

POLE BUILDING PLANS

BUILDING OWNER: RICH PEPPE
 BUILDING LOCATION: 4754 NORTH PRINCETON ST.
 PORTLAND, OR. 97203
 COUNTY: MULTNOMAH
 BUILDING DIMENSIONS: 30' X 30' X 10'
 BUILDING AREA: 900 SF
 ROOF PITCH: 4 IN 12 CONSTRUCTION TYPE: V-B
 ROOF FRAMING SYSTEM: PRE-ENGINEERED TRUSSES BY OTHERS
 ROOF ASSEMBLY: METAL
 WALL ASSEMBLY: METAL
 BUILDING USE: PRIVATE SHOP

CONTRACTOR/BUILDER: PARKER BUILDINGS
 ADDRESS: P.O. BOX 407
 HUBBARD, OR. 97032

BUILDING DESIGN CRITERIA

GROUND SNOW LOAD, $P_g = 25$ PSF
 ROOF DEAD LOAD = 3 PSF
 WIND SPEED = 95 MPH (3 SEC. GUST)
 WIND EXPOSURE = B
 SEISMIC DESIGN PARAMETERS:
 $S_s = 0.90$; $S_1 = 0.38$

SEISMIC DESIGN CATEGORY = D
 OCCUPANCY CATEGORY = II
 SOIL BEARING CAPACITY = 1,500 PSF

BUILDING CODES & REFERENCES

-2010 OSSC
 -ASCE 7-05
 -NDS 2005

SHT DRAWING LIST

1	C1.0	COVER SHEET
2	P1.0	PLAN VIEW
3	E1.0	ELEVATION VIEWS
4	S1.0	SECTION A
5	S2.0	SECTION B
6	D1.0	TRUSS CONNECTION DETAILS
7	D2.0	POST EMBEDMENT DETAILS
8	D3.0	PURLIN & GIRT DETAILS
9	D4.0	SHEATHING DETAILS
10	T1.0	SOAKAGE TRENCH

Permit Number
 MAY 20 2013
 City of Portland
 REVIEWED FOR CODE

THE PLANS FOR THIS BUILDING ARE THE PROPERTY OF SOUTH VALLEY ENGINEERING, LLC AND SHALL BE SURRENDERED UPON REQUEST. THIS BUILDING HAS BEEN ENGINEERED TO CONFORM TO THE DIMENSIONS AND BUILDING DESIGN CRITERIA SPECIFIED, AND SHALL BE BUILT ONLY AT THE LOCATION DESIGNATED ON THESE PLANS. ANY CHANGES OR MODIFICATIONS TO THESE PLANS, INCLUDING BUT NOT LIMITED TO BUILDING DIMENSIONS, DESIGN CRITERIA, OR BUILDING LOCATION WITHOUT EXPRESS WRITTEN PERMISSION OR DOCUMENTATION BY THE ENGINEER WHOSE SEAL IS AFFIXED TO THESE PLANS SHALL RENDER THESE PLANS NULL AND VOID, AND THE ENGINEER WHOSE SEAL IS AFFIXED TO THESE PLANS SHALL NOT BE HELD LIABLE FOR THE STRUCTURAL INTEGRITY OR CODE COMPLIANCE OF ANY BUILDING CONSTRUCTED REFERENCING THESE PLANS UNDER THESE CONDITIONS.

CONSTRUCTION NOTES

- ALL DRAWINGS SHALL BE REVIEWED AND APPROVED BY OWNER AND/OR CONTRACTOR PRIOR TO BEGINNING OF SITE WORK OR BUILDING CONSTRUCTION. ALL DIMENSIONS AND SPECIFICATIONS SHALL BE VERIFIED AND ANY DISCREPANCIES, ERRORS AND/OR OMISSIONS SHALL BE REPORTED TO SOUTH VALLEY ENGINEERING PRIOR TO MATERIAL PURCHASE AND BEGINNING OF CONSTRUCTION. SOUTH VALLEY ENGINEERING SHALL NOT BE HELD LIABLE FOR ANY COSTS OR DAMAGES INCURRED DUE TO DISCREPANCIES, ERRORS AND/OR OMISSIONS DISCOVERED AFTER CONSTRUCTION HAS BEGUN.
- THE BUILDING OWNER AND OR CONTRACTOR ACCEPTS FULL RESPONSIBILITY FOR ANY WORK DONE THAT IS NOT SHOWN ON OR DOES NOT COMPLY WITH THESE PLANS. REQUESTS FOR ANY CHANGES SHALL BE MADE TO THE ENGINEER IN WRITING, AND SHALL BE RECEIVED FROM THE ENGINEER IN WRITING PRIOR TO IMPLEMENTATION.
- STRUCTURAL CONCRETE FOR SLABS AND FOUNDATIONS SHALL BE A MINIMUM OF 2,500 PSI COMPRESSIVE STRENGTH AT 28 DAYS WITH NO SPECIAL INSPECTION REQUIRED. FOOTINGS AND FOUNDATIONS SHALL BE CAST AGAINST UNDISTURBED NATIVE SOIL UNLESS OTHERWISE NOTED ON THE PLANS.
- ALL DIMENSIONAL LUMBER SHALL BE #2 DF OR BETTER UNLESS OTHERWISE NOTED ON THE PLANS. HIGHER GRADE LUMBER OF SAME SPECIES MAY BE SUBSTITUTED FOR SPECIFIED GRADE.
- ALL WOOD IN CONTACT WITH CONCRETE ABOVE GROUND SHALL BE PRESSURE TREATED FOR ABOVE GROUND CONTACT. ALL WOOD EMBEDDED IN GROUND SHALL BE PRESSURE TREATED FOR BURIAL.
- ROOF & WALL WOOD SHEATHING (IF USED)-UNLESS OTHERWISE NOTED ON THE PLANS:
 ALL ROOF AND WALL WOOD SHEATHING SHALL BE AN APPROVED APA RATED SHEATHING, EXPOSURE I, GRADE C-D, EXTERIOR GLUE, 7/16" THICK (MINIMUM). EXTERIOR GRADE SHEATHING SHALL BE RATED FOR EXTERIOR USE. WOOD ROOF SHEATHING SHALL BE NAILED WITH 8d NAILS AT 6" O.C. EDGES AND 12" O.C. FIELD. 2X BLOCKING IS NOT REQUIRED FOR ROOF SHEATHING. WALL SHEATHING SHALL BE NAILED WITH 8d NAILS AT 6" O.C. EDGES AND 12" O.C. FIELD OR AS SPECIFIED ON THE PLANS. 2X BLOCKING SHALL BE INSTALLED AT ALL PANEL EDGES ON ALL WALLS.
- ALL FASTENERS EXPOSED TO THE ELEMENTS SHALL BE GALVANIZED OR CORROSION RESISTANT. ALL FASTENERS IN PRESSURE TREATED WOOD SHALL BE HOT-DIPPED GALVANIZED OR STAINLESS STEEL.
- IF PLANS SPECIFY GRAVEL BACKFILL IN POSTHOLE, BACKFILL WITH 3/4" MINUS CLEAN CRUSHED GRAVEL TO SPECIFIED EMBEDMENT DEPTH. BACKFILL IN 6" LIFTS AND SATURATE AND COMPACT EACH LIFT.
- UNLESS OTHERWISE NOTED, THE 6" THICK PAD AT THE BOTTOM OF THE POST HOLES IS NOT REQUIRED FOR DOOR POSTS NOT SUPPORTING VERTICAL LOADS, AND MAY BE OMITTED FOR STRUCTURAL POSTS BEARING ON SOLID ROCK.
- INSTALL ALL TRUSS BRACING PER TRUSS ENGINEERING. INSTALL TEMPORARY TRUSS BRACING DURING CONSTRUCTION PER BCSI-B10 GUIDELINES FOR POST FRAME TRUSS INSTALLATION, RESTRAINT & BRACING DURING CONSTRUCTION.

COVER SHEET

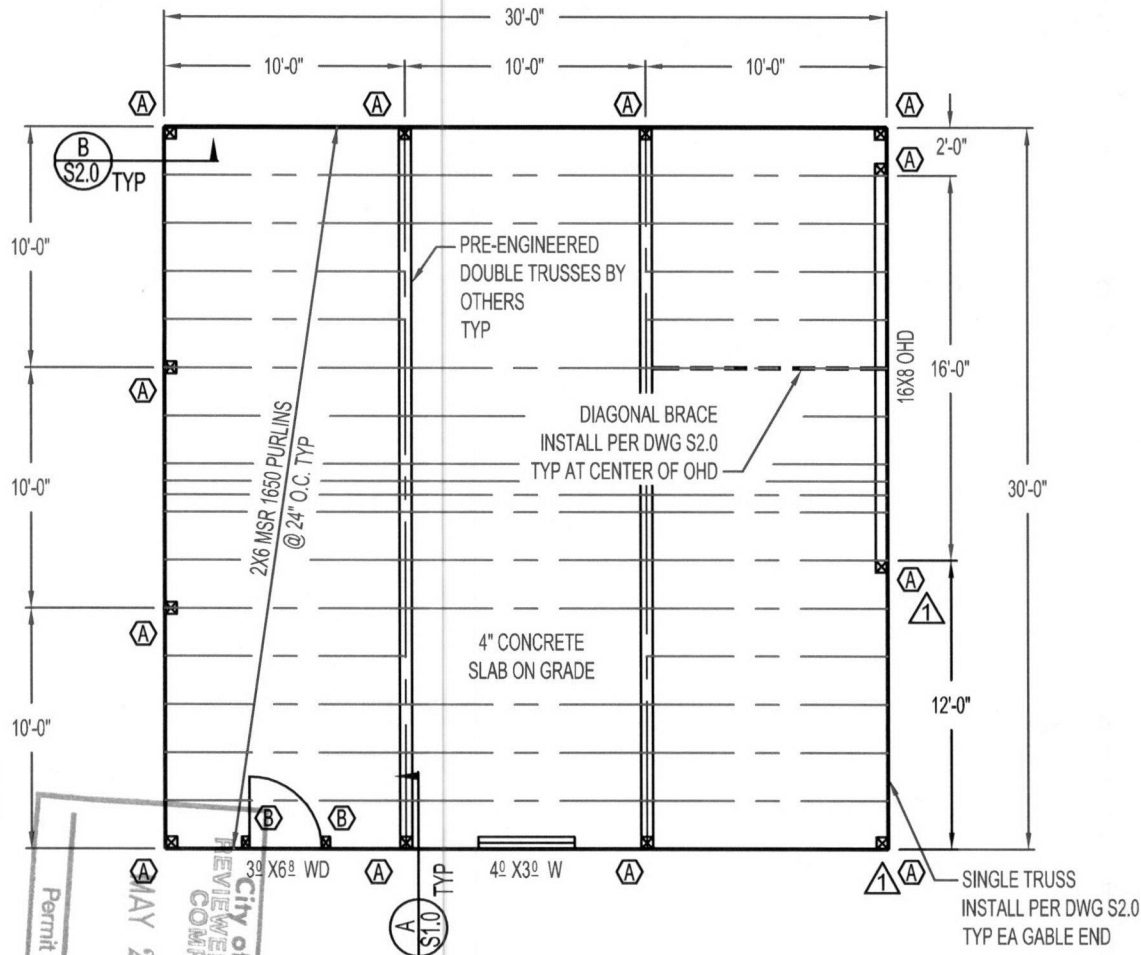
DRAWING NO.: C1.0	OWNER: RICH PEPPE
REV.: 0	SHEET: 1
PROJECT NO.: 11304003	LOCATION: 4754 NORTH PRINCETON ST. PORTLAND, OR. 97203
DATE: 9 APR 2013	SCALE: N/A
	DRAWN BY: MH



RENEWS: 6/30/13



South Valley Engineering
 4742 Liberty Rd. S #151 • Salem, OR. 97302
 Ph. (503) 302-7020 • Fax (888) 535-6341
 E-mail: southvalleyeng@comcast.net



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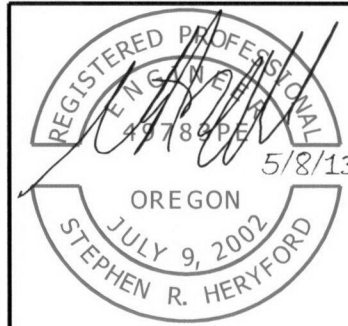
Permit Number
MAY 20 2013
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TABLE 1

POST	SIZE	HOLE DIMENSIONS		BACKFILL	NOTES
		DIAMETER	DEPTH		
(A)	6X6	24" Ø	5'-0"	CONCRETE	
(B)	4X6	18" Ø	2'-6"	CONCRETE	INSTALL PER CONSTRUCTION NOTES

GENERAL NOTES

- ALL STRUCTURAL POSTS TO BE #2 HF FULL-DIMENSIONED ROUGH-SAWN POSTS PRESSURE TREATED FOR BURIAL AND ORIENTED AS SHOWN UNLESS OTHERWISE NOTED
- UNLESS OTHERWISE DIMENSIONED, ALL WINDOWS AND WALK DOORS MAY BE LOCATED BY THE OWNER/CONTRACTOR IN THE WALLS SHOWN
- EMBED POSTS AND BACKFILL PER DWG D2.0



REV	DESCRIPTION	DATE
1	REVISED POSTHOLES, BACKFILL, NOTES 1 & 3	5/8/13

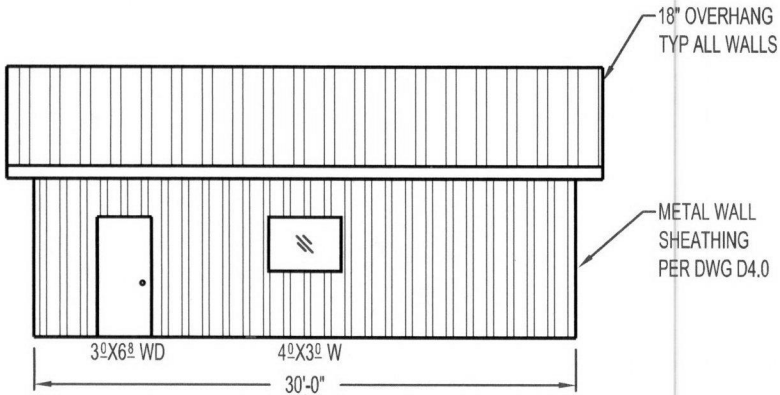
PLAN VIEW

DRAWING NO.: P1.0	OWNER: RICH PEPPE
REV.: 1	SHEET: 2
PROJECT NO.: 11304003	LOCATION: 4754 NORTH PRINCETON ST. PORTLAND, OR. 97203
DATE: 9 APR 2013	SCALE: 1/8" = 1'-0" DRAWN BY: SH

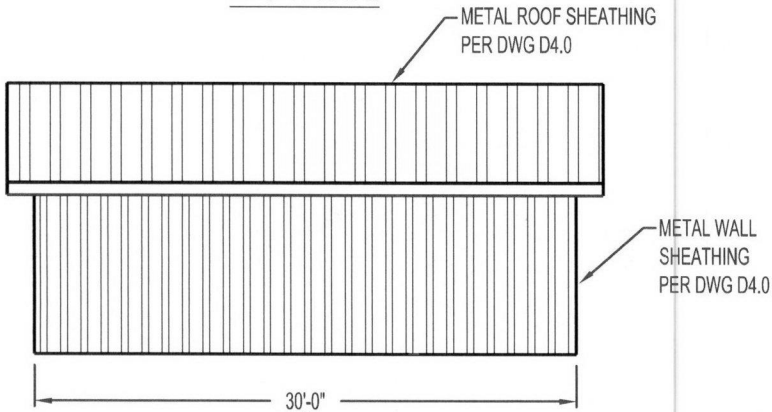


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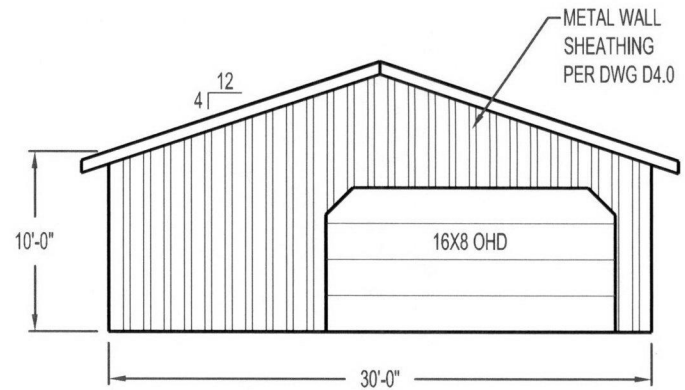
RENEWS: 6/30/13



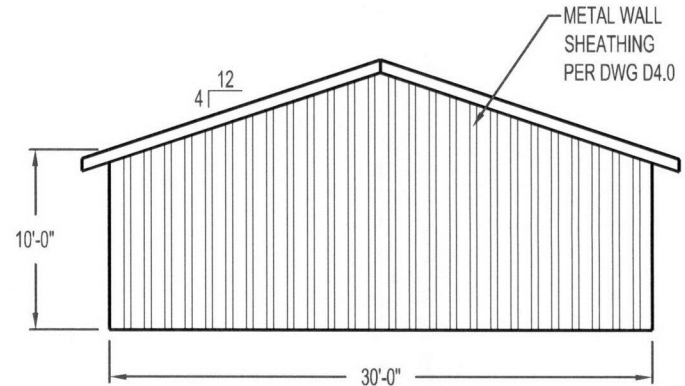
FRONT EAVE



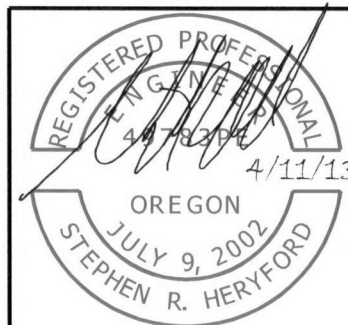
REAR EAVE



RIGHT GABLE



LEFT GABLE



RENEWS: 6/30/13

1	REVISED BUILDING LENGTH	4/11/13
REV	DESCRIPTION	DATE

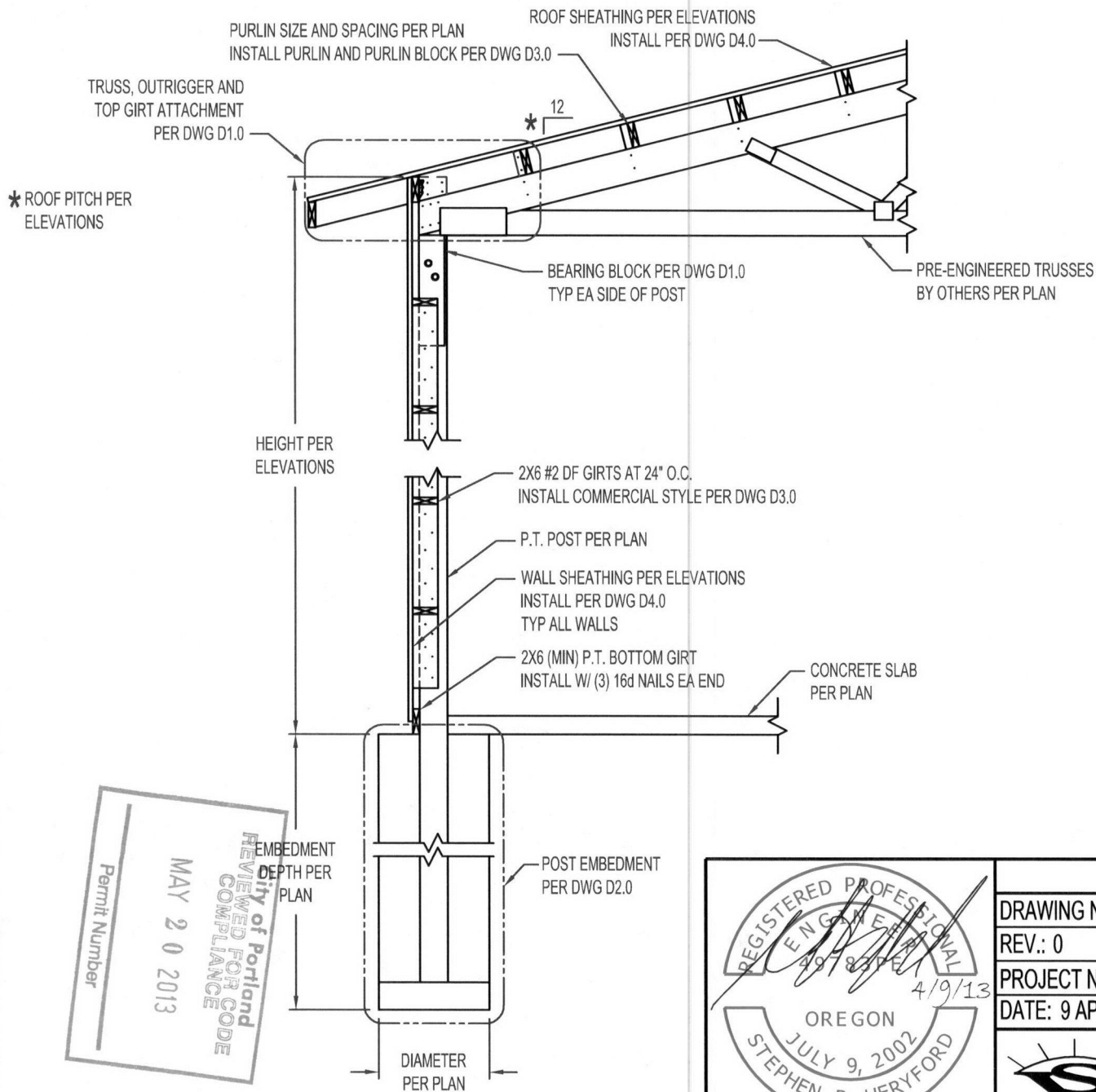
ELEVATION VIEWS

DRAWING NO.: E1.0		OWNER: RICH PEPPE	
REV.: 1	SHEET: 3	LOCATION: 4754 NORTH PRINCETON ST.	
PROJECT NO.: 11304003		PORTLAND, OR. 97203	
DATE: 9 APR 2013		SCALE: 3/32" = 1'-0"	DRAWN BY: SH



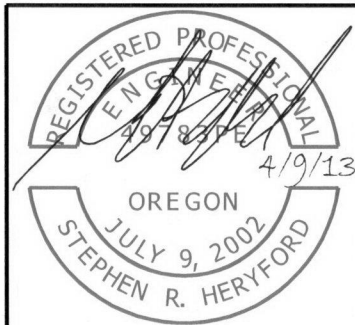
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* ROOF PITCH PER ELEVATIONS

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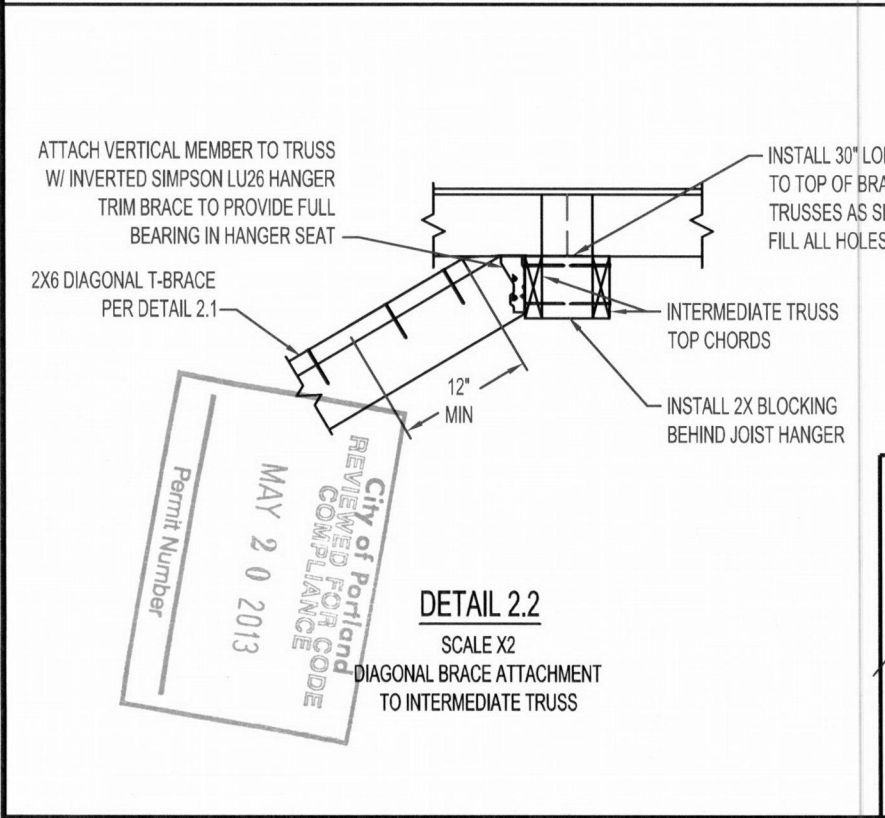
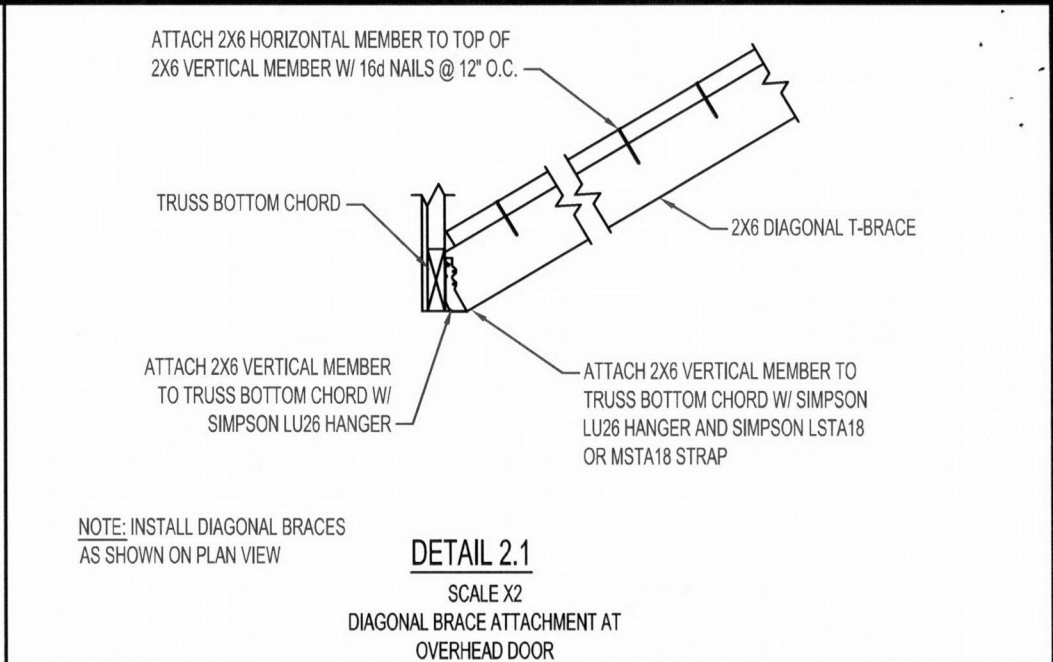
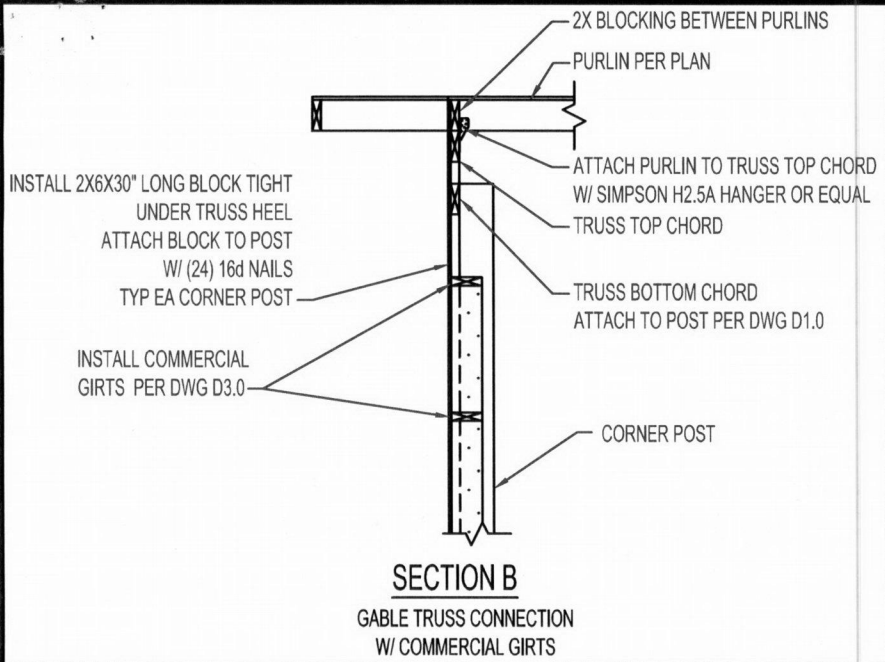
SECTION A

DRAWING NO.: S1.0		OWNER: RICH PEPPE	
REV.: 0	SHEET: 4	LOCATION: 4754 NORTH PRINCETON ST. PORTLAND, OR. 97203	
PROJECT NO.: 11304003		DRAWN BY: MH	
DATE: 9 APR 2013	SCALE: 3/8" = 1'-0"		



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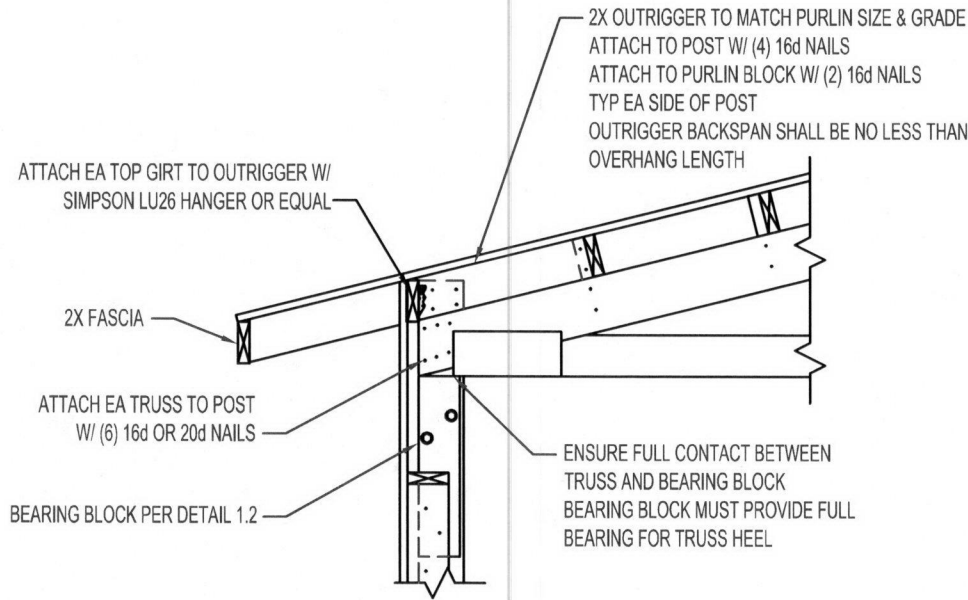
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E-mail: southvalleyeng@comcast.net



		SECTION B	
		DRAWING NO.: S2.0	OWNER: RICH PEPPE
REV.: 0	SHEET: 5	LOCATION: 4754 NORTH PRINCETON ST. PORTLAND, OR. 97203	
PROJECT NO.: 11304003		DATE: 9 APR 2013	SCALE: 3/8" = 1'-0" DRAWN BY: MH
		South Valley Engineering	
		4742 Liberty Rd. S #151 • Salem, OR. 97302 Ph. (503) 302-7020 • Fax (888) 535-6341 E-mail: southvalleyeng@comcast.net	

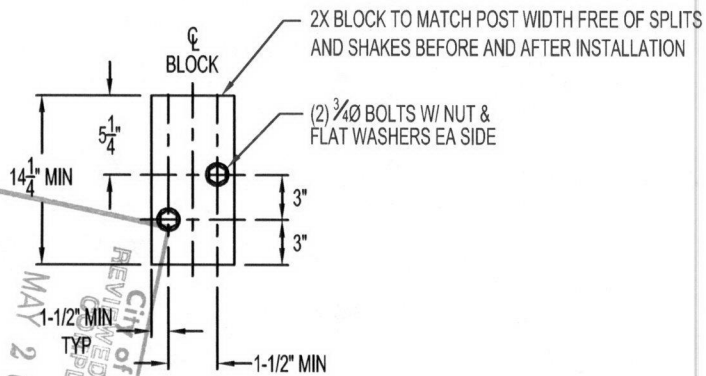
RENEWS: 6/30/13

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 MAY 20 2013
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DETAIL 1.1

TRUSS ATTACHMENT WITH COMMERCIAL GIRTS



DETAIL 1.2

SCALE: X2
BEARING BLOCK DIMENSIONS & BOLT SPACING

NOTE: ALL DIMENSIONS ARE MINIMUM DIMENSIONS AND MAY BE INCREASED BUT NOT DECREASED



RENEWS: 6/30/13

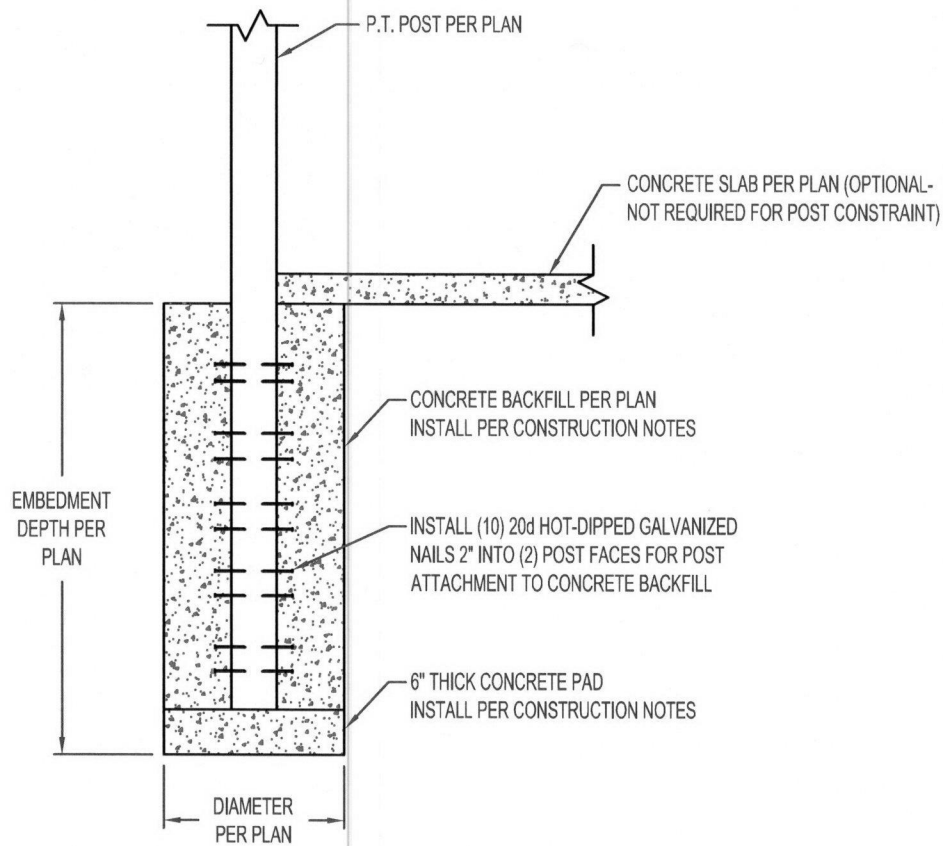
TRUSS CONNECTION DETAILS

DRAWING NO.: D1.0		OWNER: RICH PEPPE
REV.: 0	SHEET: 6	LOCATION: 4754 NORTH PRINCETON ST. PORTLAND, OR. 97203
PROJECT NO.: 11304003		
DATE: 9 APR 2013	SCALE: NTS	DRAWN BY: MH



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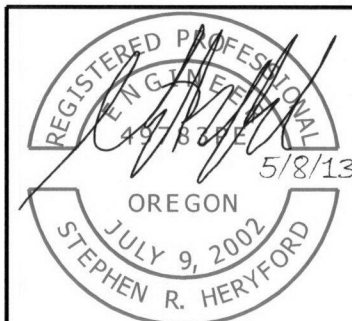


DETAIL 2.1

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1	REVISED DRAWING	5/8/13
REV	DESCRIPTION	DATE

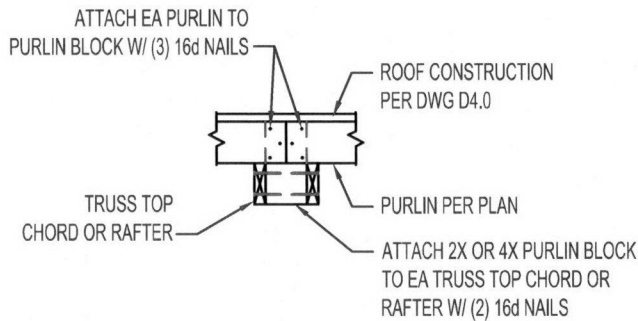
POST EMBEDMENT DETAILS

DRAWING NO.: D2.0		OWNER: RICH PEPPE
REV.: 1	SHEET: 7	LOCATION: 4754 NORTH PRINCETON ST. PORTLAND, OR. 97203
PROJECT NO.: 11304003		SCALE: NTS
DATE: 9 APR 2013	DRAWN BY: MH	



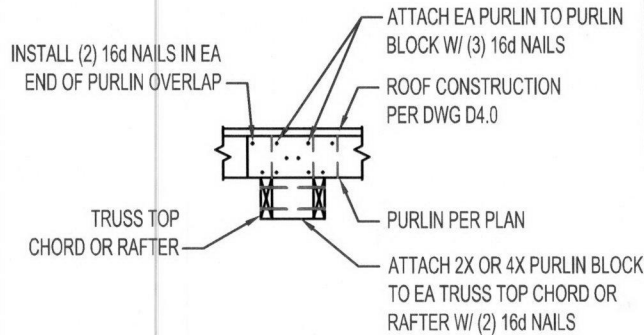
South Valley Engineering

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 E-mail: southvalleyeng@comcast.net



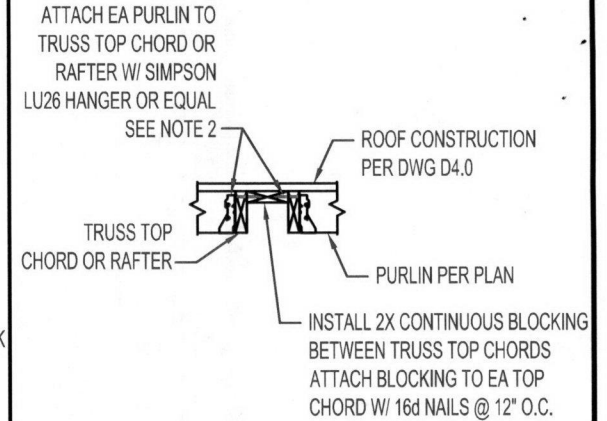
DETAIL 3.1

PURLINS STACKED AND BUTTED



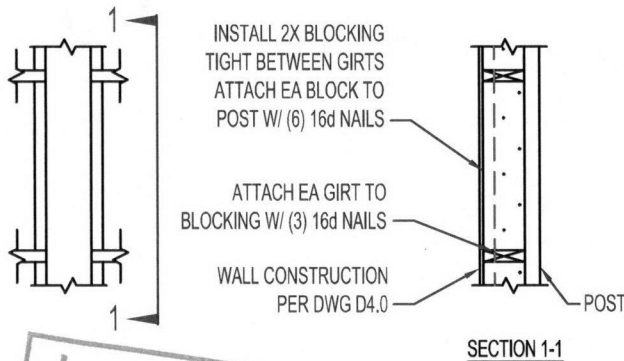
DETAIL 3.2

PURLINS STACKED AND OVERLAPPED



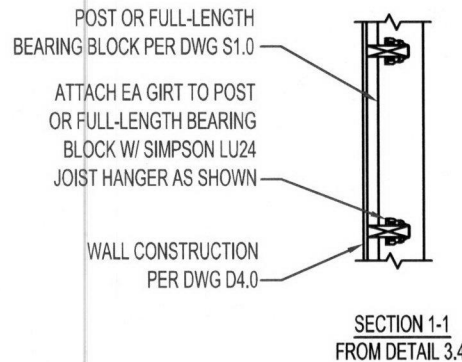
DETAIL 3.3

PURLINS INSTALLED WITH JOIST HANGERS



DETAIL 3.4

COMMERCIAL GIRTS INSTALLATION



DETAIL 3.5

ALTERNATE COMMERCIAL GIRTS INSTALLATION



GENERAL NOTES

1. ALL NAILS INTO PRESSURE TREATED LUMBER SHALL BE HOT-DIPPED GALVANIZED
2. IF PURLIN DEPTH EXCEEDS TRUSS TOP CHORD DEPTH, USE SIMPSON JB TOP FLANGE HANGER OR EQUAL



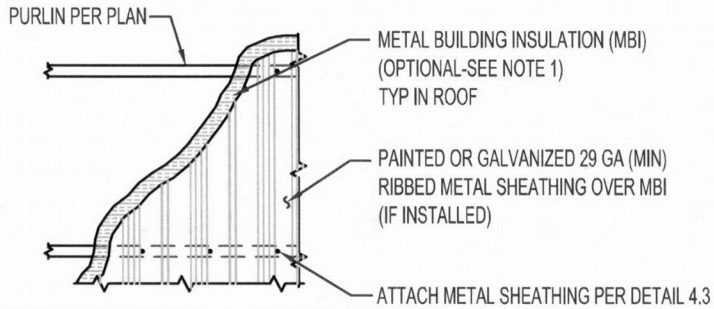
RENEWS: 6/30/13

PURLIN & GIRTS DETAILS

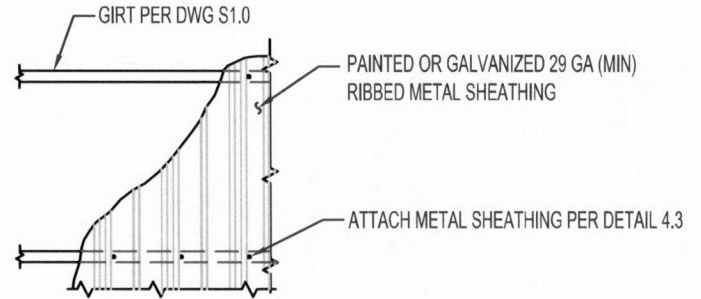
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REV.: 0	SHEET: 8
PROJECT NO.: 11304003	LOCATION: 4754 NORTH PRINCETON ST. PORTLAND, OR. 97203
DATE: 9 APR 2013	SCALE: NTS
	DRAWN BY: MH



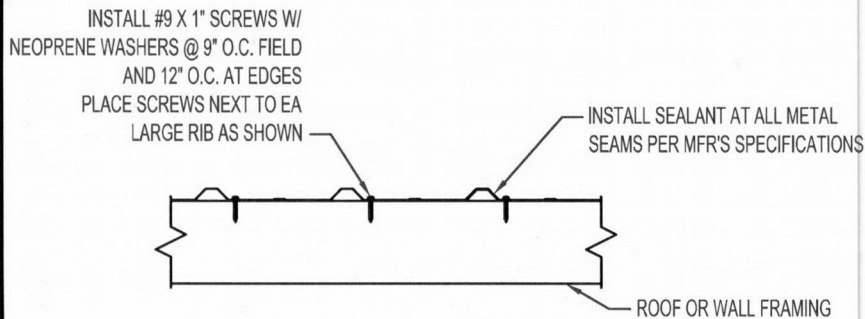
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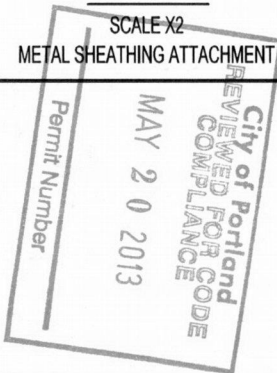
DETAIL 4.1
ROOF SHEATHING



DETAIL 4.2
WALL SHEATHING

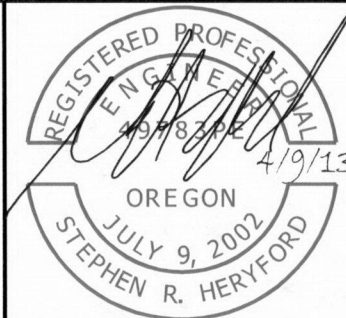


DETAIL 4.3
SCALE X2
METAL SHEATHING ATTACHMENT



GENERAL NOTES

1. METAL BUILDING INSULATION IS NOT REQUIRED BUT IS RECOMMENDED TO REDUCE OR PREVENT CONDENSATION



RENEWS: 6/30/13

SHEATHING DETAILS

DRAWING NO.: D4.0		OWNER: RICH PEPPE
REV.: 0	SHEET: 9	LOCATION: 4754 NORTH PRINCETON ST. PORTLAND, OR. 97203
PROJECT NO.: 11304003		
DATE: 9 APR 2013	SCALE: NTS	DRAWN BY: MH



South Valley Engineering

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Ph. (503) 302-7020 • Fax (888) 535-6341

E-mail: southvalleyeng@comcast.net

PLANS FOR PEPPE, RICH

DATE: 4/9/13

**PARKER BUILDINGS INC.
PO BOX 407
3540 SECOND STREET
HUBBARD OR 97032**

503-981-0890 or 1-800-331-0155

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MAY 20 2013
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13-159734RS

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MiTek USA, Inc.

7777 Greenback Lane
Suite 109
Citrus Heights, CA, 95610
Telephone 916/676-1900
Fax 916/676-1909

Re: B3-00514
PEPPE

The truss drawing(s) referenced below have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by BJS' Metal & Lumber Products.

Pages or sheets covered by this seal: R36189680 thru R36189681

My license renewal date for the state of Oregon is June 30, 2014.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3
These truss designs rely on lumber values established by others.



Digital Signature

EXPIRATION DATE: 06/30/14



April 4, 2013

Tingey, Palmer

The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI 1.



MiTek USA, Inc.

7777 Greenback Lane
Suite 109
Citrus Heights, CA, 95610
Telephone 916/676-1900
Fax 916/676-1909

Re: B3-00514
PEPPE

The truss drawing(s) referenced below have been prepared by MiTek Industries, Inc. under my direct supervision based on the parameters provided by BJS' Metal & Lumber Products.

Pages or sheets covered by this seal: R36189680 thru R36189681

My license renewal date for the state of Washington is August 1, 2014.

Lumber design values are in accordance with ANSI/TPI 1 section 6.3
These truss designs rely on lumber values established by others.



April 4, 2013

Tingey, Palmer

The seal on these drawings indicate acceptance of professional engineering responsibility solely for the truss components shown. The suitability and use of this component for any particular building is the responsibility of the building designer, per ANSI/TPI 1.



Job	Truss	Truss Type	Qty	Ply	PEPPE	R36189680
B3-00514	T30	FINK	4	1		

BJ's Metal & Lumber, Salem, OR 97303

7 250 s Aug 25 2011 MiTek Industries, Inc. Wed Apr 03 12:54:58 2013 Page 1
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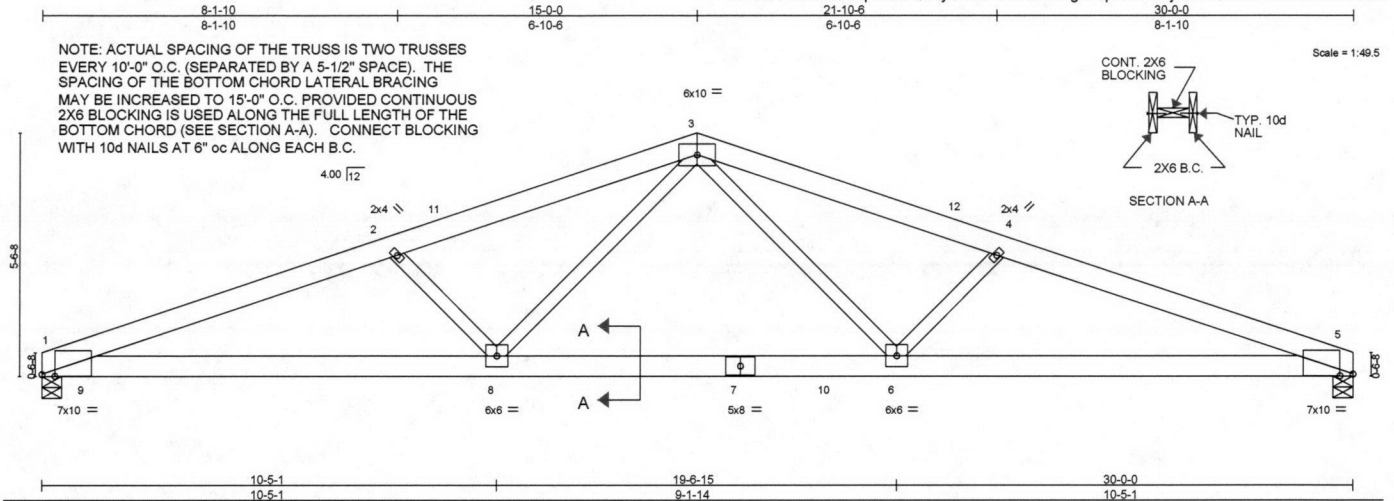


Plate Offsets (X,Y): [1:0-3-10.0-0-7], [5:0-3-10.0-0-7]

LOADING (psf)	SPACING	CSI	DEFL	PLATES	GRIP
TCLL 25.0 (Roof Snow=25.0)	5-0-0 Plates Increase 1.15 Lumber Increase 1.15 Rep Stress Incr NO Code IBC2006/TPI2002	TC 0.70 BC 0.88 WB 0.90 (Matrix)	in (loc) l/defl L/d Vert(LL) -0.28 6-8 >999 240 Vert(TL) -0.67 1-8 >529 180 Horz(TL) 0.17 5 n/a n/a	MT20	220/195
TCDL 3.0				Weight: 162 lb	FT = 20%
BCLL 0.0					
BCDL 10.0					

LUMBER	BRACING
TOP CHORD 2 X 6 DF SS G	TOP CHORD 2-0-0 oc purlins (2-7-10 max.).
BOT CHORD 2 X 6 DF SS G	BOT CHORD Rigid ceiling directly applied or 6-1-14 oc bracing.
WEBS 2 X 4 DF Stud/Std G	JOINTS 1 Brace at Jt(s): 3

REACTIONS (lb/size) 1=2903/0-5-8 (min. 0-3-2), 5=2850/0-5-8 (min. 0-3-1)
 Max Uplift1=-847(LC 5), 5=-831(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
 TOP CHORD 1-2=-6637/1961, 2-11=-5902/1768, 3-11=-5744/1786, 3-12=-5783/1797, 4-12=-5941/1779, 4-5=-6672/1972
 BOT CHORD 1-9=-1753/6148, 8-9=-1753/6148, 7-8=-1102/4262, 7-10=-1102/4262, 6-10=-1102/4262, 5-6=-1762/6180
 WEBS 2-8=-1262/490, 3-8=-477/1988, 3-6=-493/2043, 4-6=-1254/488

- NOTES**
- 1) Wind: ASCE 7-05; 95mph; TCDL=1.8psf; BCDL=0.6psf; h=25ft; Cat. II; Exp B; enclosed; C-C Interior(1); end vertical left and right exposed; Lumber DOL=1.15 plate grip DOL=1.15
 - 2) TCLL: ASCE 7-05; Pf=25.0 psf (flat roof snow); Category II; Exp B; Partially Exp.; Ct=1
 - 3) Unbalanced snow loads have been considered for this design.
 - 4) Dead loads shown include weight of truss. Top chord dead load of 5.0 psf (or less) is not adequate for a shingle roof. Architect to verify adequacy of top chord dead load.
 - 5) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 6) A plate rating reduction of 20% has been applied for the green lumber members.
 - 7) This truss is designed in accordance with the 2006 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.
 - 8) Hanger(s) or other connection device(s) shall be provided sufficient to support concentrated load(s) 70 lb down and 20 lb up at 1-0-0, and 70 lb down and 20 lb up at 18-0-0 on bottom chord. The design/selection of such connection device(s) is the responsibility of others.
 - 9) In the LOAD CASE(S) section, loads applied to the face of the truss are noted as front (F) or back (B).

LOAD CASE(S) Standard

- 1) Snow: Lumber Increase=1.15, Plate Increase=1.15
 Uniform Loads (plf)
 Vert: 1-5=-50, 1-3=-140, 3-5=-140
 Concentrated Loads (lb)
 Vert: 9=-70(F) 10=-70(F)



April 4, 2013

WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
 Design valid for use only with MiTek connectors. This design is based only upon parameters shown, and is for an individual building component. Applicability of design parameters and proper incorporation of component is responsibility of building designer - not truss designer. Bracing shown is for lateral support of individual web members only. Additional temporary bracing to insure stability during construction is the responsibility of the erector. Additional permanent bracing of the overall structure is the responsibility of the building designer. For general guidance regarding fabrication, quality control, storage, delivery, erection and bracing, consult **ANSI/TPI1 Quality Criteria, D58-89 and BCSI Building Component Safety Information** available from Truss Plate Institute, 781 N. Lee Street, Suite 312, Alexandria, VA 22314.
 If Southern Pine (SP) lumber is specified, the design values are those effective 06/01/2013 by ALSC.

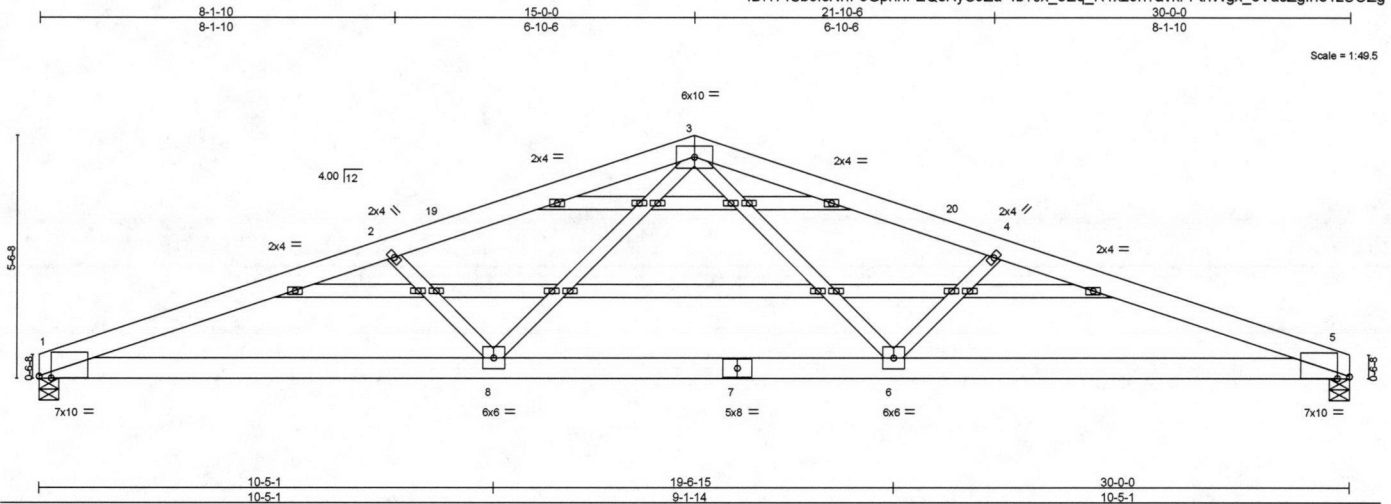
777 Greenback Lane, Suite 109
 Citrus Heights, CA, 95610

Job B3-00514	Truss T30G	Truss Type GABLE	Qty 2	Ply 1	PEPPE	R36189681
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BJ's Metal & Lumber, Salem, OR 97303

7,250 s Aug 25 2011 MiTek Industries, Inc. Wed Apr 03 12:54:59 2013 Page 1
ID:Tr4Sb9icNrtFoGphhFEQsRyS9Zu-4b?Jx_8Eq_R1xz9h?avkFFtnWgx_oVusZgfn1zUOZg

Scale = 1:49.5



LOADING (psf)		SPACING		CSI		DEFL		PLATES		GRIP	
TCLL	25.0	Plates Increase	5-0-0	TC	0.69	in (loc)	l/defl	L/d	MT20	220/195	
(Roof Snow=25.0)		Lumber Increase	1.15	BC	0.87	Vert(LL)	-0.27	6-8	>999	240	
TCDL	3.0	Rep Stress Incr	NO	WB	0.87	Vert(TL)	-0.66	1-8	>536	180	
BCLL	0.0	Code	IBC2006/TPI2002	(Matrix)		Horz(TL)	0.16	5	n/a	n/a	
BCDL	10.0										Weight: 197 lb FT = 20%

LUMBER	BRACING
TOP CHORD 2 X 6 DF SS G	TOP CHORD 2-0-0 oc purlins (2-8-1 max.).
BOT CHORD 2 X 6 DF SS G	BOT CHORD Rigid ceiling directly applied or 4-11-9 oc bracing.
WEBS 2 X 4 DF Stud/Std G	JOINTS 1 Brace at Jt(s): 3
OTHERS 2 X 4 DF Stud/Std G	

REACTIONS (lb/size) 1=2806/0-5-8 (min. 0-3-0), 5=2806/0-5-8 (min. 0-3-0)
Max Uplift1=-1225(LC 5), 5=-1225(LC 5)

FORCES (lb) - Max. Comp./Max. Ten. - All forces 250 (lb) or less except when shown.
TOP CHORD 1-2=-6533/2879, 2-19=-5799/2596, 3-19=-5641/2614, 3-20=-5641/2614, 4-20=-5799/2596, 4-5=-6533/2879
BOT CHORD 1-8=-2584/6049, 7-8=-1626/4180, 6-7=-1626/4180, 5-6=-2584/6049
WEBS 2-8=-1260/717, 3-8=-706/1964, 3-6=-706/1964, 4-6=-1260/717

- NOTES**
- 1) Wind: ASCE 7-05; 95mph; TCDL=1.8psf; BCDL=0.6psf; h=25ft; Cat. II; Exp B; enclosed; C-C Exterior(2); end vertical left and right exposed; Lumber DOL=1.15 plate grip DOL=1.15
 - 2) Truss designed for wind loads in the plane of the truss only. For studs exposed to wind (normal to the face), see Standard Industry Gable End Details as applicable, or consult qualified building designer as per ANSI/TPI 1-2002.
 - 3) TCLL: ASCE 7-05; Pf=25.0 psf (flat roof snow); Category II; Exp B; Partially Exp.; Ct= 1
 - 4) Unbalanced snow loads have been considered for this design.
 - 5) Dead loads shown include weight of truss. Top chord dead load of 5.0 psf (or less) is not adequate for a shingle roof. Architect to verify adequacy of top chord dead load.
 - 6) All plates are 1.5x4 MT20 unless otherwise indicated.
 - 7) Horizontal gable studs spaced at 2-0-0 oc.
 - 8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
 - 9) A plate rating reduction of 20% has been applied for the green lumber members.
 - 10) This truss is designed in accordance with the 2006 International Building Code section 2306.1 and referenced standard ANSI/TPI 1.

LOAD CASE(S) Standard



April 4, 2013

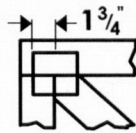
WARNING - Verify design parameters and READ NOTES ON THIS AND INCLUDED MITEK REFERENCE PAGE MII-7473 BEFORE USE.
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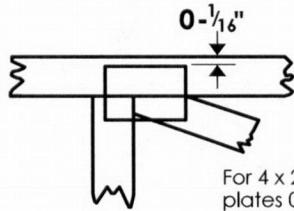
7777 Greenback Lane, Suite 109
Citrus Heights, CA, 95610

Symbols

PLATE LOCATION AND ORIENTATION



Center plate on joint unless x, y offsets are indicated. Dimensions are in ft-in-sixteenths. Apply plates to both sides of truss and fully embed teeth.



For 4 x 2 orientation, locate plates 0-¹/₁₆" from outside edge of truss.



This symbol indicates the required direction of slots in connector plates.

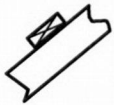
* Plate location details available in MiTek 20/20 software or upon request.

PLATE SIZE

4 x 4

The first dimension is the plate width measured perpendicular to slots. Second dimension is the length parallel to slots.

LATERAL BRACING LOCATION



Indicated by symbol shown and/or by text in the bracing section of the output. Use T or I bracing if indicated.

BEARING



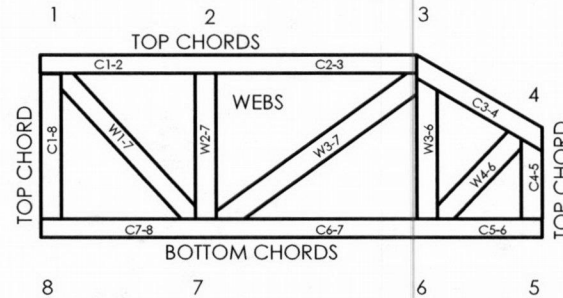
Indicates location where bearings (supports) occur. Icons vary but reaction section indicates joint number where bearings occur.

Industry Standards:

- ANSI/TPI1: National Design Specification for Metal Plate Connected Wood Truss Construction.
- DSB-89: Design Standard for Bracing.
- BCSI: Building Component Safety Information, Guide to Good Practice for Handling, Installing & Bracing of Metal Plate Connected Wood Trusses.

Numbering System

6-4-8 dimensions shown in ft-in-sixteenths (Drawings not to scale)



JOINTS ARE GENERALLY NUMBERED/LETTERED CLOCKWISE AROUND THE TRUSS STARTING AT THE JOINT FARTHEST TO THE LEFT.

CHORDS AND WEBS ARE IDENTIFIED BY END JOINT NUMBERS/LETTERS.

PRODUCT CODE APPROVALS

ICC-ES Reports:

ESR-1311, ESR-1352, ESR1988
ER-3907, ESR-2362, ESR-1397, ESR-3282

Southern Pine lumber designations are as follows:

SYP represents values as published by AWC in the 2005/2012 NDS
SP represents ALSC approved/new values with effective date of June 1, 2013

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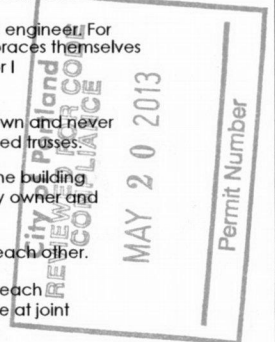
MiTek Engineering Reference Sheet: MII-7473 rev. 02/26/2013



General Safety Notes

Failure to Follow Could Cause Property Damage or Personal Injury

1. Additional stability bracing for truss system, e.g. diagonal or X-bracing, is always required. See BCSI.
2. Truss bracing must be designed by an engineer. For wide truss spacing, individual lateral braces themselves may require bracing, or alternative Tor I bracing should be considered.
3. Never exceed the design loading shown and never stack materials on inadequately braced trusses.
4. Provide copies of this truss design to the building designer, erection supervisor, property owner and all other interested parties.
5. Cut members to bear tightly against each other.
6. Place plates on each face of truss at each joint and embed fully. Knots and wane at joint locations are regulated by ANSI/TPI 1.
7. Design assumes trusses will be suitably protected from the environment in accord with ANSI/TPI 1.
8. Unless otherwise noted, moisture content of lumber shall not exceed 19% at time of fabrication.
9. Unless expressly noted, this design is not applicable for use with fire retardant, preservative treated, or green lumber.
10. Camber is a non-structural consideration and is the responsibility of truss fabricator. General practice is to camber for dead load deflection.
11. Plate type, size, orientation and location dimensions indicated are minimum plating requirements.
12. Lumber used shall be of the species and size, and in all respects, equal to or better than that specified.
13. Top chords must be sheathed or purlins provided at spacing indicated on design.
14. Bottom chords require lateral bracing at 10 ft. spacing, or less, if no ceiling is installed, unless otherwise noted.
15. Connections not shown are the responsibility of others.
16. Do not cut or alter truss member or plate without prior approval of an engineer.
17. Install and load vertically unless indicated otherwise.
18. Use of green or treated lumber may pose unacceptable environmental, health or performance risks. Consult with project engineer before use.
19. Review all portions of this design (front, back, words and pictures) before use. Reviewing pictures alone is not sufficient.
20. Design assumes manufacture in accordance with ANSI/TPI 1 Quality Criteria.



IMPERVIOUS AREA

BUILDING COVERAGE

LOT AREA

DRIVEWAY 332

BUILDING FOOTPRINT 2013

7333 SQ FT

PATIO

WALK 60

ROOF AREA (INCL. OVERHANG) 2307

TOTAL 2699

PRINCETON/AMHERST ALLEY

UTILITY POLE

66.6

144.0'

144.0'

PROPOSED POLE BUILDING
30' x 30'
18" OVERHANGS

PROPOSED GRAVEL DRIVE

28'

30'6"

14'

TREE TO BE REMOVED

5'

33'

RAIN DRAINS TO SPLASH BLOCKS APPROVED WITH BDS PLUMBING APPROVAL

30'6"

SEWER

4'

CONNECTED DOWNSPOUTS (E)

GUTTER RUN OFF

GAS

SIDE WALK

WATER

N. PRINCETON ST.

3'3"

10'

3'3"

110'

WORK IN PUBLIC RIGHT - OF - WAY IS NOT APPROVED AS PART OF THIS BUILDING PERMIT

City of Portland
REVIEWED FOR CODE COMPLIANCE
MAY 20 2013
Permit Number

City of Portland
Bureau of
Development Services
By [Signature]
Approved by [Signature]
Date 4.16.13
Planning and Zoning Review

PROJECT LEGAL

PROJECT ADDRESS

SITE PLAN

UNIVERSITY PARK

4754 N. PRINCETON ST.

BLOCK 77 LOT 9#10

PORTLAND OR 97203

R2922H

13-139734RS

SCALE 1" = 10'

21 10'



NORTH ARROW