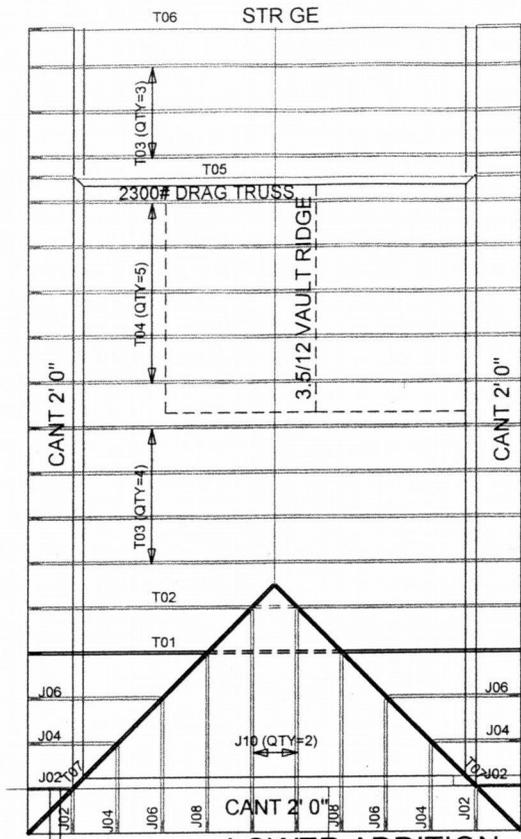


18'

27'2"

6'7"



PARR LUMBER
 HOFF CONSTRUCTION
 WESTWOOD RESIDENCE
 7/12 2' 0" CANT
 LOADING 25-7-10

| SIMPSON / ITW HANGERS | NAILS |
|-----------------------|------------|
| A=HUS28 / HDPT28 | N16 2 1/2" |
| B=HUS28 / HDTP28 | N16 2 1/2" |
| C=LUS24 / STP24 | N16 1 1/2" |
| D=HUS28-2 / HDTP28-2 | N16 2 1/2" |
| E=HGUS28-2 / HHDP28-2 | N16 2 1/2" |
| F=HGUS28-3 / HHDP28-3 | N16 2 1/2" |
| G=THJA-28 / MLJT | N16 2 1/2" |
| H=MTM / MMH | N16 2 1/2" |

ALL HANGERS LUS24 U.N.O.

LOWER ADDITION
 HAND CUT

City of Portland
 REVIEWED FOR CODE
 COMPLIANCE

OCT 30 2012

Permit Number

EXISTING ROOF

Handwritten signature

193375.Revised

TRUSWAY
 Service, Quality, Integrity
 The Builders' Choice
 3901 NE 69th Street
 Vancouver, WA 98631
 (360) 750-1470 Vancouver
 (503) 265-2615 Portland
 (360) 750-1493 Fax
 1.877-TRUSWAY Toll Free

Customer: PARR LUMBER ROCKWOOD
 Owner: HOFF CONSTRUCTION
 Plan: WESTWOOD RESIDENCE
 Salesman: Dean Harworth
 Elevation: CANTED

JOB NO:
 124062

PAGE NO:
 1 OF 1

ITW Building Components Group, Inc.

8351 Rovana Circle Sacramento, CA 95828 (916) 387-0116
Page 1 of 1 Document ID: 1UQN561-Z1826094306

Truss Fabricator: **Trus-Way, Inc**
Job Identification: **124062-PARR LUMBER ROCKWOOD WESTWOOD RESIDENCE -- 1337 SE LEXINGTON PORTLAND, O**
Model Code: **IRC**
Truss Criteria: **IRC2009/TPI-2007(STD)**
Engineering Software: **Alpine proprietary truss analysis software. Version 10.03.**
Truss Design Loads: **Roof - 42 PSF @ 1.15 Duration**
Floor - N/A
Wind - 110 MPH (ASCE 7-05-Closed)

Notes:

1. Determination as to the suitability of these truss components for the structure is the responsibility of the building designer/engineer of record, as defined in ANSI/TPI 1
2. As shown on attached drawings; the drawing number is preceded by: CAUSR561

Details: A1103005-GBLLETIN-

Submitted by CWC 09:42:57 10-26-2012 Reviewer: PBC

\$ \$

| # | Ref | Description | Drawing# | Date |
|----|------------|-------------|----------|----------|
| 1 | 89574--T01 | | 12300001 | 10/26/12 |
| 2 | 89575--T02 | | 12300002 | 10/26/12 |
| 3 | 89576--T03 | | 12300003 | 10/26/12 |
| 4 | 89577--T04 | | 12300004 | 10/26/12 |
| 5 | 89578--T05 | | 12300005 | 10/26/12 |
| 6 | 89579--T06 | | 12300006 | 10/26/12 |
| 7 | 89580--T07 | | 12300012 | 10/26/12 |
| 8 | 89581--J02 | | 12300011 | 10/26/12 |
| 9 | 89582--J04 | | 12300010 | 10/26/12 |
| 10 | 89583--J06 | | 12300009 | 10/26/12 |
| 11 | 89584--J08 | | 12300008 | 10/26/12 |
| 12 | 89585--J10 | | 12300007 | 10/26/12 |



EXP. 10/26/2012



(124062-PARR LUMBER ROCKWOOD WESTWOOD RESIDENCE -- 1337 SE LEXINGTON PORTLAND, O - T01)

Top chord 2x4 DF-L #1&Bet.(g) :T2 1.5"x5.625" DF-L SS(g):
 Bot chord 2x4 DF-L #1&Bet.(g)
 Webs 2x4 DF-L Standard(g)

110 mph wind, 20.65 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=6.0 psf.

Connectors in green lumber (g) designed using NDS/TPI reduction factors.

Left and right cantilevers are exposed to wind

Special loads

----- (Lumber Dur.Fac.=1.15 / Plate Dur.Fac.=1.15)
 TC- From 66 plf at 0.00 to 66 plf at 7.91
 TC- From 2 plf at 7.91 to 2 plf at 14.06
 TC- From 66 plf at 14.06 to 66 plf at 22.00
 BC- From 20 plf at 0.00 to 20 plf at 7.97
 BC- From 10 plf at 7.97 to 10 plf at 14.03
 BC- From 20 plf at 14.03 to 20 plf at 22.00
 TC- 450.00 lb Conc. Load at 7.91,14.06
 TC- 249.49 lb Conc. Load at 10.00,12.00
 BC- 82.75 lb Conc. Load at 7.97,10.00,12.00,14.03

(a) 1x4 #3 HEM-FIR or better continuous lateral bracing to be equally spaced. Attach with (2) 8d Box or Gun nails (0.113"x2.5",min.). Bracing material to be supplied and attached at both ends to a suitable support by erection contractor.

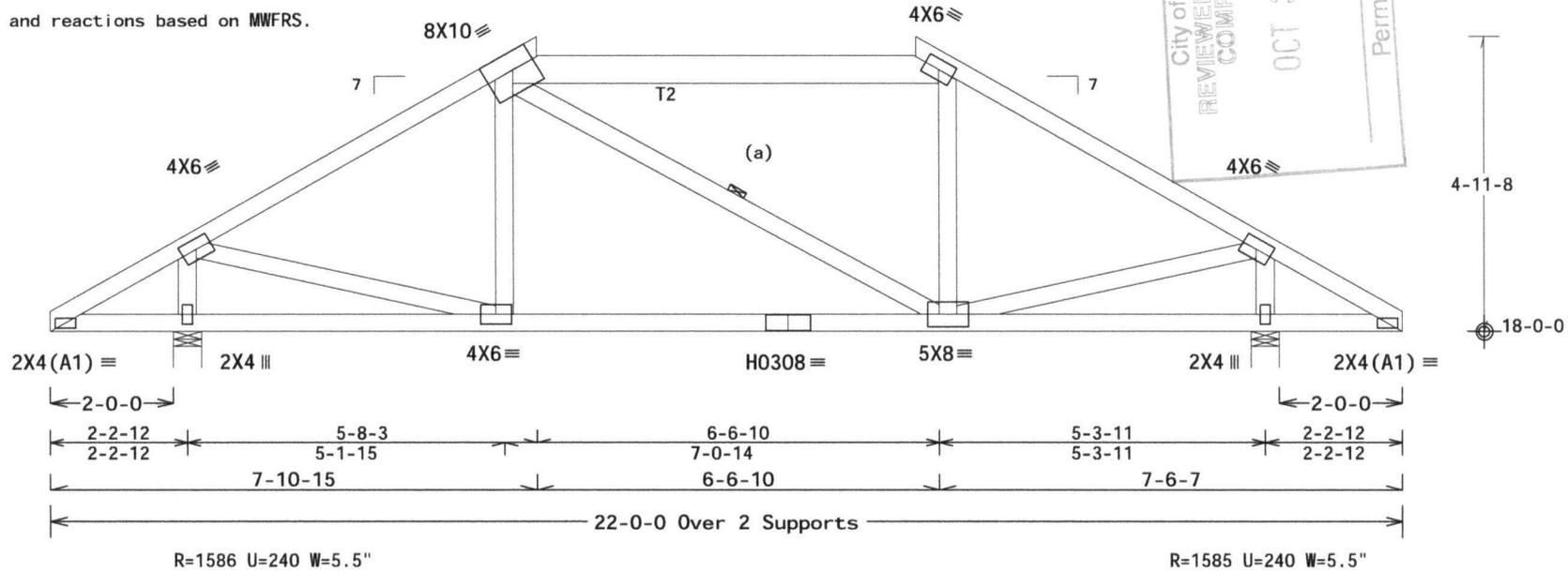
Bottom chord checked for 10.00 psf non-concurrent bottom chord live load applied per IRC-09 section 301.5.

Calculated vertical deflection is 0.08" due to live load and 0.25" due to total load at X = 10-11-2.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 2.00.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.

Wind loads and reactions based on MWFRS.



City of Portland
 REVIEWED FOR CODE
 COMPLIANCE
 OCT 30 012
 Permit Number

Design Crit: IRC2009/TPI-2007 (STD)
 FT/RT=8% (0%) / 4 (1)

PLT TYP. 20 Gauge HS, Wave

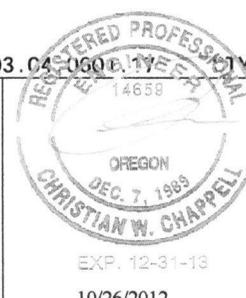
10.03.04-060.117 OR/-/1/-/1/-/1/-

Scale = .375"/Ft.

Trus-Way, Inc 360-750-1470
 3901 NE 68th St., Vancouver WA

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC31 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 8300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BUILDING COMPONENTS GROUP, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. ALPINE DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AFAPA) AND TPI. CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



| | | |
|----------|----------|-----------------------|
| TC LL | 25.0 PSF | REF R561-- 89574 |
| TC DL | 7.0 PSF | DATE 10/26/12 |
| BC DL | 10.0 PSF | DRW CAUSR561 12300001 |
| BC LL | 0.0 PSF | CA-ENG PBC/CWC |
| TOT.LD. | 42.0 PSF | SEQN- 618329 |
| DUR.FAC. | 1.15 | FROM KTC |
| SPACING | 24.0" | JREF- 1UQN561_Z18 |



ITW Building Components Group, Inc.
 Sacramento, CA 95828

(124062-PARR LUMBER ROCKWOOD WESTWOOD RESIDENCE -- 1337 SE LEXINGTON PORTLAND, O - T02)

Top chord 2x4 DF-L #1&Bet.(g) :T2 1.5"x5.625" DF-L SS(g):
 Bot chord 2x4 DF-L #1&Bet.(g)
 Webs 2x4 DF-L Standard(g)

110 mph wind, 21.23 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=6.0 psf.

Connectors in green lumber (g) designed using NDS/TPI reduction factors.

Wind loads based on both MWFRS and C&C, Reactions based on MWFRS.

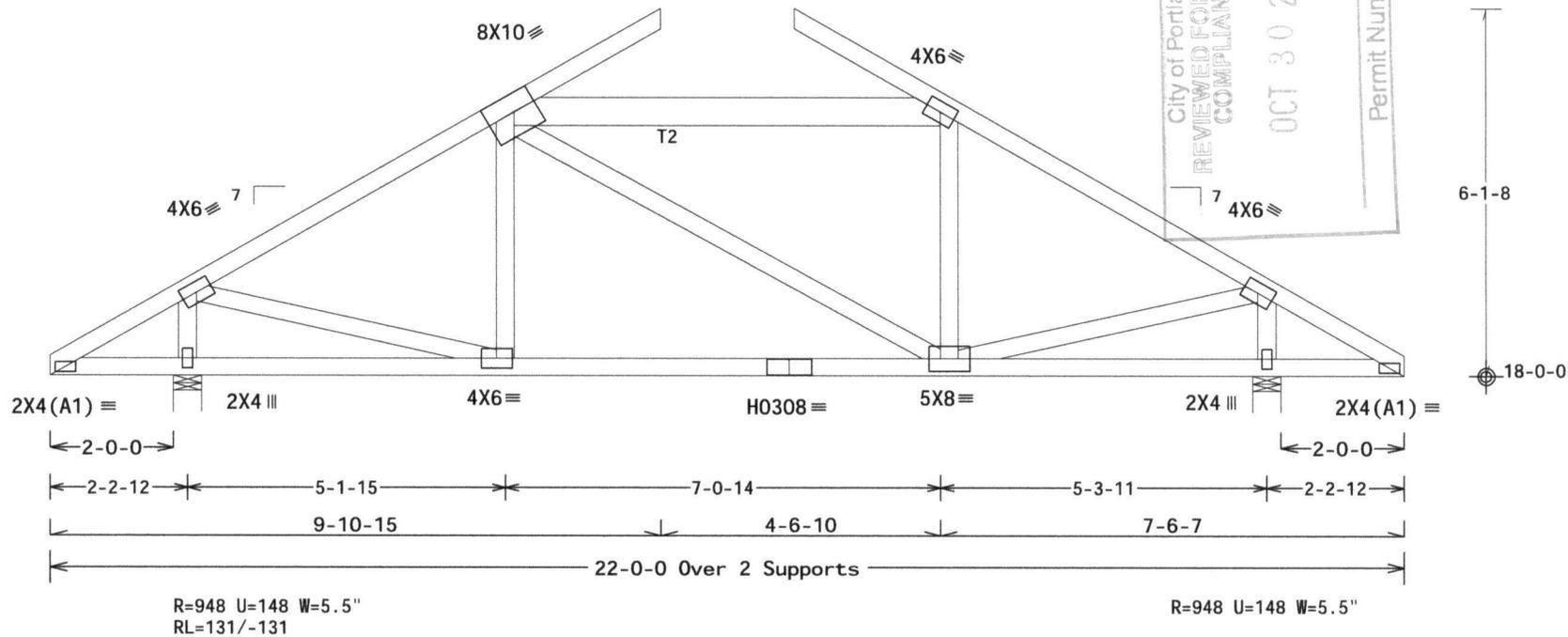
Bottom chord checked for 10.00 psf non-concurrent bottom chord live load applied per IRC-09 section 301.5.

Left and right cantilevers are exposed to wind

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 2.00.

Calculated vertical deflection is 0.05" due to live load and 0.15" due to total load at X = 10-11-2.

In lieu of structural panels use purlins to brace all flat TC @ 24" OC.



PLT TYP. 20 Gauge HS, Wave

Design Crit: IRC2009/TPI-2007(STD)
 FT/RT=8%(0%)/4(1)

10.03.04.0601.17 QTY:1 OR/-/1/-/-/R/-

Scale = .375"/Ft.

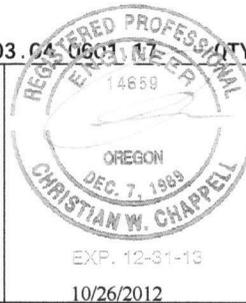
Trus-Way, Inc 360-750-1470
 3901 NE 68th St., Vancouver WA



ITW Building Components Group, Inc.
 Sacramento, CA 95828

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BUILDING COMPONENTS GROUP, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/18GA (W.H/SS/K) ASTM A653 GRADE 40/60 (W. K/H/SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



| | |
|-----------|----------|
| TC LL | 25.0 PSF |
| TC DL | 7.0 PSF |
| BC DL | 10.0 PSF |
| BC LL | 0.0 PSF |
| TOT. LD. | 42.0 PSF |
| DUR. FAC. | 1.15 |
| SPACING | 24.0" |

| | |
|--------|-------------------|
| REF | R561-- 89575 |
| DATE | 10/26/12 |
| DRW | CAUSR561 12300002 |
| CA-ENG | PBC/CWC |
| SEQN- | 618334 |
| FROM | KTC |
| JREF- | 1UQN561_Z18 |

(124062-PARR LUMBER ROCKWOOD WESTWOOD RESIDENCE -- 1337 SE LEXINGTON PORTLAND, O - T03)

Top chord 2x4 DF-L #1&Bet.(g)
 Bot chord 2x4 DF-L #1&Bet.(g)
 Webs 2x4 DF-L Standard(g)

Connectors in green lumber (g) designed using NDS/TPI reduction factors.

Bottom chord checked for 10.00 psf non-concurrent bottom chord live load applied per IRC-09 section 301.5.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 2.00.

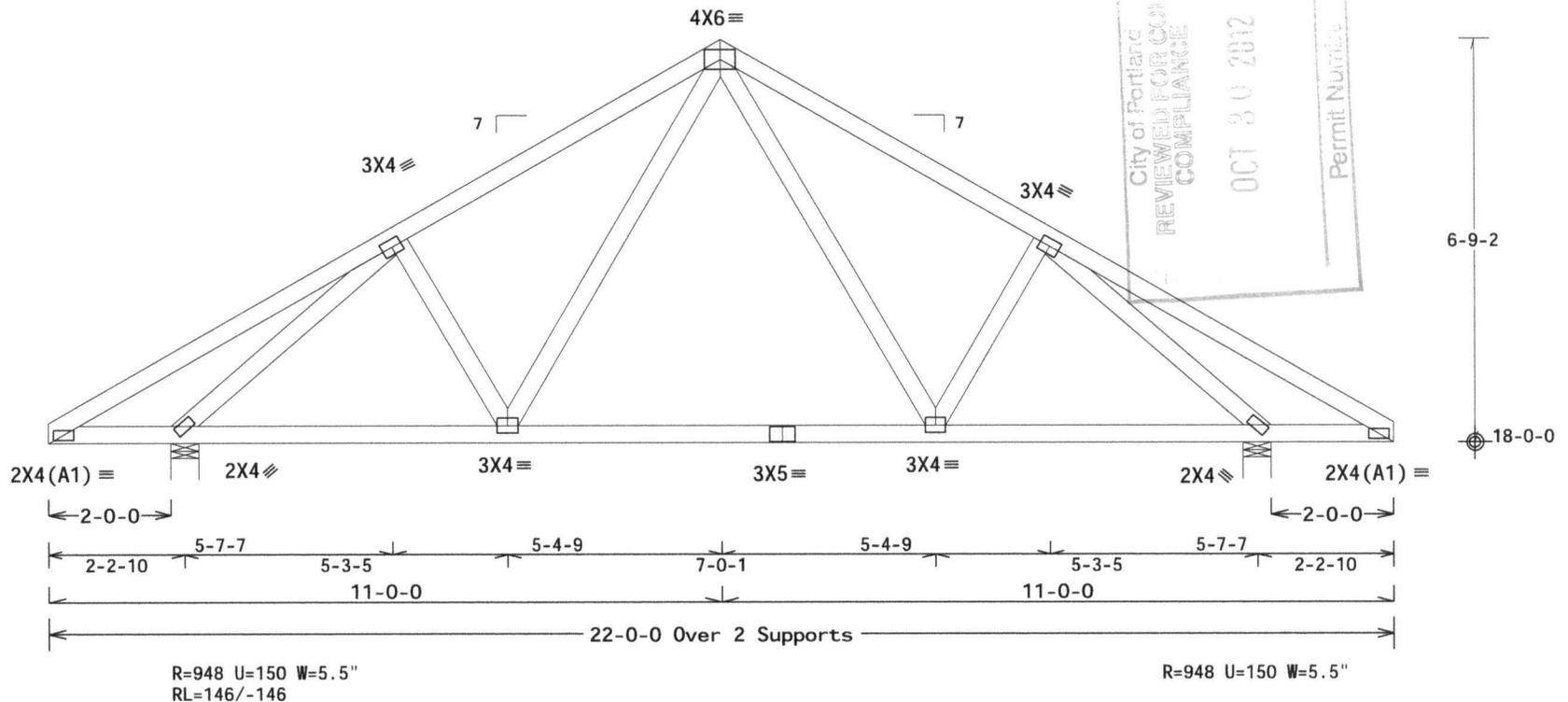
110 mph wind, 21.55 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP 'B, wind TC DL=4.2 psf, wind BC DL=6.0 psf.

Wind loads based on both MWFRS and C&C, Reactions based on MWFRS.

Left and right cantilevers are exposed to wind

Calculated vertical deflection is 0.04" due to live load and 0.12" due to total load at X = 11-1-7.

Truss designed for unbalanced snow load based on Pg=25.00 psf, Ct=1.10, Ce=1.00, CAT II & Pf=19.25 psf.



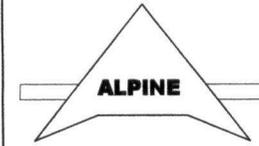
Design Crit: IRC2009/TPI-2007(STD)
 FT/RT=8%(0%)/4(1)

10.03.04.0601.17 QTY:7 OR/-/1/-/-/R/-

Scale = .375"/Ft.

PLT TYP. Wave

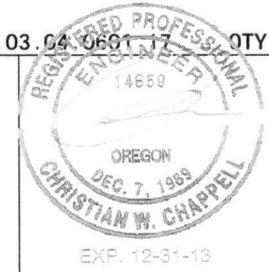
Trus-Way, Inc 360-750-1470
 3901 NE 68th St., Vancouver WA



ITW Building Components Group, Inc.
 Sacramento, CA 95828

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BUILDING COMPONENTS GROUP, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



| | |
|-----------|----------|
| TC LL | 25.0 PSF |
| TC DL | 7.0 PSF |
| BC DL | 10.0 PSF |
| BC LL | 0.0 PSF |
| TOT. LD. | 42.0 PSF |
| DUR. FAC. | 1.15 |
| SPACING | 24.0" |

| | |
|--------|-------------------|
| REF | R561-- 89576 |
| DATE | 10/26/12 |
| DRW | CAUSR561 12300003 |
| CA-ENG | PBC/CWC |
| SEQN- | 618255 |
| FROM | KTC |
| JREF- | 1UQN561_Z18 |

10/26/2012

(124062-PARR LUMBER ROCKWOOD WESTWOOD RESIDENCE -- 1337 SE LEXINGTON PORTLAND, O - T04)

Top chord 2x4 DF-L #1&Bet.(g)
 Bot chord 2x4 DF-L #1&Bet.(g)
 Webs 2x4 DF-L Standard(g)

110 mph wind, 21.55 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=6.0 psf.

Connectors in green lumber (g) designed using NDS/TPI reduction factors.

Wind loads based on both MWFRS and C&C, Reactions based on MWFRS.

(a) 1x4 #3 HEM-FIR or better continuous lateral bracing to be equally spaced. Attach with (2) 8d Box or Gun nails (0.113"x2.5",min.). Bracing material to be supplied and attached at both ends to a suitable support by erection contractor.

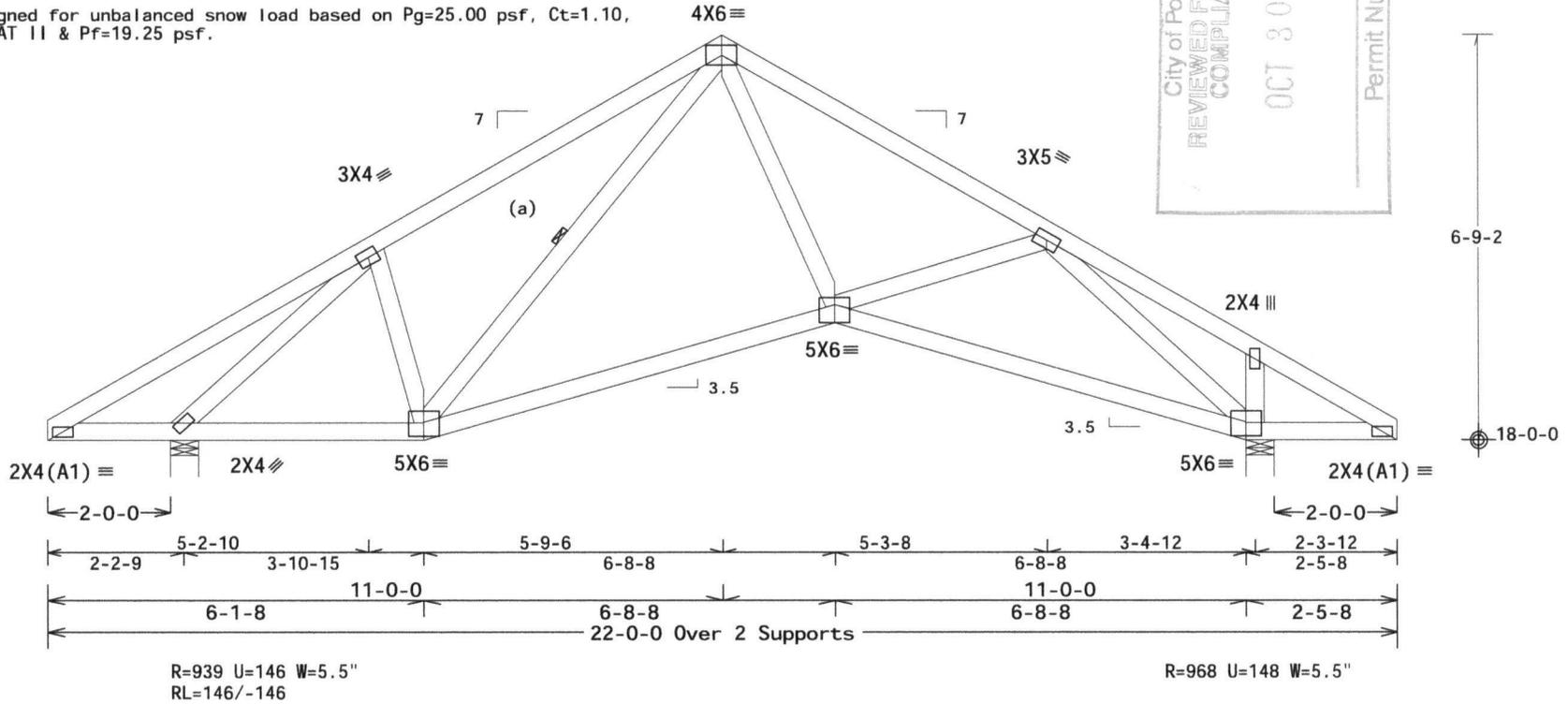
Left and right cantilevers are exposed to wind

Bottom chord checked for 10.00 psf non-concurrent bottom chord live load applied per IRC-09 section 301.5.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 2.00.

Calculated vertical deflection is 0.04" due to live load and 0.12" due to total load at X = 9-2-14.

Truss designed for unbalanced snow load based on Pg=25.00 psf, Ct=1.10, Ce=1.00, CAT II & Pf=19.25 psf.



Design Crit: IRC2009/TPI-2007(STD)
 FT/RT=8%(0%)/4(1)

10.03.04.0601.12 QTY:5 OR/-/1/-/-/R/- Scale = .375"/Ft.

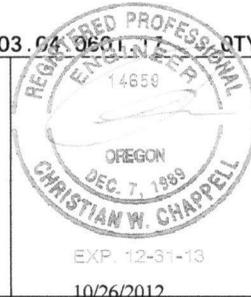
PLT TYP. Wave

Trus-Way, Inc 360-750-1470
 3901 NE 68th St., Vancouver WA

ITW Building Components Group, Inc.
 Sacramento, CA 95828

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 219 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BUILDING COMPONENTS GROUP, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AFAPA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/80 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 180A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.9. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



| | | |
|----------|----------|-----------------------|
| TC LL | 25.0 PSF | REF R561-- 89577 |
| TC DL | 7.0 PSF | DATE 10/26/12 |
| BC DL | 10.0 PSF | DRW CAUSR561 12300004 |
| BC LL | 0.0 PSF | CA-ENG PBC/CWC |
| TOT.LD. | 42.0 PSF | SEQN- 618272 |
| DUR.FAC. | 1.15 | FROM KTC |
| SPACING | 24.0" | JREF- 1UQN561_Z18 |

(124062-PARR LUMBER ROCKWOOD WESTWOOD RESIDENCE -- 1337 SE LEXINGTON PORTLAND, O - T05)

Top chord 2x4 DF-L #1&Bet.(g)
 Bot chord 2x4 DF-L #1&Bet.(g)
 Webs 2x4 DF-L Standard(g)

Connectors in green lumber (g) designed using NDS/TPI reduction factors.

110 mph wind, 21.55 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=6.0 psf.

Bottom chord checked for 10.00 psf non-concurrent bottom chord live load applied per IRC-09 section 301.5.

Deflection meets L/180 live and L/120 total load. Creep increase factor for dead load is 2.00.

Left and right cantilevers are exposed to wind

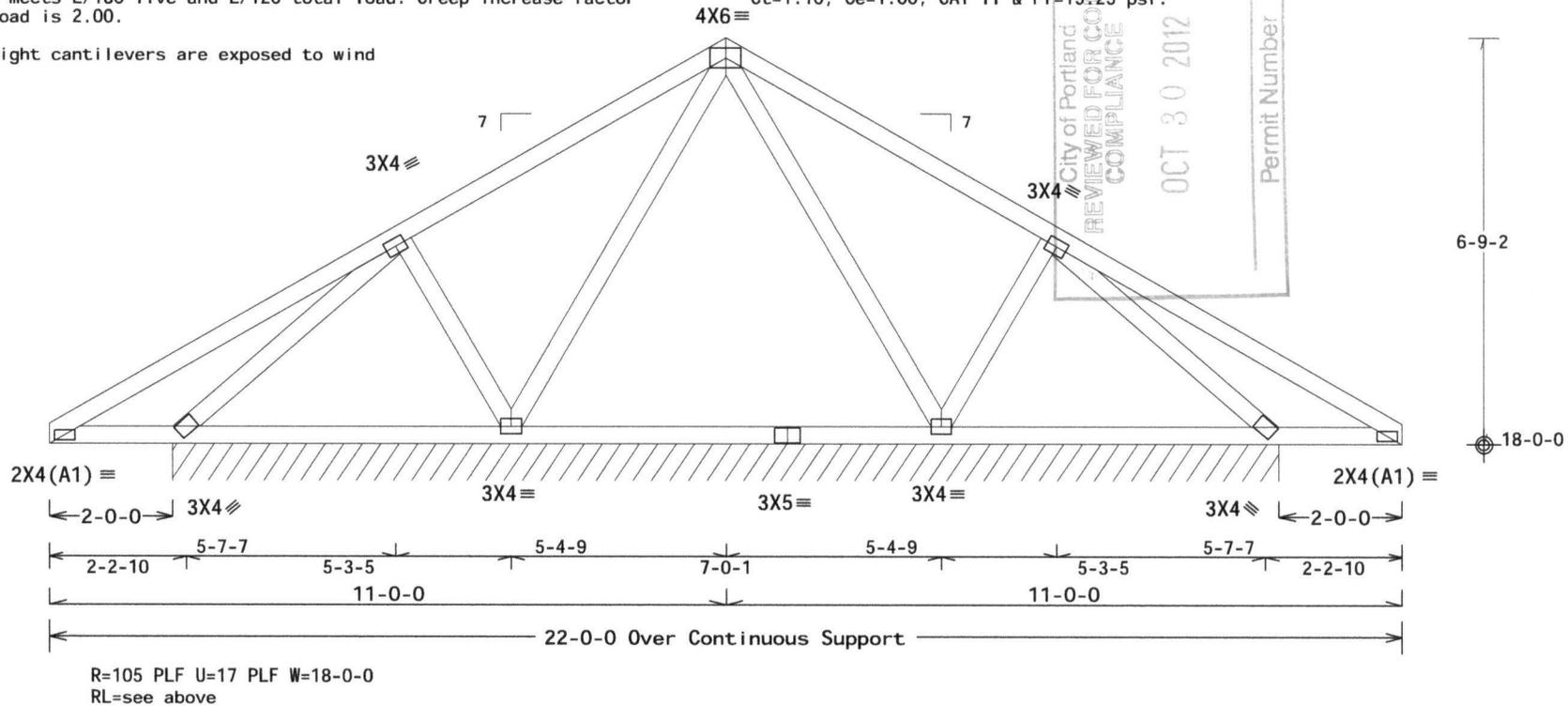
Truss transfers a maximum horizontal load of 2300 # (104.58 plf) along top chord, from either, direction to supports where indicated. Diaphragm and connections are to be designed by Engineer of Record.

| Drag Loads: | Force(#) | (PLF) | Mbr | Start | End |
|-------------|----------|--------|-----|-------|-------|
| Case 1: | 2300 | 104.58 | TC | 0.00 | 22.00 |
| | 2300 | 127.78 | BC | 2.00 | 20.00 |

Wind loads based on both MWFRS and C&C, Reactions based on MWFRS.

Calculated vertical deflection is 0.01" due to live load and 0.04" due to total load at X = 17-1-11.

Truss designed for unbalanced snow load based on Pg=25.00 psf, Ct=1.10, Ce=1.00, CAT II & Pr=19.25 psf.



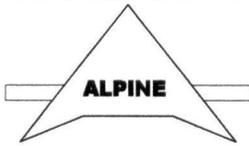
Design Crit: IRC2009/TPI-2007(STD)
 FT/RT=8%(0%)/4(1)

10.03.04.0601.17 CITY:1 OR/-/1/-/1/-/1/-/1

Scale = .375"/Ft.

PLT TYP. Wave

Trus-Way, Inc 360-750-1470
 3901 NE 68th St., Vancouver WA



ITW Building Components Group, Inc.
 Sacramento, CA 95828

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BUILDING COMPONENTS GROUP, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN, ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



| | | |
|----------|----------|-----------------------|
| TC LL | 25.0 PSF | REF R561-- 89578 |
| TC DL | 7.0 PSF | DATE 10/26/12 |
| BC DL | 10.0 PSF | DRW CAUSR561 12300005 |
| BC LL | 0.0 PSF | CA-ENG PBC/CWC |
| TOT.LD. | 42.0 PSF | SEQN- 618282 |
| DUR.FAC. | 1.15 | FROM KTC |
| SPACING | 24.0" | JREF- 1UQN561_Z18 |

(124062-PARR LUMBER ROCKWOOD WESTWOOD RESIDENCE -- 1337 SE LEXINGTON PORTLAND, O - T06)

Top chord 2x4 DF-L #1&Bet.(g)
 Bot chord 2x4 DF-L #1&Bet.(g)
 Webs 2x4 DF-L Standard(g)

Connectors in green lumber (g) designed using NDS/TPI reduction factors.

Left and right cantilevers are exposed to wind

Bottom chord checked for 10.00 psf non-concurrent bottom chord live load applied per IRC-09 section 301.5.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 2.00.

Truss spaced at 24.0" OC designed to support 1-0-0 top chord outlookers. Cladding load shall not exceed 3.00 PSF. Top chord must not be notched or cut.

(**)2 plate(s) require special positioning. Refer to scaled plate plot details for special positioning requirements.

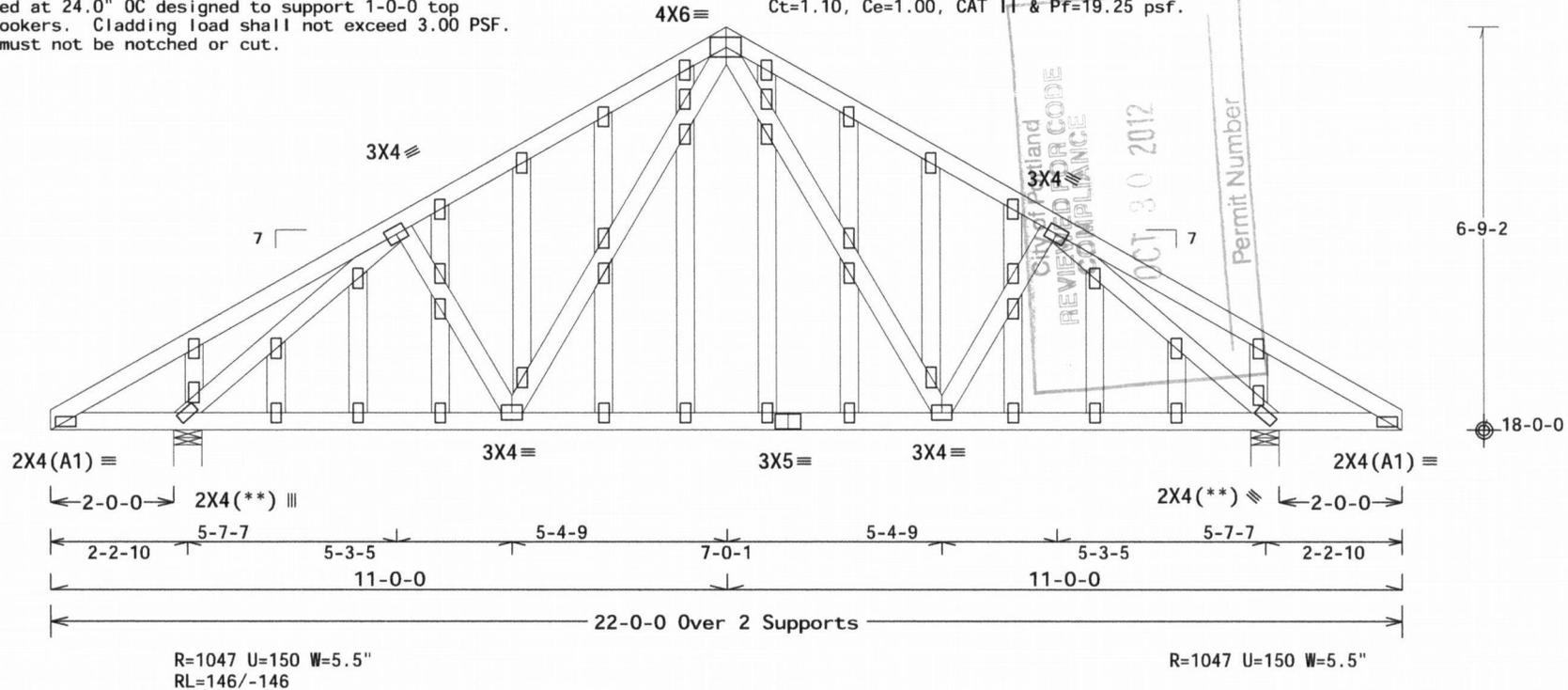
110 mph wind, 21.55 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=6.0 psf.

Wind loads based on both MWFRS and C&C, Reactions based on MWFRS.

See DWGS A11030050109 & GBLLETIN0109 for more requirements.

Calculated vertical deflection is 0.02" due to live load and 0.06" due to total load at X = 9-0-0.

Truss designed for unbalanced snow load based on Pg=25.00 psf, Ct=1.10, Ce=1.00, CAT II & Pf=19.25 psf.



Note: All Plates Are 2X4 Except As Shown.

Design Crit: IRC2009/TPI-2007(STD)
 FT/RT=8%(0%)/4(1)

PLT TYP. Wave

Trus-Way, Inc 360-750-1470
 3901 NE 68th St., Vancouver WA

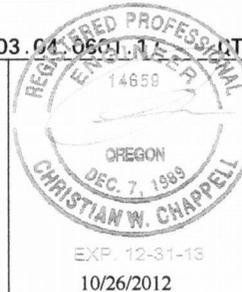


ITW Building Components Group, Inc.
 Sacramento, CA 95828

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BUILDING COMPONENTS GROUP, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W.H/SS/K) ASTM A653 GRADE 40/60 (W. K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

10.03.04.0501.115



OR/-/1/-/-/R/-

Scale = .375"/Ft.

| | | |
|----------|----------|----------------------|
| TC LL | 25.0 PSF | REF R561-- 89579 |
| TC DL | 7.0 PSF | DATE 10/26/12 |
| BC DL | 10.0 PSF | DRW CAUSR561 1230006 |
| BC LL | 0.0 PSF | CA-ENG PBC/CWC |
| TOT.LD. | 42.0 PSF | SEQN- 618286 |
| DUR.FAC. | 1.15 | FROM KTC |
| SPACING | 24.0" | JREF- 1UQN561_Z18 |

(124062-PARR LUMBER ROCKWOOD WESTWOOD RESIDENCE -- 1337 SE LEXINGTON PORTLAND, O - T07)

Top chord 2x4 DF-L #1&Bet.(g)
 Bot chord 2x4 DF-L #1&Bet.(g)
 Webs 2x4 DF-L Standard(g)

110 mph wind, 21.55 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=6.0 psf.

Connectors in green lumber (g) designed using NDS/TPI reduction factors.

Wind loads and reactions based on MWFRS.

Trusses to be spaced at 0.0" OC maximum.

Left and right cantilevers are exposed to wind

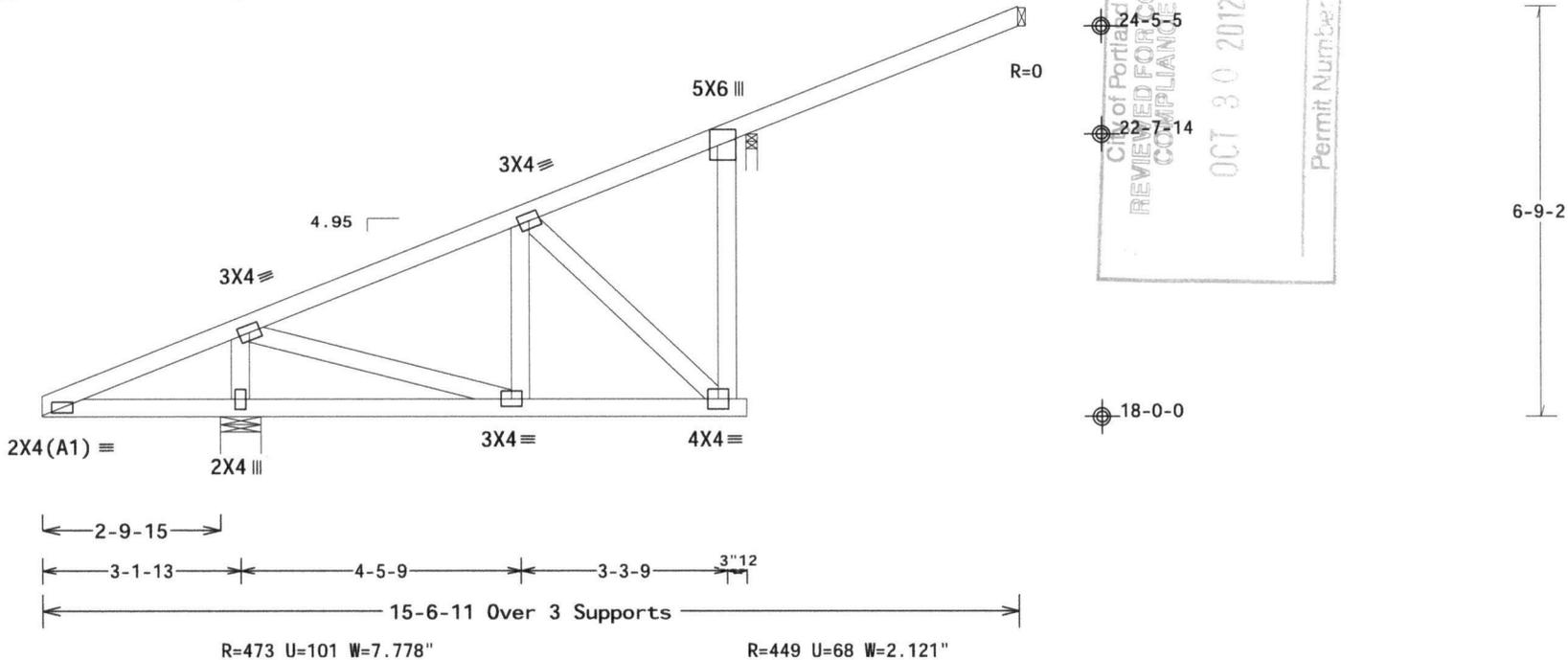
Sub-fascia beam assumptions: 4-0-0 sub-fascia beam on the 2-0-0 cantilever side. 4-0-0 sub-fascia beam on the 2-0-0 cantilever side.

The following members need concentrated loads at the heel: 4-0-0 span/setback member on the 2-0-0 cant side requires 61 lbs and the 4-0-0 span/setback member on the 2-0-0 cant side requires 61 lbs.

Hipjack supports 8-0-0 setback jacks with 2-0-0 cantilever one face; 2-0-0 cantilever opposite face.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 2.00.

Shim all supports to solid bearing.

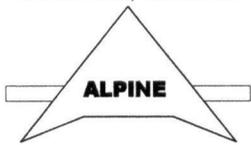


Design Crit: IRC2009/TPI-2007 (STD)
 FT/RT=8%(0%)/4(1)

10.03.04.050.17 OCT:2 OR/-/1/-/-/R/-

Scale = .375"/Ft.

PLT TYP. Wave
 Trus-Way, Inc 360-750-1470
 3901 NE 68th St., Vancouver WA



ITW Building Components Group, Inc.
 Sacramento, CA 95828

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BC51 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BUILDING COMPONENTS GROUP, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W, H/SS/K) ASTM A653 GRADE 40/60 (W, K/H, SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



EXP. 12-31-13
 10/26/2012

| | | |
|----------|-----------|-----------------------|
| TC LL | 25.0 PSF | REF R561-- 89580 |
| TC DL | 7.0 PSF | DATE 10/26/12 |
| BC DL | 10.0 PSF | DRW CAUSR561 12300012 |
| BC LL | 0.0 PSF | CA-ENG PBC/CWC |
| TOT.LD. | 42.0 PSF | SEQN- 618318 |
| DUR.FAC. | 1.15 | FROM KTC |
| LOADING | SEE ABOVE | JREF- 1UQN561_Z18 |

(124062-PARR LUMBER ROCKWOOD WESTWOOD RESIDENCE -- 1337 SE LEXINGTON PORTLAND, O - J02)

Top chord 2x4 DF-L #1&Bet.(g)
 Bot chord 2x4 DF-L #1&Bet.(g)

Connectors in green lumber (g) designed using NDS/TPI reduction factors.

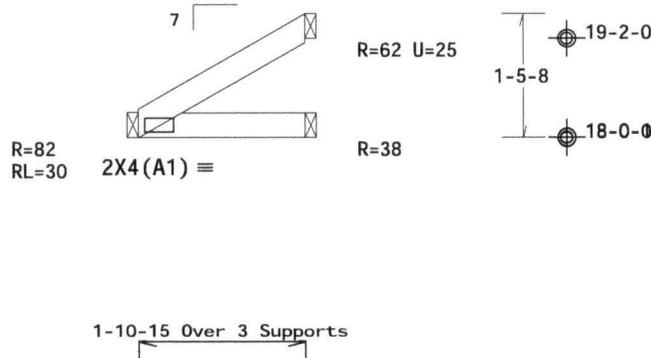
Bottom chord checked for 10.00 psf non-concurrent bottom chord live load applied per IRC-09 section 301.5.

Unbalanced snow loads have not been considered.

110 mph wind, 18.90 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=6.0 psf.

Wind loads based on both MWFRS and C&C, Reactions based on MWFRS.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 2.00.



Design Crit: IRC2009/TPI-2007(STD)
 FT/RT=8%(0%)/4(1)

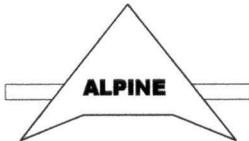
10.03.04.0601.17 QTY:4

OR/-/1/-/-/R/-

Scale =.5"/Ft.

PLT TYP. Wave

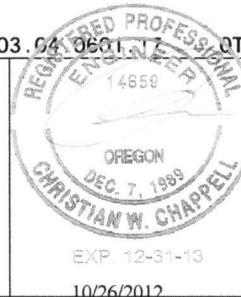
Trus-Way, Inc 360-750-1470
 3901 NE 68th St., Vancouver WA



ITW Building Components Group, Inc.
 Sacramento, CA 95828

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCS1 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BUILDING COMPONENTS GROUP, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



| | | |
|----------|----------|-----------------------|
| TC LL | 25.0 PSF | REF R561-- 89581 |
| TC DL | 7.0 PSF | DATE 10/26/12 |
| BC DL | 10.0 PSF | DRW CAUSR561 12300011 |
| BC LL | 0.0 PSF | CA-ENG PBC/CWC * |
| TOT.LD. | 42.0 PSF | SEQN- 618291 |
| DUR.FAC. | 1.15 | FROM KTC |
| SPACING | 24.0" | JREF- 1UQN561_Z18 |

(124062-PARR LUMBER ROCKWOOD WESTWOOD RESIDENCE -- 1337 SE LEXINGTON PORTLAND, O - J04)

Top chord 2x4 DF-L #1&Bet.(g)
 Bot chord 2x4 DF-L #1&Bet.(g)
 Webs 2x4 DF-L Standard(g)

110 mph wind, 19.48 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=6.0 psf.

Connectors in green lumber (g) designed using NDS/TPI reduction factors.

Left and right cantilevers are exposed to wind

Special loads

Bottom chord checked for 10.00 psf non-concurrent bottom chord live load applied per IRC-09 section 301.5.

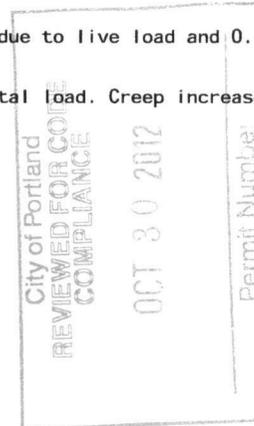
----- (Lumber Dur.Fac.=1.15 / Plate Dur.Fac.=1.15)
 TC- From 66 plf at 0.00 to 66 plf at 3.91
 BC- From 20 plf at 0.00 to 20 plf at 3.91
 TC- 101.00 lb Conc. Load at 0.00

Calculated vertical deflection is 0.08" due to live load and 0.20" due to total load at X = 0-0-0.

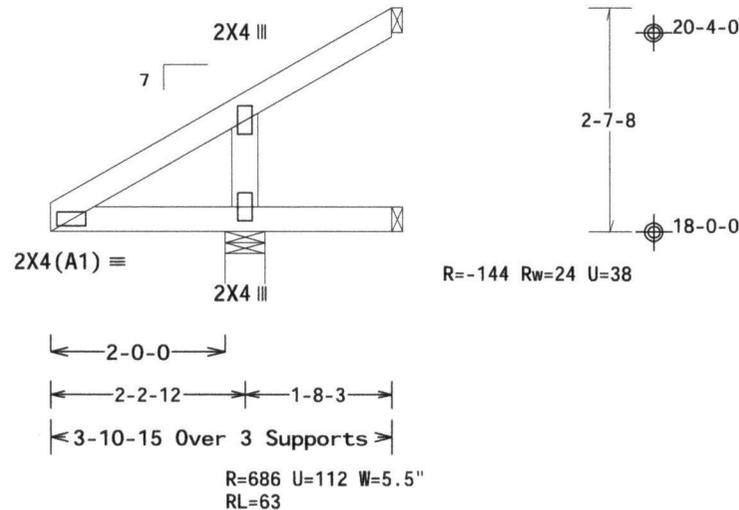
Wind loads based on both MWFRS and C&C, Reactions based on MWFRS.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 2.00.

Unbalanced snow loads have not been considered.



R=-104 Rw=17 U=31



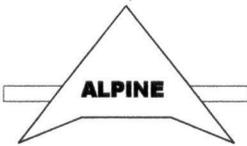
R=-144 Rw=24 U=38

Design Crit: IRC2009/TPI-2007(STD)
 FT/RT=8%(0%)/4(1)

10.03.04.0601.11 OCT/14 OR/-/1/-/-/R/- Scale =.5"/Ft.

PLT TYP. Wave

Trus-Way, Inc 360-750-1470
 3901 NE 68th St., Vancouver WA



ITW Building Components Group, Inc.
 Sacramento, CA 95828

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCS1 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BUILDING COMPONENTS GROUP, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 180A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



| | | |
|----------|----------|-----------------------|
| TC LL | 25.0 PSF | REF R561-- 89582 |
| TC DL | 7.0 PSF | DATE 10/26/12 |
| BC DL | 10.0 PSF | DRW CAUSR561 12300010 |
| BC LL | 0.0 PSF | CA-ENG PBC/CWC |
| TOT.LD. | 42.0 PSF | SEQN- 618301 |
| DUR.FAC. | 1.15 | FROM KTC |
| SPACING | 24.0" | JREF- 1UQN561_Z18 |

Top chord 2x4 DF-L #1&Bet.(g)
 Bot chord 2x4 DF-L #1&Bet.(g)
 Webs 2x4 DF-L Standard(g)

110 mph wind, 20.07 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=6.0 psf.

Connectors in green lumber (g) designed using NDS/TPI reduction factors.

Wind loads based on both MWFRS and C&C, Reactions based on MWFRS.

Bottom chord checked for 10.00 psf non-concurrent bottom chord live load applied per IRC-09 section 301.5.

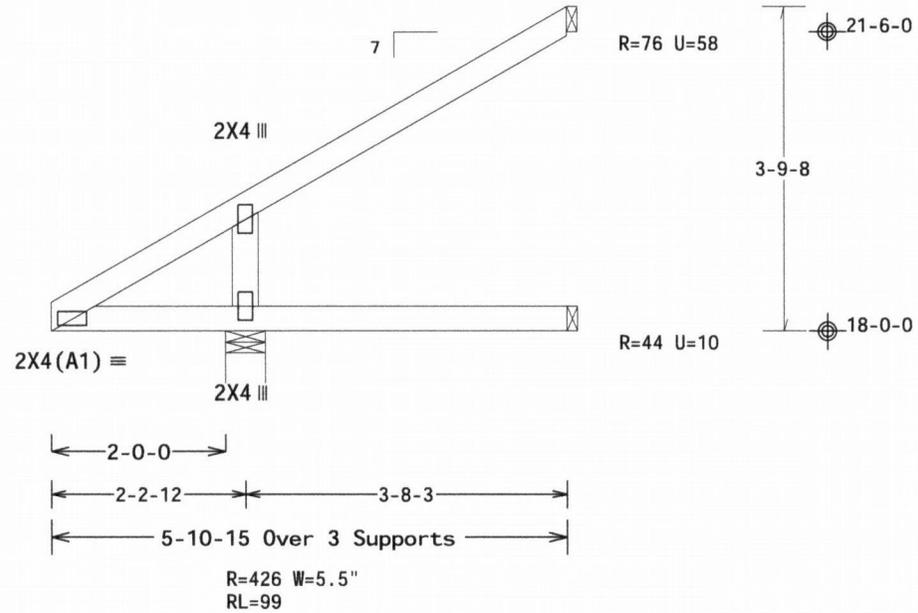
Left and right cantilevers are exposed to wind

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 2.00.

Calculated vertical deflection is 0.04" due to live load and 0.09" due to total load at X = 0-0-0.

Unbalanced snow loads have not been considered.

City of Portland
 REVIEWED FOR CODE
 COMPLIANCE
 OCT 30 2012
 Permit Number:



Design Crit: IRC2009/TPI-2007 (STD)
 FT/RT=8%(0%)/4(1)

PLT TYP. Wave

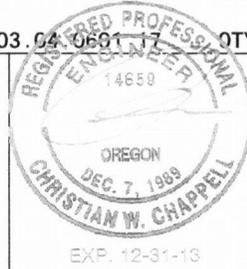
Trus-Way, Inc 360-750-1470
 3901 NE 68th St., Vancouver WA



ITW Building Components Group, Inc.
 Sacramento, CA 95828

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO RCS1 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6900 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BUILDING COMPONENTS GROUP, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 190A-2. ANY INSPECTION OF PLATES FOLLOWED BY (1) SHALL BE PER ANNEX A3 OF TPI-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.



10.03.04.0601.17 QTY:4

| | |
|----------|----------|
| TC LL | 25.0 PSF |
| TC DL | 7.0 PSF |
| BC DL | 10.0 PSF |
| BC LL | 0.0 PSF |
| TOT.LD. | 42.0 PSF |
| DUR.FAC. | 1.15 |
| SPACING | 24.0" |

Scale =.5"/Ft.

| | |
|--------|-------------------|
| REF | R561-- 89583 |
| DATE | 10/26/12 |
| DRW | CAUSR561 12300009 |
| CA-ENG | PBC/CWC |
| SEQN- | 618295 |
| FROM | KTC |
| JREF- | 1UQN561_Z18 |

10/26/2012

(124062-PARR LUMBER ROCKWOOD WESTWOOD RESIDENCE -- 1337 SE LEXINGTON PORTLAND, O - JOB)

Top chord 2x4 DF-L #1&Bet.(g)
 Bot chord 2x4 DF-L #1&Bet.(g)
 Webs 2x4 DF-L Standard(g)

Connectors in green lumber (g) designed using NDS/TPI reduction factors.

Bottom chord checked for 10.00 psf non-concurrent bottom chord live load applied per IRC-09 section 301.5.

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 2.00.

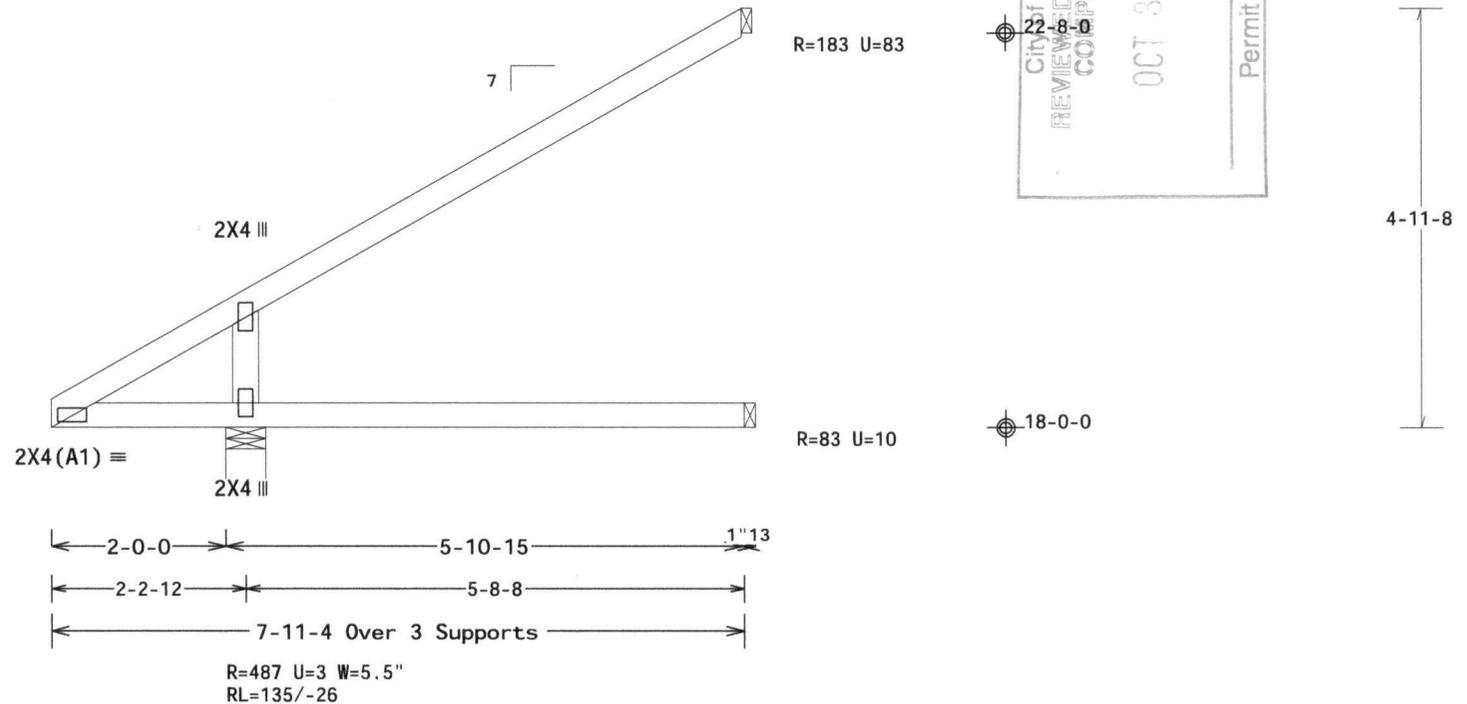
110 mph wind, 20.65 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=6.0 psf.

Wind loads based on both MWFRS and C&C, Reactions based on MWFRS.

Left and right cantilevers are exposed to wind

Calculated vertical deflection is 0.12" due to live load and 0.30" due to total load at X = 0-0-0.

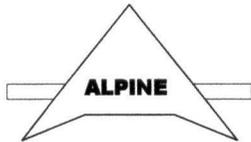
Unbalanced snow loads have not been considered.



Design Crit: IRC2009/TPI-2007(STD)
 FT/RT=8%(0%)/4(1)

PLT TYP. Wave

Trus-Way, Inc 360-750-1470
 3901 NE 68th St., Vancouver WA



ITW Building Components Group, Inc.
 Sacramento, CA 95828

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCS1 (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WIGA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BUILDING COMPONENTS GROUP, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI; OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC, BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/166A (W.H/SS/K) ASTM A653 GRADE 40/60 (W. K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-Z. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

10.03.04-0608.17



EXP. 12-31-13
 10/26/2012

OR/-/1/-/-/R/-

Scale = .5"/Ft.

| | | |
|----------|----------|-----------------------|
| TC LL | 25.0 PSF | REF R561-- 89584 |
| TC DL | 7.0 PSF | DATE 10/26/12 |
| BC DL | 10.0 PSF | DRW CAUSR561 12300008 |
| BC LL | 0.0 PSF | CA-ENG PBC/CWC |
| TOT.LD. | 42.0 PSF | SEQN- 618297 |
| DUR.FAC. | 1.15 | FROM KTC |
| SPACING | 24.0" | JREF- 1UQN561_Z18 |

(124062-PARR LUMBER ROCKWOOD WESTWOOD RESIDENCE -- 1337 SE LEXINGTON PORTLAND, O - J10)

Top chord 2x4 DF-L #1&Bet.(g)
 Bot chord 2x4 DF-L #1&Bet.(g)
 Webs 2x4 DF-L Standard(g)

110 mph wind, 21.23 ft mean hgt, ASCE 7-05, CLOSED bldg, Located anywhere in roof, CAT II, EXP B, wind TC DL=4.2 psf, wind BC DL=6.0 psf.

Connectors in green lumber (g) designed using NDS/TPI reduction factors.

Wind loads based on both MWFRS and C&C, Reactions based on MWFRS.

Bottom chord checked for 10.00 psf non-concurrent bottom chord live load applied per IRC-09 section 301.5.

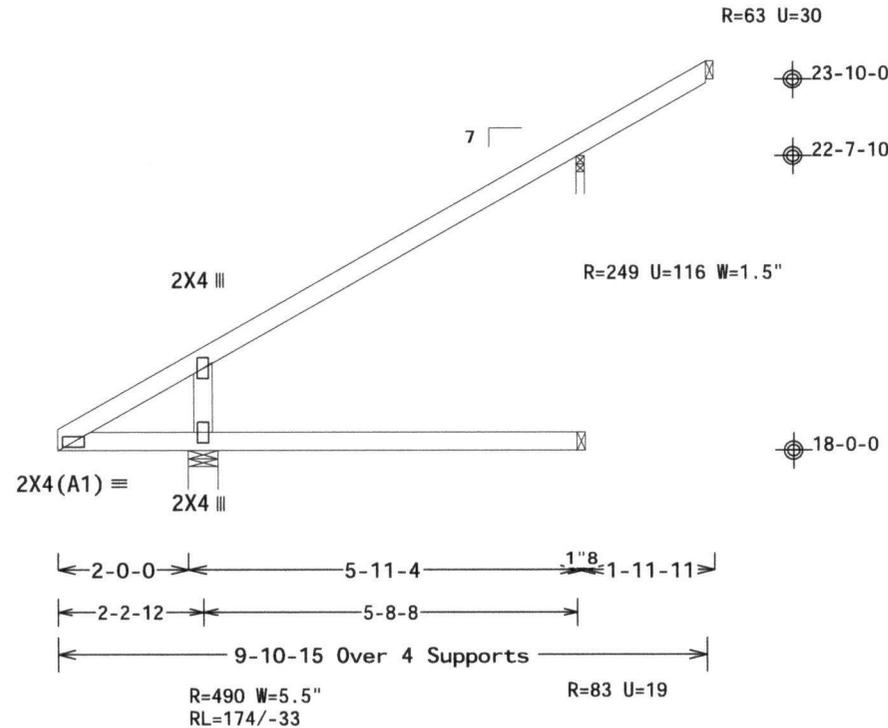
Left and right cantilevers are exposed to wind

Deflection meets L/240 live and L/180 total load. Creep increase factor for dead load is 2.00.

Calculated vertical deflection is 0.13" due to live load and 0.29" due to total load at X = 0-0-0.

Shim all supports to solid bearing.

Unbalanced snow loads have not been considered.



City of Portland
 REVIEWED FOR CODE
 COMPLIANCE
 OCT 30 2012
 Permit Number

6-1-8

Design Crit: IRC2009/TPI-2007(STD)
 FT/RT=8%(0%)/4(1)

PLT TYP. Wave

Trus-Way, Inc 360-750-1470
 3901 NE 68th St., Vancouver WA



ITW Building Components Group, Inc.
 Sacramento, CA 95828

****WARNING**** TRUSSES REQUIRE EXTREME CARE IN FABRICATION, HANDLING, SHIPPING, INSTALLING AND BRACING. REFER TO BCSI (BUILDING COMPONENT SAFETY INFORMATION), PUBLISHED BY TPI (TRUSS PLATE INSTITUTE, 218 NORTH LEE STREET, SUITE 312, ALEXANDRIA, VA, 22314) AND WTCA (WOOD TRUSS COUNCIL OF AMERICA, 6300 ENTERPRISE LANE, MADISON, WI 53719) FOR SAFETY PRACTICES PRIOR TO PERFORMING THESE FUNCTIONS. UNLESS OTHERWISE INDICATED TOP CHORD SHALL HAVE PROPERLY ATTACHED STRUCTURAL PANELS AND BOTTOM CHORD SHALL HAVE A PROPERLY ATTACHED RIGID CEILING.

****IMPORTANT**** FURNISH A COPY OF THIS DESIGN TO THE INSTALLATION CONTRACTOR. ITW BUILDING COMPONENTS GROUP, INC. SHALL NOT BE RESPONSIBLE FOR ANY DEVIATION FROM THIS DESIGN; ANY FAILURE TO BUILD THE TRUSS IN CONFORMANCE WITH TPI OR FABRICATING, HANDLING, SHIPPING, INSTALLING & BRACING OF TRUSSES. DESIGN CONFORMS WITH APPLICABLE PROVISIONS OF NDS (NATIONAL DESIGN SPEC. BY AF&PA) AND TPI. ALPINE CONNECTOR PLATES ARE MADE OF 20/18/16GA (W,H/SS/K) ASTM A653 GRADE 40/60 (W, K/H,SS) GALV. STEEL. APPLY PLATES TO EACH FACE OF TRUSS AND, UNLESS OTHERWISE LOCATED ON THIS DESIGN, POSITION PER DRAWINGS 160A-2. ANY INSPECTION OF PLATES FOLLOWED BY (I) SHALL BE PER ANNEX A3 OF TPI1-2002 SEC.3. A SEAL ON THIS DRAWING INDICATES ACCEPTANCE OF PROFESSIONAL ENGINEERING RESPONSIBILITY SOLELY FOR THE TRUSS COMPONENT DESIGN SHOWN. THE SUITABILITY AND USE OF THIS COMPONENT FOR ANY BUILDING IS THE RESPONSIBILITY OF THE BUILDING DESIGNER PER ANSI/TPI 1 SEC. 2.

10.03.C4-0601.12 CITY:2



EXP. 12-31-13

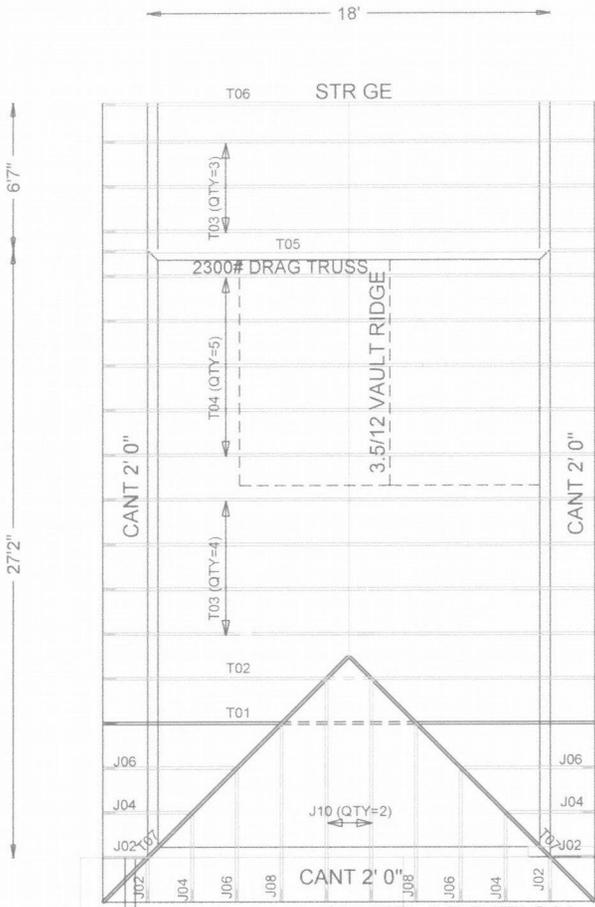
10/26/2012

OR/-/1/-/-/R/-

Scale = .375"/Ft.

| | |
|-----------|----------|
| TC LL | 25.0 PSF |
| TC DL | 7.0 PSF |
| BC DL | 10.0 PSF |
| BC LL | 0.0 PSF |
| TOT. LD. | 42.0 PSF |
| DUR. FAC. | 1.15 |
| SPACING | 24.0" |

| | |
|--------|-------------------|
| REF | R561-- 89585 |
| DATE | 10/26/12 |
| DRW | CAUSR561 12300007 |
| CA-ENG | PBC/CWC |
| SEQN- | 618299 |
| FROM | KTC |
| JREF- | 1UQN561_Z18 |



PARR LUMBER
 HOFF CONSTRUCTION
 WESTWOOD RESIDENCE
 7112 2' 0" CANT
 LOADING 25-7-10

SIMPSON / ITW HANGERS NAILS

| | |
|-----------------------|------------|
| A=HUS26 / HDPT26 | N16 2 1/2" |
| B=HUS28 / HDTP28 | N16 2 1/2" |
| C=LUS24 / STP24 | N16 1 1/2" |
| D=HHUS28-2 / HDTP28-2 | N16 2 1/2" |
| E=HGUS28-2 / HHDP28-2 | N16 2 1/2" |
| F=HGUS28-3 / HHDP28-3 | N16 2 1/2" |
| G=THJA-26 / MHJT | N16 2 1/2" |
| H=MTMH / MMTH | N16 2 1/2" |

ALL HANGERS LUS24 U N O.

LOWER ADDITION
 HAND CUT

EXISTING ROOF



Customer: PARR LUMBER ROCKWOOD
 Owner: HOFF CONSTRUCTION
 Plan: WESTWOOD RESIDENCE
 Salesman: Dean Haworth
 Elevation: CANTED

JOB NO:
 124062

PAGE NO:
 1 OF 1

Addition Drawings For:

The Parra Residence
1337 SE Lexington
Portland, Oregon 97202

Foundation Plan

REVISIONS

| | |
|--|----------|
| | 07.30.12 |
| | 08.10.12 |
| | 08.15.12 |
| | 10.09.12 |
| | 10.15.12 |
| | 10.16.12 |
| | 10.26.12 |

DATE
07.24.12

DATE LAST PRINTED

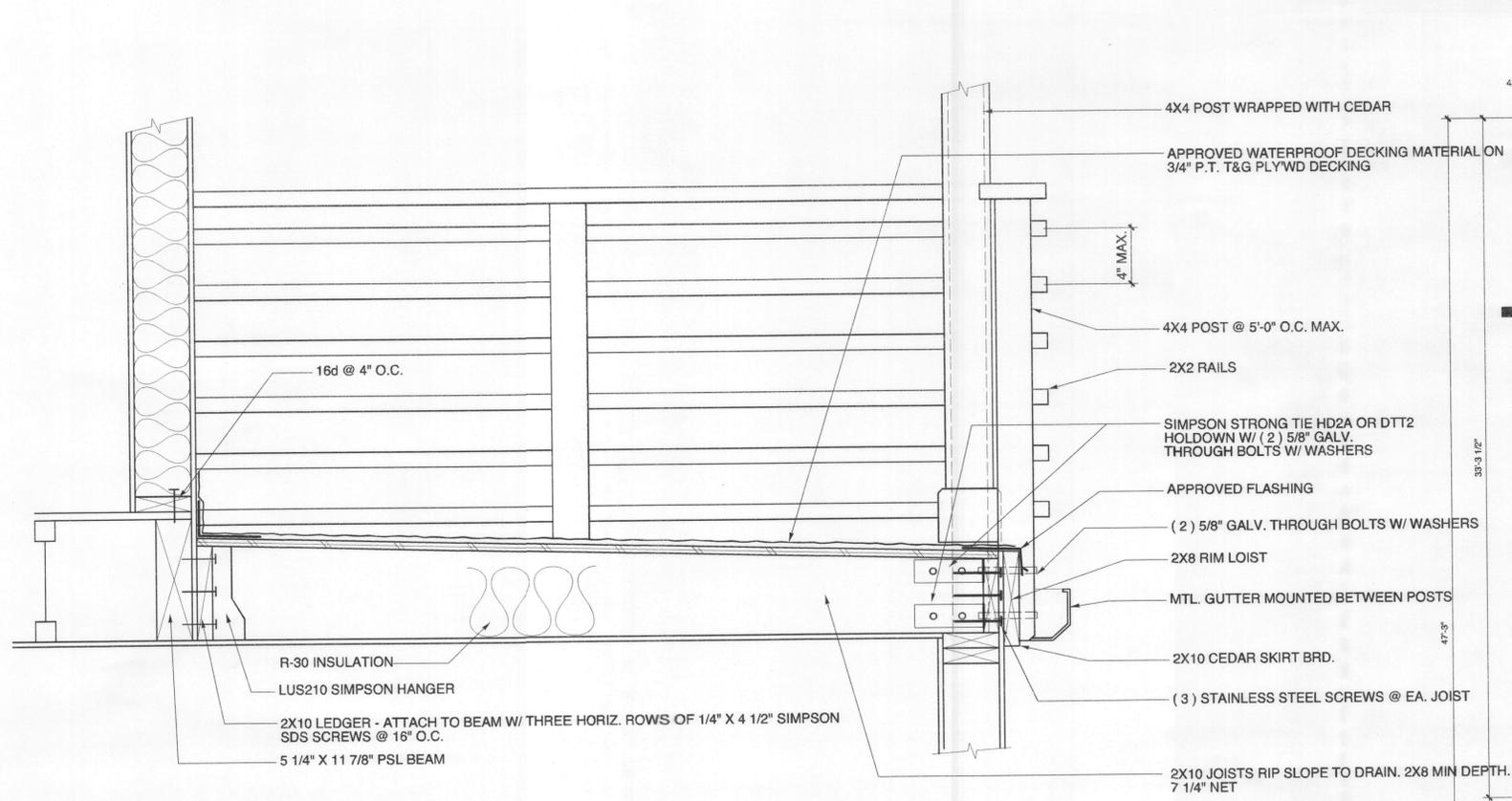
PROJECT NO:
16-12

SHEET

4

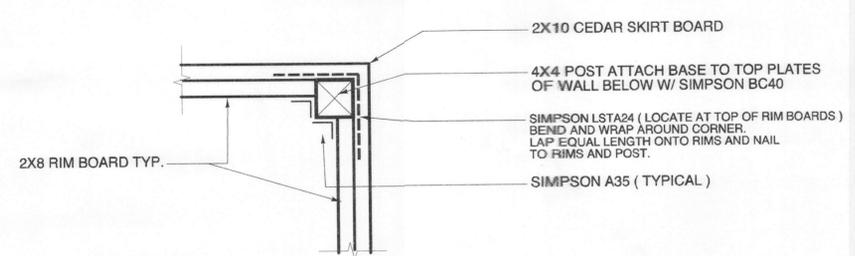
REVISED SHEET 10.26.12

18.193375.RA101-RS



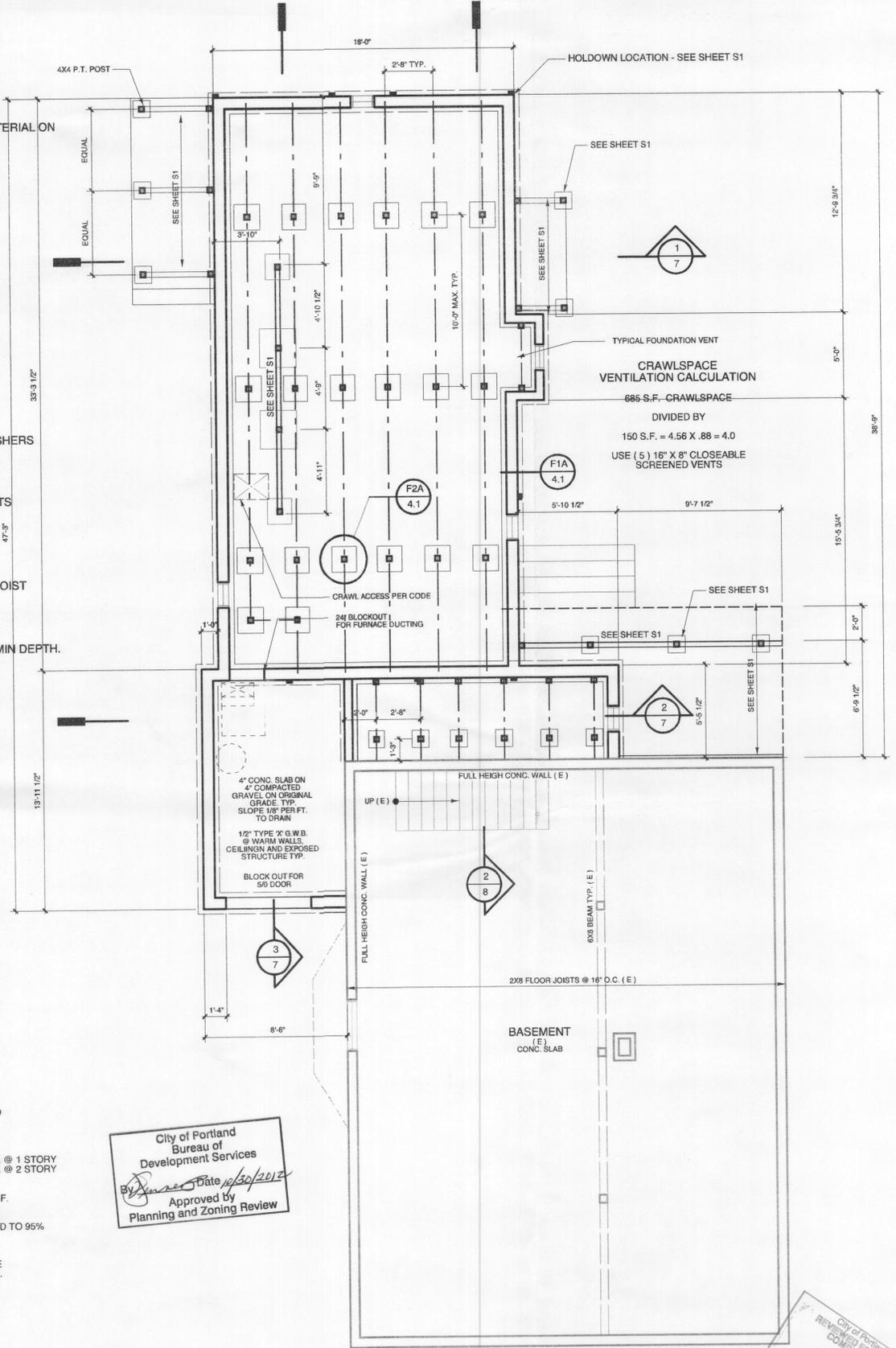
1 DECK SECTION

SCALE: 1 1/2" = 1'-0"



2 PLAN DETAIL AT CORNER

SCALE: 1 1/2" = 1'-0"



- FOOTINGS ARE TO BEAR ON UNDISTURBED SOIL DEVOID OF ANY ORGANIC MATERIAL AND STEPPED AS REQUIRED TO MAINTAIN THE REQ. DEPTH BELOW THE FINAL GRADE.
- FOUNDATION
6" CONC. STEM WALL ON 12" X 6" CONT. CONC. FTG. @ 1 STORY
8" CONC. STEM WALL ON 15" X 8" CONT. CONC. FTG. @ 2 STORY
SEE WALL SECTION ON SHEET S1
- SOIL BEARING PRESSURE ASSUMED TO BE 1500 PSF.
- ANY FILL UNDER GRADE SUPPORTED SLABS TO BE A MINIMUM OF 4" GRANULAR MATERIAL COMPACTED TO 95%
- ALL WOOD IN CONTACT W/ CONC. TO BE PRESSURE TREATED OR PROTECTED WITH 55# ROLL ROOFING.
- P.T. 2X6 SILL PLATES TYP.
- FLOOR STRUCTURE:
POST & BEAM SEE DETAILS SHEET 4.1
- 6 MIL VISQUEEN VAPOR BARRIER @ CRAWLSPACE
LAP 12" @ SEAMS AND RETURN 12" UP FOUNDATION WALL
SLOPE CRAWLSPACE TO DRAIN & PROVIDE CRAWLSPACE DRAIN.
- PROVIDE SCREENED CLOSEABLE 16" X 8" FOUNDATION VENTS
- SEE VENT CLACS. THIS SHEET
- SPACE FOUNDATION VENTS TO ALLOW FOR REQ. ANCHOR BOLT SPACING.
- PROVIDE R-30 UNDERFLOOR INSULATION USE SPRING WIRES TO HOLD INSULATION TIGHT TO UNDERSIDE OF FLOOR SHEATHING SPACE WIRES PER INSTALLERS SPECS.
- SEE GENERAL NOTES

3 FOUNDATION NOTES

SEE SHEET S1

City of Portland
Bureau of
Development Services
By *[Signature]* Date 10/30/2012
Approved by
Planning and Zoning Review

EXISTING BASEMENT PLAN
W/ ADU & MAIN HOUSE ADDITION FOUNDATION PLAN

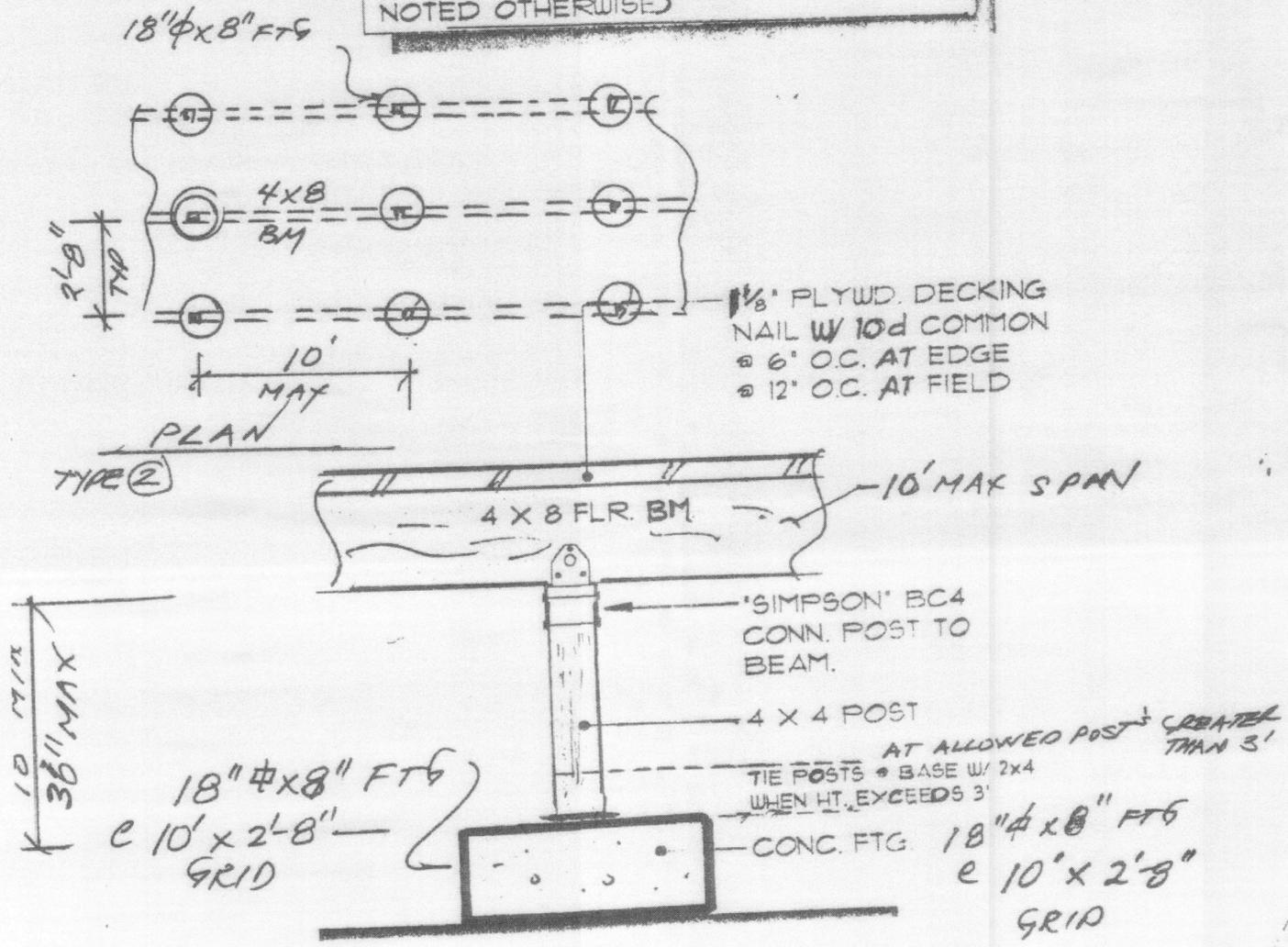
EXISTING SQUARE FEET: 868 S.F. EXCLUDING EXTERIOR WALLS
SCALE: 1/4" = 1'-0"

City of Portland
REVIEWED FOR CODE
COMPLIANCE
OCT 30 2012
Permit Number

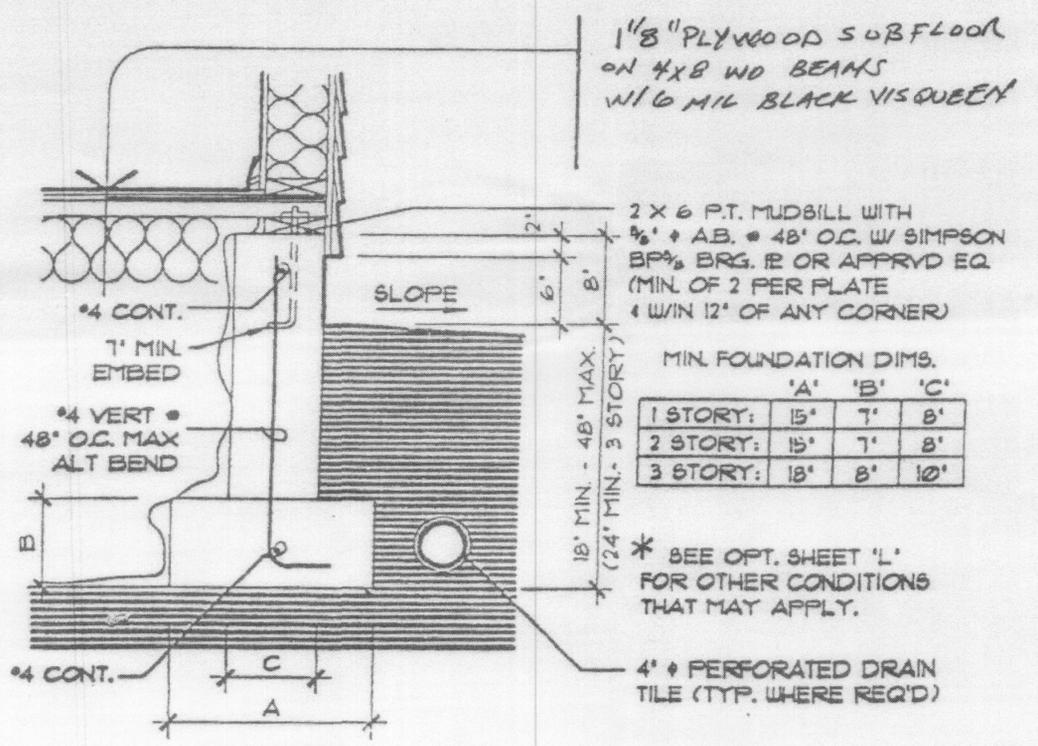
TYPE 2 POST & BEAM

TYPE 2 POST & BEAM

FLOOR SYSTEM:
 1/8" PLYWOOD SUBFLOOR ON 4 X 8 WD GIRDERS ON 4 X 4 WD POST (4 X 6 @ JOINTS) ON ASPHALT SHINGLE ON 18" x 8" CONC PAD FTG (UNLESS NOTED OTHERWISE)



F2A POST & BM. CONN.
 3/4" = 1'-0" TYPE 2 POST & BEAM
 PBCONN2



F1A TYP. WALL SECTION
 SCALE: 3/4" = 1'-0"
 (TYPE 2 POST & BEAM)
 REV. 01/02 2P694500

Addition Drawings For:
 The Parra Residence
 1337 SE Lexington
 Portland, Oregon 97202

Foundation Details

REVISIONS

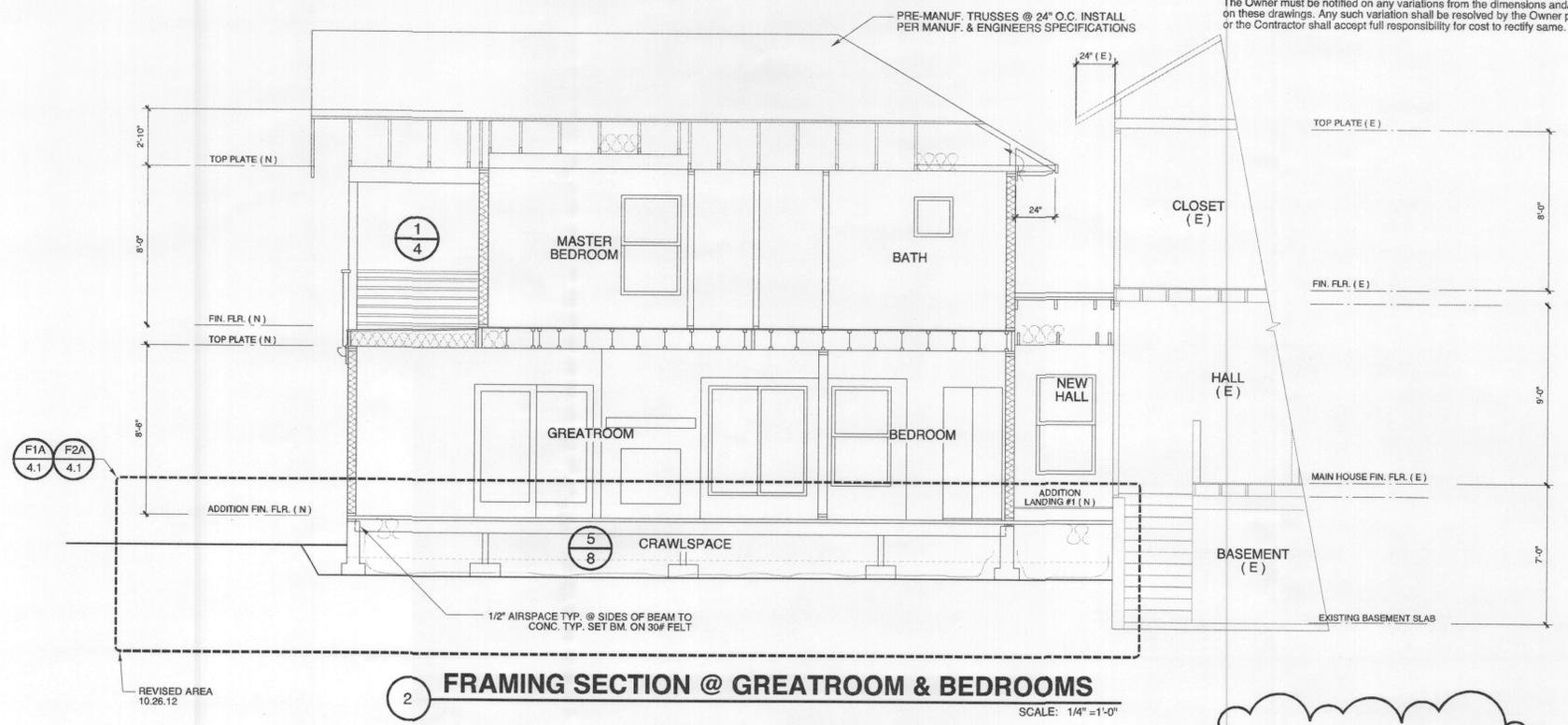
DATE 10.26.12
 DATE LAST PRINTED

PROJECT NO: 16-12

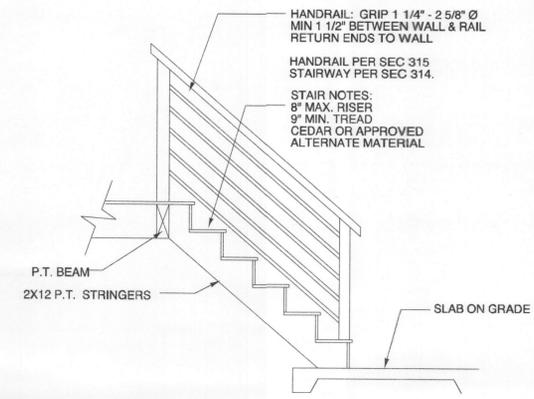
SHEET 4.1

City of Portland
 REVIEWED FOR CODE COMPLIANCE
 OCT 30 2012
 Permit Number

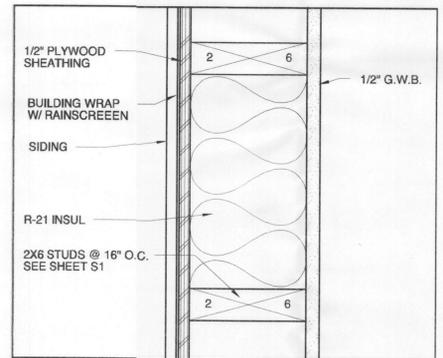
NOTE:
Written dimensions on this drawing shall take precedence over scaled dimensions.
Contractor shall verify all dimensions, conditions, etc. pertaining to the work before proceeding.
The Owner must be notified on any variations from the dimensions and/or conditions shown on these drawings. Any such variation shall be resolved by the Owner prior to proceeding with the work or the Contractor shall accept full responsibility for cost to rectify same.



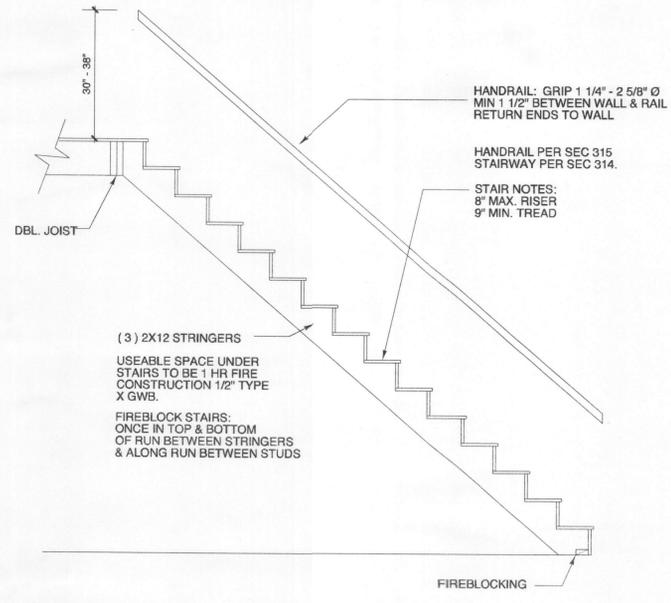
2 FRAMING SECTION @ GREATROOM & BEDROOMS
SCALE: 1/4" = 1'-0"



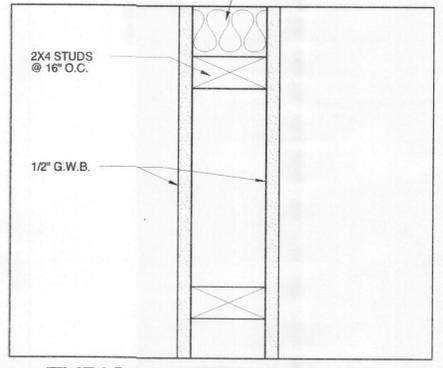
1 GENERAL DECK STAIR SECTION
SCALE: 1/2" = 1'-0"



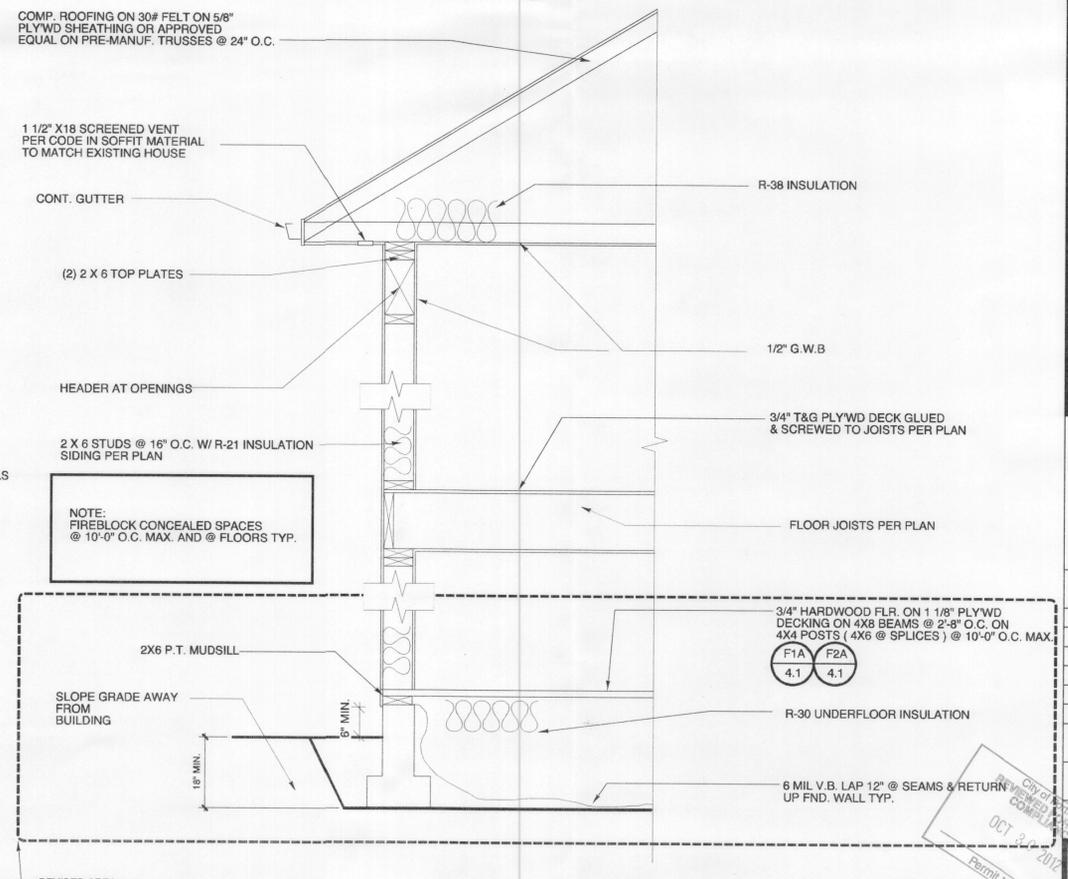
TYPICAL EXTERIOR WALL
SCALE: 3" = 1'-0"



3 TYPICAL STAIR SECTION
SCALE: 1/2" = 1'-0"



4 WALL DETAILS
SCALE: 3" = 1'-0"



5 TYPICAL WALL SECTION
NO SCALE

SEE SHEET S1 FOR STRUCTURAL INFORMATION

Revisions

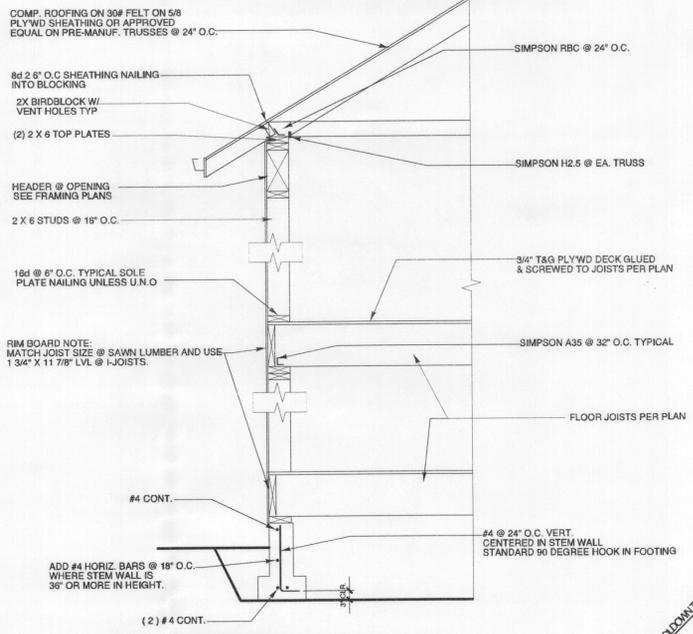
| NO. | DATE | DESCRIPTION |
|-----|----------|-------------|
| | 08.10.12 | |
| | 10.09.12 | |
| | 10.15.12 | |
| | 10.26.12 | |

Project Information

| | |
|-------------------|----------|
| DATE | 07.24.12 |
| DATE LAST PRINTED | |
| PROJECT NO. | 16-12 |

TYPICAL EXTERIOR WALL CONSTRUCTION SHALL BE 15/32" APA RATED WOOD STRUCTURAL PANEL SHEATHING APPLIED TO ONE FACE WITH 10D @ 6" O.C. EDGES AND 10D @ 12" O.C. FIELD TYPICAL UNLESS NOTED.

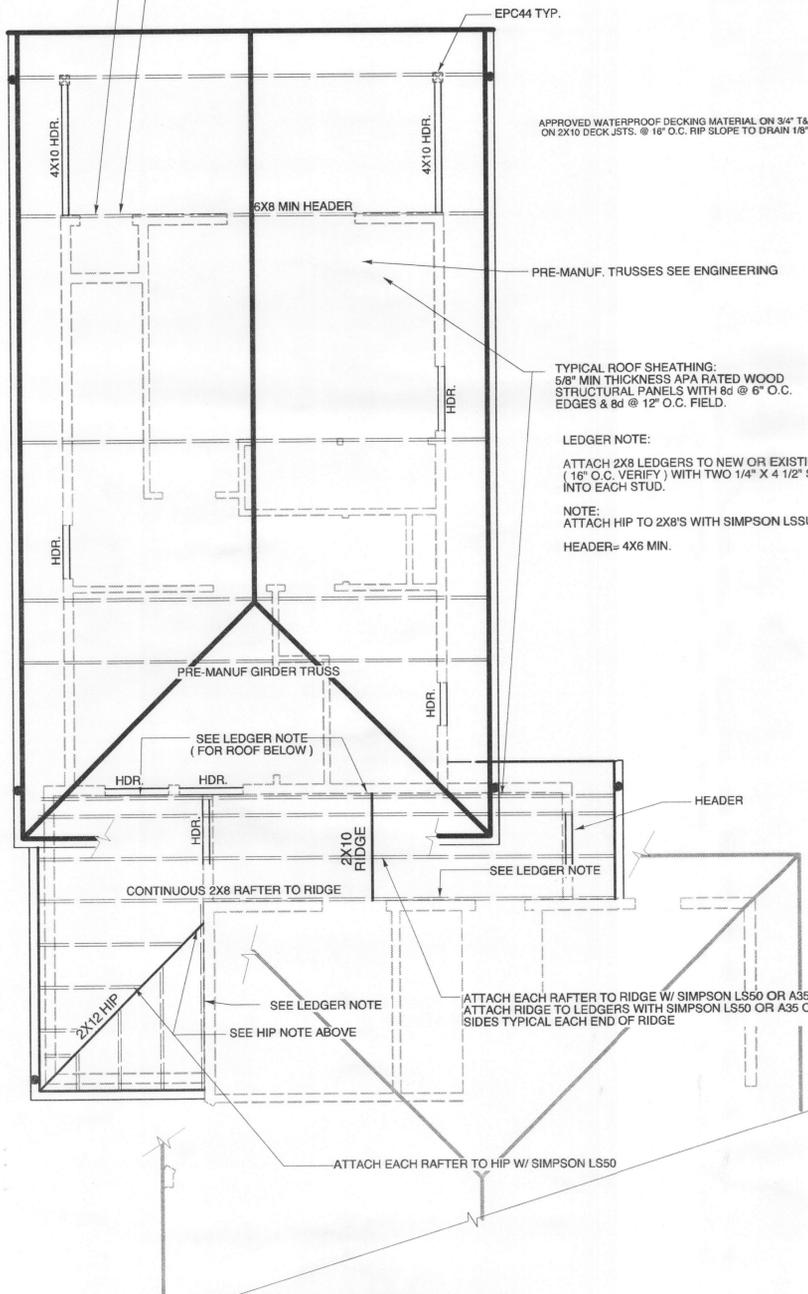
TYPICAL MUDSILL ANCHOR BOLTS SHALL BE 5/8" DIAMETER WITH 7" MIN. EMBEDMENT INTO CONCRETE SPACED @ 48" O.C. EXCEPT SPACING SHALL BE 32" O.C. WHERE 10d @ 3" EDGE NAILING IS INDICATED.



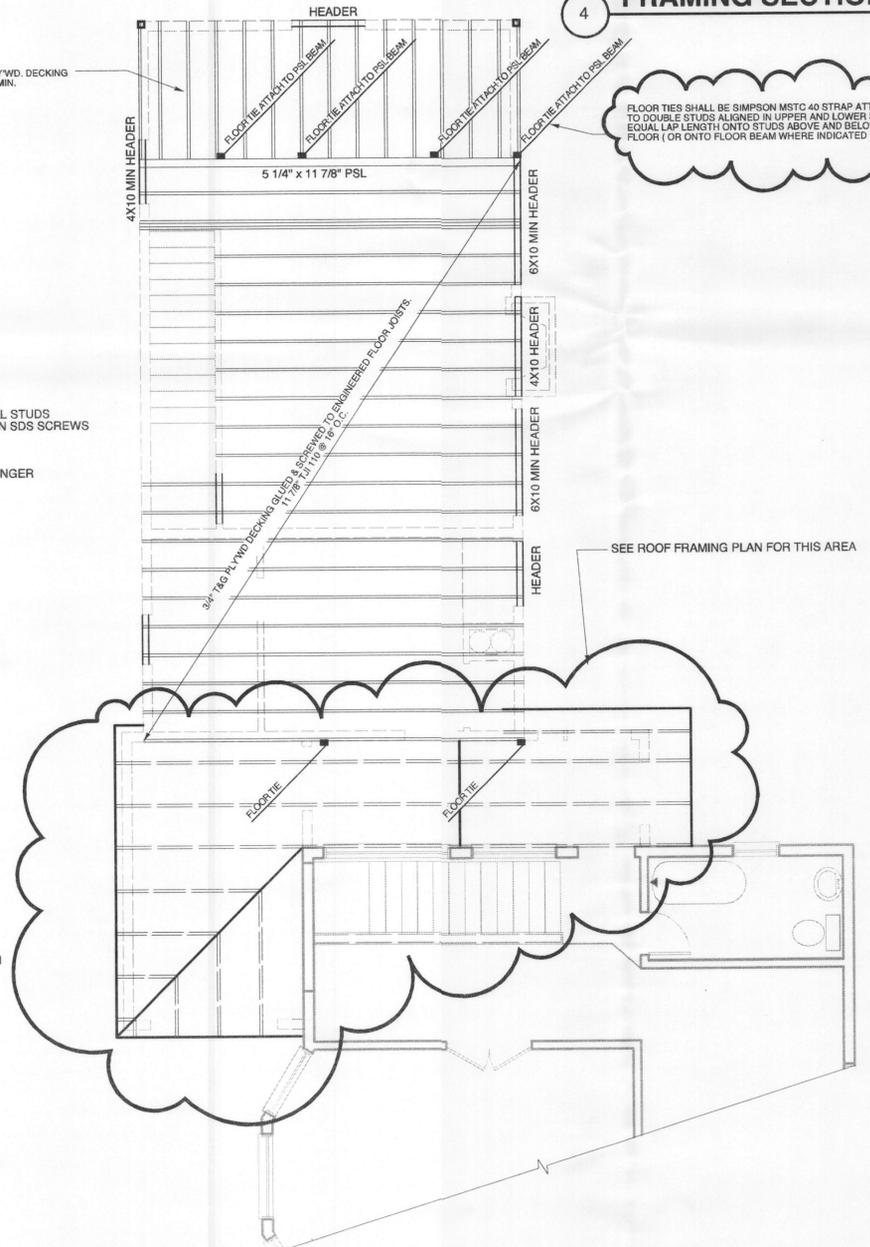
NOTE:
Written dimensions on this drawing shall take precedence over scaled dimensions.
Contractor shall verify all dimensions, conditions, etc. pertaining to the work before proceeding.
The Owner must be notified on any variations from the dimensions and/or conditions shown on these drawings. Any such variation shall be resolved by the Owner prior to proceeding with the work or the Contractor shall accept full responsibility for cost to rectify same.

SPECIAL COLLECTOR (DRAG STRUT) TRUSS- DESIGN & BUILD FOR ROOF DIAPHRAGM LATERAL TRANSFER LOAD OF 2300 LBS. ATTACH ROOF SHEATHING TO TRUSS TOP CHORD WITH 8d @ 6" O.C. (PANEL JOINTS IN SHEATHING NOT PERMITTED ABOVE THIS TRUSS)

ATTACH BOTTOM CHORD OF COLLECTOR TRUSS TO EACH SHEAR WALL PANEL TOP PLATE WITH (3) SIMPSON A35 - PROVIDE (6) A35 TOTAL FOR TWO SHEAR WALLS)



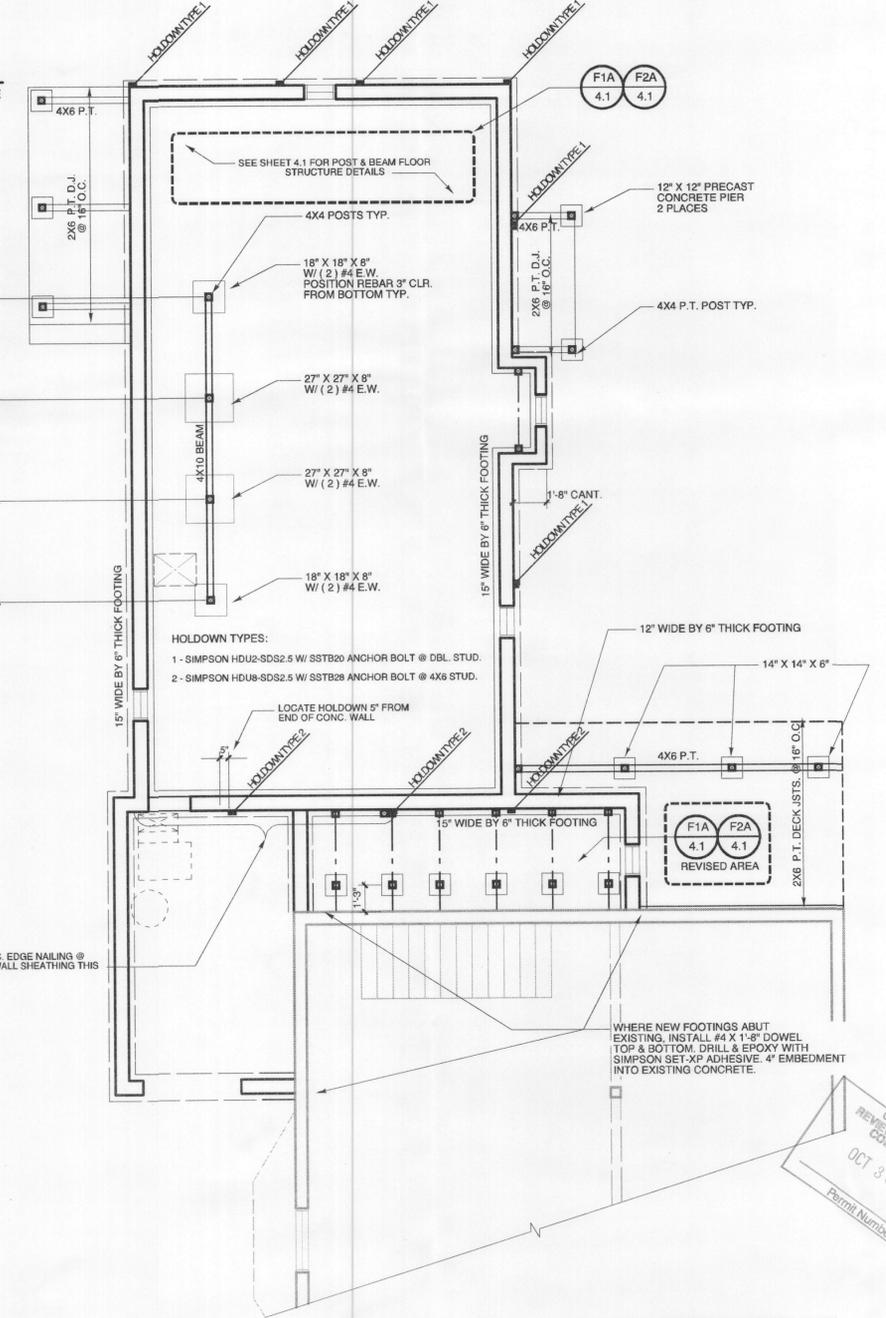
1 ROOF PLAN SCALE: 1/4" = 1'-0"



2 UPPER FLOOR FRAMING PLAN SCALE: 1/4" = 1'-0"

4 FRAMING SECTION NO SCALE

FLOOR TIES SHALL BE SIMPSON MSTC 40 STRAP ATTACHED TO DOUBLE STUDS ALIGNED IN UPPER AND LOWER STORES- EQUAL LAP LENGTH ONTO STUDS ABOVE AND BELOW UPPER FLOOR (OR ONTO FLOOR BEAM WHERE INDICATED)



3 FOUNDATION WITH MAIN FLOOR STRUCTURE SCALE: 1/4" = 1'-0"

Structural Drawings For:
The Parra Residence
1337 SE Lexington
Portland, Oregon 97202

Structural Plans

| REVISIONS | DATE |
|-----------|----------|
| | 10.09.12 |
| | 10.16.12 |
| | 10.26.12 |

DATE: 10.07.12
DATE LAST PRINTED:

PROJECT NO:

SHEET
S1

REVISED SHEET 10.26.12