 5-6=-2736/1103, 6-7=-3400/1235, 7-8=-615/225, 10-42=-1216/533, 8-10=-471/256, 14-41=-1192/579
BOT CHORD 13-14=-559/1367, 12-13=-1107/2841, 11-12=-1186/3270, 10-11=-1080/2930
WEBS 5-12=-420/1376, 3-28=-282/1101, 13-28=-283/1102, 4-13=-512/353, 4-12=-430/283, 6-12=-780/425, 7-11=0/382, 7-10=-2622/1089, 3-14=-1773/797

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 2-0-0 to 5-1-3, Exterior(2) 5-1-3 to 15-6-0, Corner(3) 15-6-0 to 18-7-3 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are MT20 plates unless otherwise indicated.
5) All plates are 2x3 MT20 unless otherwise indicated.
6) Gable studs spaced at 1-4-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) Bearing at joint(s) 41, 42 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 321 lb uplift at joint 41 and 352 lb uplift at joint 42.
11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

City of Portland
REVIEWED FOR CODE COMPLIANCE
MAY 17 2018
Permit Number

REGISTERED PROFESSIONAL ENGINEER
89200PE
OREGON
MAY 14, 2014
DAVID MERRILL BAXTER

EXPIRES: 12-31-2019
April 12,2018

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

MiTek
250 Klug Circle
Corona, CA 92880

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino	
17-OT9956	A02	SCISSORS	12	1	Job Reference (optional)	K4488195

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015 — 8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:41 2018 Page 1
ID:EKXFkE3nUaaiMJsziER1roynf_N-N1flOJg5g_6hnjRCvaP3DutX3O8NNbVNMxsSBdzRU2m

1-0-0	1-11-5	7-2-10	12-6-0	17-9-6	23-0-11	27-0-0	28-4-0
1-0-0	1-11-5	5-3-6	5-3-6	5-3-6	5-3-6	3-11-5	1-4-0

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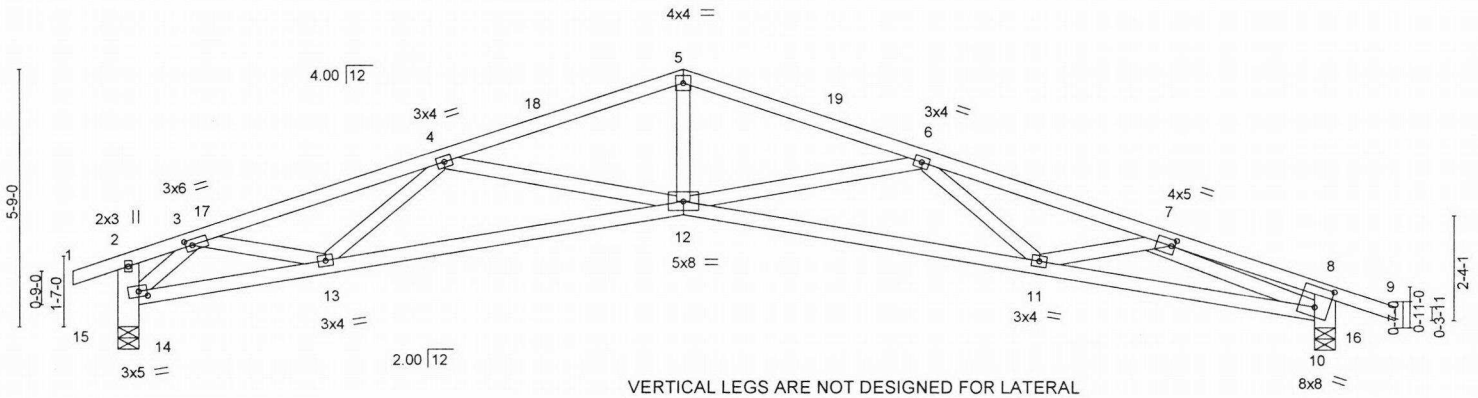


VERTICAL LEGS ARE NOT DESIGNED FOR LATERAL LOADS IMPOSED BY SUPPORTS (BEARINGS).

0-5-8	4-6-15	12-6-0	20-5-1	27-0-0
0-5-8	4-1-7	7-11-1	7-11-1	6-6-15

Plate Offsets (X,Y)-- [3:0-1-12,0-1-8], [7:0-1-0,0-1-12], [8:0-2-14,0-0-0], [10:0-3-12,0-5-8], [14:0-2-4,0-1-8]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.38	Vert(LL)	-0.25	11-12	>999	240	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	0.56	Vert(TL)	-0.59	11-12	>542	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.88	Horz(TL)	0.31	16	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-AS							Weight: 119 lb	FT = 0%

LUMBER-
TOP CHORD 2x4 DF No.1&Btr
BOT CHORD 2x4 DF No.1&Btr
WEBS 2x4 HF Std *Except*
8-16,2-15: 2x6 DF 1800F 1.6E

BRACING-
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied.

REACTIONS. (lb/size) 15=1192/0-5-8, 16=1216/0-5-8
Max Horz 15=-75(LC 17)
Max Uplift 15=-321(LC 8), 16=-352(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 3-4=-2601/526, 4-5=-2735/530, 5-6=-2736/540, 6-7=-3400/704, 7-8=-615/122, 10-16=-1216/352, 8-10=-471/188, 14-15=-1192/329
BOT CHORD 13-14=-383/1367, 12-13=-613/2841, 11-12=-646/3270, 10-11=-670/2930
WEBS 5-12=-182/1376, 3-13=-90/1102, 4-13=-512/220, 4-12=-430/283, 6-12=-780/376, 7-11=0/382, 7-10=-2622/686, 3-14=-1773/489

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 2-0-0 to 5-1-3, Interior(1) 5-1-3 to 15-6-0, Exterior(2) 15-6-0 to 18-7-3 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) Bearing at joint(s) 15, 16 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 321 lb uplift at joint 15 and 352 lb uplift at joint 16.
7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

City of Portland
REVIEWED FOR CODE COMPLIANCE
MAY 17 2018
Permit Number

REGISTERED PROFESSIONAL ENGINEER
89200PE
OREGON
MAY 14, 2014
DAVID MERRILL BAXTER

EXPIRES: 12-31-2019
April 12,2018

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

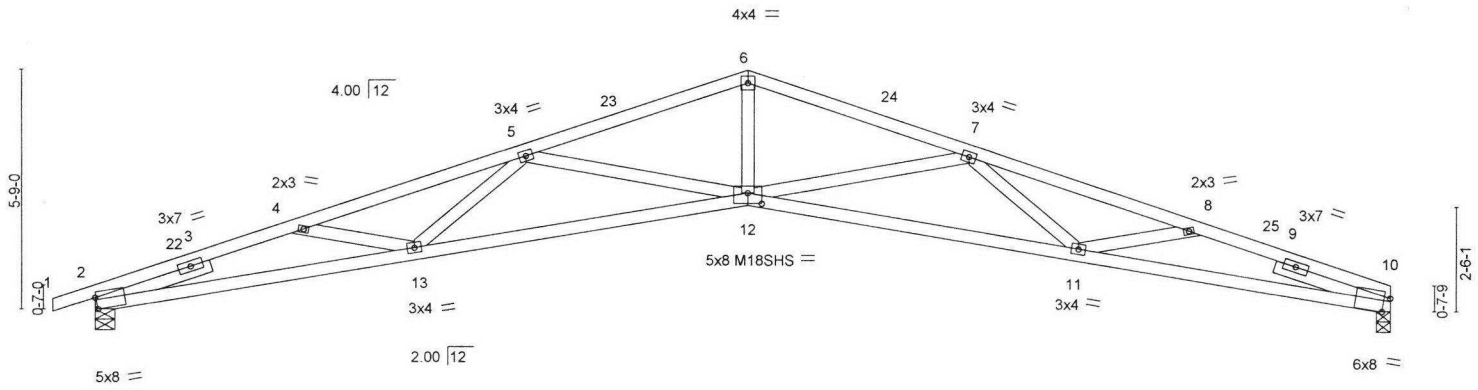
MiTek
250 Klug Circle
Corona, CA 92880

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino
17-OT9956	A03	SCISSORS	24	1	K4488196
					Job Reference (optional)

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015 — 8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:42 2018 Page 1
ID:EKXFkE3nUaaiMJsziER1roynf_N-sDCgbfgjRIEYPs0OTIwIm5QahoNc663Xabc?k4zRU2I

1-0-0	4-11-5	10-2-10	15-6-0	20-9-6	26-0-11	30-10-6
1-0-0	4-11-5	5-3-6	5-3-6	5-3-6	5-3-6	4-9-11

Scale = 1:53.1



7-6-15	15-6-0	23-5-1	30-10-6
7-6-15	7-11-1	7-11-1	7-5-5

Plate Offsets (X,Y)-- [2:0-0-5,Edge], [10:0-1-11,Edge], [12:0-4-0,0-3-0]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.89	Vert(LL)	-0.56	12-13	>659	240	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	1.00	Vert(TL)	-1.22	12-13	>303	180	M18SHS	185/148
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.60	Horz(TL)	0.55	10	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-AS							Weight: 126 lb	FT = 0%

LUMBER-
TOP CHORD 2x4 DF No.2
BOT CHORD 2x4 DF No.2
WEBS 2x4 HF Std *Except*
6-12: 2x4 DF No.2
SLIDER Left 2x4 HF Std 2-10-6, Right 2x4 HF Std 2-10-6

BRACING-
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS. (lb/size) 10=1295/0-3-14, 2=1361/0-5-8
Max Horz 2=113(LC 12)
Max Uplift 10=-309(LC 9), 2=-361(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-4=-4647/1167, 4-5=-4671/1034, 5-6=-3703/753, 6-7=-3703/763, 7-8=-4621/971, 8-10=-4554/1106
BOT CHORD 2-13=-1132/4382, 12-13=-1018/4398, 11-12=-882/4374, 10-11=-1000/4288
WEBS 6-12=-339/2008, 5-13=0/331, 5-12=-972/429, 7-12=-954/430, 7-11=0/306, 8-11=0/270

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -1-0-0 to 2-1-1, Interior(1) 2-1-1 to 15-6-0, Exterior(2) 15-6-0 to 18-7-1 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) All plates are MT20 plates unless otherwise indicated.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Bearing at joint(s) 10, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 309 lb uplift at joint 10 and 361 lb uplift at joint 2.
8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

City of Portland
REVIEWED FOR CODE COMPLIANCE
MAY 17 2018
Permit Number

REGISTERED PROFESSIONAL ENGINEER
89200PE
OREGON
MAY 14, 2014
DAVID MERRILL BAXTER

EXPIRES: 12-31-2019
April 12,2018

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
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MiTek
250 Klug Circle
Corona, CA 92880

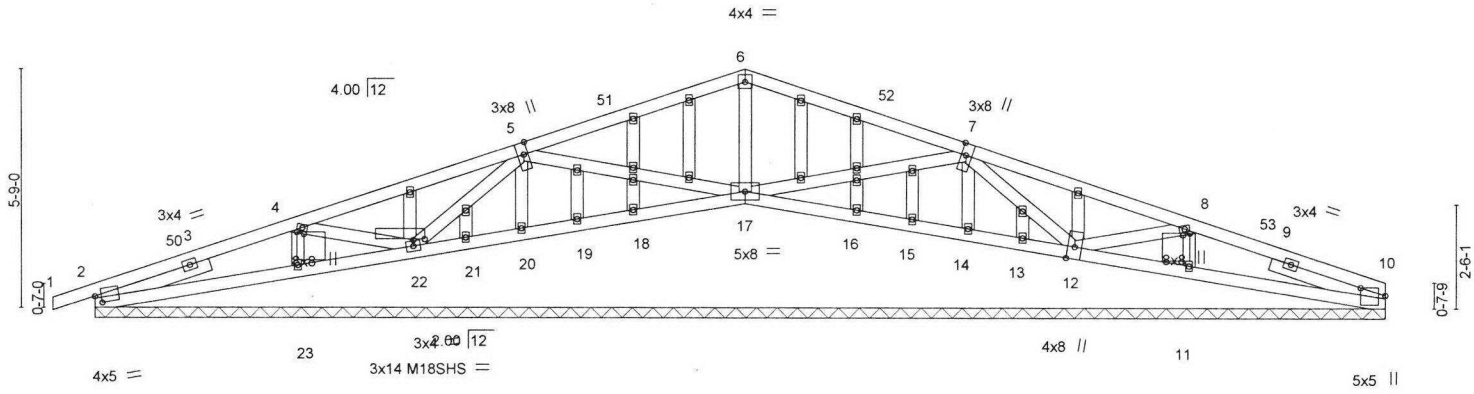
Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino
17-OT9956	A04	GABLE	4	1	K4488197
					Job Reference (optional)

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015

8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:44 2018 Page 1
ID:EKXFkE3nUaaiMJsziER1roynf_N-obKR0LizzvUGeAAnajymrWV3BbHAa6lp2v56oyzRU2j

1-0-0	4-11-5	10-2-10	15-6-0	20-9-6	26-0-11	30-10-6
1-0-0	4-11-5	5-3-6	5-3-6	5-3-6	5-3-6	4-9-11

Scale = 1:53.2



7-6-15	15-6-0	23-5-1	30-10-6
7-6-15	7-11-1	7-11-1	7-5-5

Plate Offsets (X,Y)-- [2:0-1-13,0-2-3], [5:0-0-4,0-0-1], [5:0-3-8,Edge], [7:0-3-8,Edge], [10:0-2-5,Edge], [12:0-0-14,0-0-2], [22:0-3-4,0-0-4], [32:0-0-9,0-2-0], [41:0-0-9,0-2-0]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.32	Vert(LL)	-0.00	1	n/r	120	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	0.10	Vert(TL)	0.00	1	n/r	90	M18SHS	185/148
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.23	Horz(TL)	-0.00	10	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-AS							Weight: 147 lb	FT = 0%

LUMBER-
TOP CHORD 2x4 DF No.2
BOT CHORD 2x4 DF No.2
WEBS 2x4 HF Std
OTHERS 2x4 HF Std
SLIDER Left 2x4 HF Std 2-10-6, Right 2x4 HF Std 2-10-6

BRACING-
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS. All bearings 30-10-6.
(lb) - Max Horz 10=-352(LC 1), 2=352(LC 1), 10=-352(LC 1), 2=352(LC 1)
Max Uplift All uplift 100 lb or less at joint(s) 10 except 2=-141(LC 8), 22=-286(LC 12), 17=-142(LC 8), 12=-278(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 18, 19, 20, 21, 23, 16, 15, 14, 13, 11 except 10=328(LC 1), 2=399(LC 1), 22=563(LC 23), 17=438(LC 1), 12=550(LC 24), 10=328(LC 1), 2=399(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-4=-428/272, 8-10=-463/320
WEBS 6-17=-300/227, 4-22=-576/415, 5-22=-572/375, 5-17=-255/252, 7-17=-266/258, 7-12=-552/362, 8-12=-552/437

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -1-0-0 to 2-1-1, Exterior(2) 2-1-1 to 15-6-0, Corner(3) 15-6-0 to 18-7-1 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are MT20 plates unless otherwise indicated.
5) All plates are 2x3 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 1-4-0 oc.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10, 10 except (jt=lb) 2=141, 22=286, 17=142, 12=278, 2=141.
11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 22, 17, 12, 18, 19, 20, 21, 23, 16, 15, 14, 13, 11.
12) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

City of Portland
REVIEWED FOR CODE COMPLIANCE
MAY 17 2018
Permit Number

REGISTERED PROFESSIONAL ENGINEER
89200PE
OREGON
MAY 14, 2014
DAVID MERRILL BAXTER

EXPIRES: 12-31-2019
April 12,2018

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
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MiTek
250 Klug Circle
Corona, CA 92880

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino	
17-OT9956	A05	GABLE	3	1	Job Reference (optional)	K4488198

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015 — 8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:47 2018 Page 1
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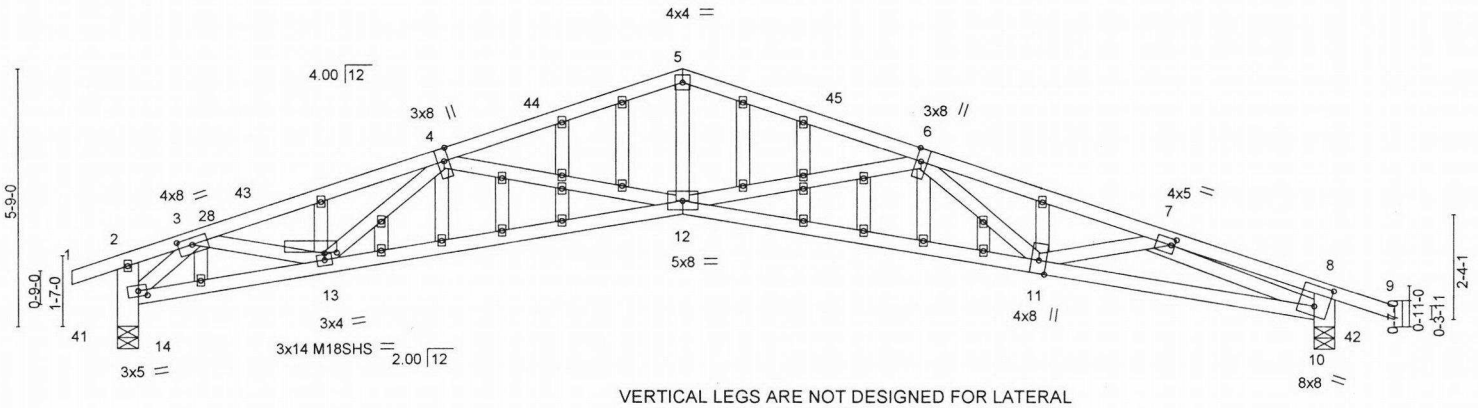
1-0-0	1-11-5	7-2-10	12-6-0	17-9-6	23-0-11	27-0-0	28-4-0
1-0-0	1-11-5	5-3-6	5-3-6	5-3-6	5-3-6	3-11-5	1-4-0

Scale = 1:49.4



VERTICAL LEGS ARE NOT DESIGNED FOR LATERAL LOADS IMPOSED BY SUPPORTS (BEARINGS).

0-5-8	4-6-15	12-6-0	20-5-1	27-0-0
0-5-8	4-1-7	7-11-1	7-11-1	6-6-15

Plate Offsets (X,Y)-- [3:0-3-12,0-1-12], [4:0-0-4,0-0-1], [4:0-3-8,Edge], [6:0-3-8,Edge], [7:0-1-0,0-1-12], [8:0-2-14,0-0-0], [10:0-3-12,0-5-8], [11:0-0-14,0-0-2], [13:0-3-4,0-0-4], [14:0-2-4,0-1-8]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.38	Vert(LL)	-0.25	11-12	>999	240	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	0.56	Vert(TL)	-0.59	11-12	>542	180	M18SHS	185/148
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.88	Horz(TL)	0.31	42	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-AS							Weight: 141 lb	FT = 0%

LUMBER-

TOP CHORD	2x4 DF No.1&Btr
BOT CHORD	2x4 DF No.1&Btr
WEBS	2x4 HF Std *Except*
	8-42,2-41: 2x6 DF 1800F 1.6E
OTHERS	2x4 HF Std

BRACING-

TOP CHORD	Structural wood sheathing directly applied, except end verticals.
BOT CHORD	Rigid ceiling directly applied.

REACTIONS. (lb/size) 41=1192/0-5-8, 42=1216/0-5-8
Max Horz 41=-75(LC 17)
Max Uplift 41=-321(LC 8), 42=-352(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	3-4=-2601/976, 4-5=-2735/1083, 5-6=-2736/1103, 6-7=-3400/1235, 7-8=-615/225, 10-42=-1216/533, 8-10=-471/256, 14-41=-1192/579
BOT CHORD	13-14=-559/1367, 12-13=-1107/2841, 11-12=-1186/3270, 10-11=-1080/2930
WEBS	5-12=-420/1376, 3-28=-282/1101, 13-28=-283/1102, 4-13=-512/353, 4-12=-430/283, 6-12=-780/425, 7-11=0/382, 7-10=-2622/1089, 3-14=-1773/797

NOTES-

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 2-0-0 to 5-1-3, Exterior(2) 5-1-3 to 15-6-0, Corner(3) 15-6-0 to 18-7-3 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are MT20 plates unless otherwise indicated.
5) All plates are 2x3 MT20 unless otherwise indicated.
6) Gable studs spaced at 1-4-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) Bearing at joint(s) 41, 42 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 41=321, 42=352.
11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

City of Portland
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MAY 17 2018
Permit Number

REGISTERED PROFESSIONAL ENGINEER
89200PE
OREGON
MAY 14, 2014
DAVID MERRILL BAXTER

EXPIRES: 12-31-2019
April 12,2018

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

MiTek
250 Klug Circle
Corona, CA 92880

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino
17-OT9956	A07	Roof Special Supported Gable	1	1	K4488199
					Job Reference (optional)

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015

8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:50 2018 Page 1
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1-0-0 | 15-6-0 | 30-10-8
1-0-0 | 15-6-0 | 15-4-8

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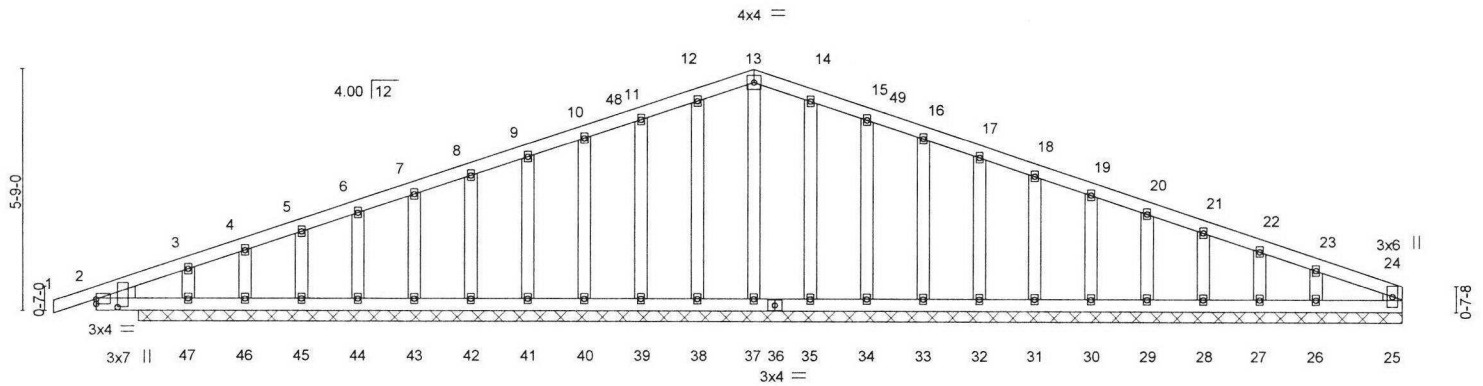

30-10-8
30-10-8

Plate Offsets (X,Y)-- [2:0-0-0,0-1-6], [2:0-2-5,0-6-1]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.21	Vert(LL)	-0.00	1	n/r	120	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	0.19	Vert(TL)	-0.01	1-2	n/r	90		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.11	Horz(TL)	-0.01	25	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-R							Weight: 159 lb	FT = 0%

LUMBER-
TOP CHORD 2x4 DF No.2
BOT CHORD 2x4 DF No.2
WEBS 2x6 DF No.2
OTHERS 2x4 HF Std
WEDGE
Left: 2x4 HF Std

BRACING-
TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 29-10-8.
(lb) - Max Horz 47=108(LC 16)
Max Uplift All uplift 100 lb or less at joint(s) 25, 38, 39, 40, 41, 42, 43, 44, 45, 46, 35, 34, 33, 32, 31, 30, 29, 28, 27, 26 except 47=-128(LC 8)
Max Grav All reactions 250 lb or less at joint(s) 25, 37, 38, 39, 40, 41, 42, 43, 44, 45, 46, 35, 34, 33, 32, 31, 30, 29, 28, 27, 26 except 47=413(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -1-0-0 to 2-2-0, Exterior(2) 2-2-0 to 15-6-0, Corner(3) 15-6-0 to 18-7-1 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are 2x3 MT20 unless otherwise indicated.
5) Gable studs spaced at 1-4-0 oc.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 25, 38, 39, 40, 41, 42, 43, 44, 45, 46, 35, 34, 33, 32, 31, 30, 29, 28, 27, 26 except (jt=lb) 47=128.
9) NA

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OREGON
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EXPIRES: 12-31-2019
April 12,2018

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MiTek
250 Klug Circle
Corona, CA 92880

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino
17-OT9956	B01	GABLE	2	1	K4488200
					Job Reference (optional)

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015 8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:51 2018 Page 1
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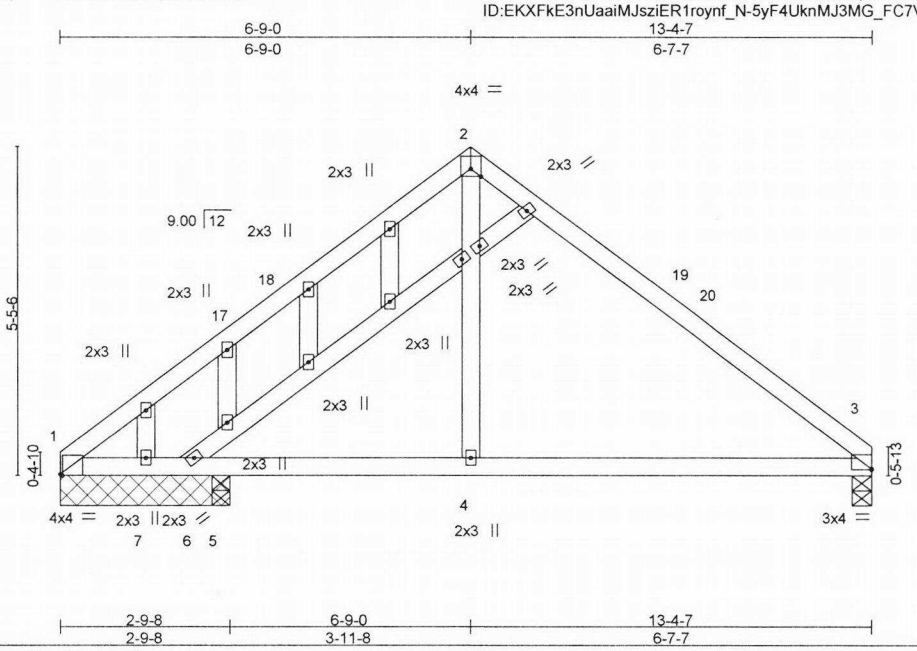

Plate Offsets (X,Y)-- [1:0-0-7,Edge], [3:0-0-0,0-0-3]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.48	Vert(LL)	0.06	3-4	>999	240	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	0.49	Vert(TL)	-0.13	3-4	>979	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.12	Horz(TL)	0.01	3	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-R							Weight: 61 lb	FT = 0%

LUMBER-
TOP CHORD 2x4 DF No.2
BOT CHORD 2x4 DF No.2
WEBS 2x4 HF Std
OTHERS 2x4 HF Std

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 2-9-8 except (jt=length) 3=0-3-14, 5=0-3-8.
(lb) - Max Horz 1=-146(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 3=-112(LC 13), 6=-261(LC 23), 7=-322(LC 12)
Max Grav All reactions 250 lb or less at joint(s) 6, 5 except 1=346(LC 20), 3=530(LC 1), 7=420(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-624/172, 2-3=-618/168
BOT CHORD 1-7=-41/398, 6-7=-41/398, 5-6=-41/398, 4-5=-41/398, 3-4=-41/398
WEBS 2-4=0/253

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 6-9-0, Exterior(2) 6-9-0 to 9-9-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) Gable studs spaced at 1-4-0 oc.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 3=112, 6=261, 7=322.

City of Portland
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MAY 17 2018
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REGISTERED PROFESSIONAL ENGINEER
89200PE
OREGON
MAY 14, 2014
DAVID MERRILL BAXTER

EXPIRES: 12-31-2019
April 12,2018

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
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MiTek
250 Klug Circle
Corona, CA 92880

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino
17-OT9956	B02	Roof Special	7	1	K4488201
					Job Reference (optional)

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015 — 8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:52 2018 Page 1
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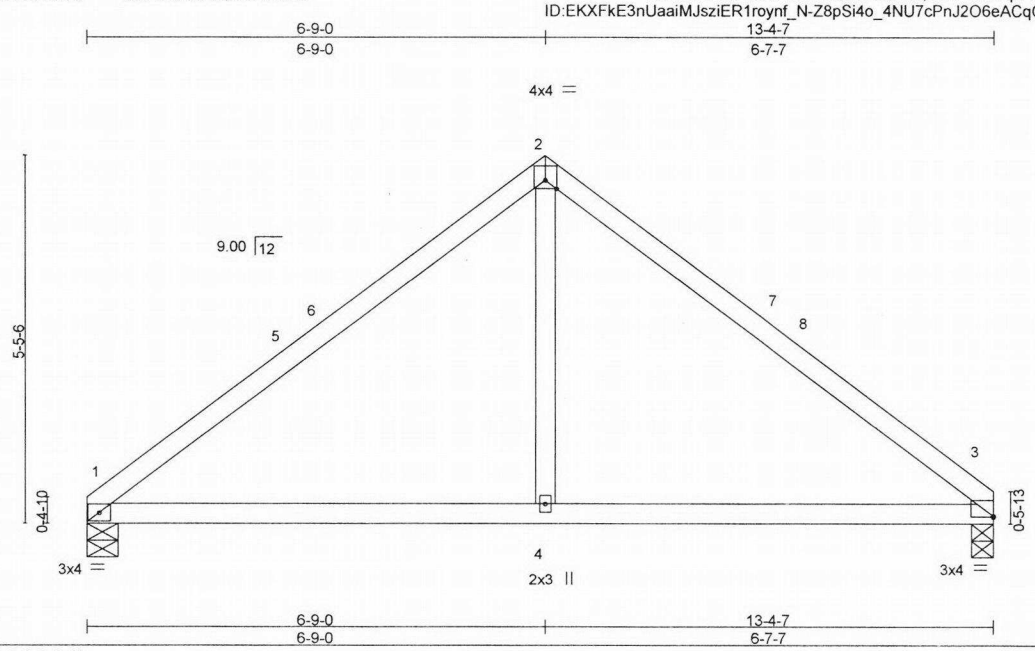

Plate Offsets (X,Y)-- [3:0-0-0,0-0-3]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.49	Vert(LL)	-0.06	1-4	>999	240	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	0.37	Vert(TL)	-0.12	1-4	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.14	Horz(TL)	0.01	3	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-R							Weight: 47 lb	FT = 0%

LUMBER-
TOP CHORD 2x4 DF No.2
BOT CHORD 2x4 DF No.2
WEBS 2x4 HF Std

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=545/0-5-8, 3=545/0-3-14
Max Horz 1=146(LC 9)
Max Uplift 1=-106(LC 12), 3=-105(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-657/171, 2-3=-658/171
BOT CHORD 1-4=-56/430, 3-4=-56/430
WEBS 2-4=0/306

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-2-12 to 3-2-12, Interior(1) 3-2-12 to 6-9-0, Exterior(2) 6-9-0 to 9-9-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=106, 3=105.

City of Portland
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MAY 17 2018
Permit Number

REGISTERED PROFESSIONAL ENGINEER
89200PE
OREGON
MAY 14, 2014
DAVID MERRILL BAXTER

EXPIRES: 12-31-2019
April 12,2018

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MiTek
250 Klug Circle
Corona, CA 92880

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino	
17-OT9956	B03	Roof Special	5	1	Job Reference (optional)	K4488202

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015 — 8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:53 2018 Page 1
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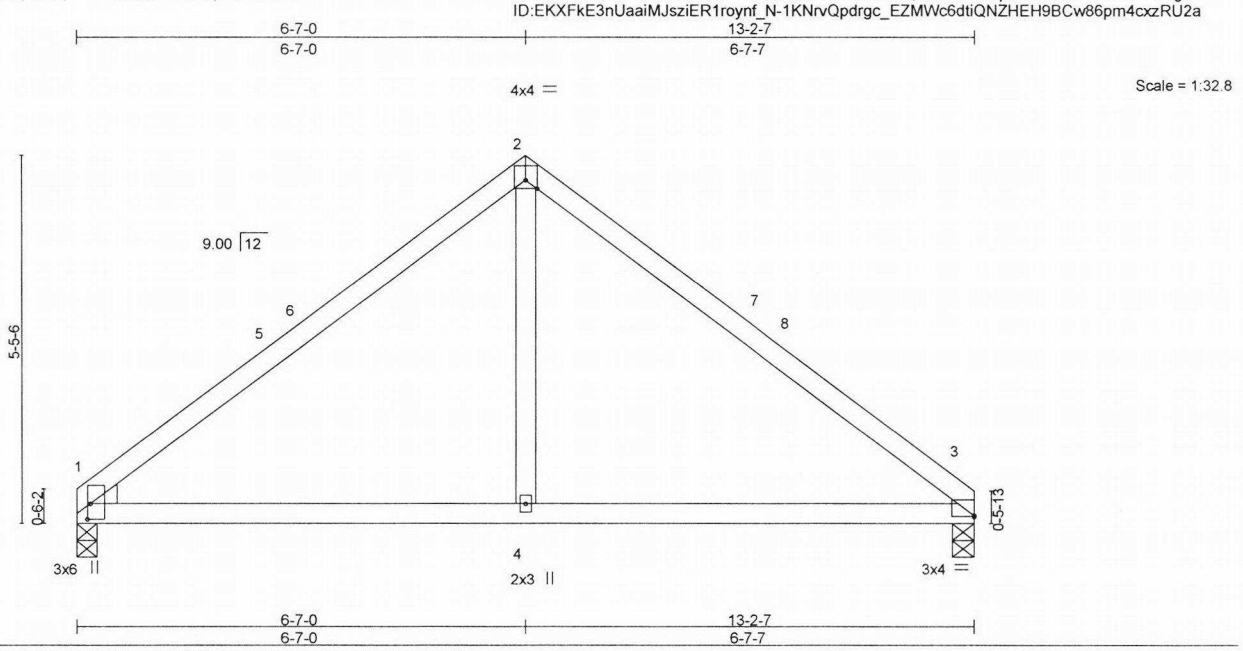

Plate Offsets (X,Y)-- [1:0-2-12,0-0-8], [1:0-1-12,0-5-7], [1:0-0-14,0-1-3], [3:0-0-0,0-0-3]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.49	Vert(LL)	-0.06	3-4	>999	240	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	0.37	Vert(TL)	-0.12	3-4	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.14	Horz(TL)	0.01	3	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-R							Weight: 47 lb	FT = 0%

LUMBER-
TOP CHORD 2x4 DF No.2
BOT CHORD 2x4 DF No.2
WEBS 2x4 HF Std
WEDGE
Left: 2x4 HF Std

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=542/0-3-14, 1=542/0-3-8
Max Horz 1=-146(LC 8)
Max Uplift 3=-105(LC 13), 1=-104(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-652/171, 2-3=-652/171
BOT CHORD 1-4=-55/425, 3-4=-55/425
WEBS 2-4=0/305

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-3-12 to 3-3-12, Interior(1) 3-3-12 to 6-9-0, Exterior(2) 6-9-0 to 9-9-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 3=105, 1=104.

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OREGON
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MiTek
250 Klug Circle
Corona, CA 92880

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino
17-OT9956	C01	Roof Special Supported Gable	1	1	K4488203
					Job Reference (optional)

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015 — 8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:54 2018 Page 1
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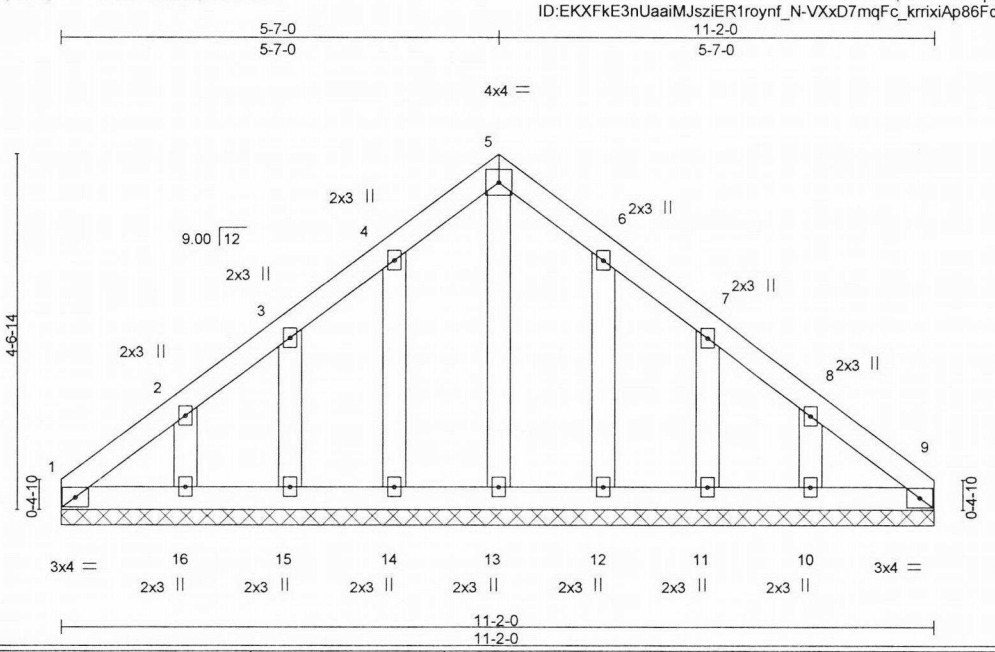

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.04	Vert(LL)	n/a	-	n/a	999	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	0.02	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.03	Horz(TL)	0.00	9	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-R							Weight: 53 lb	FT = 0%

LUMBER-
TOP CHORD 2x4 DF No.2
BOT CHORD 2x4 DF No.2
OTHERS 2x4 HF Std

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 11-2-0.
(lb) - Max Horz 1=-121(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 1, 9, 14, 15, 16, 12, 11, 10
Max Grav All reactions 250 lb or less at joint(s) 1, 9, 13, 14, 15, 16, 12, 11, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-0-0 to 2-11-0, Exterior(2) 2-11-0 to 5-7-0, Corner(3) 5-7-0 to 8-7-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 1-4-0 oc.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9, 14, 15, 16, 12, 11, 10.
9) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 9.

City of Portland
REVIEWED FOR CODE COMPLIANCE
MAY 17 2018
Permit Number

REGISTERED PROFESSIONAL ENGINEER
89200PE
OREGON
MAY 14, 2014
DAVID MERRILL BAXTER

EXPIRES: 12-31-2019
April 12,2018

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

MiTek
250 Klug Circle
Corona, CA 92880

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino
17-OT9956	C02	Roof Special Supported Gable	1	1	K4488204
					Job Reference (optional)

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015

8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:55 2018 Page 1
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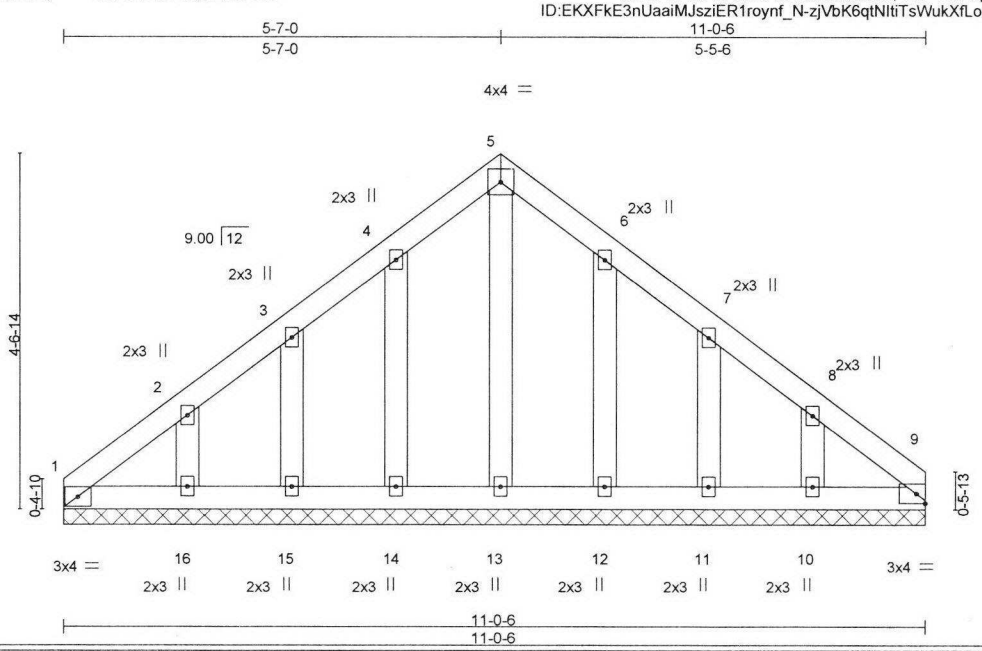

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.04	Vert(LL)	n/a	-	n/a	999	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	0.02	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00	9	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-R							Weight: 53 lb	FT = 0%

LUMBER-
TOP CHORD 2x4 DF No.2
BOT CHORD 2x4 DF No.2
OTHERS 2x4 HF Std

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 11-0-6.
(lb) - Max Horz 1=-121(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 1, 9, 14, 15, 16, 12, 11, 10
Max Grav All reactions 250 lb or less at joint(s) 1, 9, 13, 14, 15, 16, 12, 11, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-0-0 to 2-11-0, Exterior(2) 2-11-0 to 5-7-0, Corner(3) 5-7-0 to 8-7-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 1-4-0 oc.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9, 14, 15, 16, 12, 11, 10.

City of Portland
REVIEWED FOR CODE COMPLIANCE
MAY 17 2018
Permit Number

REGISTERED PROFESSIONAL ENGINEER
89200PE
OREGON
MAY 14, 2014
DAVID MERRILL BAXTER

EXPIRES: 12-31-2019
April 12,2018

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

MiTek
250 Klug Circle
Corona, CA 92880

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino	
17-OT9956	D01	COMMON SUPPORTED GAB	4	1	Job Reference (optional)	K4488205

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015 — 8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:56 2018 Page 1
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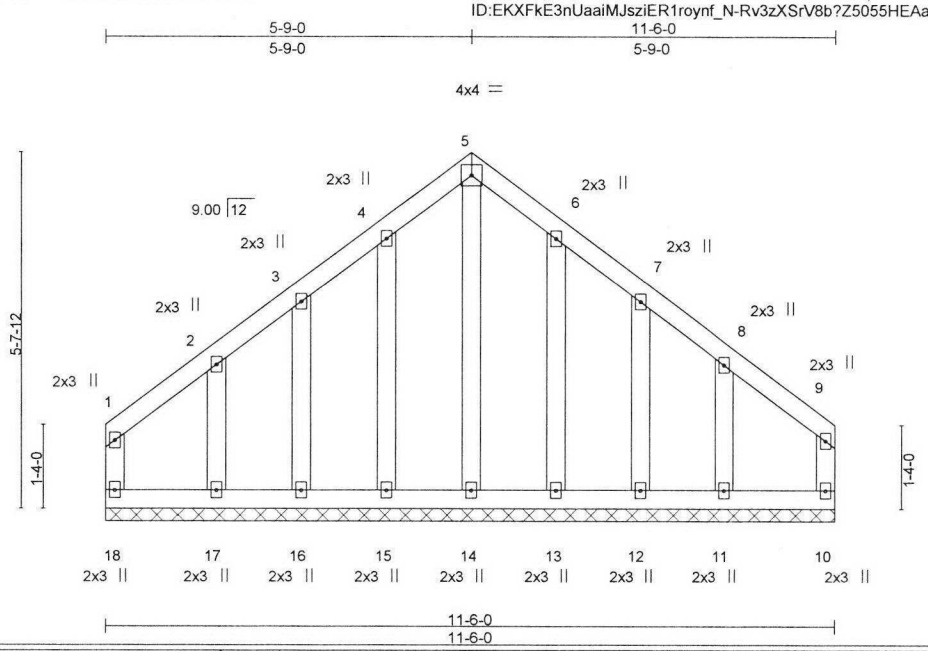

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.16	Vert(LL)	n/a	-	n/a	999	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	0.07	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.00	10	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-R							Weight: 65 lb	FT = 0%

LUMBER-
TOP CHORD 2x4 DF No.2
BOT CHORD 2x4 DF No.2
WEBS 2x4 HF Std
OTHERS 2x4 HF Std

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 11-6-0.
(lb) - Max Horz 18=-166(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 18, 10, 15, 16, 13, 12 except 17=-130(LC 12), 11=-128(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 18, 10, 14, 15, 16, 17, 13, 12, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-1-12 to 3-1-0, Exterior(2) 3-1-0 to 5-9-0, Corner(3) 5-9-0 to 8-9-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) Gable requires continuous bottom chord bearing.
5) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
6) Gable studs spaced at 1-4-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 18, 10, 15, 16, 13, 12 except (jt=lb) 17=130, 11=128.

City of Portland
REVIEWED FOR CODE COMPLIANCE
MAY 17 2018
Permit Number

REGISTERED PROFESSIONAL ENGINEER
89200PE
OREGON
MAY 14, 2014
DAVID MERRILL BAXTER

EXPIRES: 12-31-2019
April 12,2018

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

MiTek
250 Klug Circle
Corona, CA 92880

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino
17-OT9956	E01	GABLE	1	1	K4488206
					Job Reference (optional)

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015

8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:57 2018 Page 1
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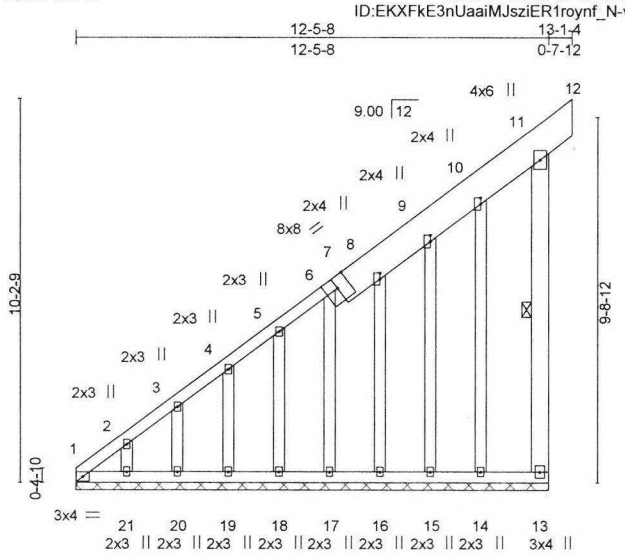

Plate Offsets (X,Y)-- [6:0-2-3,0-0-0], [7:0-0-0,0-1-12], [8:0-2-0,0-0-0], [9:0-2-0,0-0-0], [10:0-2-0,0-0-0]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.16	Vert(LL)	-0.00	11	n/r	120	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	0.26	Vert(TL)	0.00	11	n/r	90		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.18	Horz(TL)	0.00	13	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-R							Weight: 114 lb	FT = 0%

LUMBER-
TOP CHORD 2x10 DF No.2 *Except* 1-7: 2x4 DF No.2
BOT CHORD 2x4 DF No.2
WEBS 2x6 DF 1800F 1.6E
OTHERS 2x4 HF Std

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 11-13

REACTIONS. All bearings 12-5-8.
(lb) - Max Horz 1=414(LC 9)
Max Uplift All uplift 100 lb or less at joint(s) 21, 20, 19, 18, 17, 16, 15, 14 except 13=-134(LC 9), 1=-125(LC 10)
Max Grav All reactions 250 lb or less at joint(s) 13, 21, 20, 19, 18, 17, 16, 15, 14 except 1=256(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-652/620, 2-3=-584/561, 3-4=-528/516, 4-5=-474/472, 5-6=-414/423, 6-8=-351/369, 8-9=-314/340, 9-10=-253/281

NOTES-
1) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-0-0 to 3-0-0, Exterior(2) 3-0-0 to 13-1-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
3) Gable requires continuous bottom chord bearing.
4) Gable studs spaced at 1-4-0 oc.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 21, 20, 19, 18, 17, 16, 15, 14 except (jt=lb) 13=134, 1=125.

City of Portland
REVIEWED FOR CODE COMPLIANCE
MAY 17 2018
Permit Number

EXPIRES: 12-31-2019
April 12,2018

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
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MiTek
250 Klug Circle
Corona, CA 92880

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino	
17-OT9956	E02	MONOPITCH	3	1	Job Reference (optional)	K4488207

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015

8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:58 2018 Page 1
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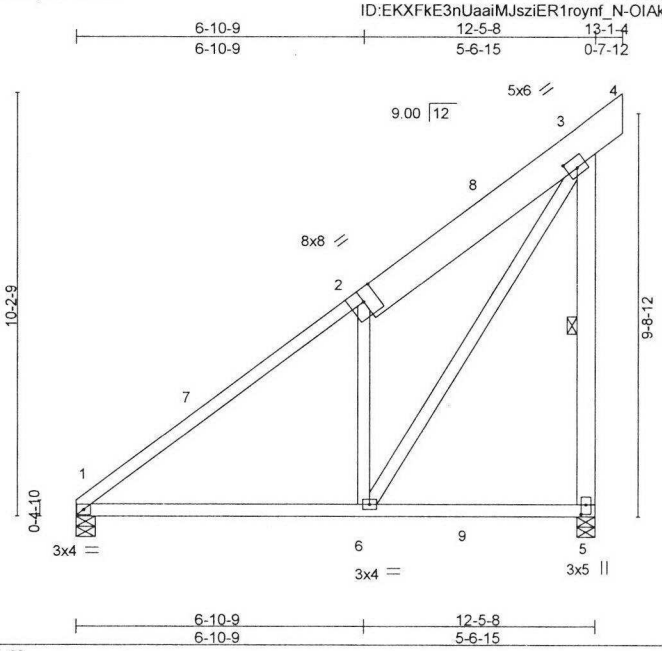

Plate Offsets (X,Y)-- [3:0-3-0,0-3-0], [5:0-2-12,0-1-8]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.56	Vert(LL)	0.07	1-6	>999	240	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	0.38	Vert(TL)	-0.14	1-6	>999	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.73	Horz(TL)	0.00	5	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-R							Weight: 92 lb	FT = 0%

LUMBER-
TOP CHORD 2x10 DF No.2 *Except*
1-2: 2x4 DF No.2
BOT CHORD 2x4 DF No.2
WEBS 2x4 HF Std *Except*
3-5: 2x6 DF 1800F 1.6E

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 3-5

REACTIONS. (lb/size) 5=1259/0-5-8, 1=455/0-5-8
Max Horz 1=413(LC 9)
Max Uplift 5=-465(LC 12), 1=-35(LC 12)
Max Grav 5=1440(LC 19), 1=455(LC 1)

City of Portland
REVIEWED FOR CODE COMPLIANCE
MAY 17 2018
Permit Number

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-571/85, 2-3=-548/256, 3-4=-441/111, 3-5=-1332/586
BOT CHORD 1-6=-265/471
WEBS 2-6=-453/359, 3-6=-370/707

NOTES-
1) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-2-12 to 3-2-12, Interior(1) 3-2-12 to 13-1-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 5=465.
5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 689 lb down and 186 lb up at 13-1-4 on top chord. The design/selection of such connection device(s) is the responsibility of others.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)
Vert: 1-3=-64, 3-4=-64, 1-5=-20
Concentrated Loads (lb)
Vert: 4=-650(F)

EXPIRES: 12-31-2019
April 12,2018

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MiTek
250 Klug Circle
Corona, CA 92880

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino
17-OT9956	J01	GABLE	2	1	K4488208
					Job Reference (optional)

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015 8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:59 2018 Page 1
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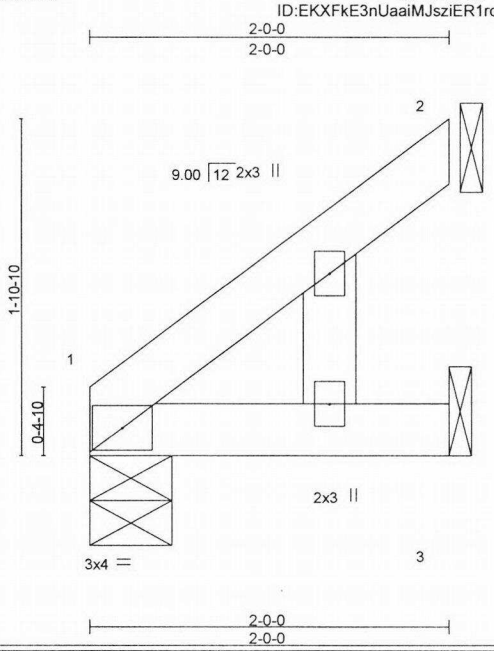


LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.06	Vert(LL)	-0.00	1	>999	240	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	0.03	Vert(TL)	-0.00	1-3	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-P							Weight: 7 lb	FT = 0%

LUMBER-
TOP CHORD 2x4 DF No.2
BOT CHORD 2x4 DF No.2
OTHERS 2x4 HF Std

BRACING-
TOP CHORD Structural wood sheathing directly applied or 2-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=82/0-5-8, 2=62/Mechanical, 3=19/Mechanical
Max Horz 1=69(LC 12)
Max Uplift 2=-62(LC 12)
Max Grav 1=82(LC 1), 2=69(LC 19), 3=39(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-
1) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
3) Gable studs spaced at 1-4-0 oc.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.

City of Portland
REVIEWED FOR CODE COMPLIANCE
MAY 17 2018
Permit Number

REGISTERED PROFESSIONAL ENGINEER
89200PE
OREGON
MAY 14, 2014
DAVID MERRILL BAXTER

EXPIRES: 12-31-2019
April 12,2018

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

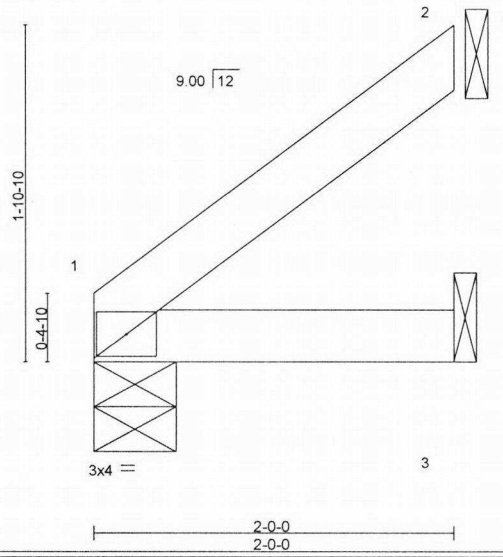
MiTek
250 Klug Circle
Corona, CA 92880

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino
17-OT9956	J02	JACK	10	1	K4488209
					Job Reference (optional)

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015 8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:59 2018 Page 1
ID:EKXFkE3nUaaiMJsziER1roynf_N-sUk6ATuORWN8yUpfzMkHyhdhVePIbxd1UlDPqbzRU2U

2-0-0
2-0-0

Scale = 1:12.4



2-0-0
2-0-0

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.06	Vert(LL)	-0.00	1	>999	240	MT20	220/195
TCDL	7.0	Lumber DOL	1.15	BC	0.03	Vert(TL)	-0.00	1-3	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-P							Weight: 6 lb	FT = 0%

LUMBER-
TOP CHORD 2x4 DF No.2
BOT CHORD 2x4 DF No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 2-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=82/0-5-8, 2=62/Mechanical, 3=19/Mechanical
Max Horz 1=69(LC 12)
Max Uplift 2=-62(LC 12)
Max Grav 1=82(LC 1), 2=69(LC 19), 3=39(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-
1) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.

City of Portland
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REGISTERED PROFESSIONAL ENGINEER
89200PE
OREGON
MAY 14, 2014
DAVID MERRILL BAXTER

EXPIRES: 12-31-2019
April 12,2018

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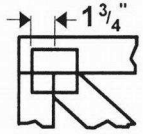
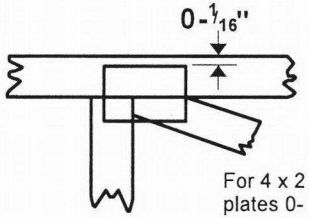
250 Klug Circle
Corona, CA 92880

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

*** Plate location details available in MiTek 20/20 software or upon request.**

PLATE SIZE

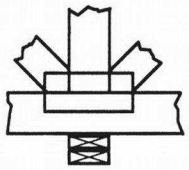
4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION

Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

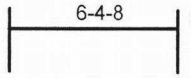


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

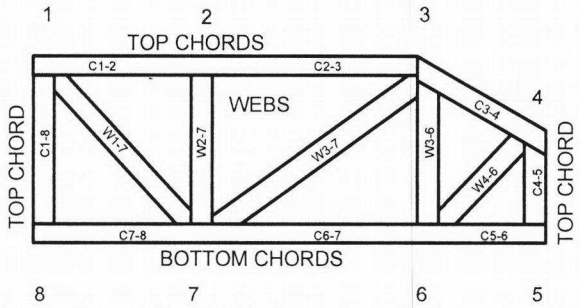
Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



dimensions shown in ft-in-sixteenths
(Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek®

MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

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MAY 17 2018
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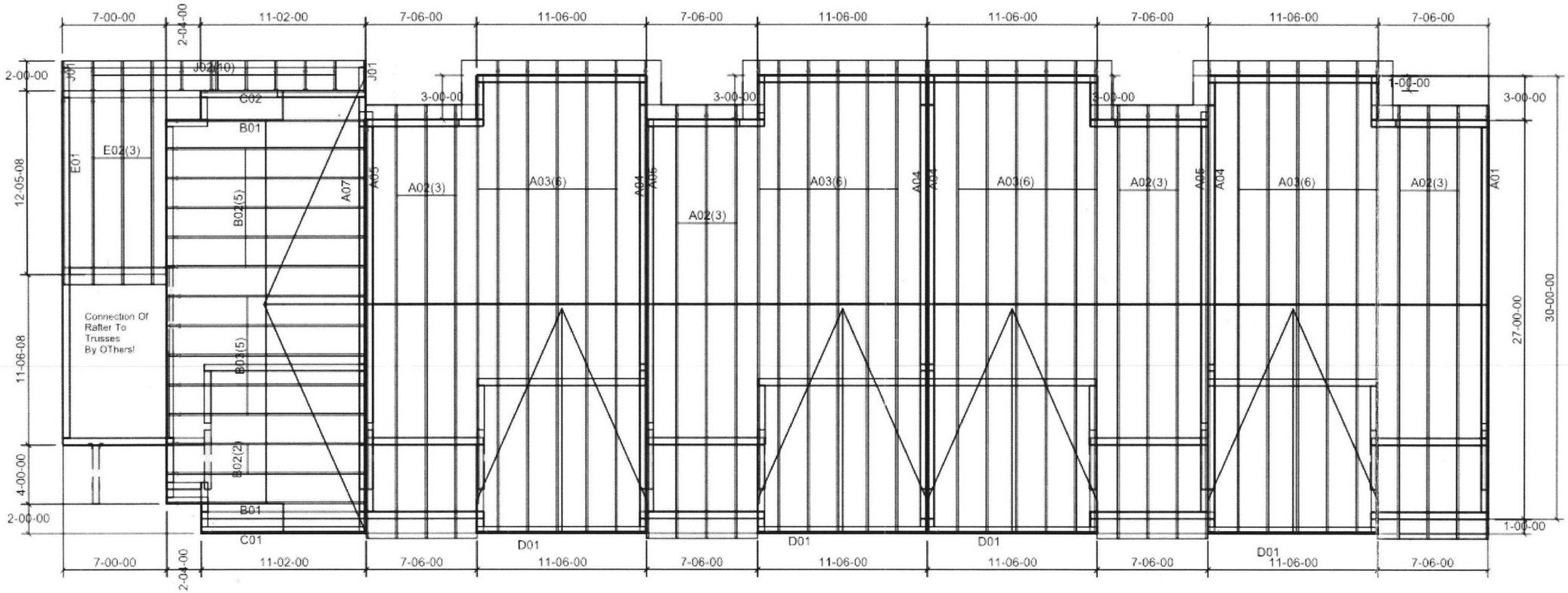

Client: Jessey Cereghino	Quote #:
Plan Name: 5 Plex	17-OT9956
Sales Person: Oscar Trigueros x131	Order #: [???]
Site :	Lot : .
Pitch: 4 & 9/12	Loading: 25-7-0-10
Overhang: Varies	Date: 4-12-18

PRECISION TRUSS & TIMBER
11550 SE Jennifer St
Clackamas, OR 97015
(503) 656-2983
(503) 656-2647

17-258825/847/853/857/858-RS

7622 SE Washington St,17-258825 / 847 / 853 / 857 / 858 - RS

MiTek USA, Inc.
250 Klug Circle
Corona, CA 92880
951-245-9525

Re: 17-OT9956
Jessey Cereghino

The truss drawing(s) referenced below have been prepared by MiTek USA, Inc. under my direct supervision based on the parameters provided by Precision Roof Trusses, Inc.

Pages or sheets covered by this seal: K4488194 thru K4488209

My license renewal date for the state of Oregon is December 31, 2019.

April 12,2018

Baxter, David

IMPORTANT NOTE: Truss Engineer's responsibility is solely for design of individual trusses based upon design parameters shown on referenced truss drawings. Parameters have not been verified as appropriate for any use. Any location identification specified is for file reference only and has not been used in preparing design. Suitability of truss designs for any particular building is the responsibility of the building designer, not the Truss Engineer, per ANSI/TPI-1, Chapter 2.

City of Portland
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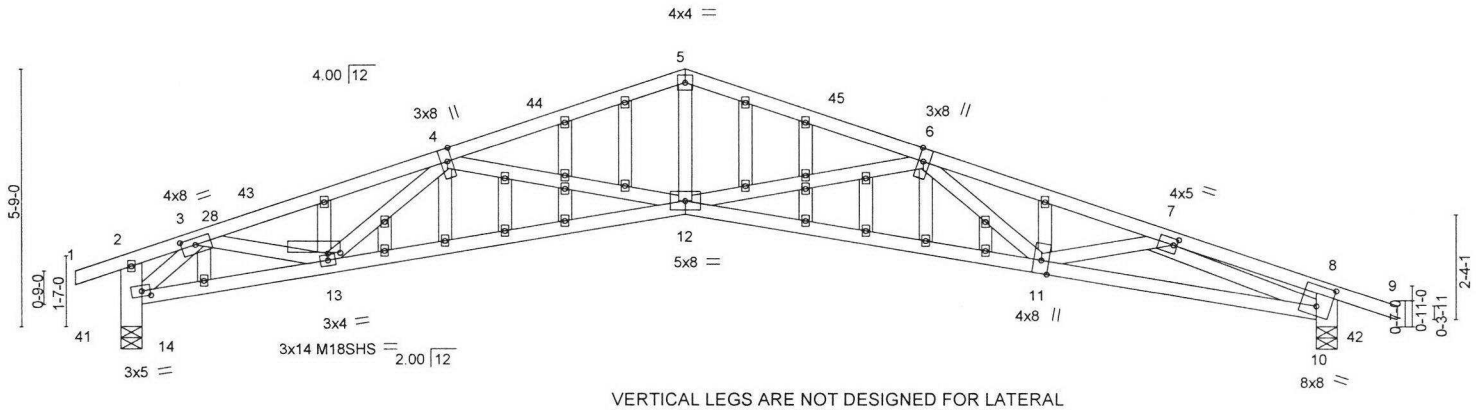
Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino	
17-OT9956	A01	GABLE	1	1	Job Reference (optional)	K4488194

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015 — 8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:40 2018 Page 1
ID:EKXFkE3nUaaiMJsziER1roynf_N-vq5wBzfTvh_qAZs0LtuqhgKMJ_o8e9FE7H7ufBzRU2n

1-0-0	1-11-5	7-2-10	12-6-0	17-9-6	23-0-11	27-0-0	28-4-0
1-0-0	1-11-5	5-3-6	5-3-6	5-3-6	5-3-6	3-11-5	1-4-0

Scale = 1:49.4



VERTICAL LEGS ARE NOT DESIGNED FOR LATERAL LOADS IMPOSED BY SUPPORTS (BEARINGS).

0-5-8	4-6-15	12-6-0	20-5-1	27-0-0
0-5-8	4-1-7	7-11-1	7-11-1	6-6-15

Plate Offsets (X,Y)-- [3:0-3-12,0-1-12], [4:0-0-4,0-0-1], [4:0-3-8,Edge], [6:0-3-8,Edge], [7:0-1-0,0-1-12], [8:0-2-14,0-0-0], [10:0-3-12,0-5-8], [11:0-0-14,0-0-2], [13:0-3-4,0-0-4], [14:0-2-4,0-1-8]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.38	Vert(LL)	-0.25	11-12	>999	240	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	0.56	Vert(TL)	-0.59	11-12	>542	180	M18SHS	185/148
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.88	Horz(TL)	0.31	42	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-AS							Weight: 141 lb	FT = 0%

LUMBER-

TOP CHORD	2x4 DF No.1&Btr
BOT CHORD	2x4 DF No.1&Btr
WEBS	2x4 HF Std *Except*
	8-42,2-41: 2x6 DF 1800F 1.6E
OTHERS	2x4 HF Std

BRACING-

TOP CHORD	Structural wood sheathing directly applied, except end verticals.
BOT CHORD	Rigid ceiling directly applied.

REACTIONS. (lb/size) 41=1192/0-5-8, 42=1216/0-5-8
Max Horz 41=-75(LC 17)
Max Uplift 41=-321(LC 8), 42=-352(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

TOP CHORD	3-4=-2601/976, 4-5=-2735/1083, 5-6=-2736/1103, 6-7=-3400/1235, 7-8=-615/225, 10-42=-1216/533, 8-10=-471/256, 14-41=-1192/579
BOT CHORD	13-14=-559/1367, 12-13=-1107/2841, 11-12=-1186/3270, 10-11=-1080/2930
WEBS	5-12=-420/1376, 3-28=-282/1101, 13-28=-283/1102, 4-13=-512/353, 4-12=-430/283, 6-12=-780/425, 7-11=0/382, 7-10=-2622/1089, 3-14=-1773/797

NOTES-

1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 2-0-0 to 5-1-3, Exterior(2) 5-1-3 to 15-6-0, Corner(3) 15-6-0 to 18-7-3 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are MT20 plates unless otherwise indicated.
5) All plates are 2x3 MT20 unless otherwise indicated.
6) Gable studs spaced at 1-4-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) Bearing at joint(s) 41, 42 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 321 lb uplift at joint 41 and 352 lb uplift at joint 42.
11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

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REGISTERED PROFESSIONAL ENGINEER
89200PE
OREGON
MAY 14, 2014
DAVID MERRILL BAXTER

EXPIRES: 12-31-2019
April 12,2018

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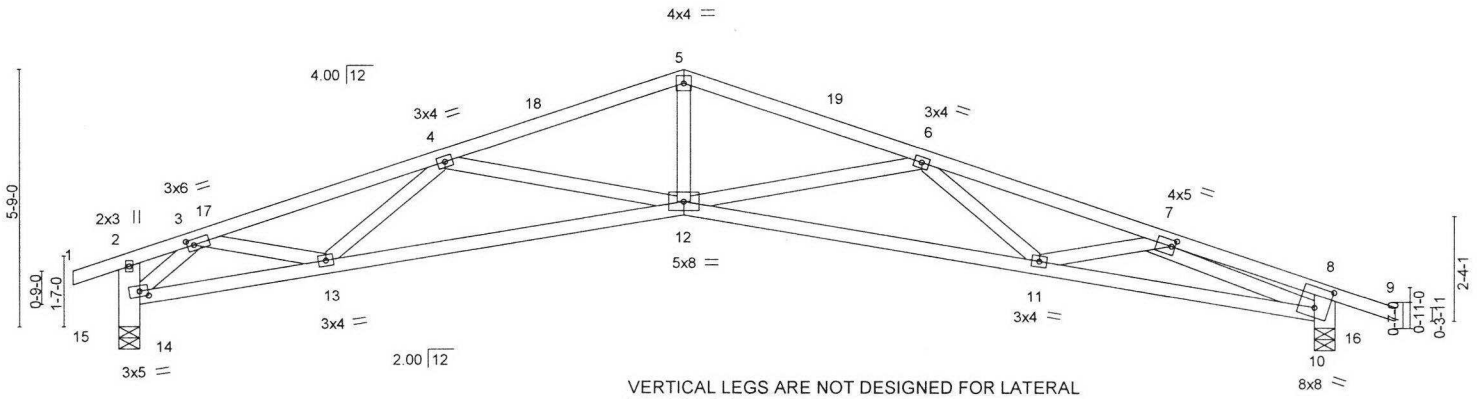
MiTek
250 Klug Circle
Corona, CA 92880

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino	
17-OT9956	A02	SCISSORS	12	1	Job Reference (optional)	K4488195

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015 — 8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:41 2018 Page 1
ID:EKXFkE3nUaaiMJsziER1roynf_N-N1flOJg5g_6hnjRCvaP3DutX3O8NNbVNMxsSBdzRU2m

1-0-0	1-11-5	7-2-10	12-6-0	17-9-6	23-0-11	27-0-0	28-4-0
1-0-0	1-11-5	5-3-6	5-3-6	5-3-6	5-3-6	3-11-5	1-4-0

Scale = 1:49.4



VERTICAL LEGS ARE NOT DESIGNED FOR LATERAL LOADS IMPOSED BY SUPPORTS (BEARINGS).

0-5-8	4-6-15	12-6-0	20-5-1	27-0-0
0-5-8	4-1-7	7-11-1	7-11-1	6-6-15

Plate Offsets (X,Y)-- [3:0-1-12,0-1-8], [7:0-1-0,0-1-12], [8:0-2-14,0-0-0], [10:0-3-12,0-5-8], [14:0-2-4,0-1-8]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.38	Vert(LL)	-0.25	11-12	>999	240	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	0.56	Vert(TL)	-0.59	11-12	>542	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.88	Horz(TL)	0.31	16	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-AS							Weight: 119 lb	FT = 0%

LUMBER-
TOP CHORD 2x4 DF No.1&Btr
BOT CHORD 2x4 DF No.1&Btr
WEBS 2x4 HF Std *Except*
8-16,2-15: 2x6 DF 1800F 1.6E

BRACING-
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied.

REACTIONS. (lb/size) 15=1192/0-5-8, 16=1216/0-5-8
Max Horz 15=-75(LC 17)
Max Uplift 15=-321(LC 8), 16=-352(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 3-4=-2601/526, 4-5=-2735/530, 5-6=-2736/540, 6-7=-3400/704, 7-8=-615/122, 10-16=-1216/352, 8-10=-471/188, 14-15=-1192/329
BOT CHORD 13-14=-383/1367, 12-13=-613/2841, 11-12=-646/3270, 10-11=-670/2930
WEBS 5-12=-182/1376, 3-13=-90/1102, 4-13=-512/220, 4-12=-430/283, 6-12=-780/376, 7-11=0/382, 7-10=-2622/686, 3-14=-1773/489

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 2-0-0 to 5-1-3, Interior(1) 5-1-3 to 15-6-0, Exterior(2) 15-6-0 to 18-7-3 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) Bearing at joint(s) 15, 16 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
6) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 321 lb uplift at joint 15 and 352 lb uplift at joint 16.
7) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

City of Portland
REVIEWED FOR CODE COMPLIANCE
MAY 17 2018
Permit Number

REGISTERED PROFESSIONAL ENGINEER
89200PE
OREGON
MAY 14, 2014
DAVID MERRILL BAXTER

EXPIRES: 12-31-2019
April 12,2018

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

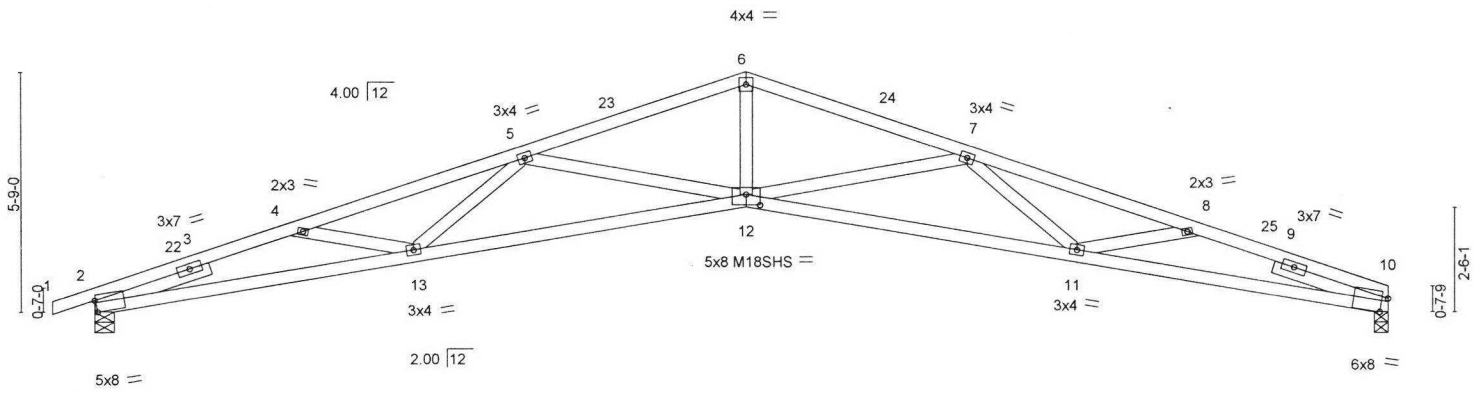
MiTek
250 Klug Circle
Corona, CA 92880

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino
17-OT9956	A03	SCISSORS	24	1	K4488196
					Job Reference (optional)

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015 — 8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:42 2018 Page 1
ID:EKXFkE3nUaaiMJsziER1roynf_N-sDCgbfgjRIEYPs0OTlwlm5QahoNc663Xabc?k4zRU2I

1-0-0	4-11-5	10-2-10	15-6-0	20-9-6	26-0-11	30-10-6
1-0-0	4-11-5	5-3-6	5-3-6	5-3-6	5-3-6	4-9-11

Scale = 1:53.1



7-6-15	15-6-0	23-5-1	30-10-6
7-6-15	7-11-1	7-11-1	7-5-5

Plate Offsets (X,Y)-- [2:0-0-5,Edge], [10:0-1-11,Edge], [12:0-4-0,0-3-0]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.89	Vert(LL)	-0.56	12-13	>659	240	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	1.00	Vert(TL)	-1.22	12-13	>303	180	M18SHS	185/148
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.60	Horz(TL)	0.55	10	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-AS							Weight: 126 lb	FT = 0%

LUMBER-
TOP CHORD 2x4 DF No.2
BOT CHORD 2x4 DF No.2
WEBS 2x4 HF Std *Except*
6-12: 2x4 DF No.2
SLIDER Left 2x4 HF Std 2-10-6, Right 2x4 HF Std 2-10-6

BRACING-
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS. (lb/size) 10=1295/0-3-14, 2=1361/0-5-8
Max Horz 2=113(LC 12)
Max Uplift 10=-309(LC 9), 2=-361(LC 8)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-4=-4647/1167, 4-5=-4671/1034, 5-6=-3703/753, 6-7=-3703/763, 7-8=-4621/971, 8-10=-4554/1106
BOT CHORD 2-13=-1132/4382, 12-13=-1018/4398, 11-12=-882/4374, 10-11=-1000/4288
WEBS 6-12=-339/2008, 5-13=0/331, 5-12=-972/429, 7-12=-954/430, 7-11=0/306, 8-11=0/270

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) -1-0-0 to 2-1-1, Interior(1) 2-1-1 to 15-6-0, Exterior(2) 15-6-0 to 18-7-1 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) All plates are MT20 plates unless otherwise indicated.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Bearing at joint(s) 10, 2 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 309 lb uplift at joint 10 and 361 lb uplift at joint 2.
8) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

City of Portland
REVIEWED FOR CODE COMPLIANCE
MAY 17 2018
Permit Number

REGISTERED PROFESSIONAL ENGINEER
89200PE
OREGON
MAY 14, 2014
DAVID MERRILL BAXTER

EXPIRES: 12-31-2019
April 12,2018

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
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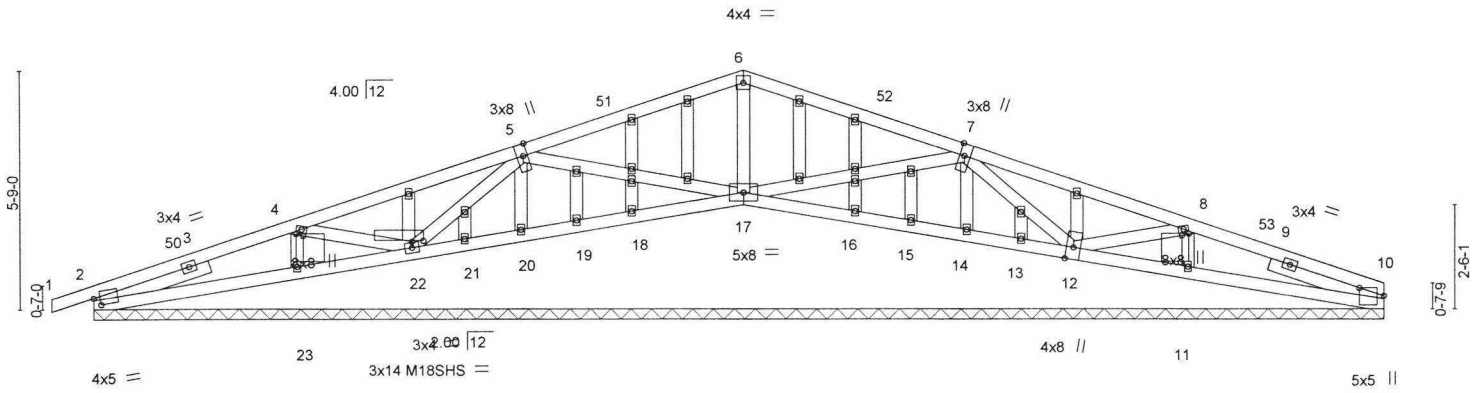
MiTek
250 Klug Circle
Corona, CA 92880

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino	
17-OT9956	A04	GABLE	4	1	Job Reference (optional)	K4488197

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015 — 8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:44 2018 Page 1
ID:EKXFkE3nUaaiMJsziER1roynf_N-obKR0LizzvUGeAAnajymrWV3BbHAa6Ip2v56oyzRU2j

1-0-0	4-11-5	10-2-10	15-6-0	20-9-6	26-0-11	30-10-6
1-0-0	4-11-5	5-3-6	5-3-6	5-3-6	5-3-6	4-9-11

Scale = 1:53.2



7-6-15	15-6-0	23-5-1	30-10-6
7-6-15	7-11-1	7-11-1	7-5-5

Plate Offsets (X,Y)-- [2:0-1-13,0-2-3], [5:0-0-4,0-0-1], [5:0-3-8,Edge], [7:0-3-8,Edge], [10:0-2-5,Edge], [12:0-0-14,0-0-2], [22:0-3-4,0-0-4], [32:0-0-9,0-2-0], [41:0-0-9,0-2-0]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.32	Vert(LL)	-0.00	1	n/r	120	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	0.10	Vert(TL)	0.00	1	n/r	90	M18SHS	185/148
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.23	Horz(TL)	-0.00	10	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-AS							Weight: 147 lb	FT = 0%

LUMBER-
TOP CHORD 2x4 DF No.2
BOT CHORD 2x4 DF No.2
WEBS 2x4 HF Std
OTHERS 2x4 HF Std
SLIDER Left 2x4 HF Std 2-10-6, Right 2x4 HF Std 2-10-6

BRACING-
TOP CHORD Structural wood sheathing directly applied.
BOT CHORD Rigid ceiling directly applied.

REACTIONS. All bearings 30-10-6.
(lb) - Max Horz 10=-352(LC 1), 2=352(LC 1), 10=-352(LC 1), 2=352(LC 1)
Max Uplift All uplift 100 lb or less at joint(s) 10 except 2=-141(LC 8), 22=-286(LC 12), 17=-142(LC 8), 12=-278(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 18, 19, 20, 21, 23, 16, 15, 14, 13, 11 except 10=328(LC 1), 2=399(LC 1), 22=563(LC 23), 17=438(LC 1), 12=550(LC 24), 10=328(LC 1), 2=399(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 2-4=-428/272, 8-10=-463/320
WEBS 6-17=-300/227, 4-22=-576/415, 5-22=-572/375, 5-17=-255/252, 7-17=-266/258, 7-12=-552/362, 8-12=-552/437

City of Portland
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NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -1-0-0 to 2-1-1, Exterior(2) 2-1-1 to 15-6-0, Corner(3) 15-6-0 to 18-7-1 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are MT20 plates unless otherwise indicated.
5) All plates are 2x3 MT20 unless otherwise indicated.
6) Gable requires continuous bottom chord bearing.
7) Gable studs spaced at 1-4-0 oc.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 10, 10 except (jt=lb) 2=141, 22=286, 17=142, 12=278, 2=141.
11) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 22, 17, 12, 18, 19, 20, 21, 23, 16, 15, 14, 13, 11.
12) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

EXPIRES: 12-31-2019
April 12,2018

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MiTek
250 Klug Circle
Corona, CA 92880

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino	
17-OT9956	A05	GABLE	3	1	Job Reference (optional)	K4488198

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015 — 8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:47 2018 Page 1
ID:EKXFkE3nUaaiMJsziER1roynf_N-CA0ZfNksGqsrVevMGrWTT97ZYpBnnJ_GktJmPHzRU2g

1-0-0	1-11-5	7-2-10	12-6-0	17-9-6	23-0-11	27-0-0	28-4-0
1-0-0	1-11-5	5-3-6	5-3-6	5-3-6	5-3-6	3-11-5	1-4-0

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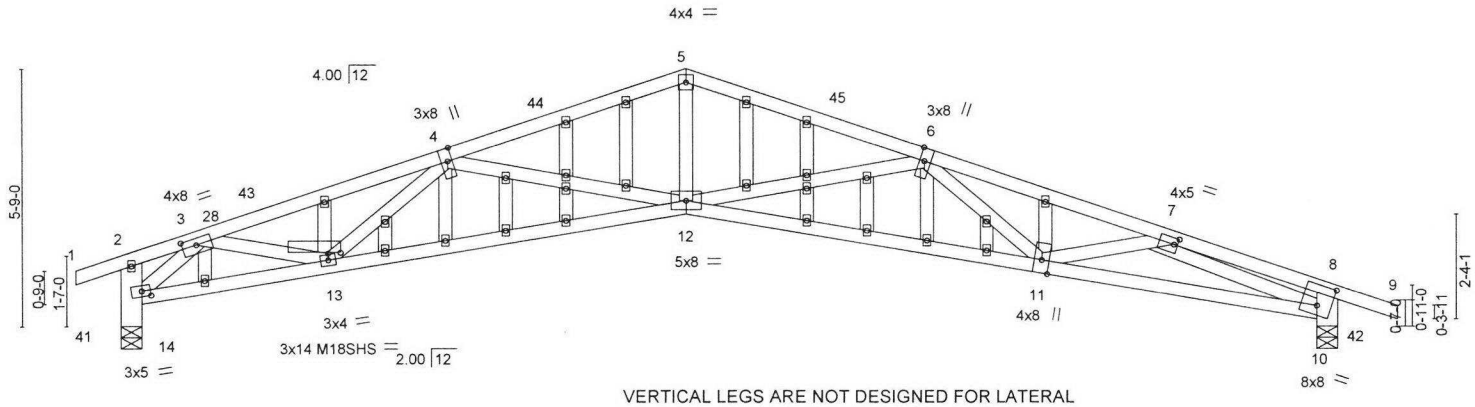

VERTICAL LEGS ARE NOT DESIGNED FOR LATERAL LOADS IMPOSED BY SUPPORTS (BEARINGS).

0-5-8	4-6-15	12-6-0	20-5-1	27-0-0
0-5-8	4-1-7	7-11-1	7-11-1	6-6-15

Plate Offsets (X,Y)-- [3:0-3-12,0-1-12], [4:0-0-4,0-0-1], [4:0-3-8,Edge], [6:0-3-8,Edge], [7:0-1-0,0-1-12], [8:0-2-14,0-0-0], [10:0-3-12,0-5-8], [11:0-0-14,0-0-2], [13:0-3-4,0-0-4], [14:0-2-4,0-1-8]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.38	Vert(LL)	-0.25	11-12	>999	240	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	0.56	Vert(TL)	-0.59	11-12	>542	180	M18SHS	185/148
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.88	Horz(TL)	0.31	42	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-AS							Weight: 141 lb	FT = 0%

LUMBER-
TOP CHORD 2x4 DF No.1&Btr
BOT CHORD 2x4 DF No.1&Btr
WEBS 2x4 HF Std *Except*
8-42,2-41: 2x6 DF 1800F 1.6E
OTHERS 2x4 HF Std

BRACING-
TOP CHORD Structural wood sheathing directly applied, except end verticals.
BOT CHORD Rigid ceiling directly applied.

REACTIONS. (lb/size) 41=1192/0-5-8, 42=1216/0-5-8
Max Horz 41=-75(LC 17)
Max Uplift 41=-321(LC 8), 42=-352(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 3-4=-2601/976, 4-5=-2735/1083, 5-6=-2736/1103, 6-7=-3400/1235, 7-8=-615/225, 10-42=-1216/533, 8-10=-471/256, 14-41=-1192/579
BOT CHORD 13-14=-559/1367, 12-13=-1107/2841, 11-12=-1186/3270, 10-11=-1080/2930
WEBS 5-12=-420/1376, 3-28=-282/1101, 13-28=-283/1102, 4-13=-512/353, 4-12=-430/283, 6-12=-780/425, 7-11=0/382, 7-10=-2622/1089, 3-14=-1773/797

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 2-0-0 to 5-1-3, Exterior(2) 5-1-3 to 15-6-0, Corner(3) 15-6-0 to 18-7-3 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are MT20 plates unless otherwise indicated.
5) All plates are 2x3 MT20 unless otherwise indicated.
6) Gable studs spaced at 1-4-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) Bearing at joint(s) 41, 42 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 41=321, 42=352.
11) This truss design requires that a minimum of 7/16" structural wood sheathing be applied directly to the top chord and 1/2" gypsum sheetrock be applied directly to the bottom chord.

City of Portland
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MAY 17 2018
Permit Number

REGISTERED PROFESSIONAL ENGINEER
89200PE
OREGON
MAY 14, 2014
DAVID MERRILL BAXTER

EXPIRES: 12-31-2019
April 12,2018

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MiTek
250 Klug Circle
Corona, CA 92880

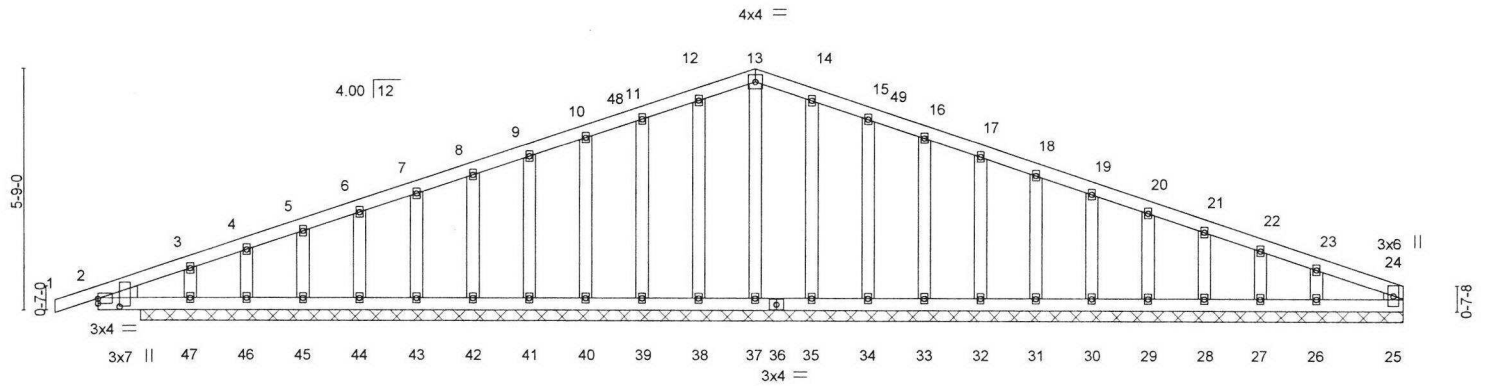
Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino
17-OT9956	A07	Roof Special Supported Gable	1	1	K4488199
					Job Reference (optional)

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015

8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:50 2018 Page 1
ID:EKXFkE3nUaaiMJsziER1roynf_N-dlhiHOnkZlEPM5dxxz3A5nl6R0JG_skiQrYQ0czRU2d

1-0-0 | 15-6-0 | 30-10-8
1-0-0 | 15-6-0 | 15-4-8

Scale = 1:52.6



30-10-8
30-10-8

Plate Offsets (X,Y)-- [2:0-0-0,0-1-6], [2:0-2-5,0-6-1]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.21	Vert(LL)	-0.00	1	n/r	120	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	0.19	Vert(TL)	-0.01	1-2	n/r	90		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.11	Horz(TL)	-0.01	25	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-R							Weight: 159 lb	FT = 0%

LUMBER-
TOP CHORD 2x4 DF No.2
BOT CHORD 2x4 DF No.2
WEBS 2x6 DF No.2
OTHERS 2x4 HF Std
WEDGE
Left: 2x4 HF Std

BRACING-
TOP CHORD Structural wood sheathing directly applied or 10-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS. All bearings 29-10-8.
(lb) - Max Horz 47=108(LC 16)
Max Uplift All uplift 100 lb or less at joint(s) 25, 38, 39, 40, 41, 42, 43, 44, 45, 46, 35, 34, 33, 32, 31, 30, 29, 28, 27, 26 except 47=-128(LC 8)
Max Grav All reactions 250 lb or less at joint(s) 25, 37, 38, 39, 40, 41, 42, 43, 44, 45, 46, 35, 34, 33, 32, 31, 30, 29, 28, 27, 26 except 47=413(LC 1)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) -1-0-0 to 2-2-0, Exterior(2) 2-2-0 to 15-6-0, Corner(3) 15-6-0 to 18-7-1 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) All plates are 2x3 MT20 unless otherwise indicated.
5) Gable studs spaced at 1-4-0 oc.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 25, 38, 39, 40, 41, 42, 43, 44, 45, 46, 35, 34, 33, 32, 31, 30, 29, 28, 27, 26 except (jt=lb) 47=128.
9) NA

City of Portland
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EXPIRES: 12-31-2019
April 12,2018

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
Design valid for use only with MiTek® connectors. This design is based only upon parameters shown, and is for an individual building component, not a truss system. Before use, the building designer must verify the applicability of design parameters and properly incorporate this design into the overall building design. Bracing indicated is to prevent buckling of individual truss web and/or chord members only. Additional temporary and permanent bracing is always required for stability and to prevent collapse with possible personal injury and property damage. For general guidance regarding the fabrication, storage, delivery, erection and bracing of trusses and truss systems, see **ANSI/TPI1 Quality Criteria, DSB-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 218 N. Lee Street, Suite 312, Alexandria, VA 22314.

MiTek
250 Klug Circle
Corona, CA 92880

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino
17-OT9956	B01	GABLE	2	1	K4488200
					Job Reference (optional)

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015 — 8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:51 2018 Page 1
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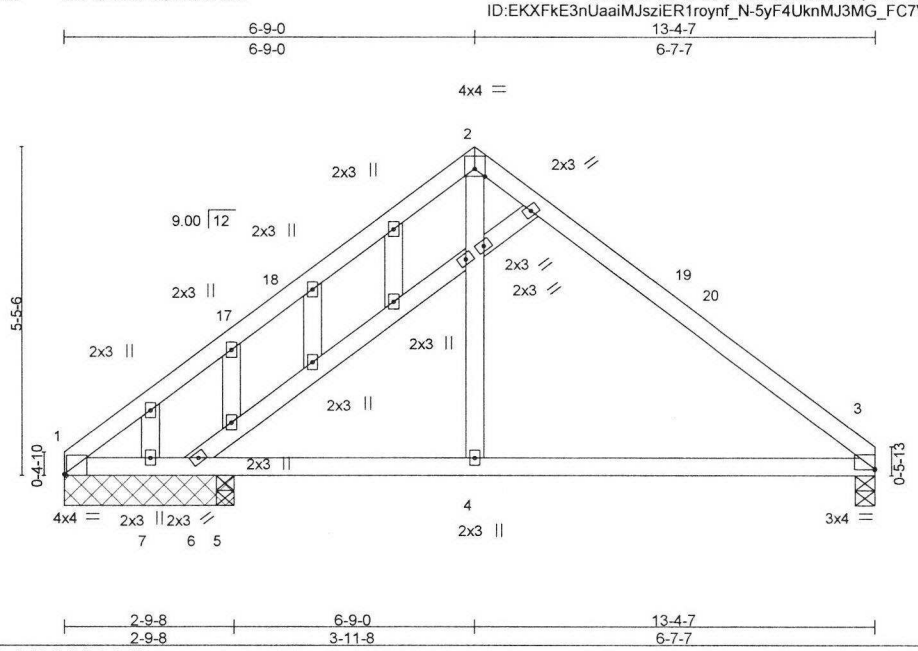

Plate Offsets (X,Y)-- [1:0-0-7,Edge], [3:0-0-0,0-0-3]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.48	Vert(LL)	0.06	3-4	>999	240	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	0.49	Vert(TL)	-0.13	3-4	>979	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.12	Horz(TL)	0.01	3	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-R							Weight: 61 lb	FT = 0%

LUMBER-
TOP CHORD 2x4 DF No.2
BOT CHORD 2x4 DF No.2
WEBS 2x4 HF Std
OTHERS 2x4 HF Std

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 2-9-8 except (jt=length) 3=0-3-14, 5=0-3-8.
(lb) - Max Horz 1=-146(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 1, 5 except 3=-112(LC 13), 6=-261(LC 23), 7=-322(LC 12)
Max Grav All reactions 250 lb or less at joint(s) 6, 5 except 1=346(LC 20), 3=530(LC 1), 7=420(LC 19)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-624/172, 2-3=-618/168
BOT CHORD 1-7=-41/398, 6-7=-41/398, 5-6=-41/398, 4-5=-41/398, 3-4=-41/398
WEBS 2-4=0/253

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-0-0 to 3-0-0, Interior(1) 3-0-0 to 6-9-0, Exterior(2) 6-9-0 to 9-9-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) Gable studs spaced at 1-4-0 oc.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 5 except (jt=lb) 3=112, 6=261, 7=322.

City of Portland
REVIEWED FOR CODE COMPLIANCE
MAY 17 2018
Permit Number

REGISTERED PROFESSIONAL ENGINEER
89200PE
OREGON
MAY 14, 2014
DAVID MERRILL BAXTER

EXPIRES: 12-31-2019
April 12,2018

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MiTek
250 Klug Circle
Corona, CA 92880

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino	
17-OT9956	B02	Roof Special	7	1	Job Reference (optional)	K4488201

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015 — 8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:52 2018 Page 1
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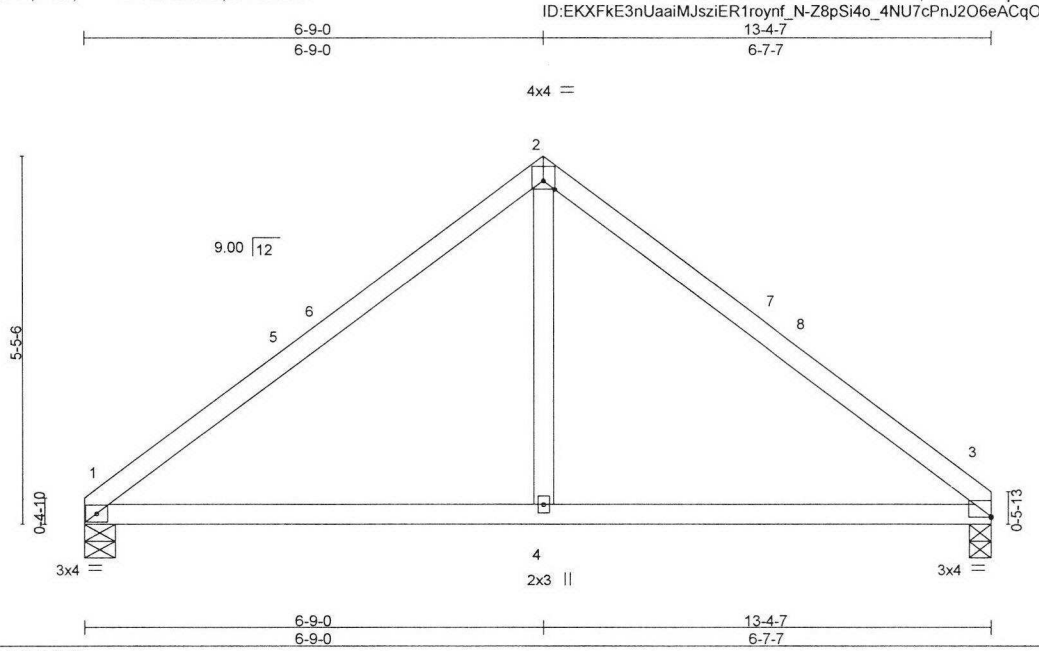

Plate Offsets (X,Y)-- [3:0-0-0,0-0-3]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.49	Vert(LL)	-0.06	1-4	>999	240	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	0.37	Vert(TL)	-0.12	1-4	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.14	Horz(TL)	0.01	3	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-R							Weight: 47 lb	FT = 0%

LUMBER-
TOP CHORD 2x4 DF No.2
BOT CHORD 2x4 DF No.2
WEBS 2x4 HF Std

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=545/0-5-8, 3=545/0-3-14
Max Horz 1=146(LC 9)
Max Uplift 1=-106(LC 12), 3=-105(LC 13)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-657/171, 2-3=-658/171
BOT CHORD 1-4=-56/430, 3-4=-56/430
WEBS 2-4=0/306

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-2-12 to 3-2-12, Interior(1) 3-2-12 to 6-9-0, Exterior(2) 6-9-0 to 9-9-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 1=106, 3=105.

City of Portland
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MAY 17 2018
Permit Number

REGISTERED PROFESSIONAL ENGINEER
89200PE
OREGON
MAY 14, 2014
DAVID MERRILL BAXTER

EXPIRES: 12-31-2019
April 12,2018

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MiTek
250 Klug Circle
Corona, CA 92880

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino
17-OT9956	B03	Roof Special	5	1	K4488202
					Job Reference (optional)

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015 — 8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:53 2018 Page 1
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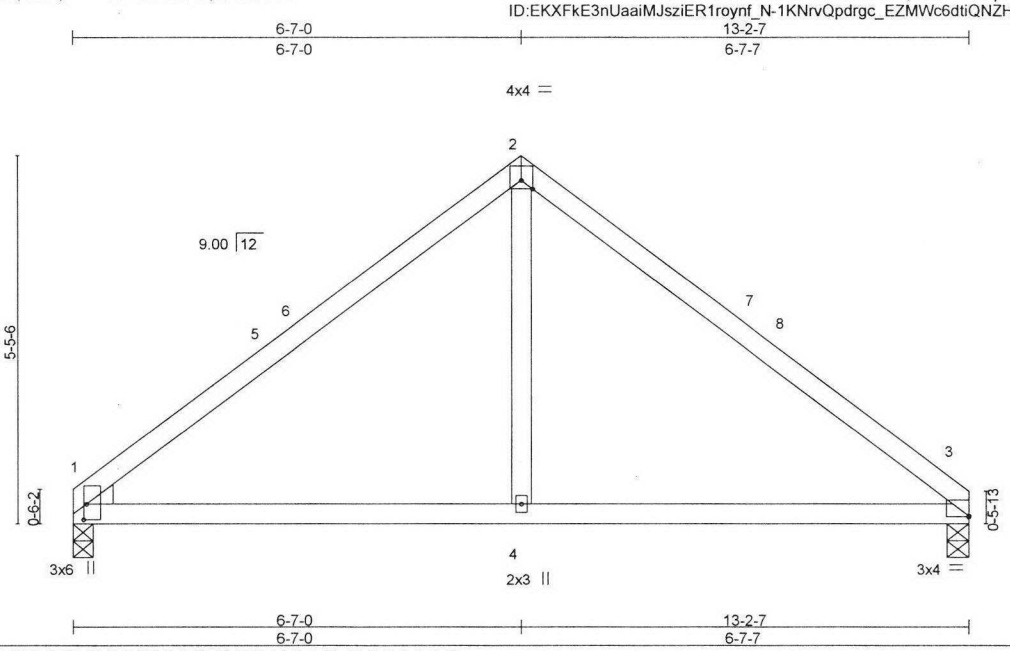

Plate Offsets (X,Y)-- [1:0-2-12,0-0-8], [1:0-1-12,0-5-7], [1:0-0-14,0-1-3], [3:0-0-0,0-0-3]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.49	Vert(LL)	-0.06	3-4	>999	240	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	0.37	Vert(TL)	-0.12	3-4	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.14	Horz(TL)	0.01	3	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-R							Weight: 47 lb	FT = 0%

LUMBER-
TOP CHORD 2x4 DF No.2
BOT CHORD 2x4 DF No.2
WEBS 2x4 HF Std
WEDGE
Left: 2x4 HF Std

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 3=542/0-3-14, 1=542/0-3-8
Max Horz 1=-146(LC 8)
Max Uplift 3=-105(LC 13), 1=-104(LC 12)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-652/171, 2-3=-652/171
BOT CHORD 1-4=-55/425, 3-4=-55/425
WEBS 2-4=0/305

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-3-12 to 3-3-12, Interior(1) 3-3-12 to 6-9-0, Exterior(2) 6-9-0 to 9-9-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
4) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) except (jt=lb) 3=105, 1=104.

City of Portland
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OREGON
MAY 14, 2014
DAVID MERRILL BAXTER

EXPIRES: 12-31-2019
April 12,2018

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MiTek
250 Klug Circle
Corona, CA 92880

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino
17-OT9956	C01	Roof Special Supported Gable	1	1	K4488203
					Job Reference (optional)

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015

8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:54 2018 Page 1
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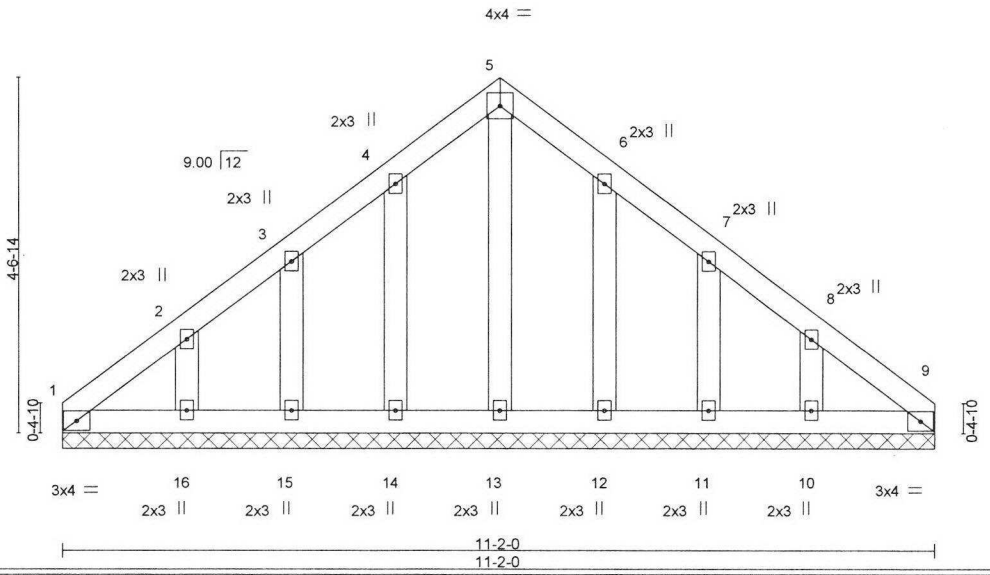

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.04	Vert(LL)	n/a	-	n/a	999	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	0.02	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.03	Horz(TL)	0.00	9	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-R							Weight: 53 lb	FT = 0%

LUMBER-
TOP CHORD 2x4 DF No.2
BOT CHORD 2x4 DF No.2
OTHERS 2x4 HF Std

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 11-2-0.
(lb) - Max Horz 1=-121(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 1, 9, 14, 15, 16, 12, 11, 10
Max Grav All reactions 250 lb or less at joint(s) 1, 9, 13, 14, 15, 16, 12, 11, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-0-0 to 2-11-0, Exterior(2) 2-11-0 to 5-7-0, Corner(3) 5-7-0 to 8-7-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 1-4-0 oc.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9, 14, 15, 16, 12, 11, 10.
9) Beveled plate or shim required to provide full bearing surface with truss chord at joint(s) 9.

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89200PE
OREGON
MAY 14, 2014
DAVID MERRILL BAXTER

EXPIRES: 12-31-2019
April 12,2018

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MiTek
250 Klug Circle
Corona, CA 92880

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino
17-OT9956	C02	Roof Special Supported Gable	1	1	K4488204
					Job Reference (optional)

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015 8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:55 2018 Page 1
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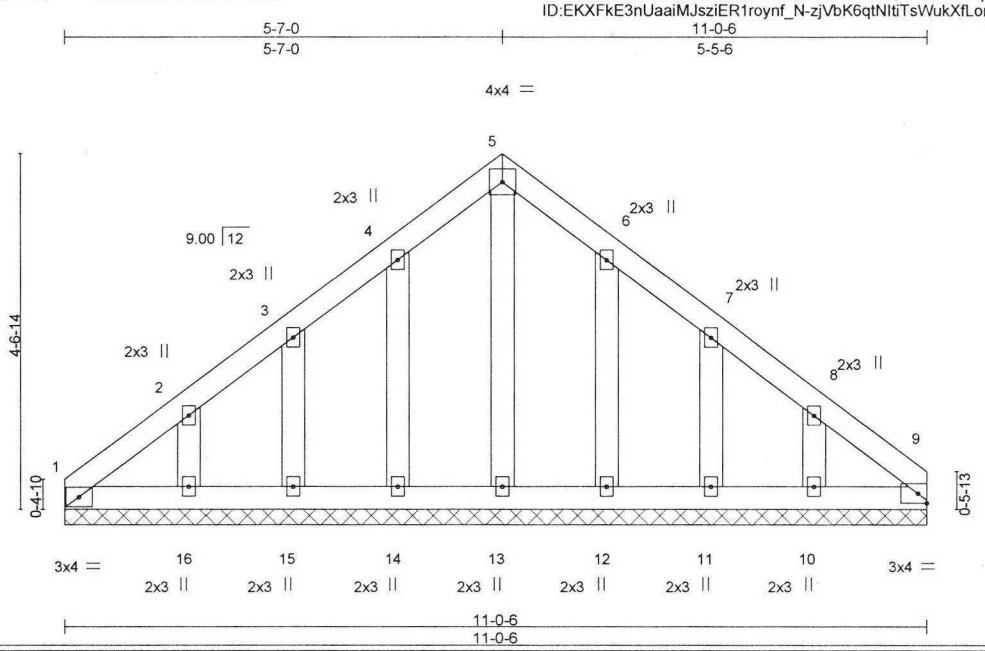

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.04	Vert(LL)	n/a	-	n/a	999	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	0.02	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.04	Horz(TL)	0.00	9	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-R							Weight: 53 lb	FT = 0%

LUMBER-
TOP CHORD 2x4 DF No.2
BOT CHORD 2x4 DF No.2
OTHERS 2x4 HF Std

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 11-0-6.
(lb) - Max Horz 1=-121(LC 8)
Max Uplift All uplift 100 lb or less at joint(s) 1, 9, 14, 15, 16, 12, 11, 10
Max Grav All reactions 250 lb or less at joint(s) 1, 9, 13, 14, 15, 16, 12, 11, 10

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-0-0 to 2-11-0, Exterior(2) 2-11-0 to 5-7-0, Corner(3) 5-7-0 to 8-7-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) Gable requires continuous bottom chord bearing.
5) Gable studs spaced at 1-4-0 oc.
6) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
7) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1, 9, 14, 15, 16, 12, 11, 10.

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OREGON
MAY 14, 2014
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EXPIRES: 12-31-2019
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MiTek
250 Klug Circle
Corona, CA 92880

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino	
17-OT9956	D01	COMMON SUPPORTED GAB	4	1	Job Reference (optional)	K4488205

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015 8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:56 2018 Page 1
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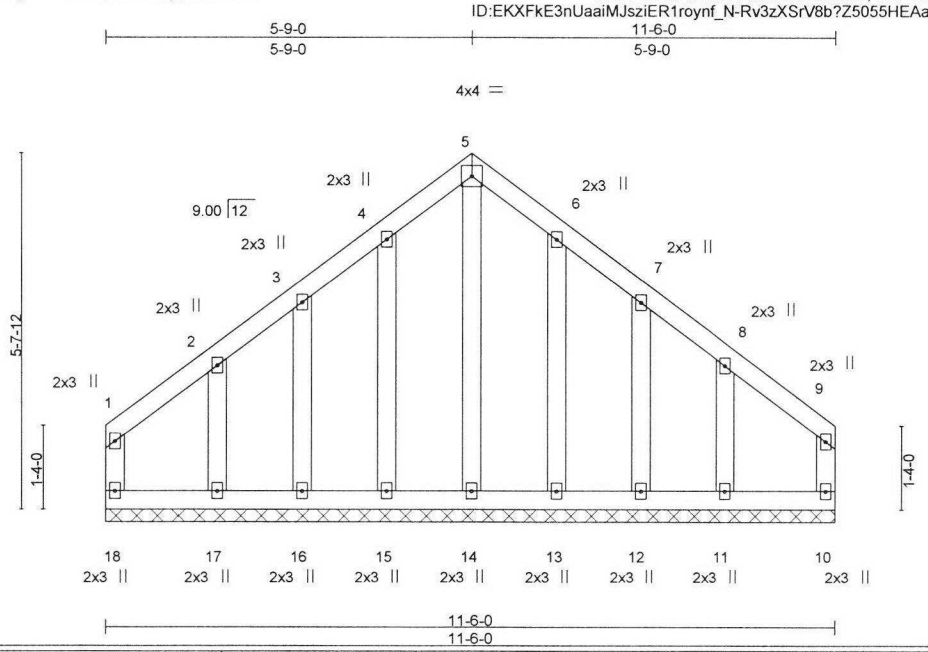

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.16	Vert(LL)	n/a	-	n/a	999	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	0.07	Vert(TL)	n/a	-	n/a	999		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.08	Horz(TL)	0.00	10	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-R							Weight: 65 lb	FT = 0%

LUMBER-
TOP CHORD 2x4 DF No.2
BOT CHORD 2x4 DF No.2
WEBS 2x4 HF Std
OTHERS 2x4 HF Std

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. All bearings 11-6-0.
(lb) - Max Horz 18=-166(LC 10)
Max Uplift All uplift 100 lb or less at joint(s) 18, 10, 15, 16, 13, 12 except 17=-130(LC 12), 11=-128(LC 13)
Max Grav All reactions 250 lb or less at joint(s) 18, 10, 14, 15, 16, 17, 13, 12, 11

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-1-12 to 3-1-0, Exterior(2) 3-1-0 to 5-9-0, Corner(3) 5-9-0 to 8-9-0 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
4) Gable requires continuous bottom chord bearing.
5) Truss to be fully sheathed from one face or securely braced against lateral movement (i.e. diagonal web).
6) Gable studs spaced at 1-4-0 oc.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
9) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 18, 10, 15, 16, 13, 12 except (jt=lb) 17=130, 11=128.

City of Portland
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MAY 17 2018
Permit Number

REGISTERED PROFESSIONAL ENGINEER
89200PE
OREGON
MAY 14, 2014
DAVID MERRILL BAXTER

EXPIRES: 12-31-2019
April 12,2018

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 rev. 10/03/2015 BEFORE USE.
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MiTek
250 Klug Circle
Corona, CA 92880

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino
17-OT9956	E01	GABLE	1	1	K4488206
					Job Reference (optional)

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015

8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:57 2018 Page 1
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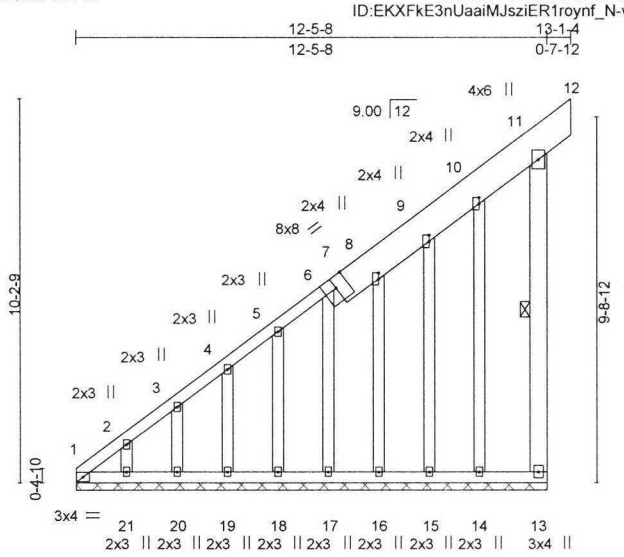

Plate Offsets (X,Y)-- [6:0-2-3,0-0-0], [7:0-0-0,0-1-12], [8:0-2-0,0-0-0], [9:0-2-0,0-0-0], [10:0-2-0,0-0-0]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.16	Vert(LL)	-0.00	11	n/r	120	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	0.26	Vert(TL)	0.00	11	n/r	90		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.18	Horz(TL)	0.00	13	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-R							Weight: 114 lb	FT = 0%

LUMBER-
TOP CHORD 2x10 DF No.2 *Except* 1-7: 2x4 DF No.2
BOT CHORD 2x4 DF No.2
WEBS 2x6 DF 1800F 1.6E
OTHERS 2x4 HF Std

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.
WEBS 1 Row at midpt 11-13

REACTIONS. All bearings 12-5-8.
(lb) - Max Horz 1=414(LC 9)
Max Uplift All uplift 100 lb or less at joint(s) 21, 20, 19, 18, 17, 16, 15, 14 except 13=-134(LC 9), 1=-125(LC 10)
Max Grav All reactions 250 lb or less at joint(s) 13, 21, 20, 19, 18, 17, 16, 15, 14 except 1=256(LC 9)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-652/620, 2-3=-584/561, 3-4=-528/516, 4-5=-474/472, 5-6=-414/423, 6-8=-351/369, 8-9=-314/340, 9-10=-253/281

NOTES-
1) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) 0-0-0 to 3-0-0, Exterior(2) 3-0-0 to 13-1-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
3) Gable requires continuous bottom chord bearing.
4) Gable studs spaced at 1-4-0 oc.
5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
6) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 21, 20, 19, 18, 17, 16, 15, 14 except (jt=lb) 13=134, 1=125.

City of Portland
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MAY 17 2018
Permit Number

REGISTERED PROFESSIONAL ENGINEER
89200PE
OREGON
MAY 14, 2014
DAVID MERRILL BAXTER

EXPIRES: 12-31-2019
April 12,2018

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MiTek
250 Klug Circle
Corona, CA 92880

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino
17-OT9956	E02	MONOPITCH	3	1	K4488207
					Job Reference (optional)

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015

8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:58 2018 Page 1
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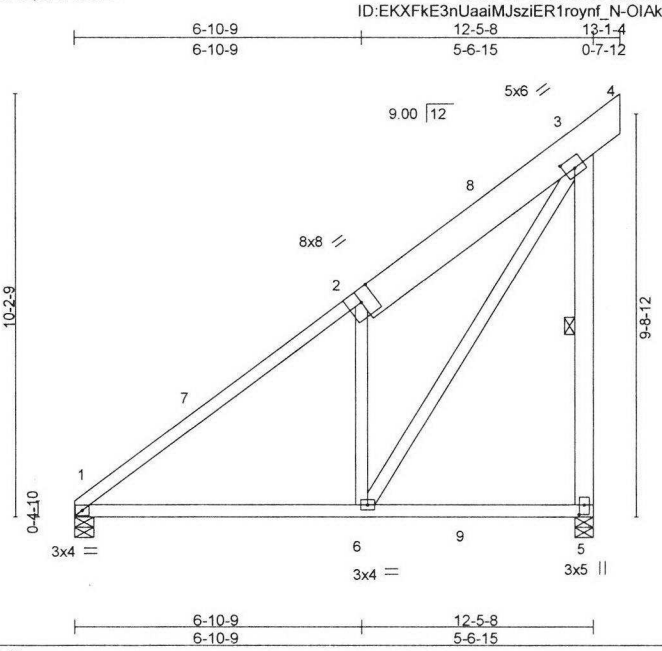

Plate Offsets (X,Y)-- [3:0-3-0,0-3-0], [5:0-2-12,0-1-8]

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.56	Vert(LL)	0.07	1-6	>999	240	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	0.38	Vert(TL)	-0.14	1-6	>999	180		
BCLL	0.0 *	Rep Stress Incr	NO	WB	0.73	Horz(TL)	0.00	5	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-R							Weight: 92 lb	FT = 0%

LUMBER-
TOP CHORD 2x10 DF No.2 *Except*
1-2: 2x4 DF No.2
BOT CHORD 2x4 DF No.2
WEBS 2x4 HF Std *Except*
3-5: 2x6 DF 1800F 1.6E

BRACING-
TOP CHORD Structural wood sheathing directly applied or 6-0-0 oc purlins, except end verticals.
BOT CHORD Rigid ceiling directly applied or 6-0-0 oc bracing.
WEBS 1 Row at midpt 3-5

REACTIONS. (lb/size) 5=1259/0-5-8, 1=455/0-5-8
Max Horz 1=413(LC 9)
Max Uplift 5=-465(LC 12), 1=-35(LC 12)
Max Grav 5=1440(LC 19), 1=455(LC 1)

City of Portland
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MAY 17 2018
Permit Number

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-571/85, 2-3=-548/256, 3-4=-441/111, 3-5=-1332/586
BOT CHORD 1-6=-265/471
WEBS 2-6=-453/359, 3-6=-370/707

NOTES-
1) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) 0-2-12 to 3-2-12, Interior(1) 3-2-12 to 13-1-4 zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members, with BCDL = 10.0psf.
4) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 1 except (jt=lb) 5=465.
5) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 689 lb down and 186 lb up at 13-1-4 on top chord. The design/selection of such connection device(s) is the responsibility of others.
6) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard
1) Dead + Roof Live (balanced): Lumber Increase=1.15, Plate Increase=1.15
Uniform Loads (plf)
Vert: 1-3=-64, 3-4=-64, 1-5=-20
Concentrated Loads (lb)
Vert: 4=-650(F)

REGISTERED PROFESSIONAL ENGINEER
89200PE
OREGON
MAY 14, 2014
DAVID MERRILL BAXTER

EXPIRES: 12-31-2019
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MiTek
250 Klug Circle
Corona, CA 92880

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino
17-OT9956	J01	GABLE	2	1	K4488208
					Job Reference (optional)

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015

8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:59 2018 Page 1
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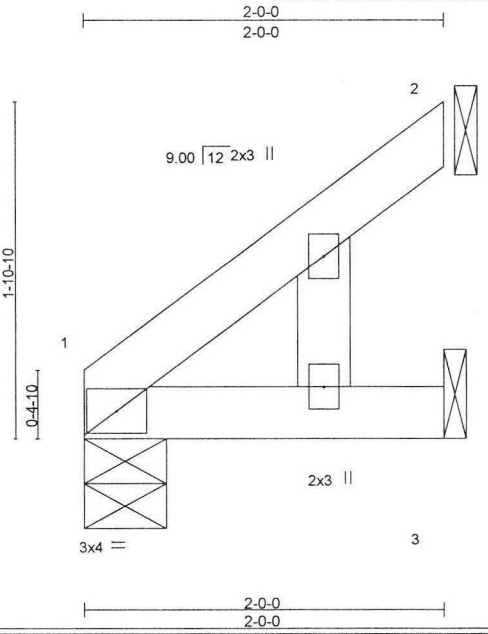

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.06	Vert(LL)	-0.00	1	>999	240	MT20	185/148
TCDL	7.0	Lumber DOL	1.15	BC	0.03	Vert(TL)	-0.00	1-3	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-P							Weight: 7 lb	FT = 0%

LUMBER-
TOP CHORD 2x4 DF No.2
BOT CHORD 2x4 DF No.2
OTHERS 2x4 HF Std

BRACING-
TOP CHORD Structural wood sheathing directly applied or 2-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=82/0-5-8, 2=62/Mechanical, 3=19/Mechanical
Max Horz 1=69(LC 12)
Max Uplift 2=-62(LC 12)
Max Grav 1=82(LC 1), 2=69(LC 19), 3=39(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-
1) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Corner(3) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1.
3) Gable studs spaced at 1-4-0 oc.
4) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
5) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
6) Refer to girder(s) for truss to truss connections.
7) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.

City of Portland
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MAY 17 2018
Permit Number

REGISTERED PROFESSIONAL ENGINEER
89200PE
OREGON
MAY 14, 2014
DAVID MERRILL BAXTER

EXPIRES: 12-31-2019
April 12,2018

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MiTek
250 Klug Circle
Corona, CA 92880

Job	Truss	Truss Type	Qty	Ply	Jessey Cereghino
17-OT9956	J02	JACK	10	1	K4488209
					Job Reference (optional)

PRECISION TRUSS & LUMBER, INC., CLACKAMAS,OR. 97015 8.210 s Feb 12 2018 MiTek Industries, Inc. Thu Apr 12 11:07:59 2018 Page 1
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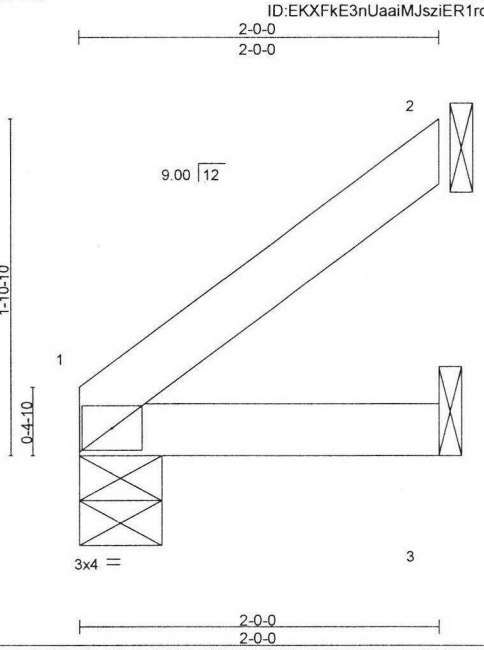

LOADING (psf)		SPACING-	2-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	25.0	Plate Grip DOL	1.15	TC	0.06	Vert(LL)	-0.00	1	>999	240	MT20	220/195
TCDL	7.0	Lumber DOL	1.15	BC	0.03	Vert(TL)	-0.00	1-3	>999	180		
BCLL	0.0 *	Rep Stress Incr	YES	WB	0.00	Horz(TL)	-0.00	2	n/a	n/a		
BCDL	10.0	Code IRC2012/TPI2007		Matrix-P							Weight: 6 lb	FT = 0%

LUMBER-
TOP CHORD 2x4 DF No.2
BOT CHORD 2x4 DF No.2

BRACING-
TOP CHORD Structural wood sheathing directly applied or 2-0-0 oc purlins.
BOT CHORD Rigid ceiling directly applied or 10-0-0 oc bracing.

REACTIONS. (lb/size) 1=82/0-5-8, 2=62/Mechanical, 3=19/Mechanical
Max Horz 1=69(LC 12)
Max Uplift 2=-62(LC 12)
Max Grav 1=82(LC 1), 2=69(LC 19), 3=39(LC 3)

FORCES. (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.

NOTES-
1) Wind: ASCE 7-10; Vult=120mph (3-second gust) V(IRC2012)=95mph; TCDL=4.2psf; BCDL=6.0psf; h=25ft; Cat. II; Exp C; enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2) zone; cantilever left and right exposed ; end vertical left and right exposed;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
2) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
3) * This truss has been designed for a live load of 20.0psf on the bottom chord in all areas where a rectangle 3-6-0 tall by 2-0-0 wide will fit between the bottom chord and any other members.
4) Refer to girder(s) for truss to truss connections.
5) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 100 lb uplift at joint(s) 2.

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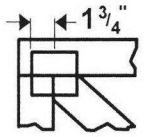
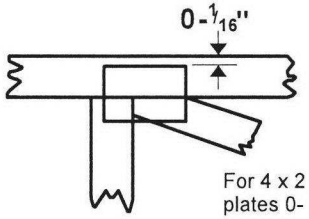
MiTek
250 Klug Circle
Corona, CA 92880

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0- 1/16" from outside edge of truss.

This symbol indicates the required direction of slots in connector plates.

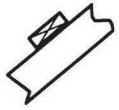
*** Plate location details available in MiTek 20/20 software or upon request.**

PLATE SIZE

4 x 4

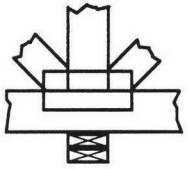
The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING

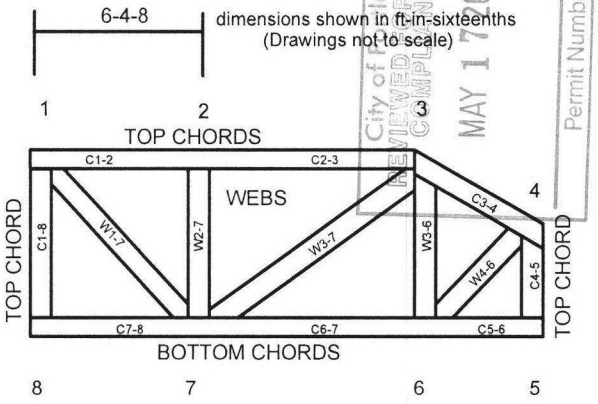


Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur. Min size shown is for crushing only.

Industry Standards:

ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
DSB-89: Design Standard for Bracing.
BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Trusses are designed for wind loads in the plane of the truss unless otherwise shown.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3 These truss designs rely on lumber values established by others.

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MiTek Engineering Reference Sheet: MII-7473 rev. 10/03/2015

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General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.

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