

City of Portland
Bureau of
Development Services
By *[Signature]* Date 12/6/10
Approved by
Planning & Zoning Review

- SHEET NOTES:
- 1. EXISTING LANDSCAPING TO REMAIN, UNO
 - 2. REGRADE AND PREP AS NEEDED FOR NEW CONSTRUCTION
 - 3. RAISED PATIO
 - 4. TRELLIS AND FIREPLACE
 - 5. NEW PORCH
 - 6. NEW GARAGE

SITE PLAN

SCALE 1/8" = 1'-0"

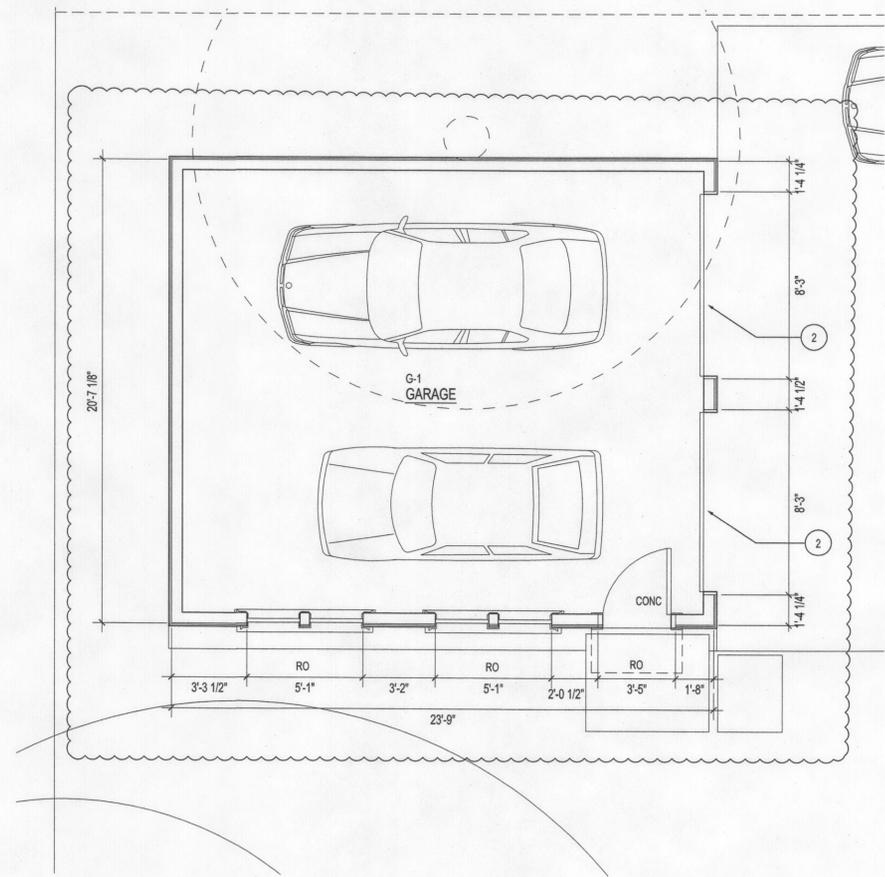
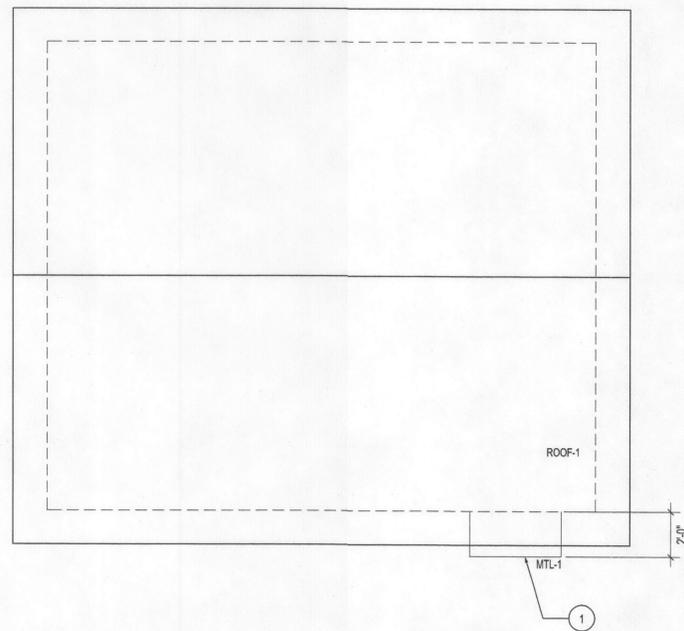
City of Portland
Reviewed for Code Compliance
DEC 08 2010
Permit Number

81543-25
grant park residence
portland, oregon

issues / revisions

permit set	9.29.2010
foundation	10.15.2010
framing	10.29.2010
revisions	11.17.2010

sheet title
SITE PLAN
sheet #
a1.01



GENERAL NOTES:
 1. ALL DIMENSIONS TO FACE OF FRAMING, UNO
 2. SEE SITE PLAN FOR LOCATION

SHEET NOTES:
 1. ROOF BELOW, ALIGN WITH DOOR TRIM
 2. GARAGE DOOR

GARAGE FLOOR PLAN

SCALE 1/4" = 1'-0"



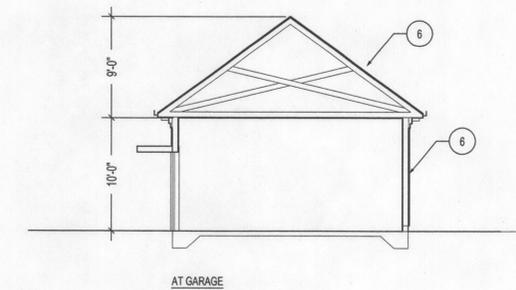
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 GARAGE

sheet #
a2.05



BUILDING SECTIONS

SCALE 1/8"=1'-0"

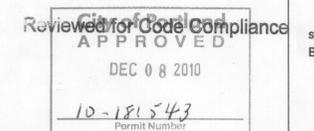
SHEET NOTES:

- 1. TYPICAL EXTERIOR PARTITION:
- 1.7. SIDING TO MATCH EXIST.
- 1.8. MOISTURE BARRIER.
- 1.11. EXTERIOR SHEATHING, SEE STRUCTURAL.
- 1.12. STUDS, SEE STRUCTURAL.
- 1.13. R-19 BATT INSULATION.
- 1.14. VAPOR BARRIER.
- 1.15. 5/8" GWB INTERIOR.
- 2. TYPICAL ROOF PARTITION:
- 2.1. ROOF-1
- 2.2. EXTERIOR SHEATHING, SEE STRUCTURAL.
- 2.3. ROOF VENTS.

- 2.4. PRE-ENGINEERED ROOF TRUSSES, SEE STRUCTURAL.
- 2.5. R-38 BATT INSULATION.
- 2.1. VAPOR BARRIER.
- 3. TYPICAL DECK PARTITION:
- 3.1. WD-5 DECKING
- 3.2. DECK BARRIER
- 3.3. EXTERIOR SHEATHING, SEE STRUCTURAL
- 3.4. SLOPED FRAMING, SEE STRUCTURAL
- 3.5. R-38 BATT INSULATION
- 3.6. VAPOR BARRIER.
- 4. TYPICAL FLOOR PARTITION:
- 4.1. R-25 BATT INSULATION
- 4.2. VAPOR BARRIER

- 4.3. FRAMING PER STRUCTURAL
- 4.4. SHEATHING PER STRUCTURAL
- 5. TYPICAL FLAT ROOF PARTITION:
- 5.1. ROOF-2
- 5.2. PROTECTION BOARD
- 5.3. SLOPED RIGID INSULATION
- 5.4. SHEATHING PER STRUCTURAL
- 5.5. FRAMING PER STRUCTURAL
- 5.6. R-38 BATT INSULATION
- 5.7. VAPOR BARRIER
- 6. FRAMING, WEATHERPROOFING, AND SIDING/ROOFING ONLY, NO INTERIOR FINISHES OR INSULATION
- 7. MATCH EXIST FLOOR HEIGHT

- 8. MATCH EXIST CEILING HEIGHT
- 9. MATCH EAVE ELEVATION

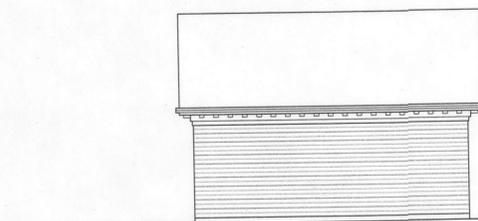


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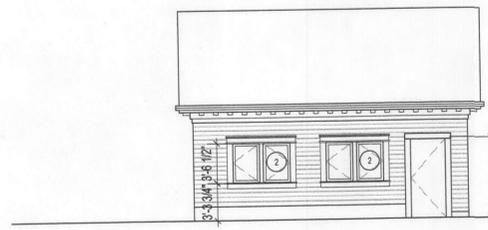
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 BUILDING SECTIONS

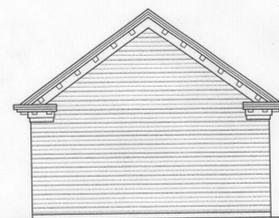
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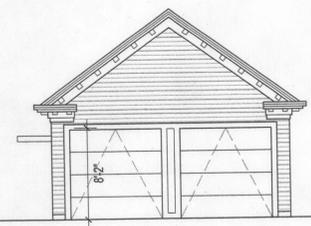
NORTH Elevation



SOUTH Elevation



WEST Elevation



EAST Elevation

GARAGE ELEVATIONS

SCALE 1/8"=1'-0"

SHEET NOTES:

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2. NEW WINDOW

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 GARAGE ELEVATIONS

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GENERAL STRUCTURAL NOTES

GENERAL NOTES:

- ALL CONSTRUCTION AND DESIGN SHALL CONFORM TO THE 2009 INTERNATIONAL BUILDING CODE AS AMENDED BY THE STATE OF OREGON.
- THE STRUCTURAL DRAWINGS SHALL BE UTILIZED IN CONJUNCTION WITH OTHER DESIGN CONSULTANT'S DRAWINGS (ARCHITECTURAL, MECHANICAL, ETC.). IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO COORDINATE THE REQUIREMENTS OF THE DRAWINGS INTO THEIR SHOP DRAWINGS AND CONSTRUCTION.
- THE GENERAL STRUCTURAL NOTES ARE INTENDED TO SERVE AS THE PROJECT STRUCTURAL SPECIFICATIONS.
- CONSTRUCTION SEQUENCE AND METHODS:**
 - THE STRUCTURAL DRAWINGS ARE INTENDED FOR THE STRUCTURE TO ACT AS A WHOLE ONCE CONSTRUCTION IS COMPLETE. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO ENSURE SAFETY AND STABILITY (I.E. TEMPORARY BRACING IF REQUIRED) DURING CONSTRUCTION AS A RESULT OF CONSTRUCTION METHODS AND SEQUENCES.
 - THE CONTRACTOR SHALL TAKE INTO ACCOUNT COLD WEATHER CONSTRUCTION AND THE EFFECTS OF THERMAL MOVEMENT DURING THE CONSTRUCTION SCHEDULE.
- THE CONTRACTOR SHALL FIELD VERIFY ALL EXISTING CONDITIONS. THE ARCHITECT AND/OR ENGINEER SHALL BE NOTIFIED OF ANY DISCREPANCY BETWEEN THE EXISTING CONDITIONS AND CONSTRUCTION DOCUMENTS.
- SUBMITTALS:**
 - SHOP DRAWINGS FOR ALL STRUCTURAL ITEMS SHALL BE SUBMITTED TO THE ARCHITECT PRIOR TO FABRICATION AND CONSTRUCTION. SUCH ITEMS INCLUDE:
 - STRUCTURAL STEEL, WOOD I-JOISTS AND GLUE-LAMINATED MEMBERS.
 SHOP DRAWINGS OR CONTRACTOR ENGINEERED DETAILS SHALL BEAR THE SEAL AND SIGNATURE OF A REGISTERED STRUCTURAL ENGINEER IN THE STATE OF OREGON IF IT DIFFERS FROM THE DESIGN OF THE STRUCTURAL DRAWINGS. ANY REVISION FROM THE STRUCTURAL DRAWINGS SHALL BE SUBMITTED TO THE ARCHITECT FOR REVIEW AND IS SUBJECT TO THE REVIEW AND ACCEPTANCE BY THE ENGINEER.
 - CALCULATIONS, DESIGN DRAWINGS, AND SHOP DRAWINGS FOR THE DESIGN, FABRICATION, AND CONSTRUCTION OF BIDDER DESIGN ITEMS SHALL BEAR THE SEAL AND SIGNATURE OF A REGISTERED STRUCTURAL ENGINEER IN THE STATE OF OREGON AND SHALL BE SUBMITTED TO THE ARCHITECT PRIOR TO FABRICATION. BIDDER DESIGN ITEMS FOR THIS PROJECT INCLUDE:
 - PREMANUFACTURED WOOD ROOF TRUSSES
 CALCULATIONS AND BIDDER DESIGN DRAWINGS SHALL INCLUDE THE DESIGN, CONNECTION TO THE STRUCTURE, AND ACCOUNTING OF ANY LOCALIZED EFFECTS THE CONNECTIONS OR SYSTEMS MAY INDUCE ON THE STRUCTURE. ALL SUCH BIDDER DESIGNED ITEMS SHALL BE BASED ON THE DESIGN REQUIREMENTS AS SPECIFIED IN THE GENERAL STRUCTURAL NOTES.

DESIGN CRITERIA:

- CODE: 2009 INTERNATIONAL BUILDING CODE AS AMENDED BY THE STATE OF OREGON (2010 OSSC).
- LOADS AND DESIGN CRITERIA:** THE FOLLOWING LIVE LOADS AND CRITERIA WERE USED IN ADDITION TO THE DEAD LOAD OF THE STRUCTURE.

LIVE LOADS:

ROOF	25 PSF (PLUS ADDED SNOW DRIFT IF SHOWN ON PLANS)
FLOORS	
FIRST FLOOR	40 PSF
SECOND FLOOR	40 PSF
STAIRS	40 PSF
DECKS	40 PSF

SOIL CRITERIA: PER THE MINIMUM REQUIREMENTS OF THE 2009 IBC

ALLOW. SOIL BEARING VALUES	1500 PSF (W/ 1/3 INCREASE FOR SHORT TERM LATERAL LOADS)
FOOTINGS	
PASSIVE	150 PCF
FRICTION COEFFICIENT	0.25

LATERAL CRITERIA:

WIND SEISMIC 95 MPH (3-SECOND GUST), EXPOSURE B iw = 1.0
 Ie = 1.0
 Ss = 0.972g S1 = 0.336g
 SITE CLASS D (PER IBC 1815.1.1 DEFAULT)
 Sds = 0.720g Sd1 = 0.387g
 SEISMIC DESIGN CATEGORY D
 LIGHT FRAMED WOOD SHEATHED SHEARWALLS
 DESIGN EQ BASE SHEAR, V = 4.23K EACH DIRECTION
 SEISMIC RESPONSE COEFFICIENT, Cs = 0.1108
 R = 6.5
 EQUIVALENT LATERAL FORCE PROCEDURE USED

CONCRETE AND REINFORCING STEEL:

- CONCRETE CONSTRUCTION SHALL CONFORM TO ACI 318-08 AND THE 2009 INTERNATIONAL BUILDING CODE AS AMENDED BY THE STATE OF OREGON.
- THE MINIMUM 28 DAY CONCRETE STRENGTHS SHALL BE AS FOLLOWS:
 $f_c = 2,500$ PSI..... FOR ALL USES UNLESS NOTED OTHERWISE
- ALL EXPOSED CONCRETE SURFACES SHALL HAVE MIX DESIGNS WITH AN AIR ENTRAINMENT ADMIXTURE INCLUDED.
- ADDITIONAL WATER SHALL NOT BE ADDED TO THE CONCRETE MIX AT THE JOBSITE. WATER REDUCING ADMIXTURES CONFORMING TO ASTM C494 MAY BE UTILIZED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS.
- IF CONCRETE IS TO BE POURED AGAINST AN EXISTING CONCRETE SURFACE, THE EXISTING SURFACE SHALL BE CLEANED AND ROUGHENED TO A MIN. 1/4" AMPLITUDE.
- SLEEVES, OPENINGS, CONDUITS, AND OTHER EMBEDDED ITEMS NOT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE APPROVED BY THE STRUCTURAL ENGINEER BEFORE POURING. CONDUITS EMBEDDED IN SLABS SHALL NOT BE LARGER IN OUTSIDE DIAMETER THAN ONE THIRD THE THICKNESS OF THE SLAB AND SHALL NOT BE SPACED CLOSER THAN THREE DIAMETERS ON CENTER. PROVIDE 3/4" CHAMFERS ON ALL EXPOSED CONCRETE EDGES UNLESS NOTED OTHERWISE.
- REINFORCING STEEL:**
 - REINFORCING STEEL SHALL BE DETAILED, FABRICATED, AND INSTALLED ACCORDING TO THE "MANUAL OF STANDARD PRACTICE OF REINFORCED CONCRETE CONSTRUCTION" BY THE CONCRETE REINFORCING STEEL INSTITUTE (CRSI).
 - REINFORCING STEEL SHALL CONFORM TO ASTM A615, GRADE 60 OR WELDABLE ASTM A706 GRADE 60.
 - ALL LAP SPLICES OF REINFORCEMENT SHALL CONFORM TO CLASS B LAPS AS SHOWN ON THE LAP SPLICE SCHEDULE PER THIS SHEET UNLESS NOTED OTHERWISE.
 - UNLESS NOTED OTHERWISE, REINFORCING STEEL SHALL HAVE THE MINIMUM COVER OR PROTECTION FOR THE FOLLOWING USES AS NOTED BELOW:

BEAMS, JOISTS, AND COLUMNS	1 1/2" (TO TIES OR STIRRUPS)
SLABS	1"
WALLS	
INTERIOR FACES	3/4"
EXPOSED TO EARTH OR WEATHER	1 1/2" (#5 BARS AND SMALLER)
FOOTINGS	2" (#6 BARS AND LARGER)
	3"
- CONCRETE WALLS**
 - PROVIDE THE MINIMUM WALL REINFORCING AS SHOWN BELOW UNLESS NOTED OTHERWISE.

WALL THICKNESS	REINFORCING
8"	#5 BARS AT 12" O.C. EA. WAY AT CL OF WALL
 - HOOKED DOWELS FROM FOUNDATIONS SHALL BE PROVIDED TO MATCH VERTICAL WALL REINFORCING.
 - PROVIDE HOOKED DOWELS MATCHING SLAB REINFORCING FROM WALLS TO SLABS OR HOOK SLAB REINFORCEMENT INTO WALLS.
 - UNLESS NOTED OTHERWISE, PLACE (2) #5 BARS ON ALL SIDES OF SLAB AND WALL OPENINGS EXTENDED 2'-6" BEYOND OPENING. PROVIDE (1) OR (2) 4'-0" LONG DIAGONAL #5 BARS AT EACH CORNER OF THE OPENING MATCHING THE LAYERS OF REINFORCING.
- ADDITIONAL CONCRETE ITEMS**
 - HEADED SHEAR STUDS AND DEFORMED BAR ANCHORS SHALL BE AN APPROVED NELSON PRODUCT OR APPROVED EQUAL.
 - WEDGE ANCHORS OR EXPANSION BOLTS SHALL BE HILTI KB-TZ OR AN APPROVED EQUAL SUBMITTED WITH ICC REPORTS TO THE ENGINEER FOR REVIEW.
 - EPOXY ANCHORS OR DOWELS SHALL BE INSTALLED WITH SIMPSON SET-XP EPOXY IN CONCRETE AND HILTI HY20 EPOXY IN UNREINFORCED BRICK. AN APPROVED EQUAL WITH ICC REPORTS MAY BE SUBMITTED TO THE ENGINEER FOR APPROVAL.
 - UNLESS NOTED OTHERWISE, PERMANENTLY EXPOSED EMBEDDED PLATE AND ANGLE ASSEMBLIES SHALL BE HOT DIPPED GALVANIZED AFTER FABRICATION. WELDS OR LOADS SHALL NOT BE PLACED ON THE EMBEDDED ASSEMBLIES FOR A MINIMUM OF (7) DAYS AFTER CASTING IN CONCRETE.

LAP SPLICE SCHEDULE				
BAR SIZE	f _c = 2,500 psi			
	TOP BARS		OTHER BARS	
	CASE 1	CASE 2	CASE 1	CASE 2
#4	41	62	32	47
#5	52	77	40	59

NOTES:

- LAP LENGTHS ARE IN INCHES AND ARE BASED ON GRADE 60 REINFORCING STEEL AND NORMAL WEIGHT CONCRETE.
- TOP BARS ARE HORIZONTAL BARS WITH MORE THAN 12 INCHES OF CONCRETE CAST BELOW THE BARS.
- CASES 1 AND 2 ARE DEFINED AS FOLLOWS:
 - CASE 1: COVER AT LEAST 1.0 db AND c.c. SPACING AT LEAST 3.0 db.
 - CASE 2: COVER LESS THAN 1.0 db OR c.c. SPACING LESS THAN 3.0 db.

DRAWING INDEX

- S0.01 GENERAL STRUCTURAL NOTES AND DRAWING INDEX
- S0.02 GENERAL STRUCTURAL NOTES CONTINUED AND SPECIAL INSPECTION PROGRAM
- S2.00 BASEMENT FLOOR PLAN - EXISTING RESIDENCE
- S2.01 MAIN LEVEL FLOOR PLAN
- S2.02 SECOND FLOOR PLAN
- S2.03 ROOF PLAN - ADDITION THIRD FLOOR PLAN - EXISTING RESIDENCE
- S2.04 GARAGE - FOUNDATION PLAN AND ROOF PLAN
- S5.01 CONCRETE DETAILS
- S5.02 CONCRETE DETAILS
- S7.01 FRAMING DETAILS
- S7.02 FRAMING DETAILS
- S7.03 FRAMING DETAILS
- S7.04 FRAMING DETAILS

STRUCTURAL STEEL:

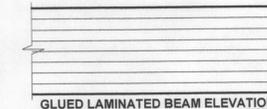
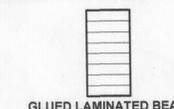
- STEEL DESIGN, FABRICATION, AND ERECTION SHALL CONFORM WITH "AISC SPECIFICATION FOR THE DESIGN, FABRICATION, AND ERECTION OF STRUCTURAL STEEL FOR BUILDINGS" AND THE "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES".
- THE GRADE AND SPECIFICATION OF THE STEEL MEMBERS SHALL BE AS FOLLOWS:

WIDE FLANGE SHAPES (WT SHAPES)	ASTM A572 GRADE 50 OR ASTM A992 GRADE 50
CHANNELS, PLATES, AND ANGLES (U.N.O.)	ASTM A36
HOLLOW STRUCTURAL SECTIONS (TUBES)	ASTM A500 GRADE B (Fy=46 KSI)
HOLLOW STRUCTURAL SECTIONS (PIPES)	ASTM A53 GRADE B (Fy=35 KSI)
- BOLTS SHALL CONFORM TO ASTM SPECIFICATIONS FOR A307 BOLTS.
- WELDING SHALL CONFORM TO THE AWS CODES FOR BUILDING CONSTRUCTION. WELDING SHALL BE PERFORMED IN ACCORDANCE WITH A WELDING PROCEDURE SPECIFICATION (WPS) AS REQUIRED IN AWS D1.1 AND APPROVED BY THE ENGINEER OF RECORD. THE WPS VARIABLES SHALL BE WITHIN THE PARAMETERS ESTABLISHED BY THE FILLER-METAL MANUFACTURER.
- WELDS SHALL UTILIZE E70XX ELECTRODES AND SHALL BE A MINIMUM OF 3/16" IN SIZE UNLESS NOTED OTHERWISE.

GLUED LAMINATED MEMBERS:

- GLUED LAMINATED MEMBERS SHALL CONFORM TO THE REQUIREMENTS OF ONE OF THE FOLLOWING STANDARDS AND PUBLICATIONS:
 - AMERICAN NATIONAL STANDARD FOR STRUCTURAL GLUED LAMINATED TIMBER.
 - ANSI STANDARD A190.1.
 - ANY CODE-APPROVED STANDARD OR PUBLICATION. APPROVAL MUST BE OBTAINED FROM ABHT STRUCTURAL ENGINEERS.
- THE MINIMUM GLUE LAMINATED MEMBER GRADES SHALL BE AS FOLLOWS:

MEMBER	GRADE
SIMPLE SPAN	24F-V4
CONTINUOUS/CANTILEVER	24F-V8
- APPEARANCE SHALL BE FRAMING INDUSTRIAL FOR HIDDEN MEMBERS AND ARCHITECTURAL FOR EXPOSED MEMBERS UNLESS NOTED OTHERWISE ON THE DRAWINGS.
- NO NOTCHING OR BORING OF HOLES IN BEAMS IS ALLOWED WITHOUT APPROVAL BY ABHT STRUCTURAL ENGINEERS.
- GLUE SHALL BE WET-USE EXTERIOR WATERPROOF GLUE.
- GLUED LAMINATED WOOD SYMBOLS:



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SAWN LUMBER:

1. ALL SAWN LUMBER SHALL CONFORM TO THE WESTERN WOOD PRODUCTS ASSOCIATION OR THE WEST COAST LUMBER INSPECTION BUREAU GRADING RULES. LUMBER SHALL BE OF THE SPECIES AND GRADE SHOWN BELOW:

MEMBER	GRADE
2x AND 4x FRAMING	DOUGLAS FIR-LARCH NO. 2
5x AND GREATER BEAMS POSTS/ COLUMNS	DOUGLAS FIR-LARCH NO. 1 DOUGLAS FIR-LARCH NO. 1
T & G DECKING	REFERENCE ARCHITECTURAL DRAWINGS

2. ALL LUMBER IN CONTACT WITH THE GROUND, CONCRETE OR CMU SHALL BE PRESSURE TREATED. CONTRACTOR MAY SUBMIT FOR APPROVAL, A MOISTURE BARRIER IN-LIEU OF THE PRESSURE TREATED WOOD.

3. FASTENERS FOR PRESERVATIVE-TREATED AND FIRE-RETARDANT-TREATED WOOD SHALL BE OF HOT-DIPPED ZINC-COATED GALVANIZED STEEL, STAINLESS STEEL, SILICON BRONZE, OR COPPER. FASTENINGS FOR WOOD FOUNDATIONS SHALL BE AS REQUIRED IN AF&PA TECHNICAL REPORT NO. 7.

4. ALL METAL HARDWARE AND FRAMING ACCESSORIES SHALL BE MANUFACTURED BY SIMPSON STRONG-TIE COMPANY OR AN APPROVED EQUAL. SUBSTITUTION OF AN APPROVED EQUAL SHALL NOT BE MADE WITHOUT THE APPROVAL OF THE ENGINEER. THE SUBMITTAL SHALL INCLUDE DOCUMENTATION SHOWING THE ALLOWABLE LOADS OF THE SPECIFIED SIMPSON ITEM ALONG WITH TABULATED ALLOWABLE LOADS FOR THE SUBSTITUTED ITEMS. ALL ITEMS SHALL BE INSTALLED PER THE MANUFACTURERS INSTALLATION REQUIREMENTS. ALL NAIL HOLES SHALL BE FILLED WITH THE RECOMMENDED FASTENER UNLESS NOTED OTHERWISE ON THE DRAWINGS.

5. WHERE FRAMING HANGERS ARE REQUIRED BUT ARE NOT SPECIFICALLY SIZED, THE FOLLOWING SIZES SHALL BE USED. SLOPE, SKEW, TURN IN FLANGES AND PROVIDE TOP FLANGE HANGERS AS REQUIRED FOR THE SPECIFIC CONDITIONS AT THE END OF THE MEMBER.

MEMBER	HANGER
2x AND 3x MEMBERS	U TYPE HANGERS
4x MEMBERS	HU TYPE HANGERS
6x MEMBERS	HUTF TYPE HANGERS
I-JOIST MEMBERS	MIT HANGERS
GLU-LAM MEMBERS	LEG HANGERS

6. ALL WALLS SHALL HAVE DOUBLE TOP PLATES AND SHALL BE SPLICED PER THE TYPICAL TOP PLATE SPLICE DETAIL (SHOWN ON 9/S7.01), UNLESS NOTED OTHERWISE. TOP PLATES AT WALL INTERSECTIONS SHALL BE LAPPED AND NAILED WITH (3) 16d NAILS.

7. HOLES FOR BOLTS SHALL BE DRILLED WITH A BIT OF THE SAME NOMINAL DIAMETER AS THE BOLT + 1/16".

8. ALL BOLTS, CARRIAGE BOLTS, LAG SCREWS, EXPANSION BOLTS AND EPOXY BOLTS SHALL BE INSTALLED WITH STANDARD CUT WASHERS UNDER THE BOLT HEADS AND NUTS THAT BEAR DIRECTLY ON THE WOOD. ALL NUTS SHALL BE TIGHTENED AT THE TIME OF INSTALLATION AND RE-TIGHTENED IF NECESSARY, DUE TO WOOD SHRINKAGE, PRIOR TO CLOSE-IN OR AT THE COMPLETION OF THE PROJECT. BOLTS AND LAG SCREWS SHALL CONFORM TO ANSI/ASME STANDARD B18.2.1-1981.

9. CUTTING AND NOTCHING OF JOISTS AND STUDS SHALL BE IN CONFORMANCE WITH 2010 OSSC §2308.9.10 AND §2308.10.4.2. BORED HOLES SHALL BE IN CONFORMANCE WITH 2010 OSSC §2308.9.11 & §2308.10.4.2.

10. WOOD SYMBOLS:



CONTINUOUS



BLOCKING

11. ALL NAILS FOR STRUCTURAL WORK SHALL BE COMMON WIRE NAILS. HOLES SHALL BE PRE-DRILLED WHERE NECESSARY TO PREVENT SPLITTING. NAILING NOT SHOWN ON THE STRUCTURAL DRAWINGS SHALL BE PER THE NAILING SCHEDULE BELOW:

NAIL TYPE	SHANK DIAMETER - INCHES	MINIMUM PENETRATION - INCHES
8d	0.131	1.31
10d	0.148	1.50
16d	0.162	1.625

NAILING SCHEDULE

- | | |
|---|---------------------------------------|
| A. JOIST SITTING ON SILL OR GIRDER | (3) 8d TOENAILS, EA. SIDE |
| B. BRIDGING TO JOIST | (2) 8d TOENAILS, EA. SIDE, EA. END |
| C. TOP PLATE TO STUD | (2) 16d |
| D. STUD TO SILL PLATE | (2) 16d END NAILS OR (4) 8d TOE NAILS |
| E. DOUBLE STUDS | 16d AT 24" o.c. |
| F. DOUBLE TOP PLATES - BETWEEN SPLICE NAILING | 16d AT 16" o.c. |
| G. DOUBLE TOP PLATES - EACH SIDE OF SPLICED PLATE | (8) 16d |
| H. BLOCKING TO TOP PLATE | (3) 8d TOE NAILS EACH SIDE |
| I. RIM JOIST TO TOP PLATE OR SILL PLATE | 8d TOENAILS AT 6" o.c. |
| J. CONTINUOUS (2) AND (3) PIECE HEADERS | 16d AT 16" o.c. ALONG EACH EDGE |
| K. CEILING JOIST LAPS OVER PARTITIONS | (3) 16d FACE NAILS |
| L. RAFTER TO TOP PLATE OR SILL PLATE | (3) 8d TOENAILS EACH SIDE |
| M. BUILT-UP CORNER STUDS | 16d AT 24" o.c. |
| N. TONGUE AND GROOVE DECKING | (2) 16d AT EACH BEARING |
| P. CROSS BRIDGING | (2) 10d EACH END |

TONGUE AND GROOVE DECKING:

- DECKING SHALL BE AT RANDOM LENGTHS. JOINTS SHALL BE LAID OUT SUCH THAT THE DISTANCE BETWEEN END JOINTS IN ADJACENT DECKING PIECES SHALL BE 2'-0".
- DECKING SHALL BE INSTALLED WITH TONGUES ORIENTED TOWARD THE RIDGE ON SLOPED ROOFS.
- DECKING SHALL BE NAILED THROUGH THE TONGUE TO THE SUPPORTING MEMBERS WITH 16d NAILS. DECKING SHALL BE FACE NAILED WITH 16d NAILS. NAILING SHALL OCCUR AT EACH SUPPORT.
- DECKING PIECES SHALL BE TOE NAILED TOGETHER WITH 8d NAILS. NAILS SHALL NOT BE SPACED MORE THAN 2'-6" o.c. AND SHALL NOT BE LOCATED MORE THAN 12" FROM THE ENDS.
- DECKING PIECES SHALL SPAN A MINIMUM OF (3) JOIST BAYS.

WOOD STRUCTURAL PANELS:

- STRUCTURAL WOOD PANELS SHALL CONFORM TO THE REQUIREMENTS OF ONE OF THE FOLLOWING STANDARDS AND PUBLICATIONS:
 - U.S. PRODUCT STANDARD PS 1 FOR CONSTRUCTION AND INDUSTRIAL PLYWOOD.
 - U.S. PRODUCT STANDARD PS 2 PERFORMANCE STANDARD FOR WOOD BASED STRUCTURAL USE PANELS
 - APA PRP-108 PERFORMANCE STANDARDS
 - ANY CODE-APPROVED STANDARD OR PUBLICATION. APPROVAL MUST BE OBTAINED FROM ABHT STRUCTURAL ENGINEERS.

2. ROOF AND WALL PANELS SHALL BE APA RATED, EXPOSURE 1, 1/2", 5 PLY PLYWOOD WITH A 32/16 SPAN RATING UNLESS NOTED OTHERWISE ON THE DRAWINGS.

3. ALL ROOF AND FLOOR SHEATHING SHALL BE INSTALLED WITH THE FACE GRAIN PERPENDICULAR TO THE SUPPORTS AND A 1/8" GAP AT ALL PANEL EDGES UNLESS RECOMMENDED OTHERWISE BY THE PANEL MANUFACTURER.

4. WHERE BLOCKING IS NOT SPECIFICALLY REQUIRED FOR THE ROOF SHEATHING, PLY CLIPS OR TONGUE AND GROOVE PLYWOOD SHALL BE USED.

5. ALL NAILS SHALL BE COMMON NAILS EXCEPT AT ROOF SHEATHING WHERE COMMON RING SHANK NAILS SHALL BE USED. GALVANIZED NAILS SHALL BE USED AT PERMANENTLY EXPOSED EXTERIOR AREAS. GALVANIZED NAILS SHALL BE HOT DIPPED OR TUMBLED ONLY.

COMPOSITE WOODS:

1. COMPOSITE WOODS SHALL BE OF THE TYPE AND SIZE AS SHOWN ON THE DRAWINGS. THE MATERIAL TYPE AND GRADE SHALL BE AS SHOWN BELOW:

TYPE	GRADE
LSL	E = 1,500,000 PSI, Fb = 2,250 PSI
LVL	E = 1,900,000 PSI, Fb = 2,600 PSI
PSL	E = 2,000,000 PSI, Fb = 2,900 PSI

2. COMPOSITE WOOD SYMBOLS:



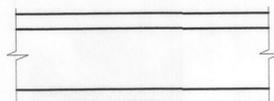
COMPOSITE MEMBER

MANUFACTURED WOOD I-JOISTS:

- THE JOISTS SHALL BE OF THE SAME SIZE AND TYPE AS SHOWN ON THE DRAWINGS MANUFACTURED BY TRUS-JOIST OR AN APPROVED EQUAL. THE JOISTS SHALL BE MANUFACTURED IN CONFORMANCE WITH APA EWS STANDARD PRI-400, PERFORMANCE STANDARD FOR APA EWS I-JOISTS.
- BRIDGING SHALL BE PROVIDED IN CONFORMANCE WITH THE MANUFACTURERS RECOMMENDATIONS.
- WHERE JOIST HANGERS ARE REQUIRED BUT NOT SPECIFICALLY IDENTIFIED ON THE DRAWINGS, IUT TYPE HANGERS (WITH MAXIMUM NAILING) SHALL BE USED AT FACE MOUNT CONDITIONS AND MIT TYPE HANGERS AT TOP FLANGE ONLY CONDITIONS.
- I-JOIST SYMBOLS:



I-JOIST



I-JOIST ELEVATION

PREMANUFACTURED WOOD ROOF TRUSSES:

- THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE DESIGN OF THE PREMANUFACTURED WOOD ROOF TRUSSES AS NOTED ON THE DRAWINGS.
- THE DESIGN SHALL CONFORM TO THE TRUSS PROFILES AS SHOWN ON THE DRAWINGS.
- THE DESIGN SHALL CONFORM TO THE 2009 INTERNATIONAL BUILDING CODE SECTIONS 2303.4, 2303.4.1, AND 2308.10.7 AS AMENDED BY THE STATE OF OREGON AND THE RECOMMENDATIONS OF THE TRUSS PLATE INSTITUTE, INC.
- THE CONTRACTOR/TRUSS MANUFACTURER SHALL PROVIDE SHOP DRAWING WITH THE FOLLOWING INFORMATION:
 - TRUSS LAYOUT, SIZE, SPACING, AND GRADE OF ALL MEMBERS ALONG WITH ANY DETAILING REQUIRED FOR THE TRUSS CONNECTIONS OR CONNECTIONS TO THE SUPPORTING STRUCTURE.
 - SUPPORTING CALCULATIONS FOR THE TRUSS SHOP DRAWINGS. BOTH THE SHOP DRAWINGS AND THE CALCULATIONS SHALL BEAR THE SEAL OF AN ENGINEER REGISTERED IN THE STATE OF OREGON.
- ALL CONNECTIONS REQUIRED TO TRANSFER THE TRUSS LOADS TO THE SUPPORTING STRUCTURE SHALL BE DESIGNED AND PROVIDED BY THE CONTRACTOR/TRUSS MANUFACTURER.
- PERMANENT OR ERECTION TRUSS BRIDGING SHALL BE DESIGNED AND PROVIDED BY THE CONTRACTOR/TRUSS MANUFACTURER AS REQUIRED.
- THE TRUSS MANUFACTURER SHALL VISIT THE JOBSITE AS REQUIRED TO ENSURE PROPER INSTALLATION OF THE WOOD TRUSSES.
- TOP CHORD DIMENSION LUMBER SHALL BE DOUGLAS FIR-LARCH.
- THE TRUSSES SHALL BE DESIGNED TO RESIST THE MINIMUM DESIGN LOADS AS FOLLOWS:

TOP CHORD	10 psf
DEAD LOAD	25 psf (REFERENCE PLANS FOR ADDED DRIFT LOADS)
SNOW LOAD	
BOTTOM CHORD	5 psf
DEAD LOAD	10 psf (NON-CONCURRENT w/ TOP CHORD SNOW LOAD)
LIVE LOAD	
WIND UPLIFT LOAD	DESIGN PER THE REQUIREMENTS OF THE 2010 OREGON STRUCTURAL SPECIALTY CODE (OSSC)

SPECIAL INSPECTION PROGRAM

TABLE 1 - REQUIRED STRUCTURAL SPECIAL INSPECTIONS

SYSTEM OR MATERIAL	IBC CODE REFERENCE	CODE OR STANDARD REFERENCE	FREQUENCY		REMARKS
			CONTINUOUS	PERIODIC	
POST INSTALLED CONCRETE ANCHORS					
INSTALLATION IN HARDENED CONCRETE AND COMPLETED MASONRY	1703.4.2 1704.13.3	ICC EVALUATION REPORT	X		SPECIAL INSPECTIONS APPLY TO ANCHOR PRODUCT NAME, TYPE AND DIMENSIONS, HOLE DIMENSIONS, COMPLIANCE WITH DRILL BIT REQUIREMENTS. CLEANLINESS OF THE HOLE AND ANCHOR, ADHESIVE EXPIRATION DATE, ANCHOR/ADHESIVE INSTALLATION, ANCHOR EMBEDMENT, AND TIGHTENING TORQUE.
STEEL					
FABRICATION OF STRUCTURAL ELEMENTS	1704.2			X	REFER TO INSPECTION OF FABRICATOR REQUIREMENTS
MATERIAL VERIFICATION OF STRUCTURAL STEEL	1704.3 2203.1	ASTM A6 AISC 360 A3.1		X	CERTIFIED MILL TEST REPORTS
MATERIAL VERIFICATION OF ANCHOR BOLTS AND THREADED RODS	1704.3	AISC 360 A3.4		X	MANUFACTURERS CERTIFIED TEST REPORTS
MATERIAL VERIFICATION OF WELD FILLER METALS	1704.3	AISC 360 A3.5		X	MANUFACTURERS CERTIFIED TEST REPORTS
VERIFYING USE OF PROPER WPS'S				X	COPY OF WELDING PROCEDURE SPECIFICATIONS
VERIFYING WELDER QUALIFICATIONS				X	COPY OF QUALIFICATION CARDS
COMPLETE AND PARTIAL PENETRATION GROOVE WELDS	1704.3.1	AWS D1.1 SECTION 6		X	ALL WELDS VISUALLY INSPECTED PER AWS D1.1 6.9
SINGLE PASS FILLET WELDS LESS THAN OR EQUAL TO 5/16"				X	

SPECIAL INSPECTION FOOTNOTES:

SPECIAL INSPECTIONS SHALL CONFORM TO CHAPTER 17 OF THE 2009 "INTERNATIONAL BUILDING CODE" AND OREGON AMENDMENTS. REFER TO THE TABLES 1 THROUGH 8 FOR SPECIAL INSPECTION AND TESTING REQUIREMENTS.

SPECIAL INSPECTIONS AND ASSOCIATED TESTING SHALL BE PERFORMED BY AN APPROVED ACCREDITED INDEPENDENT AGENCY MEETING THE REQUIREMENTS OF ASTM E329 (MATERIALS), ASTM D3740 (SOILS), ASTM C1077 (CONCRETE), ASTM A980 (STEEL), AND ASTM E543 (NON-DESTRUCTIVE). THE INSPECTION AND TESTING AGENCY SHALL FURNISH TO THE STRUCTURAL ENGINEER A COPY OF THEIR SCOPE OF ACCREDITATION. SPECIAL INSPECTORS SHALL BE CERTIFIED BY THE BUILDING OFFICIAL. WELDING INSPECTORS SHALL BE QUALIFIED PER SECTION 6.1.4.1.1 OF AWS D1.1. THE OWNER SHALL SECURE AND PAY FOR SERVICES OF THE INSPECTION AND TESTING AGENCY TO PERFORM ALL SPECIAL INSPECTIONS AND TESTS.

THE SPECIAL INSPECTOR SHALL OBSERVE THE INDICATED WORK FOR COMPLIANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS. ALL DISCREPANCIES SHALL BE BROUGHT TO THE IMMEDIATE ATTENTION OF THE CONTRACTOR FOR CORRECTION. NOTED IN THE INSPECTION REPORTS, AND IF NOT CORRECTED, THEY SHALL BE BROUGHT TO THE ATTENTION OF THE STRUCTURAL ENGINEER AND THE BUILDING OFFICIAL.

THE SPECIAL INSPECTOR SHALL FURNISH INSPECTION REPORTS FOR EACH INSPECTION TO THE BUILDING OFFICIAL, STRUCTURAL ENGINEER, CONTRACTOR, AND OWNER. THE SPECIAL INSPECTION AGENCY SHALL SUBMIT A FINAL REPORT INDICATING THE WORK REQUIRING SPECIAL INSPECTION WAS INSPECTED AND IS IN CONFORMANCE WITH THE APPROVED CONSTRUCTION DOCUMENTS AND THAT ALL DISCREPANCIES NOTED IN THE INSPECTION REPORTS HAVE BEEN CORRECTED.

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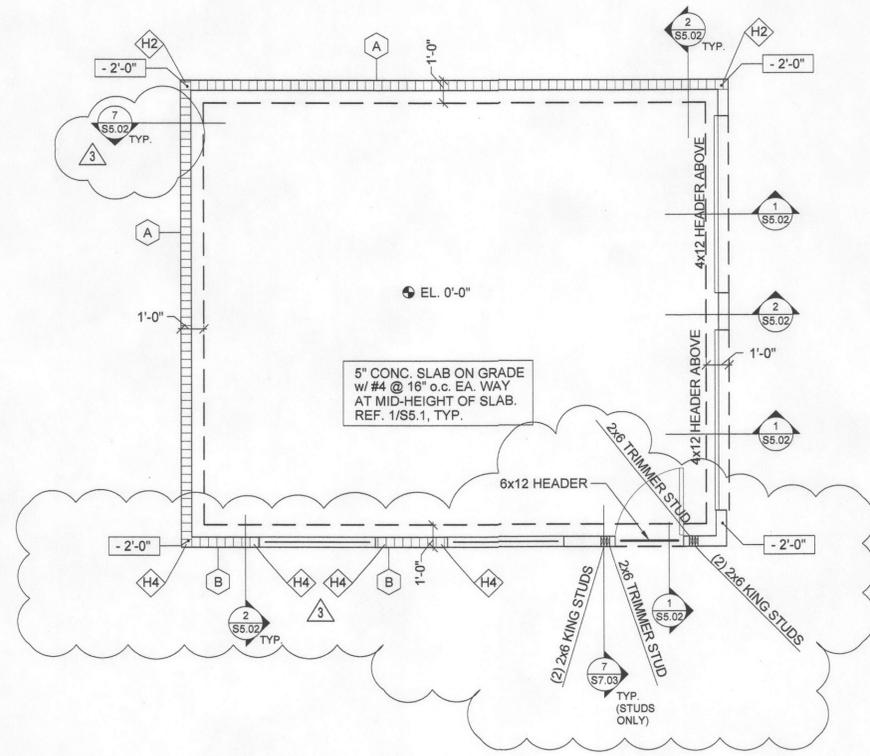
Permit Number

sheet title
general structural
notes cont.
special inspection
program

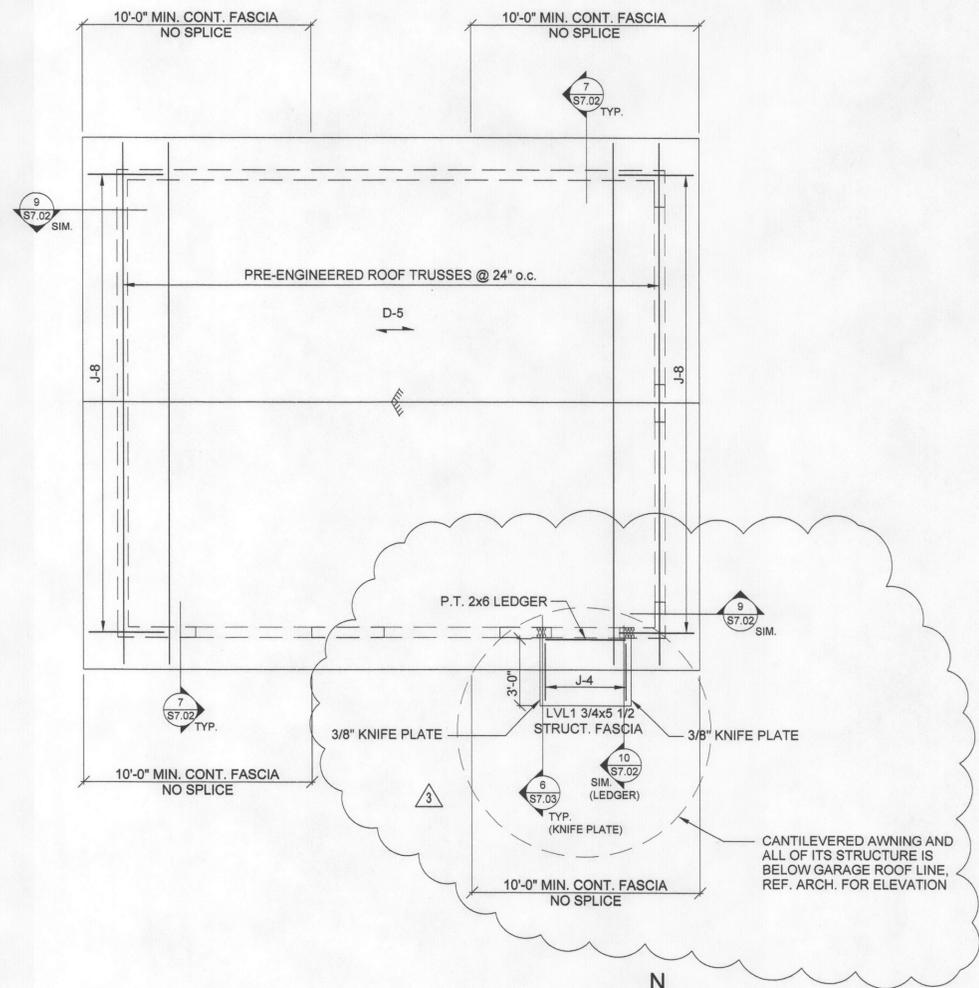
sheet #



EXPIRES 12-31-11



1 GARAGE FOUNDATION PLAN
 1/4"=1'-0" N



2 GARAGE ROOF PLAN
 1/4"=1'-0" N

NOTES:

- | | | |
|--|--|---|
| <p>1. INDICATES ELEVATION AT TOP OF SLAB.</p> <p>2. INDICATES ELEVATION AT BOTTOM OF FOOTING.</p> <p>3. INDICATES BEARING STUD WALL w/ 2x6 STUDS @ 16" o.c. WALL STUDS SHALL ALIGN FROM FLOOR TO FLOOR.</p> <p>4. INDICATES PLYWOOD SHEARWALL w/ 2x6 STUDS @ 16" o.c. UNLESS NOTED OTHERWISE. REF. 6/S7.01 FOR SHEARWALL SCHEDULE. WALL STUDS SHALL ALIGN FLOOR TO FLOOR.</p> <p>5. INDICATES HOLDOWN TYPE. REF. 5/S7.01 FOR HOLDOWN SCHEDULE.</p> <p>6. REFERENCE 1/S7.02 FOR HEADER SCHEDULE, TYPICAL UNLESS NOTED OTHERWISE.</p> | <p>7. REFERENCE GENERAL NOTES FOR PRE-ENGINEERED ROOF TRUSS DESIGN LOADS AND DEFLECTION CRITERIA.</p> <p>8. J-4 INDICATES P.T. 2x6 @ 16" o.c. DECKING JOISTS w/ LUS26-ZMAX.</p> <p>9. J-8 INDICATES 2x4 @ 24" o.c. OUTRIGGERS, FLATWISE. REF. 9/S7.02-SIM. FOR OUTRIGGER DETAILS.</p> <p>10. D-5 INDICATES SPAN DIRECTION OF 5/8" T&G PLYWOOD ROOF SHEATHING (SPAN RATED 32/16) NAILED w/ 8d COMMON RING-SHANK NAILS @ 6" o.c. AT PANEL EDGES AND 12" o.c. IN FIELD.</p> <p>11. INDICATES WALL BELOW.</p> <p>12. INDICATES RIDGE LINE.</p> | <p>13. CONTRACTOR TO VERIFY ALL EXISTING CONDITIONS, DIMENSIONS AND ELEVATIONS PRIOR TO FABRICATION AND ERECTION AND SHALL NOTIFY THE ARCHITECT AND ENGINEER OF ANY SIGNIFICANT DISCREPANCIES FROM THAT SHOWN ON THE DRAWINGS.</p> <p>14. CONTRACTOR TO SHORE EXISTING FRAMING AS REQUIRED FOR DEMOLITION AND REFRAMING WORK.</p> <p>15. REFERENCE ARCHITECTURAL DRAWINGS FOR ALL WALL LOCATIONS AND DIMENSIONS.</p> <p>16. REFERENCE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR SLEEVES, BLOCKOUTS AND OTHER ITEMS TO BE COORDINATED WITH THE STRUCTURAL DRAWINGS.</p> <p>17. VERIFY ALL ROOF SLOPES, CRICKETS, ELEVATION STEPS AND DOWNSPOUTS WITH ARCHITECTURAL DRAWINGS.</p> <p>18. REFERENCE ARCHITECTURAL DRAWINGS FOR LOCATION OF GARAGE.</p> |
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City of Portland
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 DEC 08 2010
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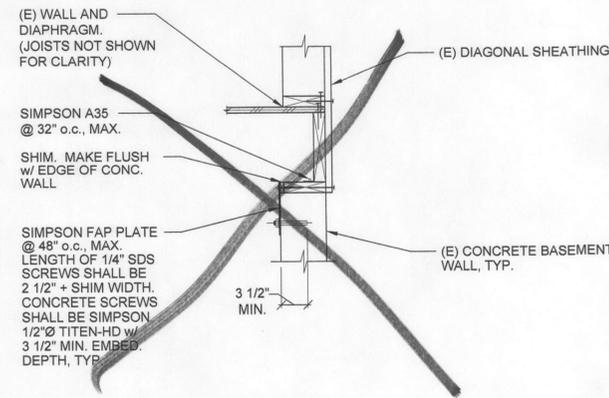
grant park residence
 portland, oregon

issues / revisions	
permit set 9.29.2010	
foundation 10.15.2010	
framing 10.29.2010	
revisions 11.17.2010	

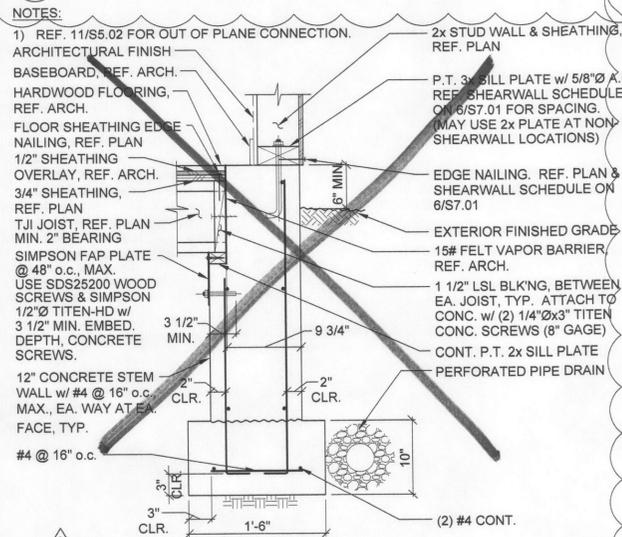
sheet title
 garage - foundation
 plan and roof plan



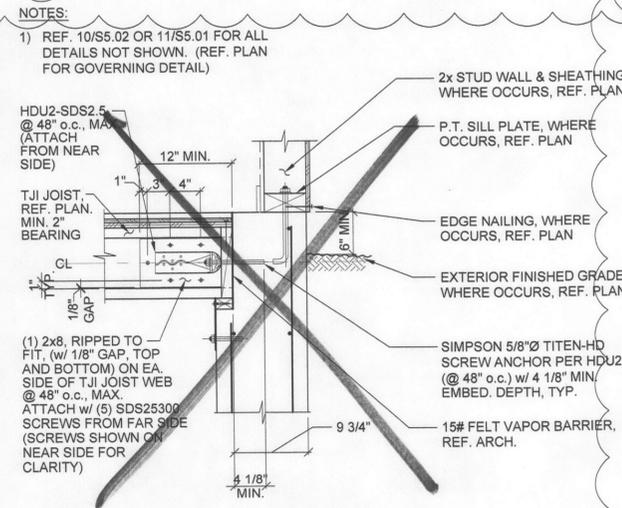
EXPIRES 12-31-11



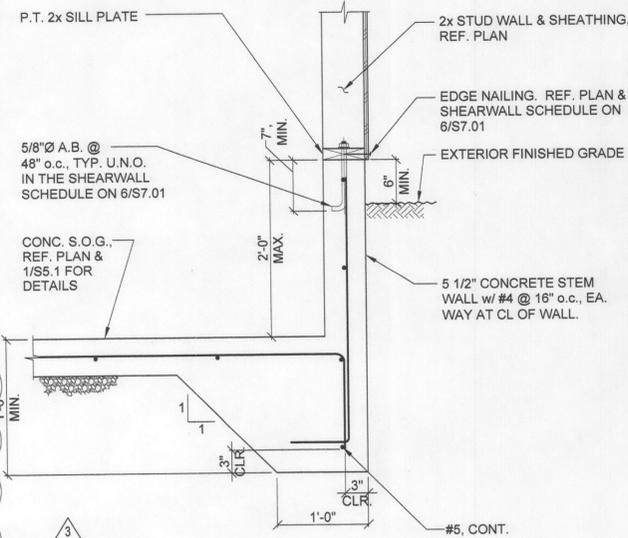
9 (E) EXTERIOR WALL SHEAR RETROFIT
1"=1'-0"



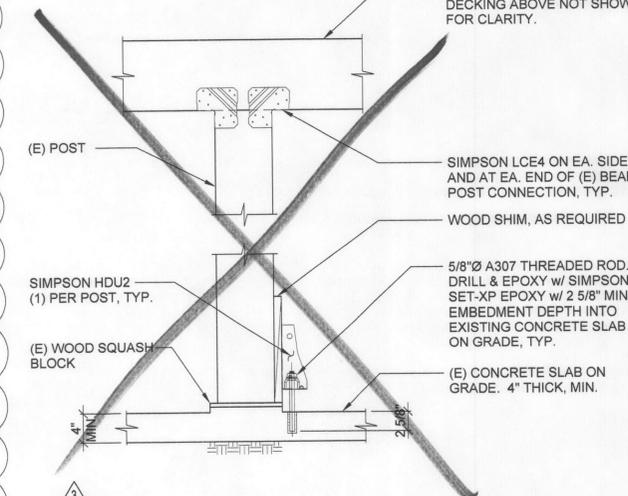
10 FOOTING AT SOUTH WALL w/ STUD WALL
1"=1'-0"



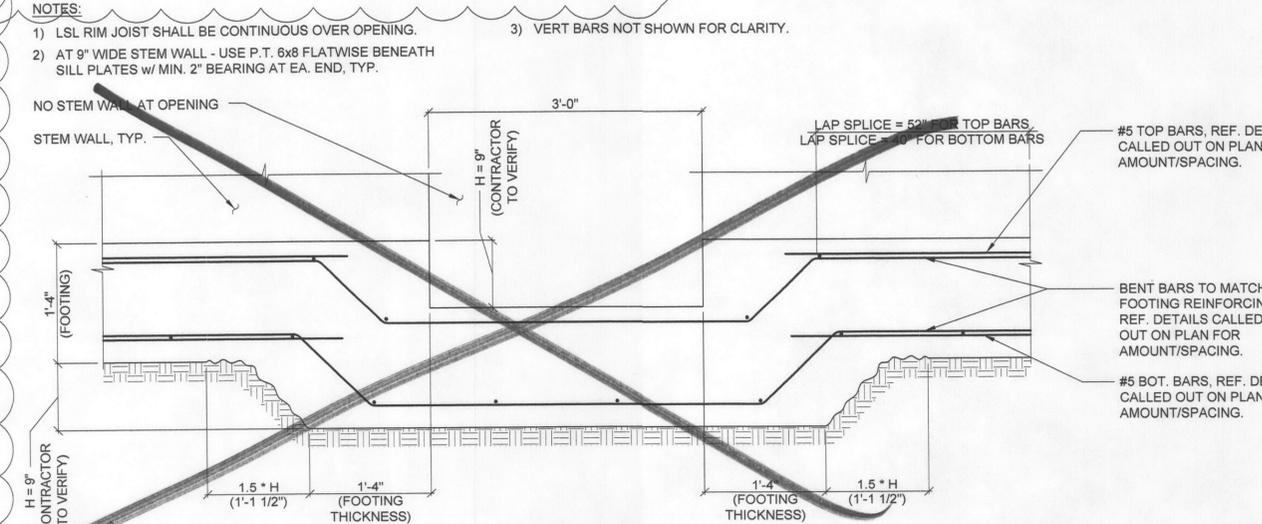
11 OUT OF PLANE BRACING FOR SOUTH WALL
1"=1'-0"



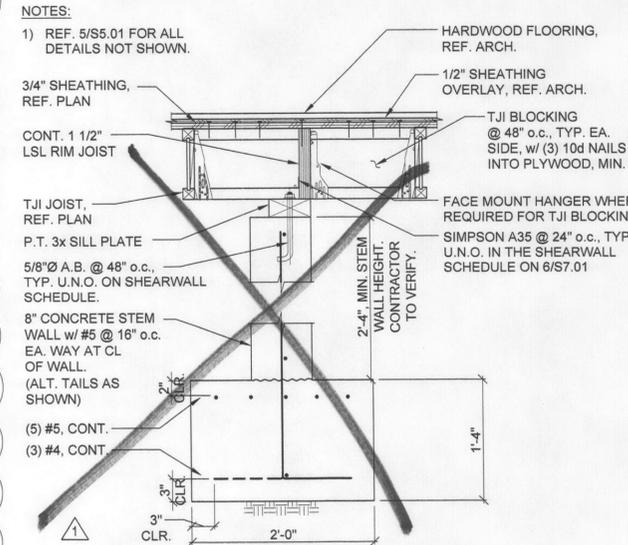
7 TYPICAL GARAGE PERIMETER STEM WALL
1"=1'-0"



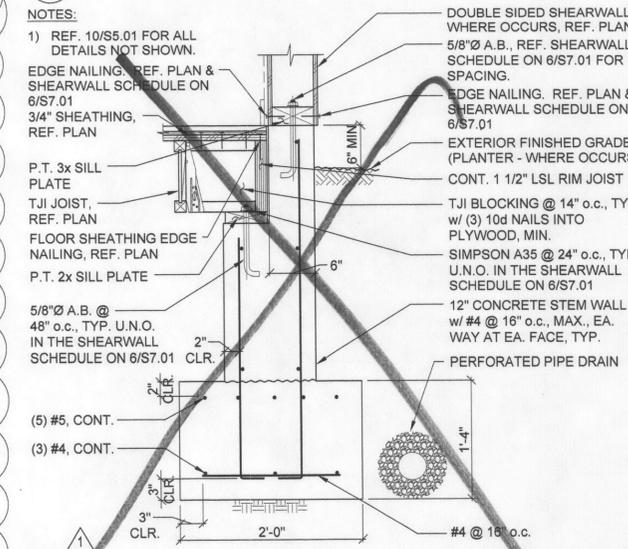
8 (E) COLUMN RETROFIT BRACING
1"=1'-0"



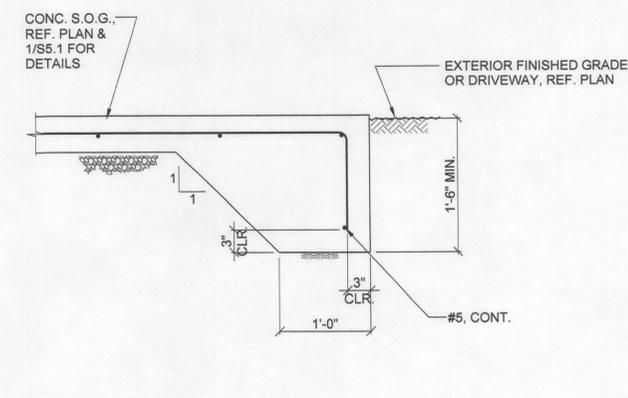
6 STEPPED FOOTING FOR ACCESS
1"=1'-0"



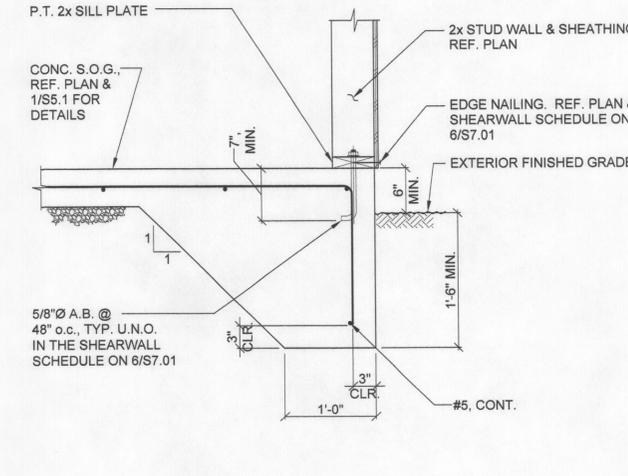
4 GRADE BEAM FOOTING AT CORRIDOR
1"=1'-0"



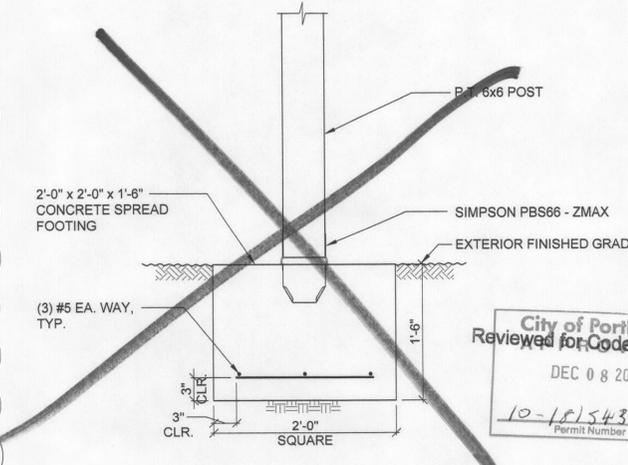
5 GRADE BEAM FOOTING - 'BB' SHEARWALL
1"=1'-0" (SOUTH SECTION)



1 THICKENED SLAB EDGE DETAIL
1"=1'-0"

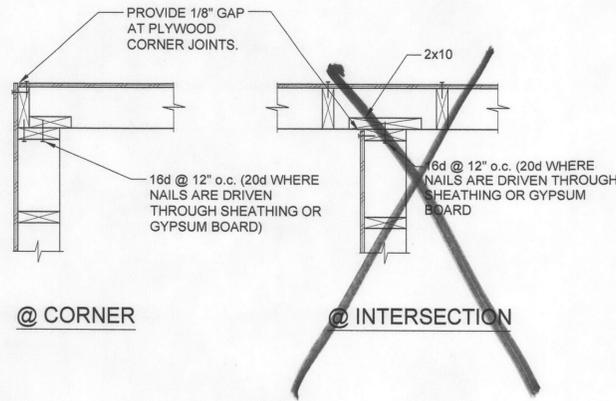


2 TYPICAL GARAGE PERIMETER WALL
1"=1'-0"



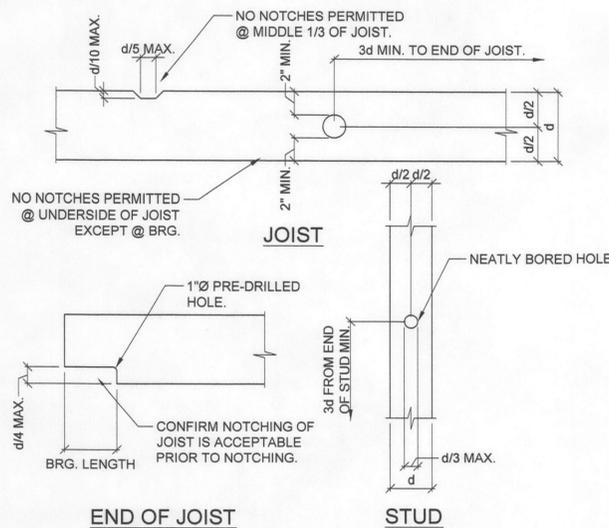
3 EXTERIOR COLUMN SPREAD FOOTING
1"=1'-0"

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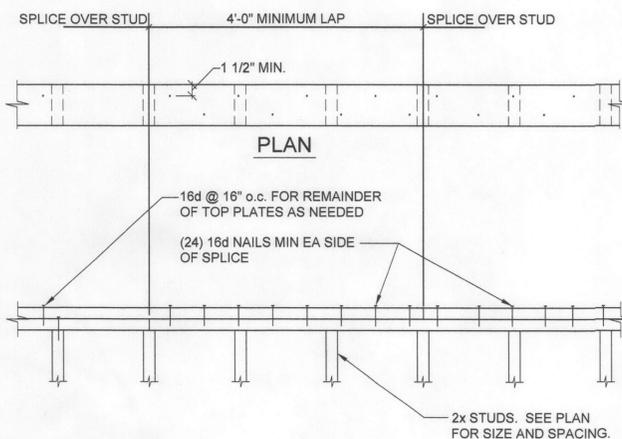


7 STUD FRAMING @ CORNERS/INTERSECTIONS
1"=1'-0"

7 STUD FRAMING @ CORNERS/INTERSECTIONS
1"=1'-0"



8 NOTCHES AND HOLES @ STUDS AND JOISTS
1"=1'-0"



9 NAILED TOP PLATE SPLICE
1"=1'-0"

HOLDOWN SCHEDULE				
MARK	HOLDOWN	ANCHOR BOLT	HOLDOWN POST	ATTACHMENT TO POST
H2	HDU2	SSTB16	(2) 2x	(6) SDS 1/4x2 1/2 WOOD SCREWS
H4	HDU4	SSTB20	(2) 2x	(10) SDS 1/4x2 1/2 WOOD SCREWS
H8	HDU8	SSTB28	(3) 2x OR 4x4 POST	(20) SDS 1/4x2 1/2 WOOD SCREWS
H8*	HDU8	REF. DETAIL 12/S5.01	(3) 2x OR 4x4 POST	(20) SDS 1/4x2 1/2 WOOD SCREWS
H14	HDU14	REF. DETAIL 9/S5.01	6x6 POST	(36) SDS 1/4x2 1/2 WOOD SCREWS
M28	MSTC28	N/A	(2) 2x	NAILING PER SIMPSON
M40	MSTC40	N/A	(2) 2x	NAILING PER SIMPSON
MB3	MSTC48B3	N/A	4x4 POST	NAILING PER SIMPSON

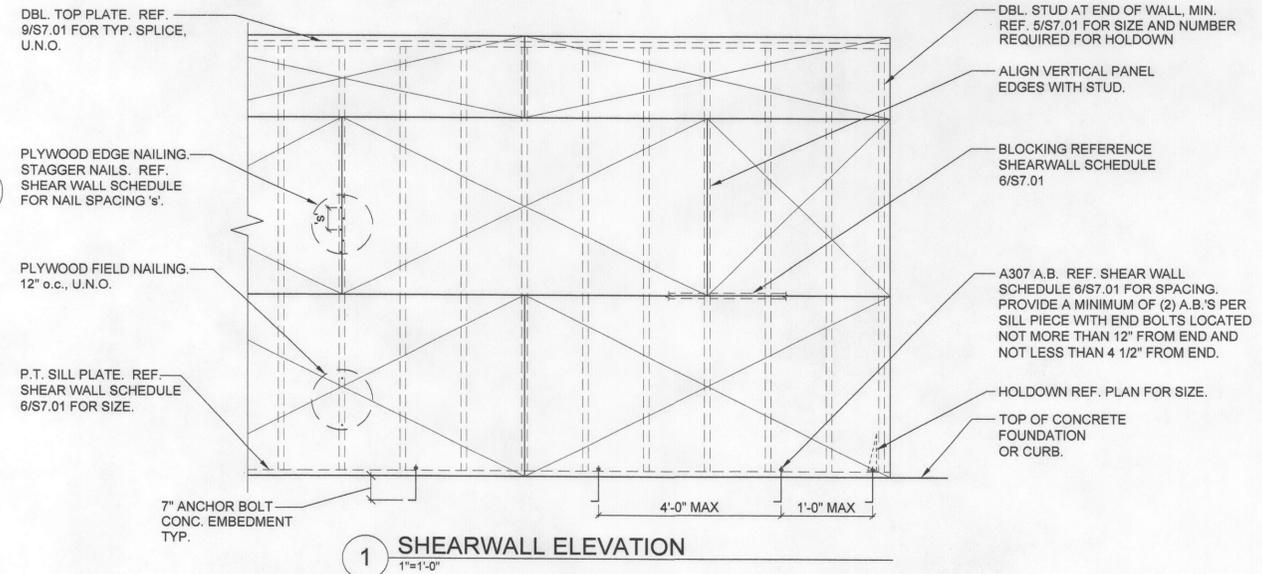
- NOTES:**
1. PROVIDE SHEARWALL EDGE NAILING TO POST - FULL HEIGHT.
 2. HOLDDOWNS SHALL BE INSTALLED IN CONFORMANCE WITH THE MANUFACTURER'S REQUIREMENTS.
 3. HOLDDOWN POST SIZES SHOWN ARE MINIMUMS U.N.O. ON PLAN.

5 HOLDOWN SCHEDULE
NO SCALE

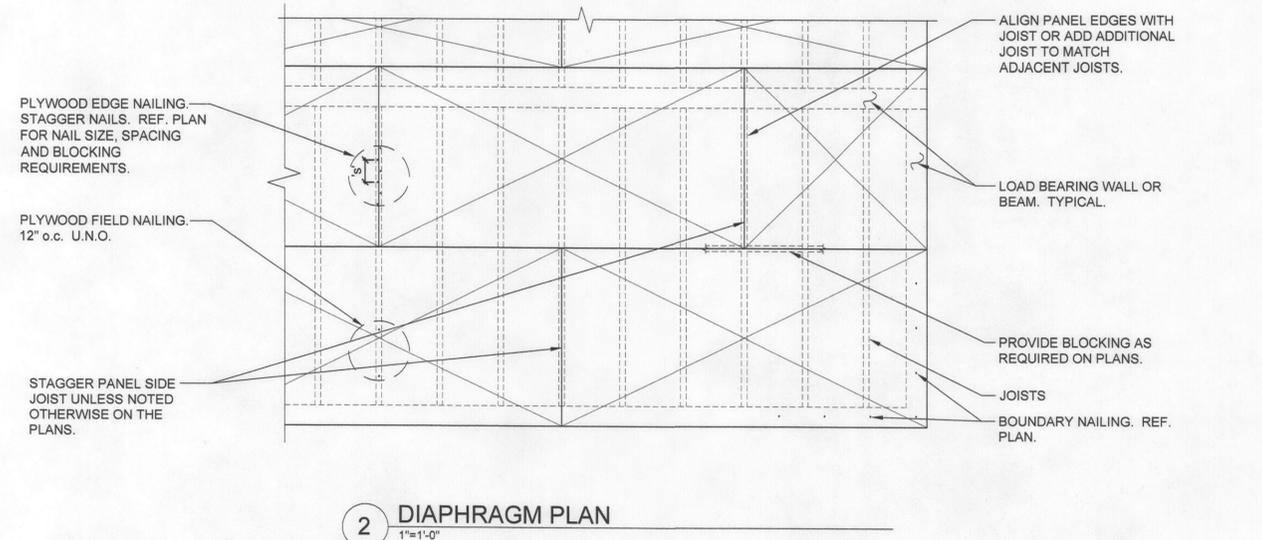
SHEARWALL SCHEDULE							
MARK	SHEATHING	EDGE NAILING	SILL ANCHORAGE	SOLE/RIM ANCHORAGE	SILL/SOLE	STUDS	REMARKS
A	15/32"	10d @ 6" o.c.	5/8" x 7" @ 4'-0" o.c.	16d @ 8" o.c./ LTP4 @ 2'-0" o.c. OR A35 @ 2'-0" o.c.	2x/2x	2x	
B	15/32"	10d @ 4" o.c.	5/8" x 7" @ 1'-6" o.c.	16d @ 6" o.c./ LTP4 @ 1'-4" o.c. OR A35 @ 1'-4" o.c.	2x/2x	2x	
C	15/32"	10d @ 3" o.c.	5/8" x 7" @ 1'-4" o.c.	16d @ 4" o.c./ LTP4 @ 1'-0" o.c. OR A35 @ 1'-0" o.c.	2x/2x	2x	REF. NOTE 2
D	15/32"	10d @ 2" o.c.	5/8" x 7" @ 1'-4" o.c.	16d @ 4" o.c./ LTP4 @ 1'-0" o.c. OR A35 @ 1'-0" o.c.	3x/2x	2x	REF. NOTE 2
BB	15/32" BOTH SIDES	10d @ 4" o.c. EA. SIDE	5/8" x 7" @ 2'-0" o.c.	SDS25600 @ 6" o.c./ HGA10KT @ 1'-0" o.c.	3x/3x	2x	REF. NOTE 2 & REF. NOTE 8
DD	15/32" BOTH SIDES	10d @ 2" o.c. EA. SIDE	5/8" x 7" @ 1'-4" o.c.	SDS25600 @ 4" o.c./ HGA10KT @ 0'-9" o.c.	3x/3x	3x	REF. NOTE 2 & REF. NOTE 8

- NOTES:**
1. FIELD NAILING SHALL BE AT 12" o.c. AND NAILS SHALL BE THE SAME SIZE AS THE EDGE NAILING SPECIFIED ABOVE, UNLESS NOTED OTHERWISE.
 2. ALL HORIZONTAL PANEL EDGES SHALL BE BLOCKED WITH 2x BLOCKING EXCEPT, (2) 2x BLOCKING AND STUDS, OR 3x, SHALL BE PROVIDED AT ALL ADJOINING PANEL EDGES FOR TYPE "C", "D", "BB" & "DD" SHEARWALLS. WHERE (2) 2x BLOCKING OR STUDS ARE USED, FASTEN w/ NAILING TO MATCH EDGE NAILING, TYP. ((2) ROWS FOR DOUBLE SIDED WALLS, TYP.).
 3. PROVIDE EDGE NAILING AROUND ALL SHEARWALL OPENINGS.
 4. EDGE NAILING APPLIES TO ALL PANEL EDGES.
 5. UPSET THREADS ON SILL BOLTS ARE NOT ALLOWED. ALL SILL BOLTS MUST HAVE CUT THREADS.
 6. ALL NAILS SHALL BE COMMON WIRE NAILS.
 7. ALL SILL ANCHOR BOLTS SHALL HAVE A MINIMUM 1/4"x3"x3" PLATE WASHER BETWEEN SILL AND NUT. THE PLATE WASHER SHALL EXTEND TO WITHIN 1/2" OF THE EDGE OF THE SILL PLATE ON THE SIDE(S) w/ SHEARWALL SHEATHING. THE HOLE IN THE PLATE WASHER SHALL BE PERMITTED TO BE DIAGONALLY SLOTTED WITH A MAXIMUM WIDTH OF 13/16" AND A MAXIMUM LENGTH OF 1 3/4", PROVIDED A STANDARD CUT WASHER IS PLACED BETWEEN THE PLATE WASHER AND THE NUT. USE A 1/4"x3"x4 1/2" PLATE WASHER FOR TYPE "BB" & "DD" SHEARWALLS. REFERENCE 10/S7.03 FOR PLATE WASHER DETAILS.
 8. WHERE PANELS ARE APPLIED ON BOTH SIDES OF A WALL, PANEL JOINTS SHALL BE OFFSET TO FALL ON DIFFERENT FRAMING MEMBERS, REFERENCE 2/S7.02 FOR DETAILS.

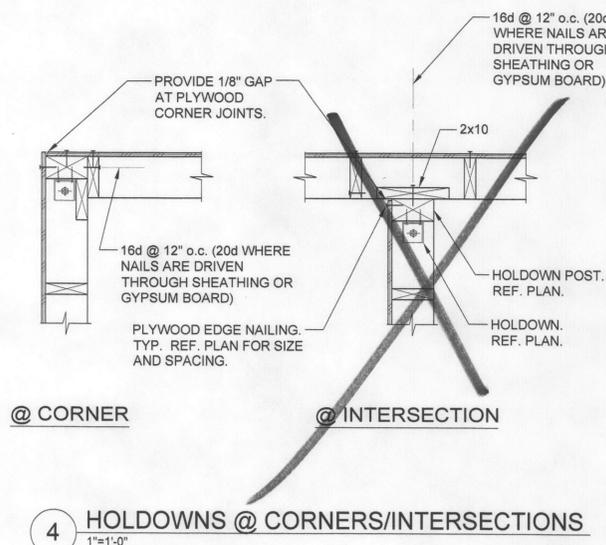
6 SHEARWALL SCHEDULE
NO SCALE



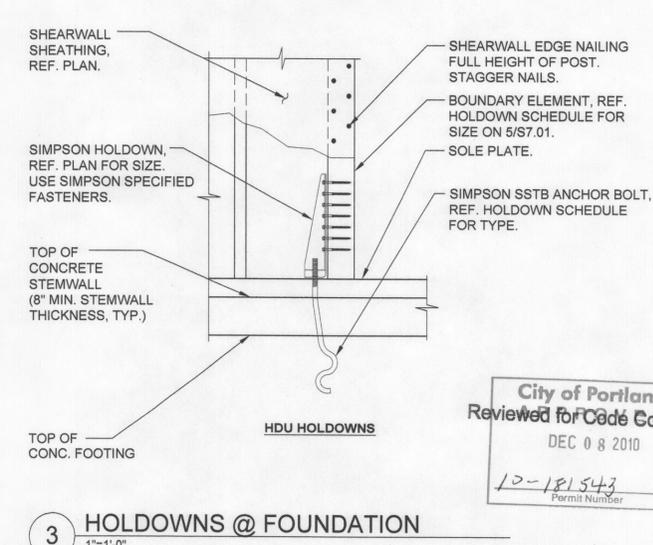
1 SHEARWALL ELEVATION
1"=1'-0"



2 DIAPHRAGM PLAN
1"=1'-0"



4 HOLDOWNS @ CORNERS/INTERSECTIONS
1"=1'-0"



3 HOLDOWNS @ FOUNDATION
1"=1'-0"

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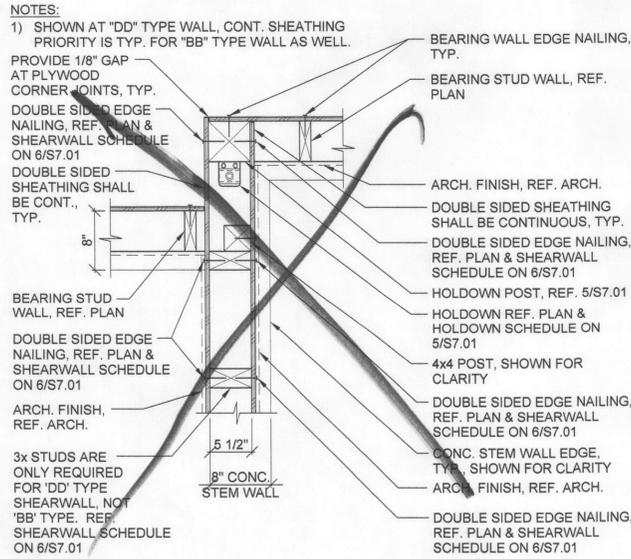
grant park residence
portland, oregon

issues / revisions
permit set 9.29.2010
foundation 10.15.2010
framing 10.29.2010
revisions 11.17.2010

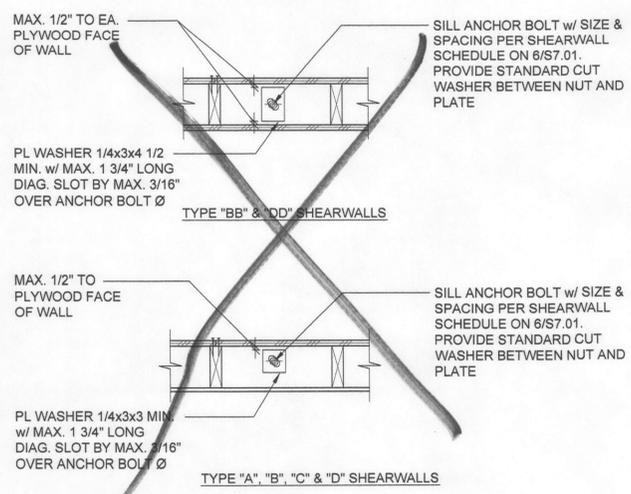
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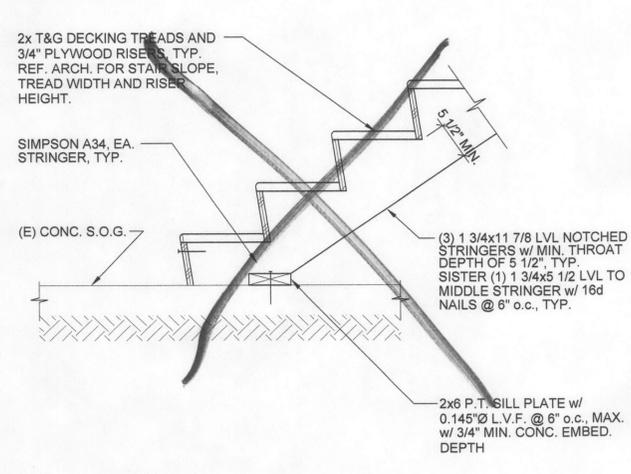
S7.01



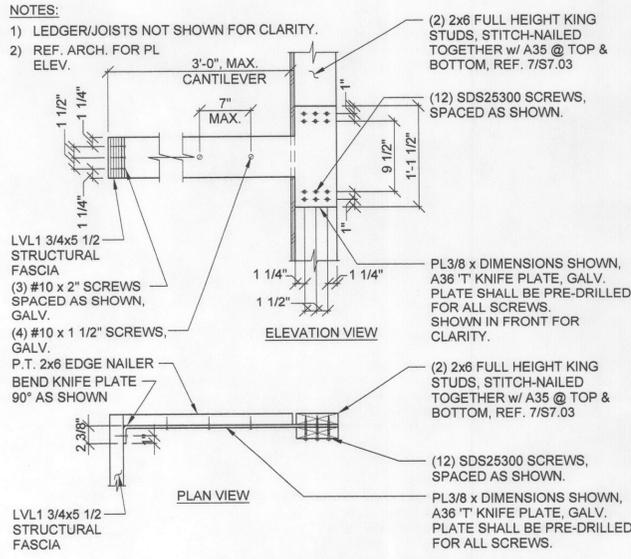
9 DOUBLE SIDED SHEARWALL - PLAN VIEW
1"=1'-0" (SHOWN AT "DD" WALL - "BB" WALL SIMILAR)



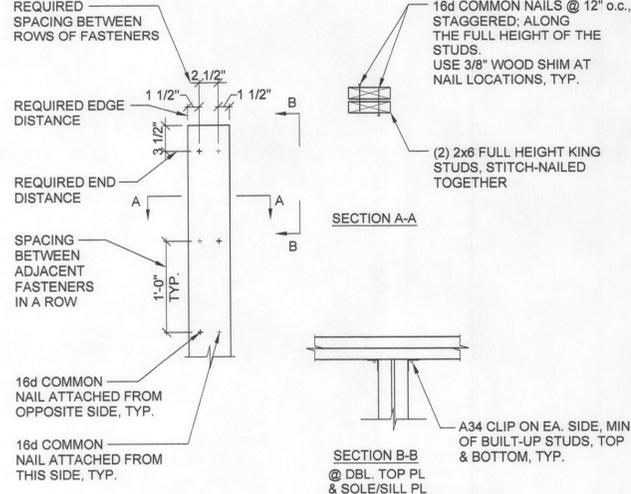
10 SILL PLATE ANCHORAGE
1"=1'-0"



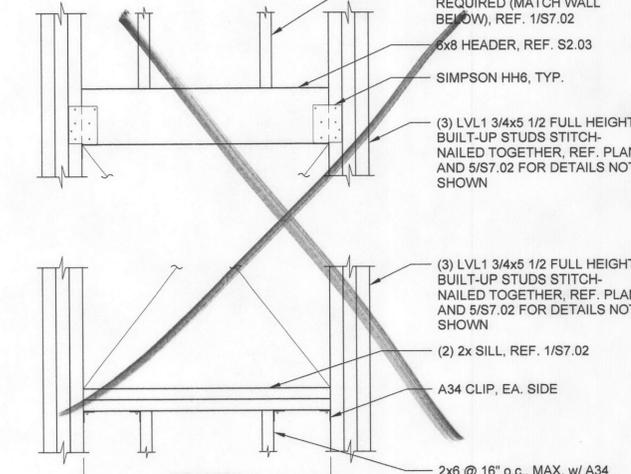
11 (E) BASEMENT STAIR BASE CONNECTION
1"=1'-0"



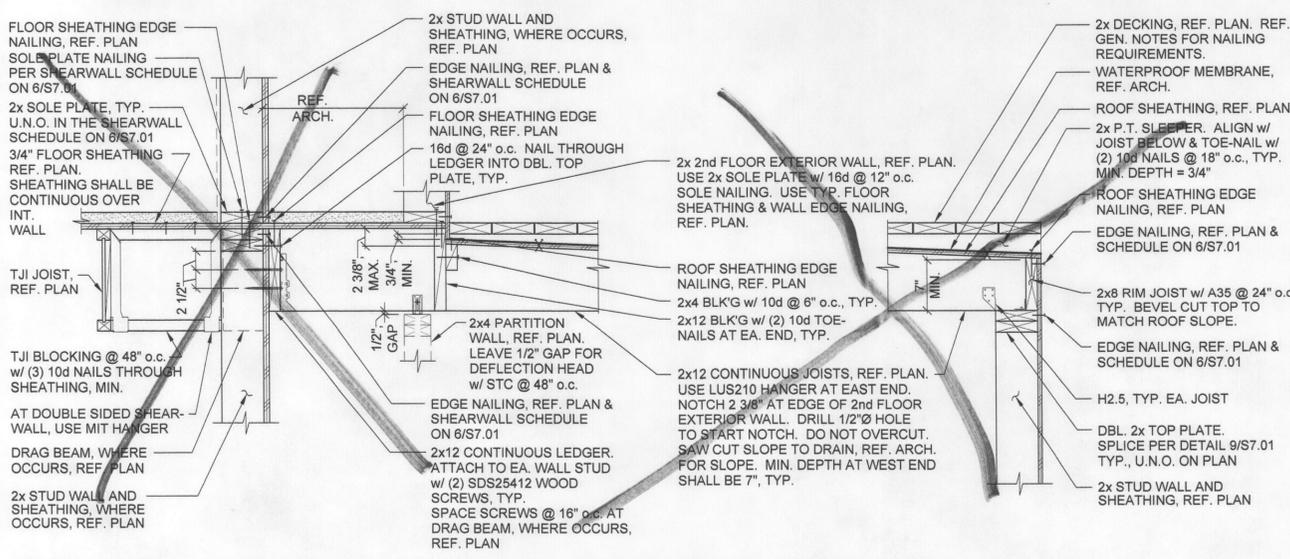
6 KNIFE PLATE FOR CANTILEVERED AWNING
1"=1'-0"



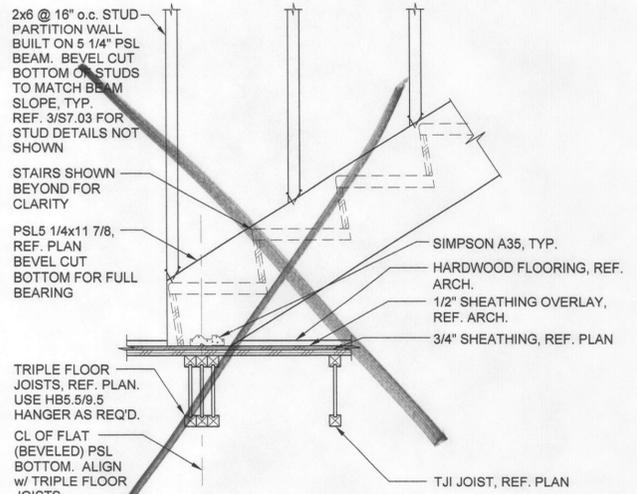
7 BUILT-UP KING STUDS FOR KNIFE PLATE
1"=1'-0"



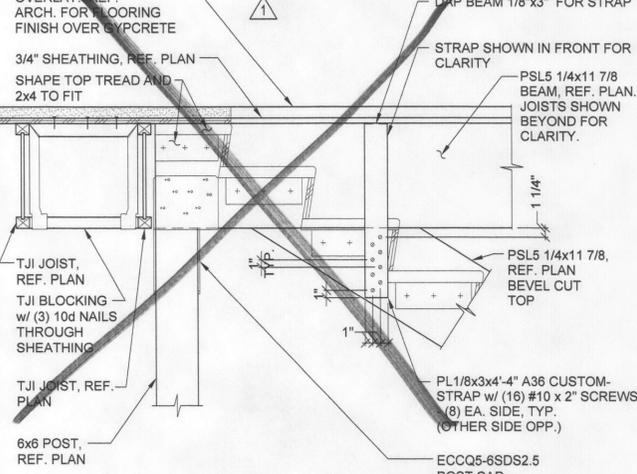
8 STAIR FULL HEIGHT WALL - OPENING DETAIL
1"=1'-0"



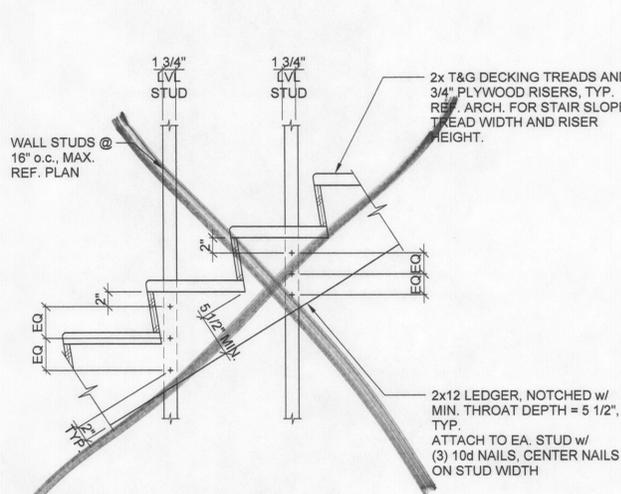
1 INTERIOR SHEAR WALL - EXTERIOR WALL - EXTERIOR SHED ROOF & SHEARWALL
1"=1'-0"



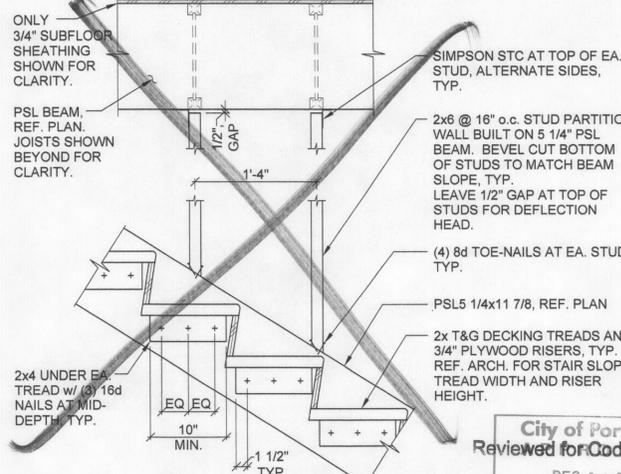
4 STAIR BASE CONNECTION DETAIL
1"=1'-0"



5 STAIR LANDING BEAM/COLUMN DETAIL
1"=1'-0"



2 STAIR LEDGER DETAIL
1"=1'-0"



3 STAIR EDGE BEAM DETAIL
1"=1'-0"



1223 sw washington street, suite 300
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503.863.2590 v
503.224.2482 f

ABHT
STRUCTURAL ENGINEERS

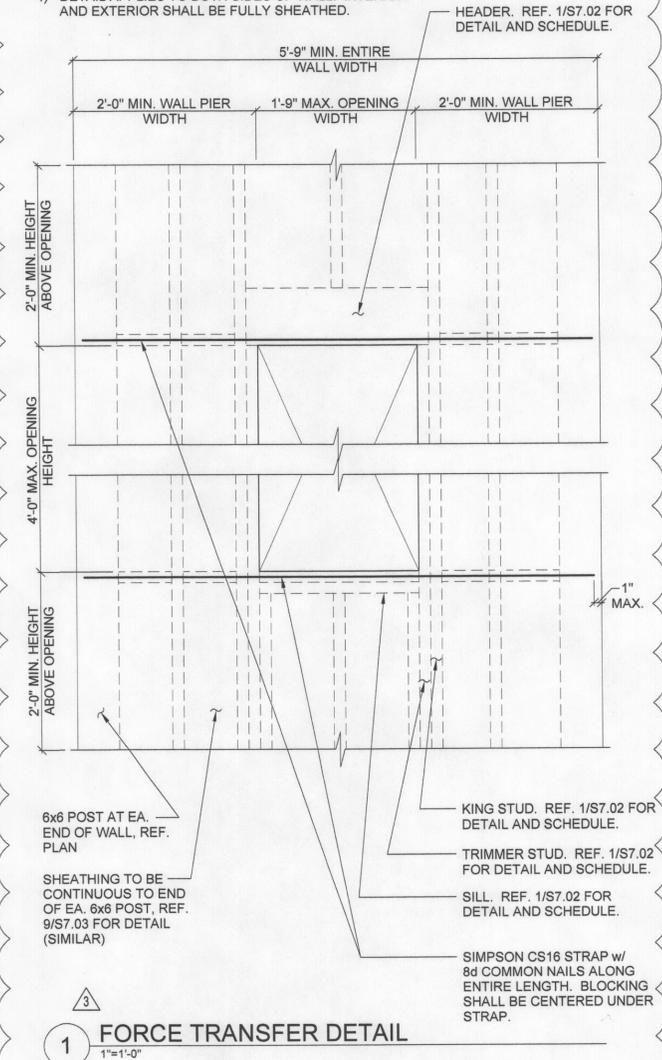
1640 NW JOHNSON STREET
PORTLAND, OR 97209
Tel 503.243.6682
Fax 503.243.6622
www.abht-structural.com



EXPIRES 12-31-11

NOTES:

- 1) MAXIMUM OPENING SIZE = 4'-0" TALL AND 1'-9" WIDE
- 2) MINIMUM HEIGHT OF SHEATHED WALL ABOVE AND BELOW OPENING SHALL BE 2'-0"
- 3) MINIMUM WIDTH OF WALL PIER ON EITHER SIDE OF OPENING SHALL BE 2'-0". WALL PIERS SHALL BE SHEATHED THEIR FULL HEIGHT.
- 4) DETAIL APPLIES TO BOTH SIDES OF WALL. INTERIOR AND EXTERIOR SHALL BE FULLY SHEATHED.



grant park
residence
portland, oregon

issues / revisions
permit set 9.29.2010
foundation 10.15.2010
framing 10.29.2010
revisions 11.17.2010

sheet title
framing details

sheet #

S7.04

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